AGENDA

Planning Commission

February 02, 2026

5:30 PM Regular Session

City Council Chamber
300 E. Chapman Avenue
Orange, CA 92866

Hayden Beckman
Planning Manager

Nathalie Adourian
City Attorney

Jennifer Scudellari
Executive Assistant

ALISON VEJAR
Chair

ISAIAH LESLIE
Vice Chair

RICK MARTINEZ
Commissioner

DAVID VAZQUEZ
Commissioner

RUBY MALDONADO
Commissioner

SHANNON TUCKER
Commissioner

TIM McCORMACK
Commissioner

Welcome to the Planning Commission meeting. Regular meetings of the City of Orange Planning Commission are held the first and third Monday of each month at 5:30 p.m.

Agenda Information

The agenda contains a brief general description of each item to be considered. Written materials relating to an item on the agenda that are provided to the Planning Commission after agenda packet distribution and within 72 hours before it is to consider the item will be made available for public inspection in the City Clerk's Office located at 300 E. Chapman Avenue, Orange, during normal business hours; at the Planning Commission meeting; and made available on the City's website.

Public Participation

Planning Commission meetings may be viewed on Spectrum Cable Channel 3 and AT&T U-verse Channel 99 or streamed live and on-demand on the City's website at www.cityoforange.org.

Pursuant to Government Code Section 54954.3, members of the public may address the Planning Commission on any agenda items or matters within the jurisdiction of the governing body by using any of the following methods:

1) In-person

To speak on an item on the agenda, complete a speaker card indicating your name, address, and identify the agenda item number or subject matter you wish to address. The card should be given to City staff prior to the start of the meeting. General comments are made during the "Public Comments" section at the beginning of the meeting. Public Comments are limited to three (3) minutes per speaker unless a different time limit is announced. It is requested that you state your name for the record, then proceed to address the Commission. All speakers shall observe civility, decorum, and good behavior.

(Continued on page 2)

2) Written Public Comments via eComment

Members of the public can submit their written comments electronically for Planning Commission consideration by using the eComment feature on the Agenda page of the City's website at www.cityoforange.org. To ensure distribution to the Planning Commission prior to consideration of the agenda, we encourage the public to submit written comments by 3:00 p.m. the day of the meeting. All written comments will be provided to the Commissioners for consideration and posted on the City's website after the meeting.

3) Public Comments via recorded voicemail message

Finally, the public can record their comments by calling (714) 744-7271 no later than 4:00 p.m. the day of the meeting. Recorded messages will not be played at the meeting, but will be provided to the Planning Commission and the caller's position will be summarized in the minutes.

Please contact the City Clerk's Office at (714) 744-5500 with any questions.

ADA Requirements: In compliance with the Americans with Disabilities Act, if you need accommodations to participate in this meeting, contact the Clerk's office at (714) 744-5500. Notification at least 48 hours in advance of meeting will enable the City to make arrangements to assure accessibility to this meeting.

REMINDER: Please silence all electronic devices while Planning Commission is in session.

APPEAL PROCEDURE

Any final determination by the Planning Commission may be appealed, and such appeal must be filed within 7 business days after the Planning Commission action. This appeal shall be made in written form to the Community Development Department, accompanied by an initial appeal deposit of $1,000.00.

The City Clerk, upon filing of said appeal, will set petition for public hearing before the City Council at the earliest date. All owners of property located within 300 feet of the project site will be notified by the City Clerk of said hearing. For additional information, please call (714) 744-7220.

If you challenge any City of Orange decision in court, you may be limited to raising only those issues you or someone else raised at the public hearing described on this agenda or in written correspondence delivered to the Planning Commission at, or prior to, the public hearing.

1. OPENING/CALL TO ORDER

1.1 PLEDGE OF ALLEGIANCE

Commissioner David Vazquez

1.2 ROLL CALL

2. PUBLIC COMMENTS

At this time, members of the public may address the Commission on matters not listed on the Agenda which are within the subject matter jurisdiction of the Commission, provided that NO action may be taken on off-agenda items unless authorized by law. Public Comments are limited to three (3) minutes per speaker.

3. CONSENT CALENDAR

All matters listed under the Consent Calendar are considered to be routine by the Planning Commission and will be enacted by one motion. There will be no separate discussion of said items unless members of the Planning Commission, staff or the public request specific items removed from the Consent Calendar for separate action.

3.1. Approval of meeting minutes of the City of Orange Planning Commission Regular Meeting held on January 22, 2026.

Recommended Action:

Approve minutes as presented.

Attachments: Staff Report
January 22, 2026 Regular Meeting Minutes

4. PUBLIC HEARINGS

4.1. Public Hearing to consider an amendment to the Circulation and Mobility Element of the City of Orange General Plan and finding of CEQA exemption.

Recommended Action:

1. Conduct and close the Public Hearing.
2. Adopt Planning Commission Resolution No. 05-26 recommending the City Council approve General Plan Amendment No. 26-0001 amending the City of Orange General Plan Circulation and Mobility Element to conform to the Orange County Transportation Authority's Countywide Master Plan of Arterial Highways.
3. Find the proposal exempt from the California Environmental Quality Act (CEQA) per State CEQA Guidelines Section 15061(b)(3) (Common Sense Exemption).

Attachments: Staff Report
Attachment 1 Planning Commission Resolution No. PC 05-26 including amended City Orange General Plan Circulation and Mobility Element
Attachment 2 Proposed City Master Plan of Streets and Highways Amendment Reclassification
Attachment 3 - Amended City of Orange General Plan Circulation and Mobility Element (redline version)
Attachment 4 Santiago Canyon Road (Villa Park Road) MPAH Amendment Traffic Study
Attachment 5 County of Orange Staff Report of May 6, 2025, for the Santiago Canyon Road (Villa Park Road) reclassification
Attachment 6 MPAH Amendment Staff Report of September 9, 2024, for the Santiago Canyon Road (Villa Park Road) reclassification
Attachment 7 City of Santa Ana General Plan Update Appendix K Transportation Impact Study for the Fairhaven Avenue reclassification
Attachment 8 City of Santa Ana Staff Report from April 19, 2022, for the Fairhaven Avenue reclassification
Attachment 9 Adopted City of Santa Ana General Plan Mobility Element
Attachment 10 OCTA Staff Report of November 2, 2020, for the Conditional Amendment of the Fairhaven reclassification in the Countywide Master Plan of Arterial Highways

4.2. Public Hearing to consider a request to operate a bed and breakfast inn at 616 W. Chapman Avenue and finding of CEQA Exemption (CUP-3208 and SP-1159).

Recommended Action:

1. Conduct and close the Public Hearing.
2. Adopt Planning Commission Resolution No. PC 04-26, approving Conditional Use Permit No. 3208 and Minor Site Plan Review No. 1159 for a request to operate a bed and breakfast inn at 616 W. Chapman Avenue.
3. Find the request categorically exempt from the California Environmental Quality Act (CEQA) in accordance with Section 15301 (Class 1 - Existing Facilities) of the State CEQA Guidelines.

Attachments: Staff Report
Attachment 1 Planning Commission Resolution No. PC 04-26
Attachment 2 Vicinity Map
Attachment 3 Project Plans
Attachment 4 Applicant Business Narrative Letter
Attachment 5 Site Photos

5. ADJOURNMENT

Pursuant to OMC 2.64.040, the next Regular Planning Commission Meeting will be

held on Thursday, February 19, 2026 at 5:30 p.m., in the Council Chamber.

I, Jennifer Scudellari, Executive Assistant for the City of Orange, hereby declare, under penalty of perjury, that a full and correct copy of this agenda was posted pursuant to Government Code Section 54950 et. seq., at the following locations: Orange Civic Center kiosk and Orange City Clerk's Office at 300 E. Chapman Avenue, Police facility at 1107 N. Batavia, Orange Main Public Library at 407 E. Chapman Avenue, and uploaded to the City's website www.cityoforange.org.

Date posted: January 29, 2026

Agenda Item

Planning Commission

Item #: 3.1. 2/2/2026 **File #:** 26-0056

TO: **Chair and Members of the Planning Commission**

THRU: **Hayden Beckman, Planning Manager**

FROM: **Jennifer Scudellari, Executive Assistant**

1. SUBJECT

Approval of meeting minutes of the City of Orange Planning Commission Regular Meeting held on January 22, 2026.

2. SUMMARY

Submitted for your consideration and approval are the minutes of the above meeting(s).

3. RECOMMENDED ACTION

Approve minutes as presented.

4. ATTACHMENTS

- January 22, 2026 Regular Meeting minutes

Agenda Item

Planning Commission

Item #:	2/2/2026	**File #:** 26-0056

TO: **Chair and Members of the Planning Commission**

THRU: **Hayden Beckman, Planning Manager**

FROM: **Jennifer Scudellari, Executive Assistant**

1. SUBJECT

Approval of meeting minutes of the City of Orange Planning Commission Regular Meeting held on January 22, 2026.

2. SUMMARY

Submitted for your consideration and approval are the minutes of the above meeting(s).

3. RECOMMENDED ACTION

Approve minutes as presented.

4. ATTACHMENTS

- January 22, 2026 Regular Meeting minutes

MINUTES - DRAFT

City of Orange

Planning Commission

January 22, 2026

The Planning Commission of the City of Orange, California convened on January 22, 2026, at 5:30 p.m. in a Regular Meeting in the Council Chamber, 300 E. Chapman Avenue, Orange, California.

1. OPENING/CALL TO ORDER

Chair Vejar called the meeting to order at 5:34 p.m.

1.1 PLEDGE OF ALLEGIANCE

Commissioner Rick Martinez led the flag salute.

1.2 ROLL CALL

Present: Leslie, Martinez, Maldonado, Tucker, McCormack, and Vejar
Absent: Vazquez

2. PUBLIC COMMENTS

None.

3. CONSENT CALENDAR

All matters listed under the Consent Calendar are considered to be routine by the Planning Commission and will be enacted by one motion. There will be no separate discussion of said items unless members of the Planning Commission, staff or the public request specific items removed from the Consent Calendar for separate action.

3.1. Approval of meeting minutes of the City of Orange Planning Commission Regular Meeting held on December 1, 2025.

ACTION: Approved minutes as presented.

Approval of the Consent Calendar

A motion was made by Commissioner Martinez, seconded by Commissioner McCormack, to approve the Consent Calendar. The motion carried by the following vote:

Ayes: Leslie, Martinez, Maldonado, Tucker, McCormack, and Vejar
Noes: None
Absent: Vazquez

4. PUBLIC HEARINGS

4.1. Public Hearing to consider a request to establish an alternate parking rate and construct an addition to an existing commercial building for the site located at 1012 N. Tustin Street and a finding of CEQA Exemption (CUP25-0010).

Associate Planner, Arlen Beck, presented a summary of the proposed project.

Chair Vejar opened the public hearing at 5:40 p.m.

Applicant Dominic To spoke in support of the project.

There being no public speakers, Chair Vejar closed the public hearing at 5:42 p.m.

A motion was made by Chair Vejar, seconded by Commissioner Martinez to:
1. Adopt Planning Commission Resolution No. PC 01-26, approving Conditional Use Permit No. 25-0010 and Minor Site Plan Review No. 1181, for an 834 square-foot addition, exterior facade improvements, interior remodel, new landscaping, and a revised parking lot layout and striping to accommodate a new veterinary hospital within an existing commercial building, and to establish an alternate parking rate for the two existing uses at the site located at 1012-1016 N. Tustin Street.
2. Find the request categorically exempt from the California Environmental Quality Act (CEQA) in accordance with Section 15301 of the State CEQA Guidelines.
The motion carried by the following vote:

Ayes: Leslie, Martinez, Maldonado, Tucker, McCormack, and Vejar
Noes: None
Absent: Vazquez

4.2. Public Hearing to consider a request to serve beer and wine for on-site consumption in association with a proposed restaurant, Car-nnoisseur Café, located at 426 W Almond Avenue, Suite C and finding of CEQA exemption (ENT25-0048).

Associate Planner, Arlen Beck, presented a summary of the proposed project.

Chair Vejar opened the public hearing at 5:46 p.m.

Applicant, Arturo Zalaya spoke in support of the project

Donnie DeWees and Ruben Marchan spoke in opposition of the project.

Chair Vejar closed the public hearing at 5:57 p.m.

Staff responded to questions regarding parking, vehicle access, trash enclosures and adjacent sensitive uses.

A motion was made by Commissioner McCormack, seconded by Commissioner Martinez to:
1. Adopt Planning Commission Resolution No. PC 02-26 approving ENT No. 25-0048, allowing the sale of beer and wine for onsite consumption in conjunction with a State of California Department of Alcoholic Beverage Control Type 41 license (On-Sale Beer and Wine - Eating Place) within a proposed restaurant located at 426 W. Almond Avenue, Suite C.
2. Find the request categorically exempt from the California Environmental Quality Act (CEQA) in accordance with Section 15301 of the State CEQA Guidelines.
The motion carried by the following vote:

Ayes: Leslie, Martinez, Maldonado, Tucker, McCormack, and Vejar
Noes: None

Absent: Vazquez

4.3. Public Hearing to consider a request to convert a historic warehouse building to a church use at 527 W. Almond Avenue and 180 S. Cypress Street and finding of CEQA Exemption (CUP-3236).

Associate Planner, Arlen Beck, presented a summary of the proposed project.

Chair Vejar opened the public hearing at 6:24 p.m.

Applicant Pastor Kyle Zimmerman spoke in support of the project. The applicant presented letters in support of the project to staff.

Donnie DeWees spoke in support of the project.

Chair Vejar closed the public hearing at 6:32 p.m.

Commissioners briefly discussed conditions of approval related to landscape.

Chair Vejar re-opened the public hearing at 6:40 p.m.

Landscape Architect Gabriel Choi responded to questions regarding shade canopies in the proposed courtyard.

Commissioner Vejar re-closed the public hearing at 6:44 p.m.

A motion was made by Commissioner Martinez, seconded by Commissioner McCormack to:

1. Adopt Planning Commission Resolution No. 03-26, approving Conditional Use Permit No. 3236, Design Review No. 5159, and Minor Site Plan Review No. 1191, to permit the adaptive re-use of a historic steel frame warehouse to a church/religious assembly use and the remodel of an existing office building on a single site that includes improvements within the connecting plaza located at 527 W. Almond Avenue and 180 S. Cypress Street.
2. Find the request categorically exempt from the California Environmental Quality Act (CEQA) in accordance with Section 15301 and Section 15331 of the State CEQA Guidelines.

The motion carried by the following vote:

Ayes: Leslie, Martinez, Maldonado, Tucker, McCormack, and Vejar
Noes: None
Absent: Vazquez

5. ADJOURNMENT

There being no further business, the meeting was adjourned at 6:50 p.m.

The next Regular Planning Commission Meeting will be held on Monday, February 2, 2026 at 5:30 p.m., in the Council Chamber.

Hayden Beckman
Planning Manager

Agenda Item

Planning Commission

Item #: 4.1. 2/2/2026 **File #:** 26-0055

TO: **Chair and Members of the Planning Commission**

THRU: **Hayden Beckman, Planning Manager**

FROM: **Arlen Beck, Associate Planner**

1. SUBJECT

Public Hearing to consider an amendment to the Circulation and Mobility Element of the City of Orange General Plan and finding of CEQA exemption.

2. SUMMARY

The City is proposing an amendment to the City of Orange General Plan Circulation and Mobility Element to reclassify the roadway designation for Santiago Canyon Road (Villa Park Road) between the shared Orange-Villa Park boundary and Cannon Street, and Fairhaven Avenue between the shared Orange-Santa-Ana boundary and Tustin Street (Tustin Avenue) to reflect changing development and traffic patterns.

3. RECOMMENDED ACTION

1. Conduct and close the Public Hearing.
2. Adopt Planning Commission Resolution No. 05-26 recommending the City Council approve General Plan Amendment No. 26-0001 amending the City of Orange General Plan Circulation and Mobility Element to conform to the Orange County Transportation Authority's Countywide Master Plan of Arterial Highways.
3. Find the proposal exempt from the California Environmental Quality Act (CEQA) per State CEQA Guidelines Section 15061(b)(3) (Common Sense Exemption).

4. PROJECT BACKGROUND

The City of Orange was formally requested by adjacent agencies to update roadway classifications for two streets located along the City's jurisdictional boundaries:

- Santiago Canyon Road (Villa Park Road) between the shared Orange-Villa Park boundary and Cannon Street; and
- Fairhaven Avenue between the shared Orange-Santa Ana boundary and Tustin Street (Tustin Avenue).

These requests were initiated by the County of Orange and the City of Santa Ana, respectively, following updated traffic studies that evaluated long-term growth trends, traffic volumes, and roadway design objectives.

Both agencies have already adopted the revised roadway classifications within their own General Plans. The Orange County Transportation Authority (OCTA) has also reviewed and conditionally approved the changes as amendments to the Countywide Master Plan of Arterial Highways (MPAH).

To complete the regional planning process and maintain consistency with neighboring jurisdictions and OCTA, the City of Orange must amend its Circulation and Mobility Element to reflect the same roadway lane configuration.

Although the City of Orange, the County of Orange, the City of Santa Ana, and the OCTA are coordinating to achieve the same roadway reclassification objectives, each agency uses slightly different roadway classification terminology. The intent is to achieve equivalency in terms of number of lanes and median type. Therefore, for clarity and consistency, this staff report uses the roadway classification naming conventions established in the Circulation and Mobility Element of the City of Orange General Plan.

5. PROJECT DESCRIPTION

Santiago Canyon Road (Villa Park Road)

A transportation assessment prepared by Fehr & Peers evaluated Santiago Canyon Road (Villa Park Road) from Wanda Street to Cannon Street, spanning multiple jurisdictions. The study analyzed:

- Traffic operations and level of service;
- Vehicle queueing;
- Safety conditions; and
- User comfort for pedestrians and bicyclists.

The analysis determined that the roadway's existing Major Arterial (6-Lane Divided) designation does not align with current traffic forecasts or the available 100-foot right-of-way. Instead, an Asymmetric Major Arterial (5-Lane Divided) configuration, three westbound lanes and two eastbound lanes, would adequately serve future demand while fitting within existing constraints and meeting General Plan level of service requirements for roadway performance.

This revised designation also allows space for buffered Class II bicycle lanes, improving multimodal safety and comfort.

Fairhaven Avenue

Through their comprehensive General Plan Update, the City of Santa Ana conducted a Transportation Impact Study which included the evaluation of Fairhaven Avenue between Tustin Street (Tustin Avenue) and Yorba Street as a distinct roadway segment for both Existing Year (2020) and Future Year (2045) conditions. The analysis provided:

- Average Daily Traffic volumes
- Roadway Level of Service based on volume-to-capacity (V/C) ratios
- Consistency between observed/forecasted traffic demand and the Circulation Element roadway classification

The Transportation Impact Study determined that Fairhaven Avenue's existing Secondary Arterial (4-Lane Undivided) designation is inconsistent with both observed and forecasted traffic volumes,

supporting its reclassification to a Two-Lane Secondary Arterial (2-Lane Divided) to better align with operational performance and Complete Streets objectives, which supports lower design speeds, and allows for enhanced pedestrian, bicycle, and streetscape features without compromising roadway performance while also meeting General Plan level of service requirements for roadway performance.

Subsequently, OCTA conditionally approved the reclassification from a Secondary Arterial (4-Lane Undivided) to a Two-Lane Secondary Arterial (2-Lane Divided). This change reflects lower than anticipated growth, updated traffic volumes, and surrounding land use context.

The amendment requires the City of Orange to update our General Plan to achieve consistency (number of lanes and median type) in order for the OCTA MPAH Amendment to become final.

6. PROJECT ANALYSIS

Reclassification of Roadways:

The City of Orange's Circulation and Mobility Element (Circulation Element) identifies goals, objectives, policies and implementation programs that affect the transportation system. The Circulation Element contains policies on the development of the transportation facilities necessary to accommodate orderly growth within the City, provides guidance for future planning efforts, and ensures a balanced transportation system. As a part of the Circulation Element, the Circulation & Mobility Plan documents existing transportation facilities and identifies the desired future conditions of these facilities. The Circulation Element defines the different classifications of roadways to accommodate various daily capacity and functions.

The existing Circulation & Mobility Plan includes six roadway classifications. For each roadway classification, the level of service (LOS) is identified to indicate the performance measure for an arterial that meets the City's required daily capacity minimum of LOS D. Finally, the City Master Plan of Streets and Highways illustrates the proposed roadway classification for each major roadway within Orange.

The proposed General Amendment will reclassify two specific roadway segments within the City as follows:

1. Santiago Canyon Road between the shared Orange-Villa Park boundary and Cannon Street to be reclassified from Major Arterial (6-lane divided) to Asymmetric Major Arterial (5-lane divided); and

2. Fairhaven Avenue, between the shared Orange-Santa Ana boundary and Tustin Street from Secondary Arterial (4-Lane Undivided) to Two-Lane Secondary Arterial (2-Lane Divided).

Attachment 2 identifies the two roadway segments to be reclassified. Additionally, this proposed General Plan Amendment will create two more roadway classifications:
1) Asymmetric Major Arterial (5-Lane Divided) and 2) Two-Lane Secondary Arterial (2-Lane Divided). Adding these two classifications will result in a total of eight roadway classifications within the Circulation & Mobility Plan. Attachment 3 contains the proposed changes to the Circulation Element, including an updated City Master Plan of Streets and Highways. The requested reclassifications for both Santiago Canyon Road and Fairhaven Avenue follow adopted reclassifications by neighboring agencies and OCTA.

The County of Orange prepared a traffic analysis for the reclassification of Santiago Canyon Road (named Villa Park Road within County limits) between Hewes and Cannon Street (Attachment 4) from a divided 6-lane to a divided 5-lane roadway. The results of this analysis support the proposed reclassification, and in May 2025, the County of Orange adopted the reclassification of Villa Park Road (Santiago Canyon Road) (Attachment 5). OCTA, in anticipation of the County of Orange's General Plan amendment, adopted this reclassification in OCTA's Master Plan of Arterial Highways in September 2024 (Attachment 6). OCTA's adoption of the reclassification of Santiago Canyon Road requires the City of Orange to update their General Plan to reclassify this road segment in kind.

As part of a recent General Plan update, the City of Santa Ana evaluated various roadway segments, including Fairhaven Avenue, for reclassification. The results of Santa Ana's traffic analysis supports the reclassification of Fairhaven Avenue, between Grand Avenue to Tustin Street, from an undivided 4-lane to a divided 2-lane roadway (Attachment 7). Santa Ana adopted an updated General Plan in April 2022 (Attachment 8), which includes an updated Mobility Element showing the reclassification of Fairhaven Avenue (Attachment 9). OCTA, in response to Santa Ana's request to reclassify Fairhaven Avenue, conditionally approved the adoption of this reclassification in OCTA's Master Plan of Arterial Highways (Attachment 10). OCTA's conditional adoption of the reclassification of Fairhaven Avenue stipulates that the City of Orange must also reclassify this segment of Fairhaven Avenue in kind.

It is important that the City's Master Plan of Streets and Highways is consistent with neighboring jurisdictions, such as the County of Orange and Santa Ana, to ensure the City's circulation system develops in a manner that promotes regional mobility. This amendment will achieve this consistency.

7. AUTHORIZING GUIDELINES

Orange Municipal Code Section 17.08.020 and 17.10.010 authorizes the Planning Commission to review and make advisory recommendations to the City Council on General Plan Amendments.

8. PUBLIC NOTICE

On January 22, 2026, a notice was published in the Orange City News newspaper for a public hearing before the Planning Commission on February 2, 2026.

9. ENVIRONMENTAL REVIEW

The proposed project is categorically exempt (Common Sense Exemption) from the provisions of the California Environmental Quality Act (CEQA) per State CEQA Guidelines Section 15061(b)(3) because there is no possibility that the revisions to the roadway designations may have a significant effect on the environment.

10. ADVISORY BOARD ACTION

None.

11. ATTACHMENTS

- Attachment 1 Planning Commission Resolution No. PC 05-26 (including amended City of Orange General Plan Circulation and Mobility Element)
- Attachment 2 Proposed City Master Plan of Streets and Highways Amendment Reclassification

- Attachment 3 Amended City of Orange General Plan Circulation and Mobility Element (redline version)
- Attachment 4 Santiago Canyon Road (Villa Park Road) MPAH Amendment Traffic Study
- Attachment 5 County of Orange Staff Report of May 6, 2025, for the Santiago Canyon Road (Villa Park Road) reclassification
- Attachment 6 MPAH Amendment Staff Report of September 9, 2024, for the Santiago Canyon Road (Villa Park Road) reclassification
- Attachment 7 City of Santa Ana General Plan Update Appendix K Transportation Impact Study for the Fairhaven Avenue reclassification
- Attachment 8 City of Santa Ana Staff Report from April 19, 2022, for the Fairhaven Avenue reclassification
- Attachment 9 Adopted City of Santa Ana General Plan Mobility Element
- Attachment 10 OCTA Staff Report of November 2, 2020, for the Conditional Amendment of the Fairhaven reclassification in the Countywide Master Plan of Arterial Highways

Agenda Item

Planning Commission

Item #: 4.1. 2/2/2026 **File #:** 26-0055

TO: **Chair and Members of the Planning Commission**

THRU: **Hayden Beckman, Planning Manager**

FROM: **Arlen Beck, Associate Planner**

1. SUBJECT

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2. SUMMARY

The City is proposing an amendment to the City of Orange General Plan Circulation and Mobility Element to reclassify the roadway designation for Santiago Canyon Road (Villa Park Road) between the shared Orange-Villa Park boundary and Cannon Street, and Fairhaven Avenue between the shared Orange-Santa-Ana boundary and Tustin Street (Tustin Avenue) to reflect changing development and traffic patterns.

3. RECOMMENDED ACTION

1. Conduct and close the Public Hearing.
2. Adopt Planning Commission Resolution No. 05-26 recommending the City Council approve General Plan Amendment No. 26-0001 amending the City of Orange General Plan Circulation and Mobility Element to conform to the Orange County Transportation Authority's Countywide Master Plan of Arterial Highways.
3. Find the proposal exempt from the California Environmental Quality Act (CEQA) per State CEQA Guidelines Section 15061(b)(3) (Common Sense Exemption).

4. PROJECT BACKGROUND

The City of Orange was formally requested by adjacent agencies to update roadway classifications for two streets located along the City's jurisdictional boundaries:

- Santiago Canyon Road (Villa Park Road) between the shared Orange-Villa Park boundary and Cannon Street; and
- Fairhaven Avenue between the shared Orange-Santa Ana boundary and Tustin Street (Tustin Avenue).

These requests were initiated by the County of Orange and the City of Santa Ana, respectively, following updated traffic studies that evaluated long-term growth trends, traffic volumes, and roadway design objectives.

Both agencies have already adopted the revised roadway classifications within their own General Plans. The Orange County Transportation Authority (OCTA) has also reviewed and conditionally approved the changes as amendments to the Countywide Master Plan of Arterial Highways (MPAH).

To complete the regional planning process and maintain consistency with neighboring jurisdictions and OCTA, the City of Orange must amend its Circulation and Mobility Element to reflect the same roadway lane configuration.

Although the City of Orange, the County of Orange, the City of Santa Ana, and the OCTA are coordinating to achieve the same roadway reclassification objectives, each agency uses slightly different roadway classification terminology. The intent is to achieve equivalency in terms of number of lanes and median type. Therefore, for clarity and consistency, this staff report uses the roadway classification naming conventions established in the Circulation and Mobility Element of the City of Orange General Plan.

5. PROJECT DESCRIPTION

Santiago Canyon Road (Villa Park Road)

A transportation assessment prepared by Fehr & Peers evaluated Santiago Canyon Road (Villa Park Road) from Wanda Street to Cannon Street, spanning multiple jurisdictions. The study analyzed:

- Traffic operations and level of service;
- Vehicle queueing;
- Safety conditions; and
- User comfort for pedestrians and bicyclists.

The analysis determined that the roadway's existing Major Arterial (6-Lane Divided) designation does not align with current traffic forecasts or the available 100-foot right-of-way. Instead, an Asymmetric Major Arterial (5-Lane Divided) configuration, three westbound lanes and two eastbound lanes, would adequately serve future demand while fitting within existing constraints and meeting General Plan level of service requirements for roadway performance.

This revised designation also allows space for buffered Class II bicycle lanes, improving multimodal safety and comfort.

Fairhaven Avenue

Through their comprehensive General Plan Update, the City of Santa Ana conducted a Transportation Impact Study which included the evaluation of Fairhaven Avenue between Tustin Street (Tustin Avenue) and Yorba Street as a distinct roadway segment for both Existing Year (2020) and Future Year (2045) conditions. The analysis provided:

- Average Daily Traffic volumes
- Roadway Level of Service based on volume-to-capacity (V/C) ratios
- Consistency between observed/forecasted traffic demand and the Circulation Element roadway classification

The Transportation Impact Study determined that Fairhaven Avenue's existing Secondary Arterial (4-Lane Undivided) designation is inconsistent with both observed and forecasted traffic volumes,

supporting its reclassification to a Two-Lane Secondary Arterial (2-Lane Divided) to better align with operational performance and Complete Streets objectives, which supports lower design speeds, and allows for enhanced pedestrian, bicycle, and streetscape features without compromising roadway performance while also meeting General Plan level of service requirements for roadway performance.

Subsequently, OCTA conditionally approved the reclassification from a Secondary Arterial (4-Lane Undivided) to a Two-Lane Secondary Arterial (2-Lane Divided). This change reflects lower than anticipated growth, updated traffic volumes, and surrounding land use context.

The amendment requires the City of Orange to update our General Plan to achieve consistency (number of lanes and median type) in order for the OCTA MPAH Amendment to become final.

6. PROJECT ANALYSIS

Reclassification of Roadways:

The City of Orange's Circulation and Mobility Element (Circulation Element) identifies goals, objectives, policies and implementation programs that affect the transportation system. The Circulation Element contains policies on the development of the transportation facilities necessary to accommodate orderly growth within the City, provides guidance for future planning efforts, and ensures a balanced transportation system. As a part of the Circulation Element, the Circulation & Mobility Plan documents existing transportation facilities and identifies the desired future conditions of these facilities. The Circulation Element defines the different classifications of roadways to accommodate various daily capacity and functions.

The existing Circulation & Mobility Plan includes six roadway classifications. For each roadway classification, the level of service (LOS) is identified to indicate the performance measure for an arterial that meets the City's required daily capacity minimum of LOS D. Finally, the City Master Plan of Streets and Highways illustrates the proposed roadway classification for each major roadway within Orange.

The proposed General Amendment will reclassify two specific roadway segments within the City as follows:

1. Santiago Canyon Road between the shared Orange-Villa Park boundary and Cannon Street to be reclassified from Major Arterial (6-lane divided) to Asymmetric Major Arterial (5-lane divided); and

2. Fairhaven Avenue, between the shared Orange-Santa Ana boundary and Tustin Street from Secondary Arterial (4-Lane Undivided) to Two-Lane Secondary Arterial (2-Lane Divided).

Attachment 2 identifies the two roadway segments to be reclassified. Additionally, this proposed General Plan Amendment will create two more roadway classifications:
1) Asymmetric Major Arterial (5-Lane Divided) and 2) Two-Lane Secondary Arterial (2-Lane Divided). Adding these two classifications will result in a total of eight roadway classifications within the Circulation & Mobility Plan. Attachment 3 contains the proposed changes to the Circulation Element, including an updated City Master Plan of Streets and Highways. The requested reclassifications for both Santiago Canyon Road and Fairhaven Avenue follow adopted reclassifications by neighboring agencies and OCTA.

The County of Orange prepared a traffic analysis for the reclassification of Santiago Canyon Road (named Villa Park Road within County limits) between Hewes and Cannon Street (Attachment 4) from a divided 6-lane to a divided 5-lane roadway. The results of this analysis support the proposed reclassification, and in May 2025, the County of Orange adopted the reclassification of Villa Park Road (Santiago Canyon Road) (Attachment 5). OCTA, in anticipation of the County of Orange's General Plan amendment, adopted this reclassification in OCTA's Master Plan of Arterial Highways in September 2024 (Attachment 6). OCTA's adoption of the reclassification of Santiago Canyon Road requires the City of Orange to update their General Plan to reclassify this road segment in kind.

As part of a recent General Plan update, the City of Santa Ana evaluated various roadway segments, including Fairhaven Avenue, for reclassification. The results of Santa Ana's traffic analysis supports the reclassification of Fairhaven Avenue, between Grand Avenue to Tustin Street, from an undivided 4-lane to a divided 2-lane roadway (Attachment 7). Santa Ana adopted an updated General Plan in April 2022 (Attachment 8), which includes an updated Mobility Element showing the reclassification of Fairhaven Avenue (Attachment 9). OCTA, in response to Santa Ana's request to reclassify Fairhaven Avenue, conditionally approved the adoption of this reclassification in OCTA's Master Plan of Arterial Highways (Attachment 10). OCTA's conditional adoption of the reclassification of Fairhaven Avenue stipulates that the City of Orange must also reclassify this segment of Fairhaven Avenue in kind.

It is important that the City's Master Plan of Streets and Highways is consistent with neighboring jurisdictions, such as the County of Orange and Santa Ana, to ensure the City's circulation system develops in a manner that promotes regional mobility. This amendment will achieve this consistency.

7. AUTHORIZING GUIDELINES

Orange Municipal Code Section 17.08.020 and 17.10.010 authorizes the Planning Commission to review and make advisory recommendations to the City Council on General Plan Amendments.

8. PUBLIC NOTICE

On January 22, 2026, a notice was published in the Orange City News newspaper for a public hearing before the Planning Commission on February 2, 2026.

9. ENVIRONMENTAL REVIEW

The proposed project is categorically exempt (Common Sense Exemption) from the provisions of the California Environmental Quality Act (CEQA) per State CEQA Guidelines Section 15061(b)(3) because there is no possibility that the revisions to the roadway designations may have a significant effect on the environment.

10. ADVISORY BOARD ACTION

None.

11. ATTACHMENTS

- Attachment 1 Planning Commission Resolution No. PC 05-26 (including amended City of Orange General Plan Circulation and Mobility Element)
- Attachment 2 Proposed City Master Plan of Streets and Highways Amendment Reclassification

- Attachment 3 Amended City of Orange General Plan Circulation and Mobility Element (redline version)
- Attachment 4 Santiago Canyon Road (Villa Park Road) MPAH Amendment Traffic Study
- Attachment 5 County of Orange Staff Report of May 6, 2025, for the Santiago Canyon Road (Villa Park Road) reclassification
- Attachment 6 MPAH Amendment Staff Report of September 9, 2024, for the Santiago Canyon Road (Villa Park Road) reclassification
- Attachment 7 City of Santa Ana General Plan Update Appendix K Transportation Impact Study for the Fairhaven Avenue reclassification
- Attachment 8 City of Santa Ana Staff Report from April 19, 2022, for the Fairhaven Avenue reclassification
- Attachment 9 Adopted City of Santa Ana General Plan Mobility Element
- Attachment 10 OCTA Staff Report of November 2, 2020, for the Conditional Amendment of the Fairhaven reclassification in the Countywide Master Plan of Arterial Highways

GENERAL PLAN AMENDMENT NO. 26-0001

RESOLUTION NO. PC 05-26

A RESOLUTION OF THE PLANNING COMMISSION OF THE CITY OF ORANGE RECOMMENDING THE CITY COUNCIL APPROVE GENERAL PLAN AMENDMENT NO. 26-0001 AMENDING THE CITY OF ORANGE GENERAL PLAN CIRCULATION AND MOBILITY ELEMENT TO CONFORM TO THE ORANGE COUNTY TRANSPORTATION AUTHORITY'S COUNTYWIDE MASTER PLAN OF ARTERIAL HIGHWAYS

APPLICANT: CITY OF ORANGE

WHEREAS, the Planning Commission has authority per Orange Municipal Code Sections 17.08.020 and 17.10.010 to make recommendations to the City Council to take action on General Plan Amendments; and

WHEREAS, the City of Orange adopted a comprehensive update to the General Plan on March 9, 2010; and

WHEREAS, the City of Orange has prepared General Plan Amendment No. 26-0001 to amend the Circulation and Mobility Element of the City of Orange General Plan to reclassify roadway designations for certain segments of Santiago Canyon Road (Villa Park Road) and Fairhaven Avenue to reflect updated traffic conditions, roadway constraints, and regional planning efforts; and

WHEREAS, the proposed roadway reclassifications are based on updated traffic analyses that evaluated long-term growth trends, traffic volumes, safety conditions, roadway design feasibility, and multimodal considerations, and determined that the existing classifications do not align with projected demand or right-of-way constraints; and

WHEREAS, the County of Orange and the City of Santa Ana have already adopted the proposed roadway reclassifications within their respective General Plans, and the Orange County Transportation Authority (OCTA) has reviewed and conditionally approved the changes as amendments to the Countywide Master Plan of Arterial Highways; and

WHEREAS, the Planning Commission, having considered the proposed changes to the General Plan Circulation and Mobility Element contained herein at a public hearing held on February 2, 2026, including review of the staff report and receiving public testimony on the item, has determined that General Plan Amendment No. 26-0001 is justified and recommends approval thereof.

NOW, THEREFORE, BE IT RESOLVED that the Planning Commission recommends that the City Council approve General Plan Amendment No. 26-0001, amending the City of Orange General Plan Circulation and Mobility Element as depicted in Exhibit "A" attached hereto and incorporated by this reference based on the following findings:

SECTION 1 – FINDINGS

The subject General Plan Amendment updates roadway classifications within the Circulation and Mobility Element to reflect current and projected traffic conditions, roadway constraints, and multimodal considerations, while maintaining the City's required level of service standards. The amendment supports orderly growth, promotes a balanced and efficient transportation system, and ensures consistency between the City's Master Plan of Streets and Highways and regional transportation planning efforts adopted by neighboring jurisdictions and the Orange County Transportation Authority. As such, the proposed amendment furthers the intent of the General Plan by advancing coordinated regional mobility and responsible long-range transportation planning.

SECTION 2- ENVIRONMENTAL REVIEW

The proposed project is categorically exempt (Common Sense Exemption) from the provisions of the California Environmental Quality Act (CEQA) per State CEQA Guidelines Section 15061(b)(3) because there is no possibility that the revisions to the roadway designations may have a significant effect on the environment.

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ADOPTED this 2nd day of February 2026.

_____
Alison Vejar, Planning Commission Chair

I hereby certify that the foregoing Resolution was adopted by the Planning Commission of the City of Orange at a regular meeting thereof held on the 2nd day of February 2026, by the following vote:

AYES:
NOES:
RECUSED:
ABSENT:

_____
Hayden Beckman, Planning Manager

Exhibit "A"

Amended General Plan Circulation and Mobility Element

CIRCULATION & MOBILITY

INTRODUCTION AND VISION FOR THE FUTURE

Orange's circulation system has been influenced by a variety of historical factors, including the presence of the Santa Fe Railroad, the vision of Alfred B. Chapman and William T. Glassell, the agricultural history of the area, and alternative transportation modes including a historic streetcar system.

In 1887, the Santa Fe Railroad came to Orange and built a station four blocks west of the Plaza. The coming of the railroad set off a real estate boom that brought hundreds of settlers to the area. The railroad also influenced the City's early economic success by providing a means to transport goods, especially citrus, to the entire country. Today, the railroad tracks continue to serve freight trains and provide a critical link to the region via the Metrolink heavy rail transit system.

In the 1870s, Alfred B. Chapman and William T. Glassell subdivided their land into residential and small farm lots centered on a roundabout known today as Plaza Park. Plaza Park was dedicated in 1886 and established the City's two main streets – Chapman Avenue and Glassell Street – as well as the compact street grid of Old Towne Orange. The street grid and railroad system were supported historically by a streetcar system that connected the small towns and settlements that make up the City today.

Over time, the small farms on the outer edges of Orange's core district began to disappear. Two factors influenced this change: the demand for housing after World War II and the appearance of "Quick Decline" disease that destroyed the local citrus industry. As each farm was developed independently, the grid system expanded outward and commercial corridors were established. Orange's roadways began to take on a more suburban pattern of collectors, connectors, and arterials. As development reached the eastern portion of the City, the grid gave way to curvilinear street patterns.

The historic roadways and railways that form the basis for the current circulation network have been complemented over the years by the development of a streetcar system, a transit service, an emerging and continually expanding bicycle trail and route network, and routes for equestrian use in the eastern portion of the City. The City will continue to be served by these multiple modes of transportation and other emerging mobility technologies.

Orange's ***Vision for the Future***, described in the General Plan Introduction, recognizes that the circulation system is a key component of the quality of life in the City. Accordingly, the vision includes the following objectives:

- Residential areas will be connected to commercial, recreational, and open space areas, as well as educational and cultural facilities via a balanced, multi-modal circulation network that accommodates vehicles, pedestrians, cyclists, hikers, and equestrians. This network will create additional opportunities for walking and biking, enhancing safety and well-being for neighborhoods and businesses.
- The City will work to define neighborhoods through the use of open space areas and a trail system that provides a source of aesthetic beauty and recreational opportunities. These open space areas support a healthy and active community.
- We will develop a connected multi-modal network for traveling from one end of town to the other that provides the option for residents from different neighborhoods to access parks, open spaces, and scenic areas by vehicle, transit, foot, bicycle or, where appropriate, horse.

Purpose of the Circulation & Mobility Element

California's General Plan Guidelines mandate that the Circulation & Mobility Element fulfill the following objectives:

- Show a direct relationship to the Land Use Element to ensure that any changes to land use as stated by the Land Use Element and growth occur with adequate circulation and transportation facilities in mind.
- Address relevant issues including the adequacy of "major thoroughfares, transportation routes, terminals, other local public utilities and facilities." The goal of the Circulation & Mobility Element is to identify circulation problems related to these facilities in the early stages and resolve them in local goals and policies without costly delays.

Other relevant issues discussed in the Circulation & Mobility Element include those that address streets, highways, public transit routes, railroads, bicycle and pedestrian routes, recreational trails, paratransit, parking, transportation system management, and air pollution. The hierarchy of streets within the residential areas helps to frame the urban form. Connections between neighborhoods can be achieved by a comprehensive network of sidewalks and trails. Also, the commercial corridors can be enhanced with adequate street capacity, public transit, and pedestrian-friendly environments.

The state also recommends that the Circulation & Mobility Element address coordination efforts among the local, regional, and state transportation plans to better resolve circulation issues. Since many transportation concerns are regional, addressing them requires intergovernmental and regional transportation management plans and policy implementation. These partnerships ensure the most efficient use of funding, infrastructure, and other resources. The state also recommends the "preservation of transportation corridors for future system improvements."

The Circulation & Mobility Element prioritizes the issues and opportunities that exist within Orange's transit network. It is directly responsive to proposed changes in land use and anticipates the impacts of those changes. This Element also seeks to reassure residents and businesses that the City recognizes the link between transportation and land uses, and provides a means to mitigate the impacts of growth.

Another goal of the Circulation & Mobility Element is to increase transportation options and provide increased access to the circulation system for all residents of Orange. This goal includes improved rail and bus transit connections and frequency, implementation of a *Bikeway Master Plan,* and completion of a trails system. Transforming many of Orange's historically auto-oriented commercial corridors, such as Katella Avenue, Main Street, and portions of La Veta Avenue, Chapman Avenue, and Glassell Street into more pedestrian-friendly mixed-use environments is an overarching goal. Where possible, the development of equestrian trails is also encouraged.

One of the main functions of the Circulation & Mobility Element is to guide and direct enhancement of the current circulation system for existing and future developments. Thus, circulation provisions correlate with the Land Use Element to avoid unchecked growth and unnecessary congestion.

Another key objective of the Element is to work toward a future circulation network that provides meaningful alternatives for getting around the community by less auto-dependent means. The City's topography, street and sidewalk system, transit and trail framework, and land use relationships provide an excellent foundation for pursuit of this objective.

The Circulation & Mobility Element does not simply determine automobile routes. It also guides the movement of people and goods, directly affecting Orange's physical, social, and economic environment. Since circulation permits accessibility to places and social amenities, it can either improve or cause deterioration in quality of life. Circulation efficiency also plays a major role in progress and development of the City's economy.

Scope and Content of the Circulation & Mobility Element

The Circulation & Mobility Element comprises three sections:

(1) Introduction;
(2) Issues, Goals, and Policies; and
(3) The Circulation & Mobility Plan.

The first section introduces the contents of the Circulation & Mobility Element. The second section presents issues, goals, and policies for improving circulation. The third section includes the Circulation & Mobility Plan, which designates locations and standards for roadways and non-motorized circulation facilities, and states the community's desired level of transportation service.

Implementation measures designed to promote achievement of the goals and policies are provided in an Appendix to the General Plan.

Relationship to other General Plan Elements

California planning law requires that the Circulation & Mobility Element correlate and maintain consistency with the other General Plan elements. The Circulation & Mobility Element relates most closely to the Land Use, Natural Resources, Noise, Economic Development, Housing, and Urban Design Elements.

The Circulation & Mobility Element is linked to the Land Use Element because the General Plan land use designations identified in the Land Use Element serve as a basis for the allocation of vehicle trips and establishment of capacity levels for circulation planning. The Land Use Element also provides land use designations that accommodate mixed commercial and residential development, which encourage shorter trips and improve the efficiency of the transportation network. The Circulation & Mobility Plan is established to define and provide for adequate levels of service and facilities to support future land uses. This Element recommends roadway and intersection improvements that may require land acquisition. Location of public transportation facilities will also influence pedestrian activity and transit-oriented development, and the physical size of streets will affect urban land uses and the physical appearance of the City.

The Natural Resources Element identifies regional air quality objectives and provides appropriate mitigation efforts that affect the Circulation & Mobility Element. Improving access, encouraging alternative modes of travel, and maintaining air quality and conservation standards are common objectives of the Natural Resources and the Circulation & Mobility Elements.

The Noise Element addresses future noise levels associated with roadways, rail, and other transportation facilities. Future volumes of traffic on the circulation system are directly related to future noise levels and mitigation strategies.

The Economic Development Element identifies desirable economic conditions and land uses that enhance and promote business activity, employment growth, and economic stability. The goals and policies of the Circulation & Mobility Element will determine road capacity in Orange, which will impact the type and location of uses, and parking and access considerations associated with future uses. Both elements share a common objective of planning for future transportation infrastructure needs. Maintaining roadways, bikeways and bus and rail transit facilities is critical to the success of both current and future businesses in Orange.

The Urban Design Element is a framework for shaping the future form and character of Orange. The quality of Orange's physical environment contributes to its identity, attracts new residents, and sets the stage for economic activity. The Urban Design Element builds on the foundation of Orange's already strong sense of place to preserve and strengthen the streetscape environment of commercial corridors and landmarks within the city. The Urban Design Element and Circulation & Mobility Element share a common objective to reinvent City streets as more functional and walkable public places.

ISSUES, GOALS, AND POLICIES

The goals, policies, and implementation programs of the Circulation & Mobility Element seek to achieve a better balance between vehicular, pedestrian, and bicycle travel, and to provide a wide range of viable transportation options to Orange residents. The following six issues are addressed: (1) enhancing the local circulation system; (2) maintaining the regional circulation system; (3) maintaining a viable public transportation network; (4) creating a comprehensive system of sidewalks, trails, and bikeways; (5) providing adequate parking facilities; and (6) improving circulation system aesthetics and safety.

Local Circulation System

The local roadway system serves the community's primary needs for mobility and access, and consists of a hierarchy of City streets to meet those needs. The City's original street system was established as a grid pattern long before a Master Plan of Arterial Highways (MPAH) was adopted. The Old Towne area and many postwar neighborhoods were designed in a classic grid configuration, while in newer parts of the City, physical features such as the Santa Ana River, Santiago Creek, hilly terrain, freeways and the presence of the City of Villa Park have resulted in a system without a definitive pattern. Some major roads do not connect the eastern and western portions of the City and consequently do not provide effective through circulation. A well-designed roadway system will provide convenient access to activities in Orange.

GOAL 1.0: **Provide a safe, efficient, and comprehensive circulation system that serves local needs, meets forecasted demands, and sustains quality of life in neighborhoods.**

Policy 1.1: Plan, build, and maintain an integrated, hierarchical, and multi-modal system of roadways, pedestrian walkways, and bicycle paths throughout the City.

Policy 1.2: Identify key intersections and streets with historical or projected traffic congestion problems and apply creative traffic management measures to improve overall circulation.

Policy 1.3: Consider various methods to increase safety on City arterials and neighborhood streets, including landscaping, provision of bike/transit lanes, and consideration of traffic calming on neighborhood streets in accordance with the City's Neighborhood Residential Traffic Management Program.

Policy 1.4: Prohibit on-street parking where possible to reduce bicycle/automobile conflicts in appropriate target areas as recommended by the *Bikeways Master Plan*.

Policy 1.5: Address possible safety and noise effects of increased rail activity on grade crossings throughout the City.

Policy 1.6: Maintain and repair roadways and sidewalks as necessary to improve circulation and safety.

Policy 1.7: Consolidate driveways along roadways that provide access to commercial uses to minimize side street interruption and promote smooth traffic flows. On-street parking is prohibited on commercial access streets to provide adequate curb-to-curb width for travel lanes.

Regional Circulation System

Mobility in Orange is directly related to the regional transportation network, as the City lies at the confluence of several regional freeways: the Santa Ana Freeway (Interstate 5), Orange Freeway (State Route [SR] 57), Garden Grove Freeway (SR-22), Costa Mesa Freeway (SR-55), Riverside Freeway (SR-91) and Eastern Transportation Corridor (SR-241). In addition to the freeways, other connections to the region include the commuter rail system known as Metrolink, a freight and goods rail transport system, and a regional bikeways system with connections to the Santa Ana River and other locations. Orange is also connected to the region via the Orange County Transportation Authority (OCTA) bus system. City infrastructure must accommodate regional through traffic originating in other communities in addition to providing local residents access to the regional network.

GOAL 2.0: Provide an effective regional transportation network.

Policy 2.1: Ensure consistency with the County MPAH in order to qualify for funding programs.

Policy 2.2: Coordinate with adjacent cities to plan and develop major east/west and north/south arterials and rapid transit to connect the City with the cities of Anaheim, Tustin, Santa Ana, Garden Grove, and Villa Park, as well as developing areas within the City's sphere of influence.

Policy 2.3: Cooperate with and support local and regional agencies' efforts to improve regional arterials and transit in order to address increasing traffic congestion.

Policy 2.4: Coordinate land use planning with anticipated future development of roadways and other transportation facility improvements as well as the expansion of commuter rail and bus service.

Policy 2.5: Ensure that transportation facilities and improvements do not degrade the quality of Orange's commercial and residential areas.

Policy 2.6: Encourage the use of regional rail, transit, bicycling, carpools, and vanpools for work trips to relieve traffic congestion.

Policy 2.7: Continue to support the use of rail corridors within the City for the movement of freight and goods, and work with rail operators to minimize associated traffic delays.

Public Transportation

Public transportation is a crucial component of a comprehensive circulation system. In addition to reducing air pollution and traffic congestion, a successful public transit system provides an alternative mode of travel for those with limited mobility, residents who may not have access to a car, and persons who choose not to drive.

GOAL 3.0: **Connect centers within the City to each other and to the region through efficient and accessible public transportation.**

Policy 3.1: Work with OCTA and other agencies to assess City public transportation needs and to ensure delivery of services when and where they are needed.

Policy 3.2: Enhance and encourage provision of convenient and attractive transit amenities and streetscapes to encourage use of public transportation (e.g., benches, trash cans, shelters, and lighting).

Policy 3.3: Require incorporation of transit-oriented design features within major commercial and employment areas as well as in medium density residential and mixed-use development areas.

Sidewalks, Trails, and Bikeways

In addition to offering recreational and public health benefits, non-vehicular modes of transportation offer commuting options. Also, the mixed-use environments advocated by Land Use Element policies will encourage increased pedestrian activity on City sidewalks for both business and pleasure. An effective pedestrian, bicycle, and equestrian network must be safe and accessible, and must connect key activity centers within the City with each other and with the regional trail system. A comprehensive network of on-street bicycle lanes, off-street bicycle paths, sidewalks, and trails should be developed and maintained to increase the safety and utility of the system, with a particular focus on the City's sidewalk deficient industrial areas

GOAL 4.0: **Provide efficient and accessible modes of pedestrian, bicycle, and equestrian transportation and improved facilities and amenities.**

Policy 4.1: Create a comprehensive bicycle network that is integrated with other transportation systems by establishing complementary on-street and off-street facilities as identified in the City of Orange *Bikeways Master Plan* and OCTA *Commuter Bikeways Strategic Plan*, including Santiago Creek, the Santa Ana River, and the Tustin Branch Trail.

Policy 4.2: Install racks and safe storage facilities at parking areas for City facilities, as appropriate, and encourage incorporation of such facilities within privately-developed projects.

Policy 4.3: Improve citywide awareness of automobile and bicycle safety.

Policy 4.4: Encourage use of the bikeway system by providing adequate signage, trail markings, and other amenities.

Policy 4.5: Ensure that pedestrian sidewalks, trails, and bikeways are safe environments through the use of crime prevention-oriented trail design features, lighting where appropriate, pedestrian and bicycle safety improvements at at-grade rail crossings, access for emergency vehicles, and links to the roadway signal system.

Policy 4.6: Explore opportunities to convert abandoned rail corridors into segments of the City's bikeway and pedestrian trail system.

Policy 4.7: Provide ADA accessible sidewalks and pedestrian amenities throughout the City.

Policy 4.8: Expand and maintain an equestrian trail network and provide for appropriate staging areas and infrastructure.

Parking Facilities

A shortage of parking can cause circulation problems and could lead to a reduction or loss of business activity. Old Towne Orange has been identified as an area of particular concern. As the City develops, providing adequate parking adjacent to other activity centers is increasingly important.

GOAL 5.0: Provide adequate parking to meet the needs of activity centers throughout the City.

Policy 5.1: Provide adequate parking to protect and support the economic vitality and diversity of Old Towne.

Policy 5.2: Plan for and design parking facilities throughout the City that are adequate to meet demand, but also consider land use-parking efficiencies, and the surrounding natural and built environment.

Policy 5.3: Encourage adjacent businesses to consolidate parking facilities and access points.

Policy 5.4: Encourage well-designed structured parking in commercial areas where such features would be economically feasible, safe, and visually integrated with existing development.

Circulation System Aesthetics

Streets that have been made or modified to include visual and pedestrian amenities can improve the overall look and feel of City streets, as well as enhancing functionality for all users. As major commercial corridors are beautified and changed to include a pleasant pedestrian environment, this will have positive effects on the feelings of safety and security for pedestrians, bicyclists, and motorists.

GOAL 6.0: Provide roadway corridors that are aesthetically pleasing and contribute to a feeling of safety, security, and comfort for motorists, bicyclists, and pedestrians.

Policy 6.1: Supply adequate, clear, and correctly placed signage to direct both motorists and non-motorists toward destinations and away from hazards.

Policy 6.2: Provide clear indicators in the right-of-way for where pedestrians and bicyclists are encouraged to walk, bike, or cross safely. These may include special paving, line stripes, and crosswalks.

Policy 6.3: Provide lighting, landscaping, street trees, and other appropriately scaled streetscape features that accommodate all users on commercial corridors. Where appropriate, lighting should be scaled for autos as well as pedestrians.

CIRCULATION & MOBILITY PLAN

The objective of the Circulation & Mobility Plan is to document existing transportation facilities in the planning area used for the movement of people and goods. The Element addresses the desired future condition of these facilities, and their relationship to future land uses identified in the Land Use Element. The Plan describes the circulation system, including the arterial network and intersections, the public transit system, bicycle paths, recreation facilities, parking, and railroad operations. The City's circulation network includes an extensive system of roadways, bus transit service, commuter rail, and freight rail.

Local Circulation System

A well-designed local arterial roadway system that connects to a well-developed regional circulation system provides safe and convenient access to employment, housing, recreation, and commercial areas in Orange. City arterial roadways located on the western side of SR-55 generally follow north-south and east-west orientations. On the eastern side of the freeway, arterials are characterized by curvilinear streets due to undulating geographical surroundings. Key north-south arterials include Tustin Street, Glassell Street, Main Street, and The City Drive. Key east-west arterials include Chapman Avenue, Katella Avenue, Taft Avenue, and parts of La Veta Avenue. These arterial roadways are in turn supported by a network of collector and local streets that provide access to homes and businesses throughout the City.

Roadway Classification System

The City's roadway network is distinguished by a hierarchical classification system that differentiates roads by size, function, and approximate daily capacity based upon Level of Service D (LOS D). LOS is a qualitative measure that characterizes traffic congestion on a scale of A to F with LOS A representing a free-flow condition and LOS F representing extreme congestion. LOS standards can apply to either intersections or links (a section of street between two intersections). Generally speaking, LOS represents the ability of a roadway or an intersection to accommodate traffic.

In the City, intersections are used as actual control points. City roadways consist of both divided and undivided roadways. Divided roadways generally contain a physical barrier or buffer, such as a raised median or a continuous two-way left turn lane, between each direction of travel. Divided roadways remove vehicles making a left turn from the travel lanes so as not to impede through traffic and constrict roadway capacity. Undivided roadways do not contain a buffer between each direction of travel, and therefore left-turning traffic can impede through traffic. Undivided roadways may provide turn movement pockets at intersections. The eight categories of roadways in Orange are summarized in Table CM-1. Proposed cross-sections for each type of roadway are shown in Figure CM-1.

The City's policy is to use a link capacity standard of LOS D. The following paragraphs represent link capacities of each roadway type at LOS D.

Table CM-1 Roadway Classifications		
Classification	**Facility Type**	**Characteristics**
Smart Street	Smart Street	4-8 lane divided, with possible signal coordination, intersection capacity improvements and/or grade separations
Principal Arterial	8 Lane Divided	Primarily serves through traffic with limited local access
Major Arterial	6 Lane Divided	Serves mostly through traffic with some local access allowed
Asymmetric Major Arterial	5 Lane Divided	Serves mostly through traffic with some local access allowed
Primary Arterial	4 Lane Divided	Serves through and local traffic
Secondary Arterial	4 Lane Undivided	Serves through and local traffic
Two-Lane Secondary Arterial	2 Lane Divided	Serves through and local traffic
Collector Street	2 Lane	Serves mostly local traffic

Smart Streets are typically four- to eight-lane roadways with enhanced capacity and smoother traffic flow than standard arterial streets. These streets have enhanced features such as traffic signal synchronization, bus bays, intersection improvements, and the addition of travel lanes by removing on-street parking and consolidating driveways. The traffic carrying capacities of Smart Streets can range from 60,000 to 79,000 vehicles per day, depending on the number of lanes, degree of access control, peak period loading, and the configurations of major intersections.

Principal Arterials are typically eight-lane divided roadways with medians or continuous two-way left turn lanes. They can accommodate up to 67,500 vehicles on an average weekday at LOS D conditions, depending on the degree of access control, peak period traffic loadings, and lane configurations at major intersections. Principal arterials prohibit on-street, curbside parking, and connect directly to freeways.

Major Arterials are six-lane divided roadways with medians or continuous two-way left turn lanes. They can accommodate up to 50,700 vehicles on an average weekday at LOS D conditions, depending on the degree of access control, peak period traffic loadings, and lane configurations at major intersections. Major arterials facilitate traffic circulation within Orange, and may prohibit on-street, curbside parking.

Asymmetric Major Arterial are five-lane asymmetrical divided roadways with medians or continuous two-way left turn lanes. They can accommodate up to 42,250 vehicles on an average weekday at LOS D conditions, depending on the degree of access control, peak period traffic loadings, and lane configurations at major intersections. Asymmetric major arterials facilitate traffic circulation within Orange, and may prohibit on-street, curbside parking. Occasionally, based on projected volumes, it is necessary to have a cross section with more lanes in one direction than the other (asymmetric lanes) along some Major arterials. The most typical application would be for arterial segments where adjacent area land uses result in higher demand in one of the peak periods. Where such demand creates

the need for an added lane to accommodate the peak surge of traffic, the addition of through lane(s) in one (asymmetric), rather than both directions of travel, is considered.

Primary Arterials are four-lane divided roadways with medians or continuous two-way left turn lanes. They can accommodate up to 33,750 vehicles on an average weekday at LOS D conditions, depending on the degree of access control and peak period loadings. Primary Arterials provide for easy circulation in the City, and allow for limited on-street, curbside parking.

Secondary Arterials are four-lane undivided roadways without medians. They can accommodate up to 21,600 vehicles on an average weekday at LOS D conditions, depending on the degree of access control and peak period loadings. Secondary arterials allow for on-street, curbside parking.

Two-Lane Secondary Arterials are two-lane divided roadways with medians or continuous two-way left turn lanes. They can accommodate up to 17,000 vehicles per average weekday at LOS D conditions, depending on the degree of access control and peak period loadings. Two-lane secondary arterials can allow for on-street, curbside parking.

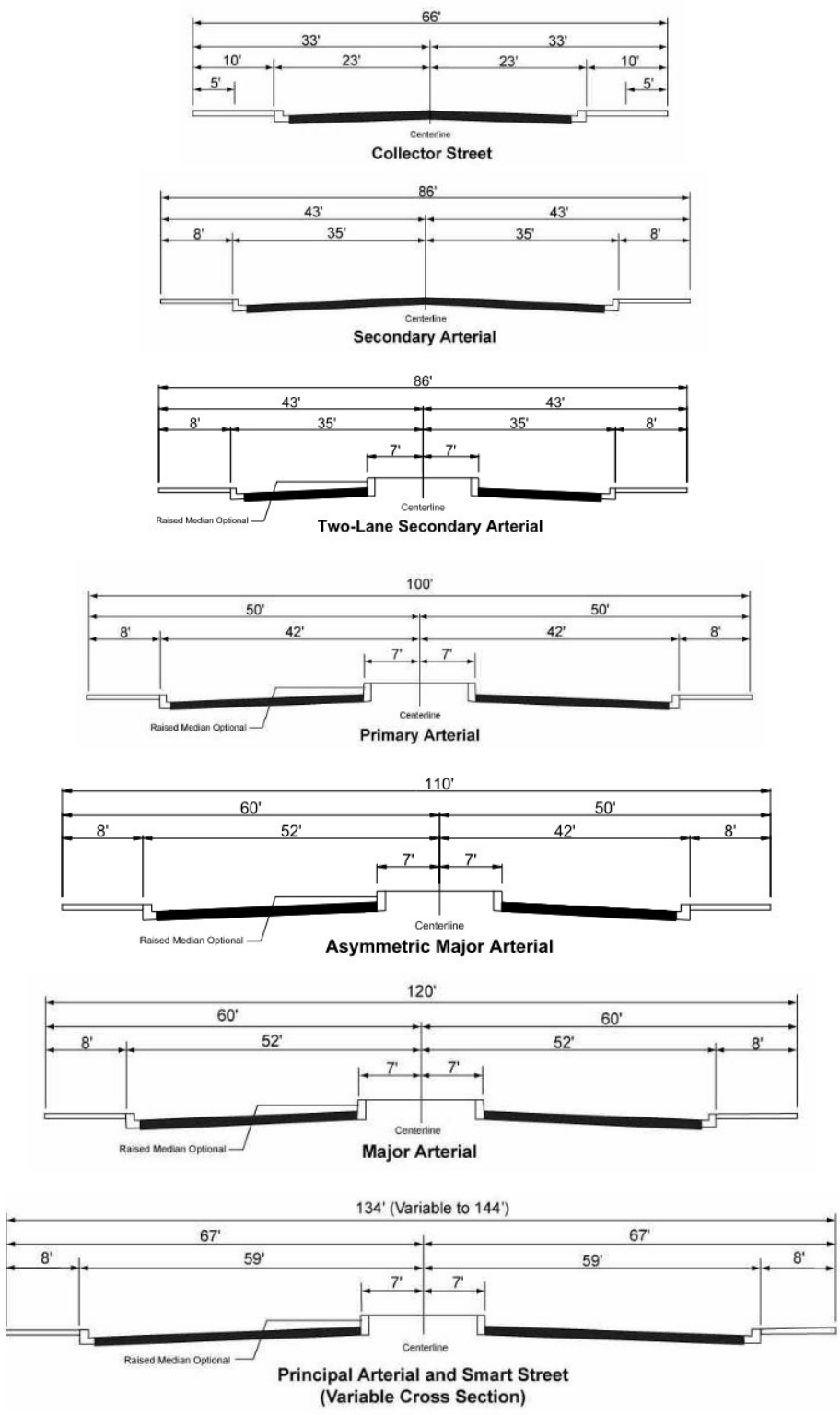


Figure CM-1
Roadway Cross Sections

Collector Streets are typically two-lane roadways without medians that gather and distribute traffic to higher-capacity arterials. They can accommodate up to 10,800 vehicles per average weekday at LOS D conditions, depending on the degree of access control and peak period traffic loadings. Centerline striping is typically not provided on collector streets, and on-street parking is allowed. There are several types of two-lane streets in the City, including divided, undivided, residential, and collector streets. Each type serves a slightly different purpose and may have different capacity thresholds based on various factors.

Performance Criteria

Evaluating the ability of the circulation system to serve residents and businesses in Orange requires establishing performance criteria. Performance criteria have a policy component that establishes a desired LOS, and a technical component that specifies how traffic forecast data can be used to measure criteria achievement.

The LOS definition for intersections is based on a volume-to-capacity (V/C) ratio and provides a more quantitative description of traffic conditions. Table CM-2 presents LOS based on traffic volumes and the design capacity of intersections.

Table CM-2
Level of Service Definitions for Intersections

Level of Service	Volume-to-Capacity Ratio	Description
A	0.00-0.60	Free Flow/Insignificant Delays: No approach phase is fully utilized by traffic and no vehicle waits longer than one red indication.
B	0.61-0.70	Stable Operation/Minimal Delays: An occasional approach phase is fully utilized. Many drivers feel somewhat restricted within platoons of vehicles.
C	0.71-0.80	Stable Operation/Acceptable Delays: Major approach phases fully utilized. Most drivers feel somewhat restricted.
D	0.81-0.90	Approaching Unstable/Tolerable Delays: Drivers may have to wait through more than one red signal indication. Queues may develop but dissipate rapidly, without excessive delays.
E	0.91-1.00	Unstable Operation/Significant Delays: Volumes at or near capacity. Vehicles may wait through several signal cycles. Long queues form upstream from intersection.
F	N/A	Forced Flow/Excessive Delays: Represents jammed conditions. Intersection operates below capacity with low volumes. Queues may block upstream intersections.

Source: Highway Capacity Manual, Transportation Research Board, Special Report No. 209, Washington DC, 2000.

Although roadway capacity is generally a function of peak hour intersection performance and the corresponding peak hour volumes, daily arterial segment capacities (link capacities) also provide a measure of the overall LOS of the arterial system. Generally, traffic impact mitigation focuses on peak hour intersection performance, since system performance is typically a function of intersection performance. The City's policy is to use a link capacity standard of LOS D. Table CM-3 presents arterial daily capacities at LOS D and LOS E.

Table CM-3
Arterial Daily Capacity Threshold Assumptions

Street Type	Daily Capacity	
	LOS D	LOS E
Smart Street – 6- to 8-lane divided	71,100	79,000
Principal – 8-lane divided	67,500	75,000
Major – 6-lane divided	50,700	56,300
Asymmetric Major – 5-lane divided	42,250	47,000
Primary – 4-lane divided	33,750	37,500
Secondary – 4- lane undivided	21,600	24,000
Two-Lane Secondary – 2-lane divided	17,000	18,750
Collector – 2-lane undivided	10,800	12,000

Source: City of Orange General Plan Update Traffic Report, 2008.

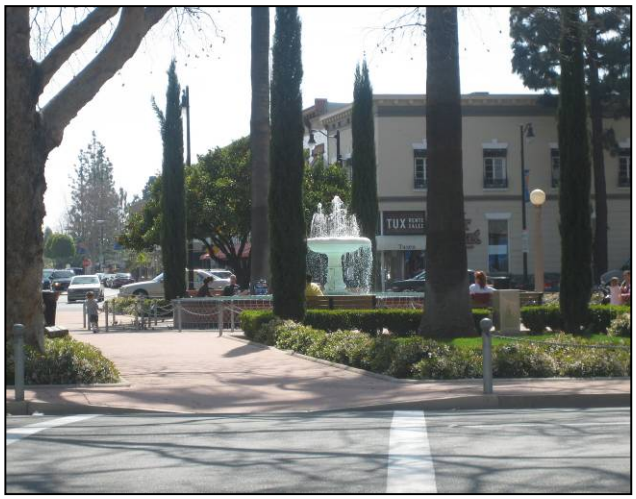
Various LOS policy standards have been established to evaluate observed traffic conditions, future development plans, and circulation system modifications. At the local level, the City of Orange has established LOS D as the lowest acceptable level of service for both roadway segments and peak-hour signalized intersection movements. At the regional planning level, Orange County's Congestion Management Plan (CMP) specifies LOS E as the operating standard for roadways and intersections on the CMP highway system. The CMP Highway System consists of the Orange County smart street network plus the state highway system. Thus, the SR-55 northbound and southbound ramps at Katella Avenue are CMP intersections within the City's jurisdiction. The City does not have an adopted LOS standard for unsignalized intersections. Performance of unsignalized intersections is evaluated on a case-by-case basis.

In addition to the LOS standards for roadways and intersections, the California Environmental Quality Act (CEQA) Guidelines (Section 15064.3), require "vehicle miles traveled" (VMT), to measure transportation impacts on the community. VMT refers to the amount and distance of automobile travel attributable to a project.

For purposes of compliance with CEQA, a significant impact would occur if the baseline and/or cumulative project-generated VMT per service population (population plus employment) exceeds the anticipated City of Orange General Plan Buildout VMT per service population. One vehicle traveling one mile would generate one VMT. Additionally, the project's effect on VMT would be considered significant if it resulted in the baseline and/or cumulative link-level boundary citywide VMT per service population increases under the plus project condition compared to the no project condition. Link-level boundary VMT is calculated by summing all weekday VMT on a roadway network within the City boundary and includes all trips including trips that pass through the City's roadway network but do not start and end with in City.

These thresholds are designed to reduce the number of miles traveled by automobiles, and are implemented within the City of Orange *Traffic Impact Analysis Guidelines*.

In order to maximize the efficiency of its circulation system, the City will look at where physical improvements to the circulation infrastructure can be made to expand capacity and increase traffic flow. To maximize efficiency of the road system, the City will support traffic signal coordination and spacing, and will also discourage on-street parking along arterials. In addition, the City will explore ways to reduce the demand for vehicular transportation, specifically through the provision and maintenance of bike lanes, bikeways, and trails, and will also encourage additional regional transit services and support facilities. The City's Transportation Demand Management (TDM) ordinance (Chapter 10.83 of the Municipal Code) further specifies a variety of techniques available to employers with 100 or more employees to advance the goals of efficiently utilizing the existing and planned transportation system and reducing vehicle emissions.



City Master Plan of Streets and Highways

Land Use Element policy will allow land use changes and intensification to occur in specific focus areas within the City. The City's Master Plan of Streets and Highways displayed in Figure CM-2 has been developed in close coordination with land use policy to ensure that traffic generated by new development will not compromise the City's goal to ensure that intersections and roadway segments operate efficiently. The map identifies components of the City's roadway circulation system. The map also indicates where augmented roads are needed, and pinpoints locations for enhanced intersections, including the future Meats Avenue interchange at SR-55. Although most of Orange is already built out, most remaining developable land is located in the eastern part of the City. New development in east Orange will require construction of new roads to provide circulation and traffic flow to residents and businesses. Land Use Element policies enabling reuse and redevelopment within established portions of the City, particularly within the focus areas, may also necessitate roadway widening and intersection enhancements. The City will continue to collect funds for necessary circulation system capital improvements through a program that sets up a fee structure for all new development and redevelopment projects. This program will require developers to pay their fair share for transportation system improvements required by new projects. The City will use the annual seven-year Capital Improvement Program (CIP) process to prioritize, fund, and complete improvements required to achieve build-out of the proposed roadway system identified in Figure CM-2.

Roadway Widening

Roadway widening in specific locations will be necessary to obtain new travel lanes. Additional travel lanes may be acquired either by obtaining additional rights-of-way as necessary or by constructing new lanes within existing rights-of-way. Parking restrictions may be applied to allow additional lanes to be provided within existing rights-of-way.

Old Towne Street Network

The Old Towne street network is a clear example of Orange's grid street pattern. Parallel roadways have been established in both the north-south and east-west direction to distribute traffic evenly. The Plaza area at Glassell Street and Chapman Avenue is a unique feature that creates discontinuous traffic flows along these two primary roadways. However, no plans have been made to modify the National Register-listed Historic Plaza to increase its traffic carrying capacity.

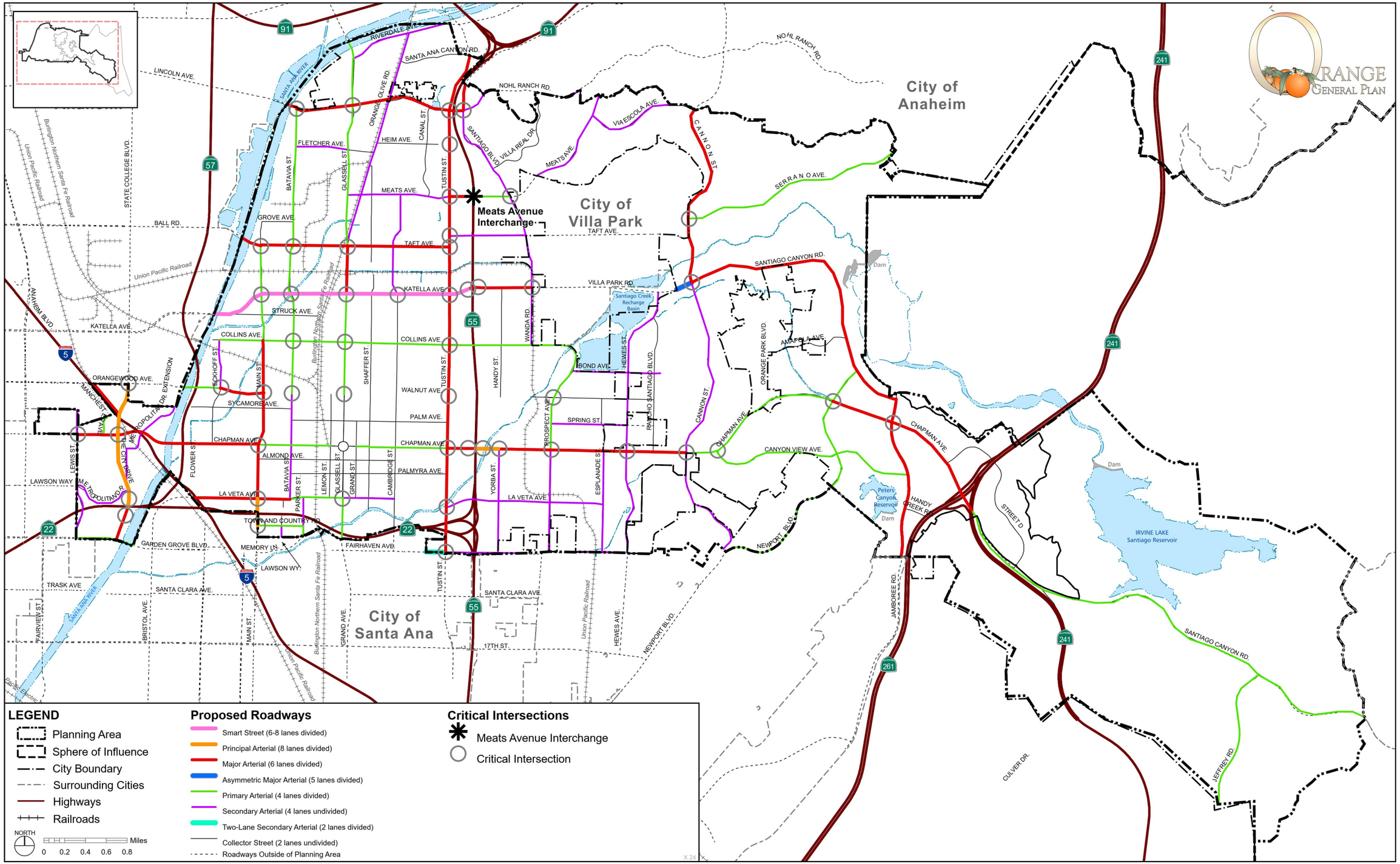


Figure CM-2 City Master Plan of Streets and Highways

GPA 2026-0001 (XX/XX/26)

GPA 2014-0001 (12/18/15)

GPA 2010-0001 (8/10/10)

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In light of these conditions, parallel roadways such as Almond Avenue, Palmyra Avenue, Lemon Street, Olive Street, Shaffer Street, Grand Street, Palm Avenue, and Maple Avenue will continue to serve as local collectors around The Plaza.

Metropolitan Drive Extension

Extending Metropolitan Drive behind the University of California, Irvine (UCI) Medical Center will help facilitate the movement of north-south traffic near The Outlets at Orange shopping area, improve access to Interstate 5 (I-5), and relieve congestion on The City Drive. Metropolitan Drive will be extended from The City Drive/State College Boulevard to the Metropolitan Drive/Rampart Street connection.

Critical Intersection Program

Intersections serve as traffic control points for the circulation system, regulating the flow of vehicles along City streets and sometimes limiting the capacity of the system. In the long term, system capacity and efficiency can both be increased if intersections are designed to handle future anticipated traffic volumes. Typically, the design of the roadways forming an intersection dictates the intersection configuration. Department of Public Works standards indicate that a left-turn pocket may or may not be provided, depending on traffic volumes through the intersection. However, one pocket may not be adequate to handle vehicles during peak hours. Traffic may back up into a through travel lane, resulting in congestion at the intersection and at other locations along the roadway.

One way of providing additional intersection capacity at critical locations is through the use of special intersection configurations known as "critical intersections." Critical intersections deviate from typical City design standards by increasing the number of lanes at an intersection beyond what typically would be required. By increasing capacity at the intersection, the circulation link increases overall system capacity.

The Master Plan of Streets and Highways (Figure CM-2) identifies the locations of critical intersections within Orange. A list of these intersections and diagrams depicting their geometries are on file in the Public Works Department.

Regional Roadway System

The City's local circulation network is connected to an efficient regional circulation system. Figure CM-2 shows the freeways that traverse the Orange planning area. The Santa Ana Freeway (I-5) provides interstate and regional access to the City. In addition, SR-57, SR-55, and SR-22 all provide connections to the City from northern Orange County and neighboring Los Angeles County, San Diego County, Riverside County, and San Bernardino County. SR-91 and SR-241 provide additional, more limited freeway access. SR-241 is a toll facility controlled by the Transportation Corridor Agency (TCA).

I-5 is a northwest-southeast freeway that passes through the southwest corner of the City, and provides direct access to Los Angeles County to the north and San Diego County to the south. I-5 has two interchanges within Orange—one located at its junction with SR-57 and SR-22 (commonly known as the Orange Crush) and the other at State College Boulevard/The City Drive. The junction at the Orange Crush currently has the most severe congestion, which directly affects the roadway system in the City. With projected future growth in Orange and in the region, traffic flow at this junction is expected to worsen. The City will continue to work with and support the efforts of local and regional agencies to mitigate the increased traffic congestion in this area.

SR-91 is an east-west freeway that provides access to key arterial facilities in Orange, including interchanges at Tustin Street and Glassell Street. SR-91 also provides regional access through interchanges with SR-55 and SR-57 and SR-241.

SR-22 is an east-west freeway that crosses through the southern portion of the City. Five SR-22 interchanges are located in the City, at Tustin Street, Glassell Street, Main Street/La Veta Avenue, Bristol Street, and The City Drive.

SR-55 is a north-south freeway that passes through the center of Orange, and provides access to the coastal communities of Orange County. SR-55 has four interchanges in Orange, located at Lincoln Avenue, Katella Avenue, Chapman Avenue, and SR-22. An additional future interchange at Meats Avenue is contemplated within this General Plan.

SR-57 is a north-south freeway that originates at the junction of I-5 and SR-22 and extends to San Dimas in Los Angeles County. It provides access for the eastern parts of Los Angeles County, and central and northern parts of Orange County. SR-57 has three interchanges in Orange, at Chapman Avenue, Orangewood Avenue, and the junctions of I-5 and SR-22.

The Eastern Transportation Corridor (SR-241) is a north-south toll facility located in the eastern portion of the planning area. This facility provides direct access to east Orange. SR-241 has three toll lanes in each direction and provides regional access through an interchange at Santiago Canyon Road.

Consistency with County Master Plan of Arterial Highways

Maintaining consistency with the County's Master Plan of Arterial Highways (MPAH) is required in order to ensure that the City's circulation system develops in a manner that promotes regional mobility. At a practical level, consistency is also required in order for the City to receive transportation funding under Measure M, also known as OC Go. Orange's Master Plan of Streets and Highways (Figure CM-2) is generally consistent with the MPAH.

While the City's Master Plan of Streets and Highways has been consistent with the County MPAH to maintain funding eligibility, both the City of Orange and OCTA have the goal of a realistic and implementable MPAH. A desire of the City is to work with OCTA to downgrade La Veta Avenue between Glassell Street and Cambridge Street from a Secondary Arterial to a

Collector Street to reflect physical constraints related to historic buildings and features in the Old Towne National Register Historic District.

To initiate the MPAH amendment process, a local agency must submit a written request to OCTA describing the amendment requested and provide documentation to support the basis for the request. A copy of the request must be submitted concurrently to the City Managers of adjoining cities. For the facilities under consideration, this would require a letter to be forwarded to the City of Santa Ana.

Once the initial request is forwarded to OCTA, a conference between the City of Orange, OCTA, and potential affected jurisdictions is held to determine whether mutual agreement exists for the MPAH amendment. If mutual agreement exists, then Orange is expected to proceed with adopting this revision to the Circulation & Mobility Element. Upon adoption, the City of Orange would submit the Circulation & Mobility Element to OCTA and request OCTA Board approval of the Orange County MPAH amendment.

Public Transportation

Effective regional transportation strategies are required to successfully implement City and County plans for accommodating future growth. Such strategies must link the City of Orange with other regional employment and commercial centers, as well as airports and other transportation hubs, and should fully integrate alternatives to the automobile. Alternative modes of transportation, including public transportation, bicycling, and walking, are important components of a comprehensive circulation system. These modes of transportation also help reduce air pollution and road congestion.

Public transportation plays a key role in future land use development and mobility. As the roadway system reaches capacity, alternative modes of transportation provide additional capacity as well as an enhanced degree of mobility for residents, workers, and visitors. Existing services are expected to continue while enhancements, many of them currently in the planning stages, will increase the viability of alternative modes of travel. The integration into the circulation system of alternative modes of transportation, such as bus, rail, bicycle, and pedestrian, is essential to maximizing mobility opportunities for residents, workers, and visitors.

Bus Service

OCTA provides public bus service for the City of Orange. In addition, the Riverside Transit Agency (RTA) provides long-distance service between The Village at Orange and the Downtown Terminal in Riverside. Table CM-4 identifies local bus routes that connect various activity centers in Orange to each other and to the region.

Table CM-5 shows the different community, station link, and inter- or intra-County routes that serve Orange. Community routes are express bus routes that provide faster connections to activity centers within and outside Orange County. Station link shuttles provide services between the Orange Transportation Center (OTC) and Orange. Both community and station link routes operate only at peak commuter times.

In addition to the fixed-route service, OCTA also offers several types of specialized community transportation services, such as standard service (curb-to-curb service), door-to-door service, subscription service, and same-day taxi service. Some of these services cater to senior citizens and people with disabilities residing in the City.

OCTA has forecast bus ridership to increase by approximately 75 percent by 2030. Some of this ridership increase will be the result of enhanced services, including express bus routes and introduction of bus rapid transit service. Much of the increase will be driven by increased arterial and freeway congestion levels in conjunction with improved local bus service.

Convenient, accessible, frequent, and easy-to-use public transit is a cornerstone element of the proposed land use plan. Planned mixed-use residential and commercial areas and intensified commercial and professional office corridors must be coupled with increases in transit service. Orange's land use plan features mixed-use districts that strategically concentrate population density near alternative transit facilities, such as the OTC in Old Towne, transit hubs at The Outlets at Orange and South Main Street, and the Anaheim Regional Transportation Intermodal Center (ARTIC) station in Anaheim near the City's western boundary at Katella Avenue.

Table CM-4
Local Bus Routes

Route #	Route Type	Route Service	Service Corridors	Key Orange Activity Centers Served
24	Local	Fullerton–Orange	Malvern Ave. / Chapman Ave. / Tustin St..	The Village at Orange Lincoln Park and Ride
42	Local	Orange–Seal Beach	Lincoln Ave. / Los Alamitos Blvd. / Seal Beach Blvd.	The Village at Orange Lincoln Park and Ride
46	Local	Los Alamitos–Orange	Ball Rd. / Taft Ave.	The Village at Orange Lincoln Park and Ride
47	Local	Brea–Newport Beach	Brea Blvd. / Anaheim Blvd. / Fairview St.	Theo Lacy Jail Orangewood Children's Home UCI Medical Center The Block at Orange
50	Local	Long Beach–Orange	Katella Ave.	The Village at Orange Lincoln Park and Ride
53	Local	Brea–Irvine	Main St.	Batavia Industrial Parks Children's Hospital – CHOC St. Joseph's Hospital OCTA Offices
54	Local	Garden Grove–Orange	Chapman Ave.	Orange Civic Center Orange Transportation Center The Plaza UCI Medical Center The Block at Orange Rancho Santiago Community College

Table CM-4
Local Bus Routes

Route #	Route Type	Route Service	Service Corridors	Key Orange Activity Centers Served
56	Local	Garden Grove–Orange	Garden Grove Blvd.	Orange Transportation Center OCTA Offices Children's Hospital–CHOC St. Joseph's Hospital
57	Local	Brea–Newport Beach	State College Blvd. / Bristol St.	The Block at Orange UCI Medical Center Theo Lacy Jail Orangewood Children's Home
59	Local	Brea–Irvine	Kraemer Blvd. / Glassell St. / Grand Ave. / Von Karman Ave.	Orange Transportation Center Chapman University Orange Plaza
71	Local	Yorba Linda–Balboa	Tustin St. / Red Hill Ave. / Newport Blvd.	The Village at Orange Lincoln Park and Ride

Source: Orange County Transportation Authority, 2006

Table CM-5
Community, Station Link, Intra- and Inter-County Bus Routes

Route #	Route Type	Route Service	Service Corridors	Key Orange Activity Centers Served
131	Community	Yorba Linda–Orange	Lakeview Ave./ Riverdale Ave. / Tustin St.	The Village at Orange Lincoln Park and Ride
147	Community	Brea–Santa Ana	Raiit St. / Greenville St. / Fairview St.	UCI Medical Center The Block at Orange OCTA Offices St. Joseph's Hospital Children's Hospital – CHOC
167	Community	Anaheim–Irvine	Santiago Blvd. / Hewes St. / Bryan Ave.	The Village at Orange Lincoln Park and Ride
453	Station Link	Orange Transportation Center –St. Joseph's Hospital	Chapman Ave. / Main St. / La Veta Ave.	Orange Transportation Center Children's Hospital–CHOC St. Joseph's Hospital OCTA Offices

Table CM-5
Community, Station Link, Intra- and Inter-County Bus Routes

Route #	Route Type	Route Service	Service Corridors	Key Orange Activity Centers Served
213	Intra County	Brea–Irvine Express	SR-55	Lincoln Park and Ride The Village at Orange
757	Inter County	Diamond Bar –Santa Ana Express	SR-57	UCI Medical Center The Block at Orange
RTA 149	Inter County	Riverside–Orange (Operated by RTA)	SR-91	The Village at Orange Downtown Riverside

Source: Orange County Transportation Authority and Riverside Transit Agency, 2006.

The City recognizes that ridership of both the bus and rail transit systems will increase, and has designed a land use plan that both enables and accommodates increased transit use. A large part of the City's role in accommodating additional transit use includes providing convenient and attractive transit amenities and streetscape features that improve user comfort and perception of safety, thus encouraging transit use. Transit-oriented Development (TOD) design features will be encouraged in major commercial and employment areas within the City, such as the Town and Country Road corridor, South Main Street, Katella Avenue, Uptown Orange, and Old Towne. Such TOD features may consist of streetscape measures such as bus turn-outs, benches, trash receptacles, shelters from wind and rain, and lighting. TOD features may also be more fundamental to the permitted uses and design within projects, such as incorporating child care centers, convenience stores, or personal services within the retail component of mixed-use projects, or near professional office concentrations. These and other measures help to make the transit system more accessible to a wide range of people.

The City will continue to work with OCTA to pursue expanded community circulators, such as the current Station Link service, that will connect people to rail transit, employment centers, residential areas, and commercial corridors. Additional options, such as jitney services that function as group taxis, will also be explored.

The City will also continue to support OCTA initiatives and services that promote the mobility of Orange's senior, disabled, and youth populations. To accommodate the needs of these groups, the City will continue to work with OCTA to offer para-transit services, and will seek ways to improve mobility for Orange youth through transit.

The City of Orange will continue to cooperate with OCTA and other regional providers to establish new bus routes and stops, and to provide transit amenities. New subdivision plans will be reviewed by OCTA to assess impacts on bus services, and to examine the need to provide bus stops or bays. Orange will also work with OCTA to maintain and, if needed, expand successful transfer stations in Old Towne, the Village at Orange, and The Outlets at Orange.

Rail Transit

Many current passenger and commuter fixed-rail transit options in the City will be expanded in the future, offering significant alternatives to automobile transit for many individuals who commute to or from the City for housing or employment. Rail transit now has a proven track record in Orange, and Metrolink seeks to expand rail services in support of land use transformations near current and future rail stations.

Metrolink

Metrolink operates seven commuter rail lines in Southern California, two of which pass through Orange. The Orange County Line offers direct connections to Union Station in Los Angeles to the north and to Oceanside to the south. The Inland Empire–Orange County Line provides direct connections to Riverside/San Bernardino to the east and San Juan Capistrano to the south. Roughly 30 Metrolink trains pass through the City on a daily basis. The Metrolink station in Orange is located three blocks from The Plaza at the OTC, close to a variety of stores and civic uses in Old Towne.

A much-anticipated rail service improvement is intra-county rail service along the County's core transit corridor. This service, which will be provided by Metrolink on behalf of OCTA, is an attempt to provide intra-county trips with a high degree of reliability. This 30-minute service is planned to operate between the Irvine Transportation Center (and possibly Laguna Niguel) and the Fullerton Transportation Center throughout the day to supplement Metrolink's peak period commuter service.

The City recognizes that plans to increase daily service on the current Metrolink lines, improvements to the OTC, and future expansion of Metrolink services to new destinations would be beneficial to Orange by providing multi-modal transportation options for people living and working in the community, thus reducing auto dependence for business and leisure purposes.

Anaheim Regional Transportation Intermodal Center



The Platinum Triangle, which borders Angel Stadium in the City of Anaheim, proposes a variety of high-density multiple-family high-rise housing, office space, and commercial uses. ARTIC is located adjacent to the proposed development, between Angel Stadium and the Honda Center. ARTIC is a major regional transportation center similar in scale to Union Station in Los Angeles and Ontario International Airport in Ontario. The intermodal center is be a stopping point for Amtrak, Metrolink, buses, and future high-speed rail systems that connect to Ontario

Airport, Las Vegas, and the Bay Area.

The City of Orange will coordinate with the City of Anaheim, OCTA, and others to ensure that the City is able to take full advantage of the regional mobility benefits offered by ARTIC. Potential benefits offered to the City by ARTIC include:

- a dynamic mixed-use commercial and residential center at the westernmost end of Katella Avenue; and
- bicycle and pedestrian connections to the Santa Ana River.

California High Speed Rail Corridor

The California High-Speed Rail Authority was established as a state agency in 1996 to direct the planning, design, construction, and operation of a future high-speed train system extending from Sacramento, San Francisco, and Oakland in the north to Los Angeles and San Diego in the south. This high-speed train is proposed to stop at ARTIC. OCTA estimates that by 2025, the California High Speed Rail Corridor will serve between 150,000 and 230,000 passengers each week.

Sidewalks, Trails and Bikeways

Walking and biking contribute to a healthy community, and play increasingly significant roles as alternatives to the automobile. The City recognizes this by providing and maintaining sidewalks, trails, and bikeways to support pedestrians and cyclists.

Pedestrian Facilities

In addition to providing basic transportation routes, sidewalks and pathways offer the opportunity to create appealing public spaces that reflect community pride and invite people to walk. Proposed mixed-use areas and reinvigorated commercial areas throughout the City will provide new and reinvented spaces for people to walk and shop. Walkability and access are essential components of a circulation system that easily and specifically accommodates pedestrians. Features that contribute to walkability include wide sidewalks, safe street crossings, design elements that encourage cautious driving, and a pleasant and safe walking environment. Sidewalks, walkways, well-designed pedestrian crossings, pathways, and pedestrian short-cuts allow people to get from one destination point to another with ease. Dedicated pedestrian paths can provide access between residential and activity areas, especially if streets do not connect. Access strategies for school children, seniors, and people with disabilities should also be incorporated into street and sidewalk plans. The City supports proactive integration of pedestrian improvements and amenities within the circulation system to improve walkability.

The City will create and implement a pedestrian-oriented streetscape master plan addressing key commercial corridors, including Tustin Street, Chapman Avenue, Main Street, Lincoln Avenue, and Katella Avenue. The master plan will address all functional aspects of the pedestrian environment. It will identify pedestrian links that need improvement and

strengthening, determine new pedestrian links to underserved areas, ensure adequate sidewalk widths to accommodate lighting and street trees, develop sidewalks in the industrial area that create links between bus stops, encourage safe routes to schools and recreation facilities, and minimize barriers to pedestrian and bicycle access.

Recreational Trails and Bikeways

A comprehensive network of recreational trails and bikeways greatly benefits Orange residents and visitors by providing popular modes of transportation for recreation. In addition to recreation activities, the City also supports walking and bicycling as viable commute alternatives to the automobile. The City's plan for recreational trails and bikeways is shown in Figure CM-3.

The plan includes trails maintained by the County and private homeowners associations, and is consistent with the OCTA *Commuter Bikeways Strategic Plan*. It is also consistent with the County's major riding and hiking trails and off-road paved bikeways. As described in the ***Vision for the Future,*** the plan will enable the City to connect parks to activity centers and residential areas using a combination of recreational trails and bikeways that truly allows people to travel from place to place within the City without needing an automobile.

Recreational Trails

As shown in Figure CM-3, over 70 miles of existing recreational trails are located within the City, connecting a large number of neighborhoods and community parks. In addition, 104 miles of proposed future trails are planned throughout Orange on land currently utilized for a variety of purposes, including flood control, railroad rights-of-way, and roadways.

The City will use the annual five-year Capital Improvement Program (CIP) process to prioritize, fund, and build proposed trail segments identified in Figure CM-3. These trails have been and will continue to be constructed and administered through cooperative efforts of the City, County, developers, and private homeowner associations.

Desired cross-sections for recreational trails are identified in Figure CM-4. The City may require construction of portions of proposed trails identified in Figure CM-3 as a condition of development approval for projects located adjacent to the proposed trail alignments. Funding for recreational trails and associated restoration projects comes from a variety of federal, state, and regional sources. Priority for funding of trail improvements will be given to projects that complete loops within the system, provide missing links for regional and local trail systems, or serve as destination links to schools, parks, retail businesses, or regional trails.

Priorities for the recreational trail system include:

- trail connections to the Santiago Oaks Regional Park extension west of Cannon Street, consisting of a connection between Calle Grande and Cannon Street, and the Jamestown trail from the Orange Park Acres equestrian arena to Cannon Street;

- trail connection from Serrano Avenue near Fred Barrera Park to Santiago Oaks Regional Park;
- Mabury Ranch Trail connection from the proposed Cerro Villa Park to the Santiago Oaks Regional Park;
- Serrano Avenue connection near Cannon Street; and
- Additional trail connection into Santiago Oaks Regional Park from Orange Park Acres.

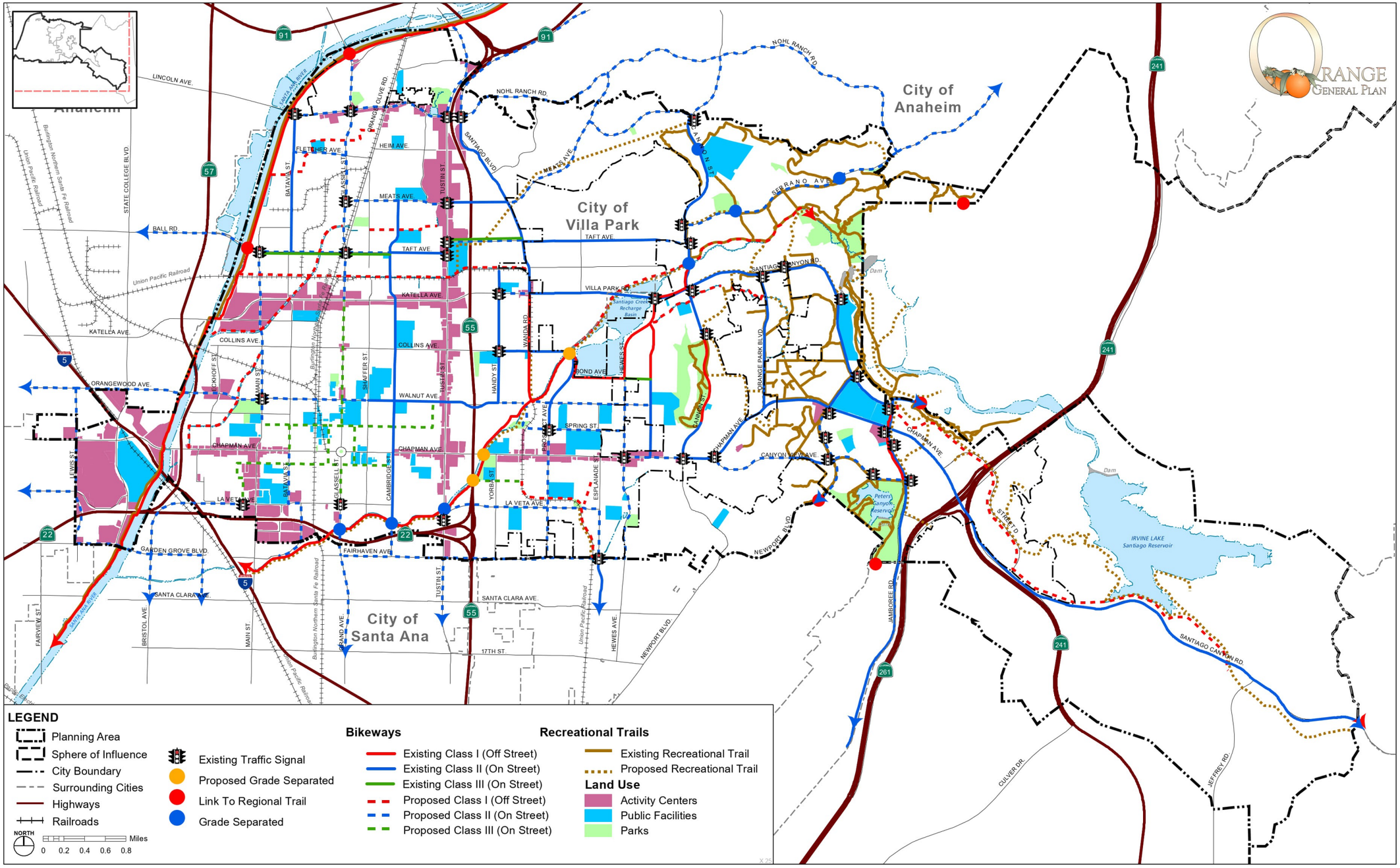
To increase the number of people using non-vehicular means of transportation, the City will encourage putting in place a safe network of crosswalks, grade separations, and walkways to ensure the safety of pedestrians, bicyclists, and equestrians. Where appropriate, traffic calming devices and methods such as median landscaping and provision of bike or transit lanes should be used to slow traffic, improve roadway capacity, and address potential safety issues. The City will continue to work towards improving the overall condition, appearance, and safety of both medians and sidewalks in Orange.

Bikeways

Orange's relatively mild climate permits bicycle riding year-round, and the growing popularity of bicycling has drawn enthusiasts onto the streets and bike trails throughout the City. The plan for recreational trails and bikeways (Figure CM-3) shows the planned system of bikeways within the City. The Orange *Bikeway Master Plan* has established three classes of bicycle routes that adhere to California Department of Transportation (Caltrans) standards:



- **Class I** Off-road bike paths are located on vacated rail lines, water corridors, or areas otherwise separated from streets.
- **Class II** On-road bike lanes are located along arterial roadways that are delineated by painted stripes and other features.
- **Class III** On-road bike routes share use with motor vehicle traffic. They provide a route that is signed but not striped.



Source: City of Orange Community Services Department, 2015.

Figure CM-3 Plan for Recreational Trails and Bikeways

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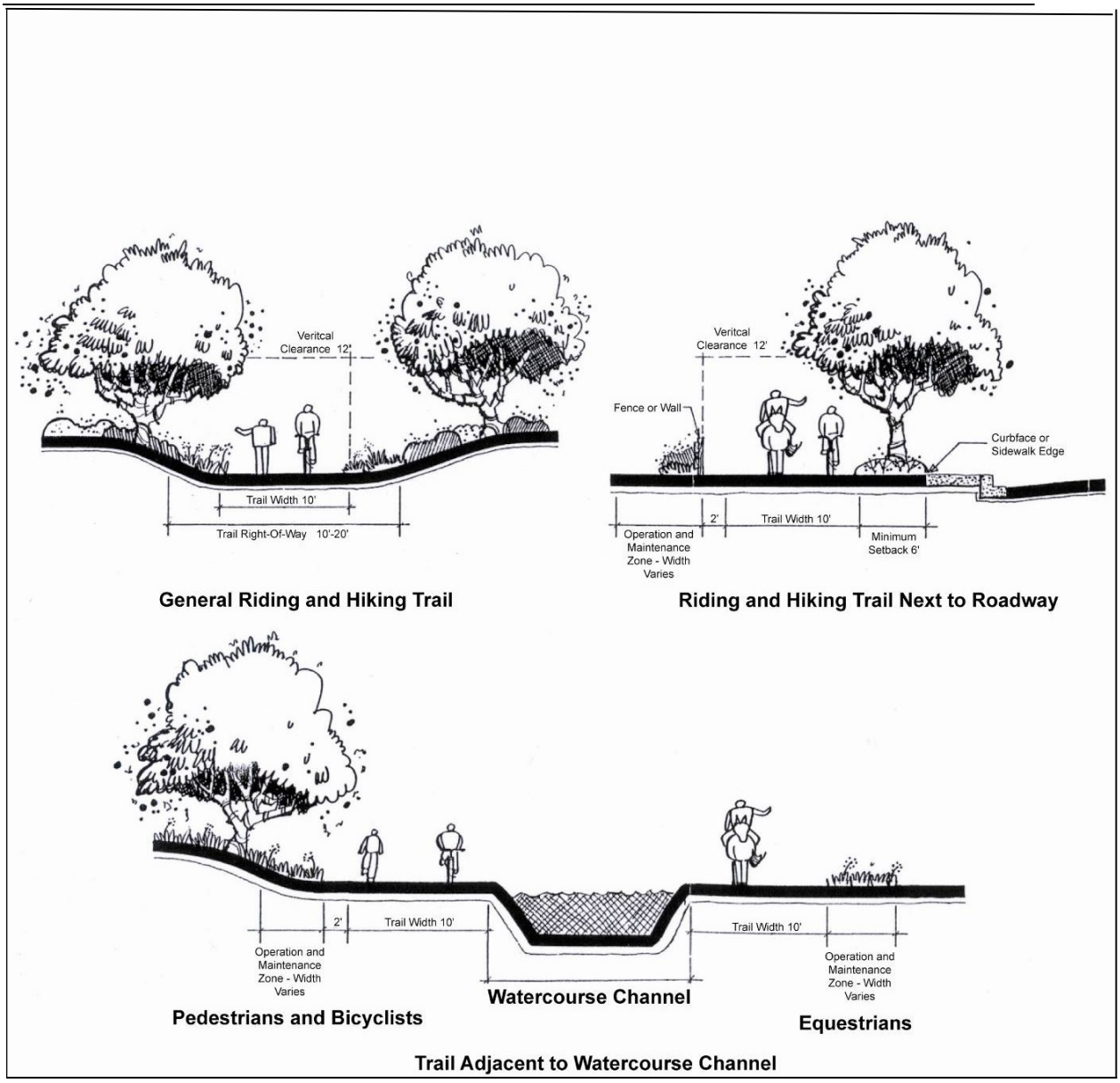


Figure CM-4
Trail Standards

Cross-sections for each type of route are shown in Figure CM-5.

As shown in Figure CM-3, several future bike routes are planned within Orange, including the following high-priority projects:

- Completing Class I bike routes along Santiago Creek and the Tustin Branch Trail
- Establishing Class II bike lanes, including:

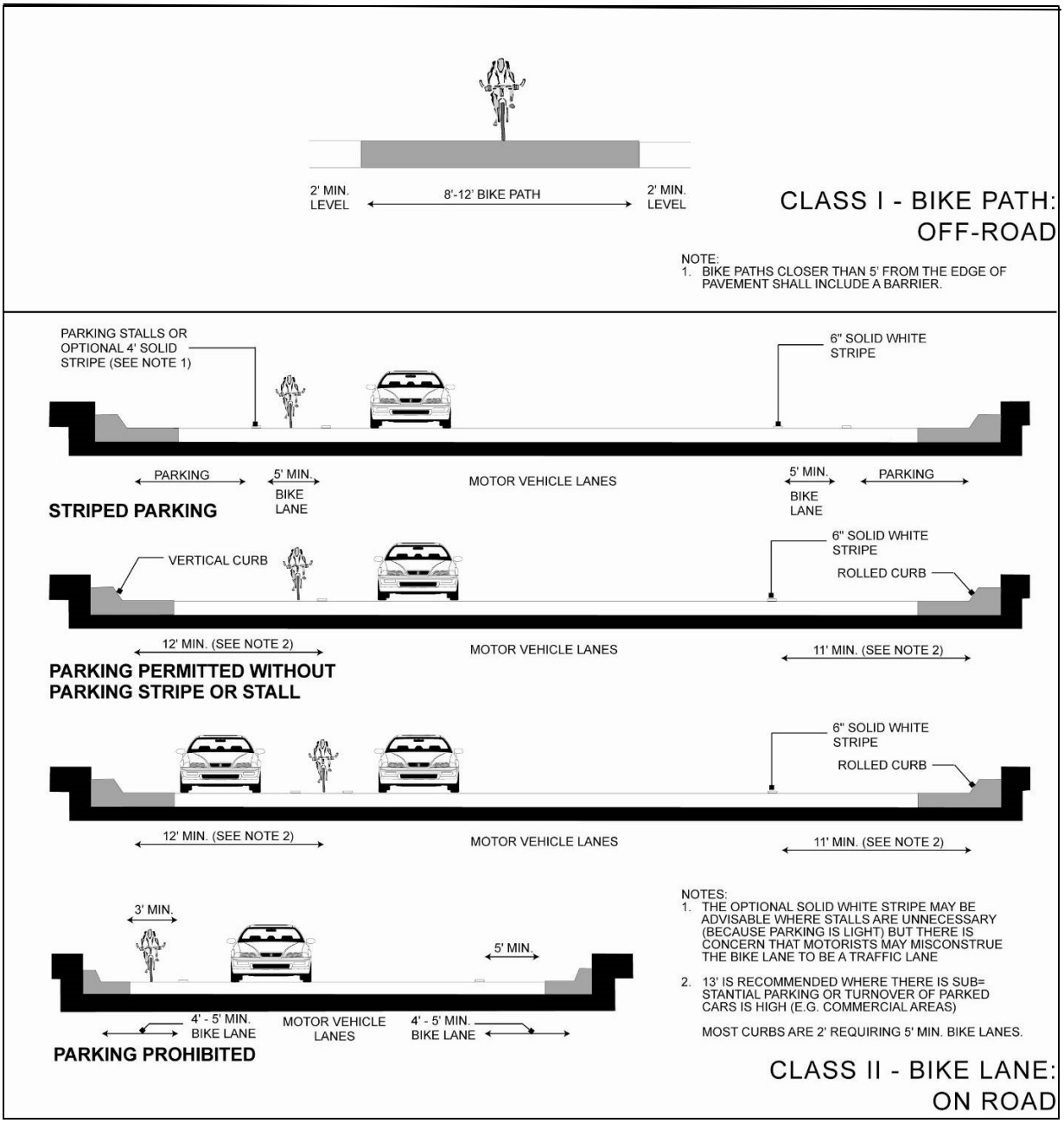


Figure CM-5
Bikeway Standards

- An east/west corridor on Walnut Avenue from the western City limits to Santiago Boulevard
- North/south corridors on Main Street from Taft Street to Palm Street, on Batavia Street from Chapman Avenue to La Veta Avenue, and on Parker Street from La Veta Avenue to the City limits

- Establishing Class III routes along Almond Avenue, Feldner Street, and Bedford Street

The City has completed a paved bike trail along Santiago Creek from Tustin Street to the western City limit that continues on to Main Place Mall and the Discovery Science Center as well as two additional segments to the bike trail, which extends the Santiago Creek Trail from Tustin Street to Grijalva Park, and also connect Grijalva Park at Santiago Creek to Collins Avenue along the City-owned portion of the Tustin Branch Trail right-of-way between Walnut and Collins Avenues. The Santiago Creek trail is planned to extend through and beyond the City, connecting the regional Santa Ana River Trail to Santiago Oaks Regional Park and wilderness areas east of Orange. The City will continue to work towards designing a comprehensive bike trail system that is highly accessible and safe for those who wish to use it.

The City has proposed Class II and III routes along many north-south and east-west arterials, all of which connect to pedestrian trails and Class I routes. The City responds to the need to provide safe and efficient bike travel by making every effort to provide bikeways separate from the roadway. When bicyclists must share the road with automobiles, the City will work to improve overall safety.

Currently, two bicycle parking facilities are located in Orange, at the OTC (Metrolink station) and the Old Towne West Metrolink Parking Structure. The City will work to provide greater bike amenities including delineated bike lanes and clear signage along bike trails. The City will also install bicycle racks and safe storage facilities at parking areas for City facilities, as appropriate, and will require privately developed projects to incorporate on-site bicycle facilities in accordance with the City's Zoning Code (Title 17 of the Orange Municipal Code).

Parking Facilities

Although parking is often considered a separate issue from vehicle circulation, it is important to address on-street parking because it has a direct effect on roadway capacity. In order to facilitate improved traffic flow along Orange's major arterials, the City generally plans to permit on-street parking only on streets classified as Primary or Secondary Arterials or Collector Streets.



Off-street parking deficiencies can reduce business activity, and can cause

vehicles to re-circulate on public streets, which increases traffic volumes and congestion by reducing capacity for through traffic. The City's Zoning Code includes parking requirements to ensure that an adequate number of spaces are provided on-site for most uses. The Code also establishes minimum stall dimensions consistent with current standards for other jurisdictions. These regulations apply to all new developments, and may be applied to current uses that are modified or expanded.

Particular concern has been expressed within the community regarding current and expected future parking shortages in Old Towne. Figure CM-6 identifies the location of existing public parking lots in Old Towne. Most Old Towne parking lots have a maximum time limit of three hours, with or without a permit. However, some parking lots, such as the OTC parking located off Chapman Avenue, have no time limit.

Orange will continue to provide sufficient parking to meet community demands. In Old Towne, the City will study the benefits of creating a parking district and the feasibility of developing underutilized parking lots surrounding The Plaza as public parking facilities. The City may encourage the use of shared parking, consolidated parking facilities, and underground parking or parking structures to provide additional off-street parking to meet future demands in areas throughout the City with higher concentrations of commercial uses.

Circulation System Aesthetics

The City has determined that it is necessary and desirable to improve certain roadways and their rights-of-way to enhance the experience for all users of these corridors. Major commercial corridors have been designated within the Urban Design Element to be improved with pedestrian-scale enhancements. Enhancements could include street trees, sidewalk improvements, lighting, bus shelters, and crosswalks. These improvements not only offer a more aesthetically-pleasing experience for all users in these corridors, they also may offer increased safety and security.

Streetscape improvements offer safety and security to both motorists and non-motorists through increased visual cues, better visibility, and increased activity. Many streetscape enhancements provide not only a comfortable environment for the pedestrian and bicyclist, but also offer drivers visual cues that a non-motorist could be expected in an area. Improvements such as striping, bus shelters, and pedestrian-scaled signage guide the non-motorist towards areas that are most appropriate for their use. These same cues help the motorist to see where the presence of pedestrians and bicyclists is most likely to occur. For example, a well-marked crosswalk guides the pedestrian towards the location in the right-of-way most appropriate for crossing, while also alerting the motorist to slow down and look out for pedestrian traffic.

Visibility offered by pedestrian-scaled lighting benefits non-motorists and motorists alike. Pedestrians and bicyclists are better able to see their way, which increases their feelings of security. Improvements that create a sense of security for pedestrians encourage increased use and activity. This increased activity, in turn, leads to a greater feeling of safety. The City has acknowledged the importance of improving the experience of users on many of its major commercial corridors.



*Note: Possession of City issued parking permit enables holder to exceed specified time limit

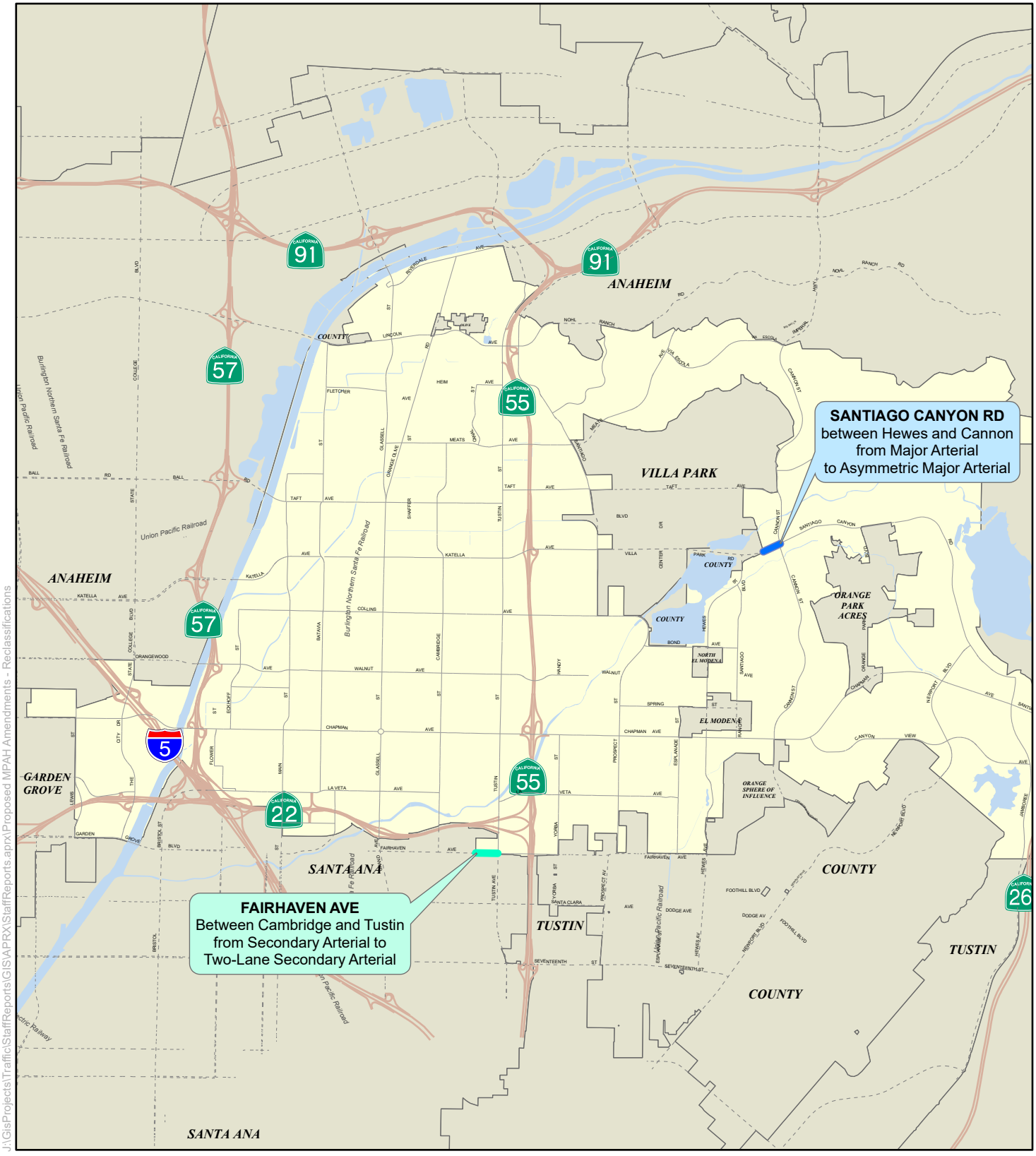
Figure CM-6
Public Parking Lots in Old Towne Orange

*Note: Possession of City issued parking permit enables holder to exceed specified time limit

GPA 2010-0001 (8/10/10)

CIRCULATION & MOBILITY IMPLEMENTATION

The goals, policies, and plans identified in this Element are implemented through a variety of City plans, ordinances, development requirements, capital improvements, and ongoing collaboration with regional agencies and neighboring jurisdictions. Specific implementation measures for this Element are contained in the General Plan Appendix.



J:\GisProjects\Traffic\StaffReports\GIS\APRX\StaffReports.aprx\Proposed MPAH Amendments - Reclassifications

City of Orange
Geographic Information System

Legend

- Reclassify as Asymmetric Major Arterial
- Reclassify as Two-Lane Secondary Arterial

Attachment 1

N
W E
S

0 3,000 6,000 Feet

1/12/2026

Source: City of Orange (2026)

CITY OF ORANGE

PROPOSED CITY MASTER PLAN OF STREETS AND HIGHWAYS AMENDMENT RECLASSIFICATIONS

CIRCULATION & MOBILITY

INTRODUCTION AND VISION FOR THE FUTURE

Orange's circulation system has been influenced by a variety of historical factors, including the presence of the Santa Fe Railroad, the vision of Alfred B. Chapman and William T. Glassell, the agricultural history of the area, and alternative transportation modes including a historic streetcar system.

In 1887, the Santa Fe Railroad came to Orange and built a station four blocks west of the Plaza. The coming of the railroad set off a real estate boom that brought hundreds of settlers to the area. The railroad also influenced the City's early economic success by providing a means to transport goods, especially citrus, to the entire country. Today, the railroad tracks continue to serve freight trains and provide a critical link to the region via the Metrolink heavy rail transit system.

In the 1870s, Alfred B. Chapman and William T. Glassell subdivided their land into residential and small farm lots centered on a roundabout known today as Plaza Park. Plaza Park was dedicated in 1886 and established the City's two main streets – Chapman Avenue and Glassell Street – as well as the compact street grid of Old Towne Orange. The street grid and railroad system were supported historically by a streetcar system that connected the small towns and settlements that make up the City today.

Over time, the small farms on the outer edges of Orange's core district began to disappear. Two factors influenced this change: the demand for housing after World War II and the appearance of "Quick Decline" disease that destroyed the local citrus industry. As each farm was developed independently, the grid system expanded outward and commercial corridors were established. Orange's roadways began to take on a more suburban pattern of collectors, connectors, and arterials. As development reached the eastern portion of the City, the grid gave way to curvilinear street patterns.

The historic roadways and railways that form the basis for the current circulation network have been complemented over the years by the development of a streetcar system, a transit service, an emerging and continually expanding bicycle trail and route network, and routes for equestrian use in the eastern portion of the City. The City will continue to be served by these multiple modes of transportation and other emerging mobility technologies.

Orange's ***Vision for the Future***, described in the General Plan Introduction, recognizes that the circulation system is a key component of the quality of life in the City. Accordingly, the vision includes the following objectives:

- Residential areas will be connected to commercial, recreational, and open space areas, as well as educational and cultural facilities via a balanced, multi-modal circulation network that accommodates vehicles, pedestrians, cyclists, hikers, and equestrians. This network will create additional opportunities for walking and biking, enhancing safety and well-being for neighborhoods and businesses.
- The City will work to define neighborhoods through the use of open space areas and a trail system that provides a source of aesthetic beauty and recreational opportunities. These open space areas support a healthy and active community.
- We will develop a connected multi-modal network for traveling from one end of town to the other that provides the option for residents from different neighborhoods to access parks, open spaces, and scenic areas by vehicle, transit, foot, bicycle or, where appropriate, horse.

Purpose of the Circulation & Mobility Element

California's General Plan Guidelines mandate that the Circulation & Mobility Element fulfill the following objectives:

- Show a direct relationship to the Land Use Element to ensure that any changes to land use as stated by the Land Use Element and growth occur with adequate circulation and transportation facilities in mind.
- Address relevant issues including the adequacy of "major thoroughfares, transportation routes, terminals, other local public utilities and facilities." The goal of the Circulation & Mobility Element is to identify circulation problems related to these facilities in the early stages and resolve them in local goals and policies without costly delays.

Other relevant issues discussed in the Circulation & Mobility Element include those that address streets, highways, public transit routes, railroads, bicycle and pedestrian routes, recreational trails, paratransit, parking, transportation system management, and air pollution. The hierarchy of streets within the residential areas helps to frame the urban form. Connections between neighborhoods can be achieved by a comprehensive network of sidewalks and trails. Also, the commercial corridors can be enhanced with adequate street capacity, public transit, and pedestrian-friendly environments.

The state also recommends that the Circulation & Mobility Element address coordination efforts among the local, regional, and state transportation plans to better resolve circulation issues. Since many transportation concerns are regional, addressing them requires intergovernmental and regional transportation management plans and policy implementation. These partnerships ensure the most efficient use of funding, infrastructure, and other resources. The state also recommends the "preservation of transportation corridors for future system improvements."

The Circulation & Mobility Element prioritizes the issues and opportunities that exist within Orange's transit network. It is directly responsive to proposed changes in land use and anticipates the impacts of those changes. This Element also seeks to reassure residents and businesses that the City recognizes the link between transportation and land uses, and provides a means to mitigate the impacts of growth.

Another goal of the Circulation & Mobility Element is to increase transportation options and provide increased access to the circulation system for all residents of Orange. This goal includes improved rail and bus transit connections and frequency, implementation of a *Bikeway Master Plan,* and completion of a trails system. Transforming many of Orange's historically auto-oriented commercial corridors, such as Katella Avenue, Main Street, and portions of La Veta Avenue, Chapman Avenue, and Glassell Street into more pedestrian-friendly mixed-use environments is an overarching goal. Where possible, the development of equestrian trails is also encouraged.

One of the main functions of the Circulation & Mobility Element is to guide and direct enhancement of the current circulation system for existing and future developments. Thus, circulation provisions correlate with the Land Use Element to avoid unchecked growth and unnecessary congestion.

Another key objective of the Element is to work toward a future circulation network that provides meaningful alternatives for getting around the community by less auto-dependent means. The City's topography, street and sidewalk system, transit and trail framework, and land use relationships provide an excellent foundation for pursuit of this objective.

The Circulation & Mobility Element does not simply determine automobile routes. It also guides the movement of people and goods, directly affecting Orange's physical, social, and economic environment. Since circulation permits accessibility to places and social amenities, it can either improve or cause deterioration in quality of life. Circulation efficiency also plays a major role in progress and development of the City's economy.

Scope and Content of the Circulation & Mobility Element

The Circulation & Mobility Element comprises three sections:

(1) Introduction;
(2) Issues, Goals, and Policies; and
(3) The Circulation & Mobility Plan.

The first section introduces the contents of the Circulation & Mobility Element. The second section presents issues, goals, and policies for improving circulation. The third section includes the Circulation & Mobility Plan, which designates locations and standards for roadways and non-motorized circulation facilities, and states the community's desired level of transportation service.

Implementation measures designed to promote achievement of the goals and policies are provided in an Appendix to the General Plan.

Relationship to other General Plan Elements

California planning law requires that the Circulation & Mobility Element correlate and maintain consistency with the other General Plan elements. The Circulation & Mobility Element relates most closely to the Land Use, Natural Resources, Noise, Economic Development, Housing, and Urban Design Elements.

The Circulation & Mobility Element is linked to the Land Use Element because the General Plan land use designations identified in the Land Use Element serve as a basis for the allocation of vehicle trips and establishment of capacity levels for circulation planning. The Land Use Element also provides land use designations that accommodate mixed commercial and residential development, which encourage shorter trips and improve the efficiency of the transportation network. The Circulation & Mobility Plan is established to define and provide for adequate levels of service and facilities to support future land uses. This Element recommends roadway and intersection improvements that may require land acquisition. Location of public transportation facilities will also influence pedestrian activity and transit-oriented development, and the physical size of streets will affect urban land uses and the physical appearance of the City.

The Natural Resources Element identifies regional air quality objectives and provides appropriate mitigation efforts that affect the Circulation & Mobility Element. Improving access, encouraging alternative modes of travel, and maintaining air quality and conservation standards are common objectives of the Natural Resources and the Circulation & Mobility Elements.

The Noise Element addresses future noise levels associated with roadways, rail, and other transportation facilities. Future volumes of traffic on the circulation system are directly related to future noise levels and mitigation strategies.

The Economic Development Element identifies desirable economic conditions and land uses that enhance and promote business activity, employment growth, and economic stability. The goals and policies of the Circulation & Mobility Element will determine road capacity in Orange, which will impact the type and location of uses, and parking and access considerations associated with future uses. Both elements share a common objective of planning for future transportation infrastructure needs. Maintaining roadways, bikeways and bus and rail transit facilities is critical to the success of both current and future businesses in Orange.

The Urban Design Element is a framework for shaping the future form and character of Orange. The quality of Orange's physical environment contributes to its identity, attracts new residents, and sets the stage for economic activity. The Urban Design Element builds on the foundation of Orange's already strong sense of place to preserve and strengthen the streetscape environment of commercial corridors and landmarks within the city. The Urban Design Element and Circulation & Mobility Element share a common objective to reinvent City streets as more functional and walkable public places.

ISSUES, GOALS, AND POLICIES

The goals, policies, and implementation programs of the Circulation & Mobility Element seek to achieve a better balance between vehicular, pedestrian, and bicycle travel, and to provide a wide range of viable transportation options to Orange residents. The following six issues are addressed: (1) enhancing the local circulation system; (2) maintaining the regional circulation system; (3) maintaining a viable public transportation network; (4) creating a comprehensive system of sidewalks, trails, and bikeways; (5) providing adequate parking facilities; and (6) improving circulation system aesthetics and safety.

Local Circulation System

The local roadway system serves the community's primary needs for mobility and access, and consists of a hierarchy of City streets to meet those needs. The City's original street system was established as a grid pattern long before a Master Plan of Arterial Highways (MPAH) was adopted. The Old Towne area and many postwar neighborhoods were designed in a classic grid configuration, while in newer parts of the City, physical features such as the Santa Ana River, Santiago Creek, hilly terrain, freeways and the presence of the City of Villa Park have resulted in a system without a definitive pattern. Some major roads do not connect the eastern and western portions of the City and consequently do not provide effective through circulation. A well-designed roadway system will provide convenient access to activities in Orange.

GOAL 1.0: **Provide a safe, efficient, and comprehensive circulation system that serves local needs, meets forecasted demands, and sustains quality of life in neighborhoods.**

Policy 1.1: Plan, build, and maintain an integrated, hierarchical, and multi-modal system of roadways, pedestrian walkways, and bicycle paths throughout the City.

Policy 1.2: Identify key intersections and streets with historical or projected traffic congestion problems and apply creative traffic management measures to improve overall circulation.

Policy 1.3: Consider various methods to increase safety on City arterials and neighborhood streets, including landscaping, provision of bike/transit lanes, and consideration of traffic calming on neighborhood streets in accordance with the City's Neighborhood Residential Traffic Management Program.

Policy 1.4: Prohibit on-street parking where possible to reduce bicycle/automobile conflicts in appropriate target areas as recommended by the *Bikeways Master Plan*.

Policy 1.5: Address possible safety and noise effects of increased rail activity on grade crossings throughout the City.

Policy 1.6: Maintain and repair roadways and sidewalks as necessary to improve circulation and safety.

Policy 1.7: Consolidate driveways along roadways that provide access to commercial uses to minimize side street interruption and promote smooth traffic flows. On-street parking is prohibited on commercial access streets to provide adequate curb-to-curb width for travel lanes.

Regional Circulation System

Mobility in Orange is directly related to the regional transportation network, as the City lies at the confluence of several regional freeways: the Santa Ana Freeway (Interstate 5), Orange Freeway (State Route [SR] 57), Garden Grove Freeway (SR-22), Costa Mesa Freeway (SR-55), Riverside Freeway (SR-91) and Eastern Transportation Corridor (SR-241). In addition to the freeways, other connections to the region include the commuter rail system known as Metrolink, a freight and goods rail transport system, and a regional bikeways system with connections to the Santa Ana River and other locations. Orange is also connected to the region via the Orange County Transportation Authority (OCTA) bus system. City infrastructure must accommodate regional through traffic originating in other communities in addition to providing local residents access to the regional network.

GOAL 2.0: Provide an effective regional transportation network.

Policy 2.1: Ensure consistency with the County MPAH in order to qualify for funding programs.

Policy 2.2: Coordinate with adjacent cities to plan and develop major east/west and north/south arterials and rapid transit to connect the City with the cities of Anaheim, Tustin, Santa Ana, Garden Grove, and Villa Park, as well as developing areas within the City's sphere of influence.

Policy 2.3: Cooperate with and support local and regional agencies' efforts to improve regional arterials and transit in order to address increasing traffic congestion.

Policy 2.4: Coordinate land use planning with anticipated future development of roadways and other transportation facility improvements as well as the expansion of commuter rail and bus service.

Policy 2.5: Ensure that transportation facilities and improvements do not degrade the quality of Orange's commercial and residential areas.

Policy 2.6: Encourage the use of regional rail, transit, bicycling, carpools, and vanpools for work trips to relieve traffic congestion.

Policy 2.7: Continue to support the use of rail corridors within the City for the movement of freight and goods, and work with rail operators to minimize associated traffic delays.

Public Transportation

Public transportation is a crucial component of a comprehensive circulation system. In addition to reducing air pollution and traffic congestion, a successful public transit system provides an alternative mode of travel for those with limited mobility, residents who may not have access to a car, and persons who choose not to drive.

GOAL 3.0: **Connect centers within the City to each other and to the region through efficient and accessible public transportation.**

Policy 3.1: Work with OCTA and other agencies to assess City public transportation needs and to ensure delivery of services when and where they are needed.

Policy 3.2: Enhance and encourage provision of convenient and attractive transit amenities and streetscapes to encourage use of public transportation (e.g., benches, trash cans, shelters, and lighting).

Policy 3.3: Require incorporation of transit-oriented design features within major commercial and employment areas as well as in medium density residential and mixed-use development areas.

Sidewalks, Trails, and Bikeways

In addition to offering recreational and public health benefits, non-vehicular modes of transportation offer commuting options. Also, the mixed-use environments advocated by Land Use Element policies will encourage increased pedestrian activity on City sidewalks for both business and pleasure. An effective pedestrian, bicycle, and equestrian network must be safe and accessible, and must connect key activity centers within the City with each other and with the regional trail system. A comprehensive network of on-street bicycle lanes, off-street bicycle paths, sidewalks, and trails should be developed and maintained to increase the safety and utility of the system, with a particular focus on the City's sidewalk deficient industrial areas

GOAL 4.0: **Provide efficient and accessible modes of pedestrian, bicycle, and equestrian transportation and improved facilities and amenities.**

Policy 4.1: Create a comprehensive bicycle network that is integrated with other transportation systems by establishing complementary on-street and off-street facilities as identified in the City of Orange *Bikeways Master Plan* and OCTA *Commuter Bikeways Strategic Plan*, including Santiago Creek, the Santa Ana River, and the Tustin Branch Trail.

Policy 4.2: Install racks and safe storage facilities at parking areas for City facilities, as appropriate, and encourage incorporation of such facilities within privately-developed projects.

Policy 4.3: Improve citywide awareness of automobile and bicycle safety.

Policy 4.4: Encourage use of the bikeway system by providing adequate signage, trail markings, and other amenities.

Policy 4.5: Ensure that pedestrian sidewalks, trails, and bikeways are safe environments through the use of crime prevention-oriented trail design features, lighting where appropriate, pedestrian and bicycle safety improvements at at-grade rail crossings, access for emergency vehicles, and links to the roadway signal system.

Policy 4.6: Explore opportunities to convert abandoned rail corridors into segments of the City's bikeway and pedestrian trail system.

Policy 4.7: Provide ADA accessible sidewalks and pedestrian amenities throughout the City.

Policy 4.8: Expand and maintain an equestrian trail network and provide for appropriate staging areas and infrastructure.

Parking Facilities

A shortage of parking can cause circulation problems and could lead to a reduction or loss of business activity. Old Towne Orange has been identified as an area of particular concern. As the City develops, providing adequate parking adjacent to other activity centers is increasingly important.

GOAL 5.0: Provide adequate parking to meet the needs of activity centers throughout the City.

Policy 5.1: Provide adequate parking to protect and support the economic vitality and diversity of Old Towne.

Policy 5.2: Plan for and design parking facilities throughout the City that are adequate to meet demand, but also consider land use-parking efficiencies, and the surrounding natural and built environment.

Policy 5.3: Encourage adjacent businesses to consolidate parking facilities and access points.

Policy 5.4: Encourage well-designed structured parking in commercial areas where such features would be economically feasible, safe, and visually integrated with existing development.

Circulation System Aesthetics

Streets that have been made or modified to include visual and pedestrian amenities can improve the overall look and feel of City streets, as well as enhancing functionality for all users. As major commercial corridors are beautified and changed to include a pleasant pedestrian environment, this will have positive effects on the feelings of safety and security for pedestrians, bicyclists, and motorists.

GOAL 6.0: Provide roadway corridors that are aesthetically pleasing and contribute to a feeling of safety, security, and comfort for motorists, bicyclists, and pedestrians.

Policy 6.1: Supply adequate, clear, and correctly placed signage to direct both motorists and non-motorists toward destinations and away from hazards.

Policy 6.2: Provide clear indicators in the right-of-way for where pedestrians and bicyclists are encouraged to walk, bike, or cross safely. These may include special paving, line stripes, and crosswalks.

Policy 6.3: Provide lighting, landscaping, street trees, and other appropriately scaled streetscape features that accommodate all users on commercial corridors. Where appropriate, lighting should be scaled for autos as well as pedestrians.

CIRCULATION & MOBILITY PLAN

The objective of the Circulation & Mobility Plan is to document existing transportation facilities in the planning area used for the movement of people and goods. The Element addresses the desired future condition of these facilities, and their relationship to future land uses identified in the Land Use Element. The Plan describes the circulation system, including the arterial network and intersections, the public transit system, bicycle paths, recreation facilities, parking, and railroad operations. The City's circulation network includes an extensive system of roadways, bus transit service, commuter rail, and freight rail.

Local Circulation System

A well-designed local arterial roadway system that connects to a well-developed regional circulation system provides safe and convenient access to employment, housing, recreation, and commercial areas in Orange. City arterial roadways located on the western side of SR-55 generally follow north-south and east-west orientations. On the eastern side of the freeway, arterials are characterized by curvilinear streets due to undulating geographical surroundings. Key north-south arterials include Tustin Street, Glassell Street, Main Street, and The City Drive. Key east-west arterials include Chapman Avenue, Katella Avenue, Taft Avenue, and parts of La Veta Avenue. These arterial roadways are in turn supported by a network of collector and local streets that provide access to homes and businesses throughout the City.

Roadway Classification System

The City's roadway network is distinguished by a hierarchical classification system that differentiates roads by size, function, and approximate daily capacity based upon Level of Service D (LOS D). LOS is a qualitative measure that characterizes traffic congestion on a scale of A to F with LOS A representing a free-flow condition and LOS F representing extreme congestion. LOS standards can apply to either intersections or links (a section of street between two intersections). Generally speaking, LOS represents the ability of a roadway or an intersection to accommodate traffic.

In the City, intersections are used as actual control points. City roadways consist of both divided and undivided roadways. Divided roadways generally contain a physical barrier or buffer, such as a raised median or a continuous two-way left turn lane, between each direction of travel. Divided roadways remove vehicles making a left turn from the travel lanes so as not to impede through traffic and constrict roadway capacity. Undivided roadways do not contain a buffer between each direction of travel, and therefore left-turning traffic can impede through traffic. Undivided roadways may provide turn movement pockets at intersections. The ~~six~~ eight categories of roadways in Orange are summarized in Table CM-1. Proposed cross-sections for each type of roadway are shown in Figure CM-1.

The City's policy is to use a link capacity standard of LOS D. The following paragraphs represent link capacities of each roadway type at LOS D.

Table CM-1 Roadway Classifications		
Classification	**Facility Type**	**Characteristics**
Smart Street	Smart Street	4-8 lane divided, with possible signal coordination, intersection capacity improvements and/or grade separations
Principal Arterial	8 Lane Divided	Primarily serves through traffic with limited local access
Major Arterial	6 Lane Divided	Serves mostly through traffic with some local access allowed
Asymmetric Major Arterial	5 Lane Divided	Serves mostly through traffic with some local access allowed
Primary Arterial	4 Lane Divided	Serves through and local traffic
Secondary Arterial	4 Lane Undivided	Serves through and local traffic
Two-Lane Secondary Arterial	2 Lane Divided	Serves through and local traffic
Collector Street	2 Lane	Serves mostly local traffic

Smart Streets are typically four- to eight-lane roadways with enhanced capacity and smoother traffic flow than standard arterial streets. These streets have enhanced features such as traffic signal synchronization, bus bays, intersection improvements, and the addition of travel lanes by removing on-street parking and consolidating driveways. The traffic carrying capacities of Smart Streets can range from 60,000 to 79,000 vehicles per day, depending on the number of lanes, degree of access control, peak period loading, and the configurations of major intersections.

Principal Arterials are typically eight-lane divided roadways with medians or continuous two-way left turn lanes. They can accommodate up to 67,500 vehicles on an average weekday at LOS D conditions, depending on the degree of access control, peak period traffic loadings, and lane configurations at major intersections. Principal arterials prohibit on-street, curbside parking, and connect directly to freeways.

Major Arterials are six-lane divided roadways with medians or continuous two-way left turn lanes. They can accommodate up to 50,700 vehicles on an average weekday at LOS D conditions, depending on the degree of access control, peak period traffic loadings, and lane configurations at major intersections. Major arterials facilitate traffic circulation within Orange, and may prohibit on-street, curbside parking.

Asymmetric Major Arterial are five-lane asymmetrical divided roadways with medians or continuous two-way left turn lanes. They can accommodate up to 42,250 vehicles on an average weekday at LOS D conditions, depending on the degree of access control, peak period traffic loadings, and lane configurations at major intersections. Asymmetric major arterials facilitate traffic circulation within Orange, and may prohibit on-street, curbside parking. Occasionally, based on projected volumes, it is necessary to have a cross section with more lanes in one direction than the other (asymmetric lanes) along some Major arterials. The most typical application would be for arterial segments where adjacent area land uses result in higher demand in one of the peak periods. Where such demand creates

the need for an added lane to accommodate the peak surge of traffic, the addition of through lane(s) in one (asymmetric), rather than both directions of travel, is considered.

Primary Arterials are four-lane divided roadways with medians or continuous two-way left turn lanes. They can accommodate up to 33,750 vehicles on an average weekday at LOS D conditions, depending on the degree of access control and peak period loadings. Primary Arterials provide for easy circulation in the City, and allow for limited on-street, curbside parking.

Secondary Arterials are four-lane undivided roadways without medians. They can accommodate up to 21,600 vehicles on an average weekday at LOS D conditions, depending on the degree of access control and peak period loadings. Secondary arterials allow for on-street, curbside parking.

Two-Lane Secondary Arterials are two-lane divided roadways with medians or continuous two-way left turn lanes. They can accommodate up to 17,000 vehicles per average weekday at LOS D conditions, depending on the degree of access control and peak period loadings. Two-lane secondary arterials can allow for on-street, curbside parking.

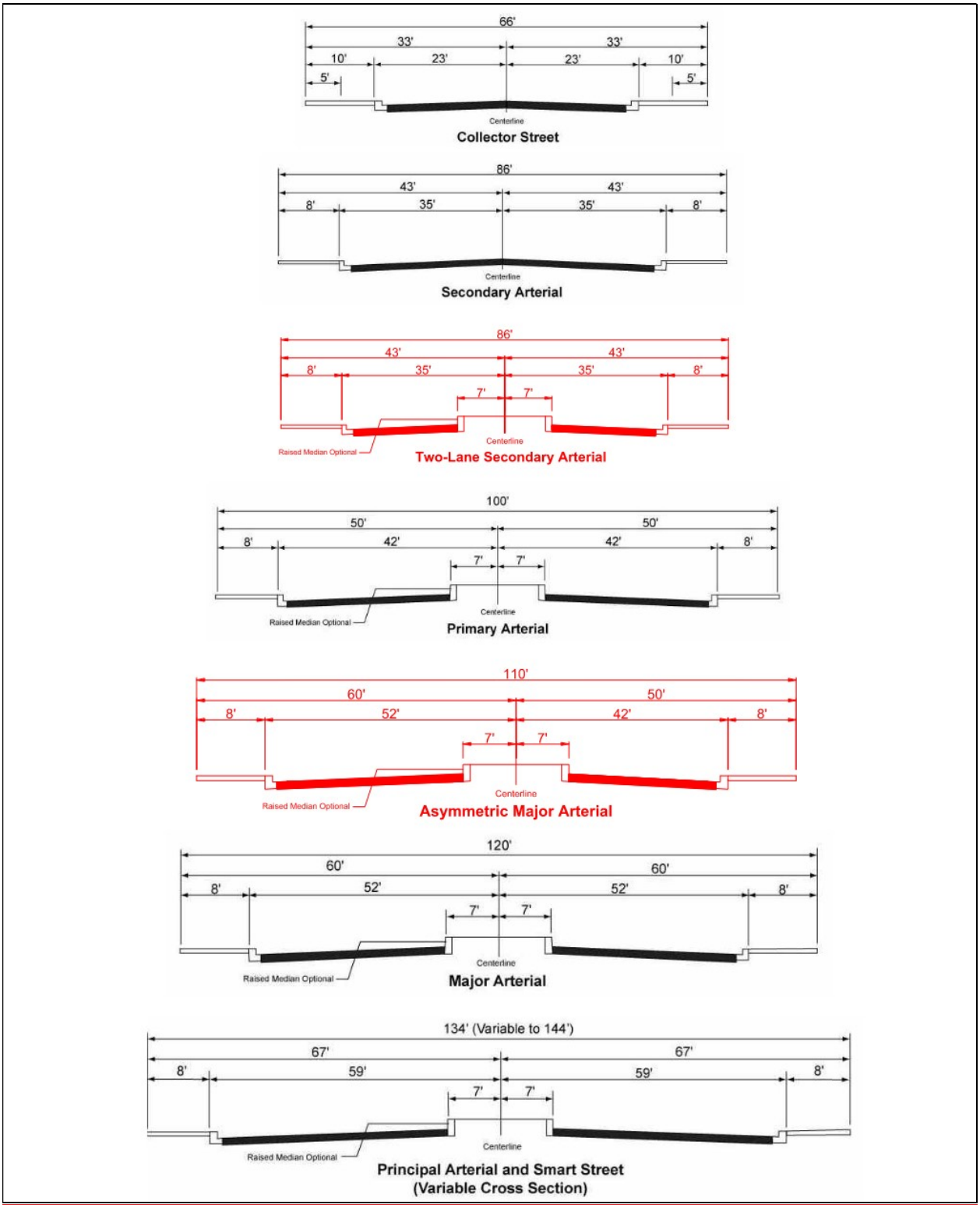


Figure CM-1
Roadway Cross Sections

Collector Streets are typically two-lane roadways without medians that gather and distribute traffic to higher-capacity arterials. They can accommodate up to 10,800 vehicles per average weekday at LOS D conditions, depending on the degree of access control and peak period traffic loadings. Centerline striping is typically not provided on collector streets, and on-street parking is allowed. There are several types of two-lane streets in the City, including divided, undivided, residential, and collector streets. Each type serves a slightly different purpose and may have different capacity thresholds based on various factors.

Performance Criteria

Evaluating the ability of the circulation system to serve residents and businesses in Orange requires establishing performance criteria. Performance criteria have a policy component that establishes a desired LOS, and a technical component that specifies how traffic forecast data can be used to measure criteria achievement.

The LOS definition for intersections is based on a volume-to-capacity (V/C) ratio and provides a more quantitative description of traffic conditions. Table CM-2 presents LOS based on traffic volumes and the design capacity of intersections.

Table CM-2
Level of Service Definitions for Intersections

Level of Service	Volume-to-Capacity Ratio	Description
A	0.00-0.60	Free Flow/Insignificant Delays: No approach phase is fully utilized by traffic and no vehicle waits longer than one red indication.
B	0.61-0.70	Stable Operation/Minimal Delays: An occasional approach phase is fully utilized. Many drivers feel somewhat restricted within platoons of vehicles.
C	0.71-0.80	Stable Operation/Acceptable Delays: Major approach phases fully utilized. Most drivers feel somewhat restricted.
D	0.81-0.90	Approaching Unstable/Tolerable Delays: Drivers may have to wait through more than one red signal indication. Queues may develop but dissipate rapidly, without excessive delays.
E	0.91-1.00	Unstable Operation/Significant Delays: Volumes at or near capacity. Vehicles may wait through several signal cycles. Long queues form upstream from intersection.
F	N/A	Forced Flow/Excessive Delays: Represents jammed conditions. Intersection operates below capacity with low volumes. Queues may block upstream intersections.

Source: Highway Capacity Manual, Transportation Research Board, Special Report No. 209, Washington DC, 2000.

Although roadway capacity is generally a function of peak hour intersection performance and the corresponding peak hour volumes, daily arterial segment capacities (link capacities) also provide a measure of the overall LOS of the arterial system. Generally, traffic impact mitigation focuses on peak hour intersection performance, since system performance is typically a function of intersection performance. The City's policy is to use a link capacity standard of LOS D. Table CM-3 presents arterial daily capacities at LOS D and LOS E.

Table CM-3
Arterial Daily Capacity Threshold Assumptions

Street Type	Daily Capacity	
	LOS D	LOS E
Smart Street – 6- to 8-lane divided	71,100	79,000
Principal – 8-lane divided	67,500	75,000
Major – 6-lane divided	50,700	56,300
Asymmetric Major – 5-lane divided	42,250	47,000
Primary – 4-lane divided	33,750	37,500
Secondary – 4- lane undivided	21,600	24,000
Two-Lane Secondary – 2-lane divided	17,000	18,750
Collector – 2-lane undivided	10,800	12,000

Source: City of Orange General Plan Update Traffic Report, 2008.

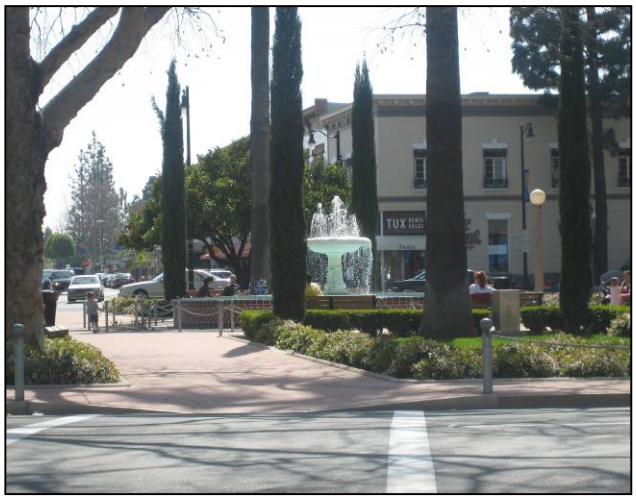
Various LOS policy standards have been established to evaluate observed traffic conditions, future development plans, and circulation system modifications. At the local level, the City of Orange has established LOS D as the lowest acceptable level of service for both roadway segments and peak-hour signalized intersection movements. At the regional planning level, Orange County's Congestion Management Plan (CMP) specifies LOS E as the operating standard for roadways and intersections on the CMP highway system. The CMP Highway System consists of the Orange County smart street network plus the state highway system. Thus, the SR-55 northbound and southbound ramps at Katella Avenue are CMP intersections within the City's jurisdiction. The City does not have an adopted LOS standard for unsignalized intersections. Performance of unsignalized intersections is evaluated on a case-by-case basis.

In addition to the LOS standards for roadways and intersections, the California Environmental Quality Act (CEQA) Guidelines (Section 15064.3), require "vehicle miles traveled" (VMT), to measure transportation impacts on the community. VMT refers to the amount and distance of automobile travel attributable to a project.

For purposes of compliance with CEQA, a significant impact would occur if the baseline and/or cumulative project-generated VMT per service population (population plus employment) exceeds the anticipated City of Orange General Plan Buildout VMT per service population. One vehicle traveling one mile would generate one VMT. Additionally, the project's effect on VMT would be considered significant if it resulted in the baseline and/or cumulative link-level boundary citywide VMT per service population increases under the plus project condition compared to the no project condition. Link-level boundary VMT is calculated by summing all weekday VMT on a roadway network within the City boundary and includes all trips including trips that pass through the City's roadway network but do not start and end with in City.

These thresholds are designed to reduce the number of miles traveled by automobiles, and are implemented within the City of Orange *Traffic Impact Analysis Guidelines*.

In order to maximize the efficiency of its circulation system, the City will look at where physical improvements to the circulation infrastructure can be made to expand capacity and increase traffic flow. To maximize efficiency of the road system, the City will support traffic signal coordination and spacing, and will also discourage on-street parking along arterials. In addition, the City will explore ways to reduce the demand for vehicular transportation, specifically through the provision and maintenance of bike lanes, bikeways, and trails, and will also encourage additional regional transit services and support facilities. The City's Transportation Demand Management (TDM) ordinance (Chapter 10.83 of the Municipal Code) further specifies a variety of techniques available to employers with 100 or more employees to advance the goals of efficiently utilizing the existing and planned transportation system and reducing vehicle emissions.



City Master Plan of Streets and Highways

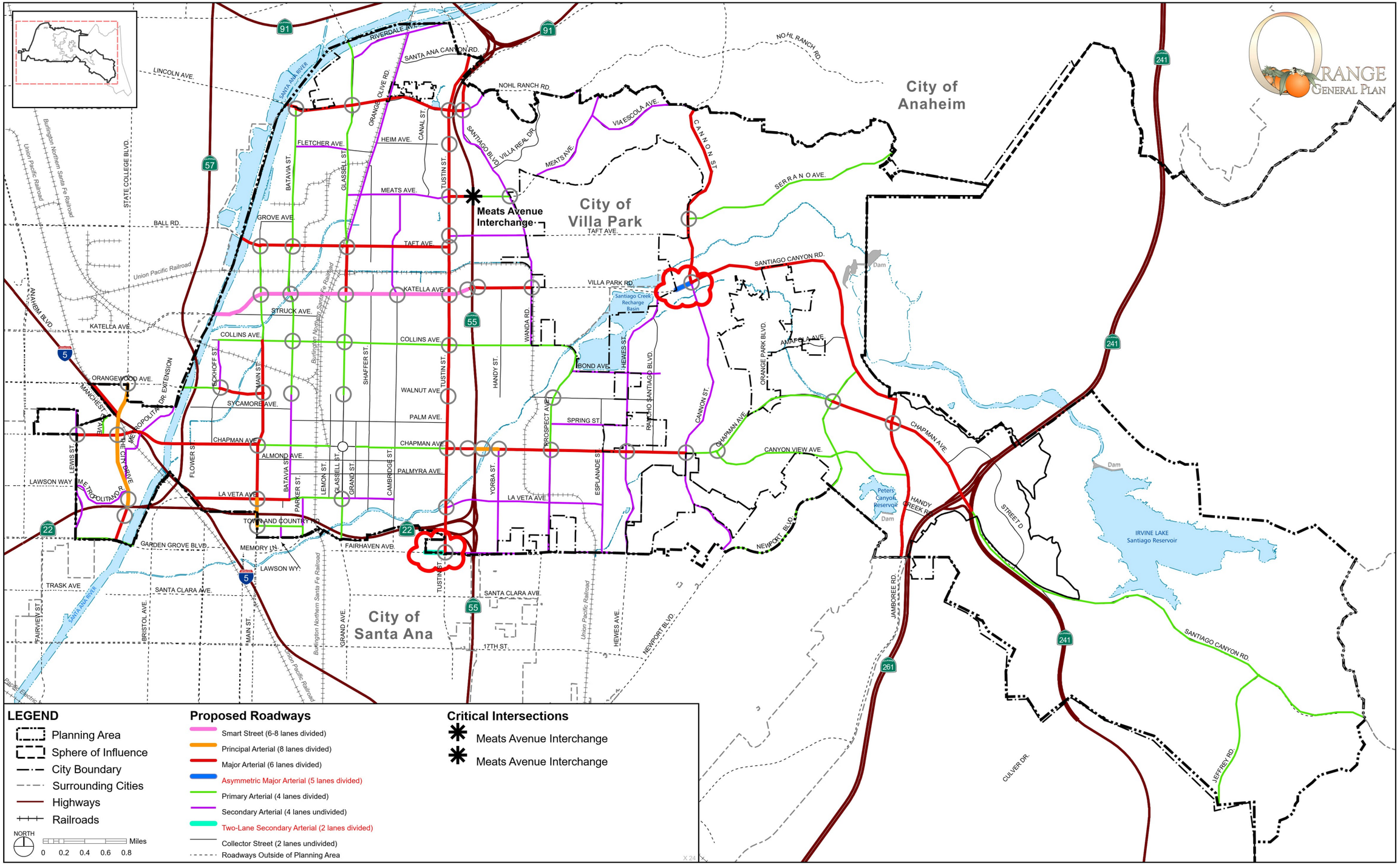
Land Use Element policy will allow land use changes and intensification to occur in specific focus areas within the City. The City's Master Plan of Streets and Highways displayed in Figure CM-2 has been developed in close coordination with land use policy to ensure that traffic generated by new development will not compromise the City's goal to ensure that intersections and roadway segments operate efficiently. The map identifies components of the City's roadway circulation system. The map also indicates where augmented roads are needed, and pinpoints locations for enhanced intersections, including the future Meats Avenue interchange at SR-55. Although most of Orange is already built out, most remaining developable land is located in the eastern part of the City. New development in east Orange will require construction of new roads to provide circulation and traffic flow to residents and businesses. Land Use Element policies enabling reuse and redevelopment within established portions of the City, particularly within the focus areas, may also necessitate roadway widening and intersection enhancements. The City will continue to collect funds for necessary circulation system capital improvements through a program that sets up a fee structure for all new development and redevelopment projects. This program will require developers to pay their fair share for transportation system improvements required by new projects. The City will use the annual seven-year Capital Improvement Program (CIP) process to prioritize, fund, and complete improvements required to achieve build-out of the proposed roadway system identified in Figure CM-2.

Roadway Widening

Roadway widening in specific locations will be necessary to obtain new travel lanes. Additional travel lanes may be acquired either by obtaining additional rights-of-way as necessary or by constructing new lanes within existing rights-of-way. Parking restrictions may be applied to allow additional lanes to be provided within existing rights-of-way.

Old Towne Street Network

The Old Towne street network is a clear example of Orange's grid street pattern. Parallel roadways have been established in both the north-south and east-west direction to distribute traffic evenly. The Plaza area at Glassell Street and Chapman Avenue is a unique feature that creates discontinuous traffic flows along these two primary roadways. However, no plans have been made to modify the National Register-listed Historic Plaza to increase its traffic carrying capacity.



(Redlined) Figure CM-2 City Master Plan of Streets and Highways

GPA 2026-0001 (XX/XX/26)

GPA 2014-0001 (12/8/15)

GPA 2010-0001 (8/10/10)

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In light of these conditions, parallel roadways such as Almond Avenue, Palmyra Avenue, Lemon Street, Olive Street, Shaffer Street, Grand Street, Palm Avenue, and Maple Avenue will continue to serve as local collectors around The Plaza.

Metropolitan Drive Extension

Extending Metropolitan Drive behind the University of California, Irvine (UCI) Medical Center will help facilitate the movement of north-south traffic near The Outlets at Orange shopping area, improve access to Interstate 5 (I-5), and relieve congestion on The City Drive. Metropolitan Drive will be extended from The City Drive/State College Boulevard to the Metropolitan Drive/Rampart Street connection.

Critical Intersection Program

Intersections serve as traffic control points for the circulation system, regulating the flow of vehicles along City streets and sometimes limiting the capacity of the system. In the long term, system capacity and efficiency can both be increased if intersections are designed to handle future anticipated traffic volumes. Typically, the design of the roadways forming an intersection dictates the intersection configuration. Department of Public Works standards indicate that a left-turn pocket may or may not be provided, depending on traffic volumes through the intersection. However, one pocket may not be adequate to handle vehicles during peak hours. Traffic may back up into a through travel lane, resulting in congestion at the intersection and at other locations along the roadway.

One way of providing additional intersection capacity at critical locations is through the use of special intersection configurations known as "critical intersections." Critical intersections deviate from typical City design standards by increasing the number of lanes at an intersection beyond what typically would be required. By increasing capacity at the intersection, the circulation link increases overall system capacity.

The Master Plan of Streets and Highways (Figure CM-2) identifies the locations of critical intersections within Orange. A list of these intersections and diagrams depicting their geometries are on file in the Public Works Department.

Regional Roadway System

The City's local circulation network is connected to an efficient regional circulation system. Figure CM-2 shows the freeways that traverse the Orange planning area. The Santa Ana Freeway (I-5) provides interstate and regional access to the City. In addition, SR-57, SR-55, and SR-22 all provide connections to the City from northern Orange County and neighboring Los Angeles County, San Diego County, Riverside County, and San Bernardino County. SR-91 and SR-241 provide additional, more limited freeway access. SR-241 is a toll facility controlled by the Transportation Corridor Agency (TCA).

I-5 is a northwest-southeast freeway that passes through the southwest corner of the City, and provides direct access to Los Angeles County to the north and San Diego County to the south. I-5 has two interchanges within Orange—one located at its junction with SR-57 and SR-22 (commonly known as the Orange Crush) and the other at State College Boulevard/The City Drive. The junction at the Orange Crush currently has the most severe congestion, which directly affects the roadway system in the City. With projected future growth in Orange and in the region, traffic flow at this junction is expected to worsen. The City will continue to work with and support the efforts of local and regional agencies to mitigate the increased traffic congestion in this area.

SR-91 is an east-west freeway that provides access to key arterial facilities in Orange, including interchanges at Tustin Street and Glassell Street. SR-91 also provides regional access through interchanges with SR-55 and SR-57 and SR-241.

SR-22 is an east-west freeway that crosses through the southern portion of the City. Five SR-22 interchanges are located in the City, at Tustin Street, Glassell Street, Main Street/La Veta Avenue, Bristol Street, and The City Drive.

SR-55 is a north-south freeway that passes through the center of Orange, and provides access to the coastal communities of Orange County. SR-55 has four interchanges in Orange, located at Lincoln Avenue, Katella Avenue, Chapman Avenue, and SR-22. An additional future interchange at Meats Avenue is contemplated within this General Plan.

SR-57 is a north-south freeway that originates at the junction of I-5 and SR-22 and extends to San Dimas in Los Angeles County. It provides access for the eastern parts of Los Angeles County, and central and northern parts of Orange County. SR-57 has three interchanges in Orange, at Chapman Avenue, Orangewood Avenue, and the junctions of I-5 and SR-22.

The Eastern Transportation Corridor (SR-241) is a north-south toll facility located in the eastern portion of the planning area. This facility provides direct access to east Orange. SR-241 has three toll lanes in each direction and provides regional access through an interchange at Santiago Canyon Road.

Consistency with County Master Plan of Arterial Highways

Maintaining consistency with the County's Master Plan of Arterial Highways (MPAH) is required in order to ensure that the City's circulation system develops in a manner that promotes regional mobility. At a practical level, consistency is also required in order for the City to receive transportation funding under Measure M, also known as OC Go. Orange's Master Plan of Streets and Highways (Figure CM-2) is generally consistent with the MPAH.

While the City's Master Plan of Streets and Highways has been consistent with the County MPAH to maintain funding eligibility, both the City of Orange and OCTA have the goal of a realistic and implementable MPAH. A desire of the City is to work with OCTA to downgrade La Veta Avenue between Glassell Street and Cambridge Street from a Secondary Arterial to a

Collector Street to reflect physical constraints related to historic buildings and features in the Old Towne National Register Historic District.

To initiate the MPAH amendment process, a local agency must submit a written request to OCTA describing the amendment requested and provide documentation to support the basis for the request. A copy of the request must be submitted concurrently to the City Managers of adjoining cities. For the facilities under consideration, this would require a letter to be forwarded to the City of Santa Ana.

Once the initial request is forwarded to OCTA, a conference between the City of Orange, OCTA, and potential affected jurisdictions is held to determine whether mutual agreement exists for the MPAH amendment. If mutual agreement exists, then Orange is expected to proceed with adopting this revision to the Circulation & Mobility Element. Upon adoption, the City of Orange would submit the Circulation & Mobility Element to OCTA and request OCTA Board approval of the Orange County MPAH amendment.

Public Transportation

Effective regional transportation strategies are required to successfully implement City and County plans for accommodating future growth. Such strategies must link the City of Orange with other regional employment and commercial centers, as well as airports and other transportation hubs, and should fully integrate alternatives to the automobile. Alternative modes of transportation, including public transportation, bicycling, and walking, are important components of a comprehensive circulation system. These modes of transportation also help reduce air pollution and road congestion.

Public transportation plays a key role in future land use development and mobility. As the roadway system reaches capacity, alternative modes of transportation provide additional capacity as well as an enhanced degree of mobility for residents, workers, and visitors. Existing services are expected to continue while enhancements, many of them currently in the planning stages, will increase the viability of alternative modes of travel. The integration into the circulation system of alternative modes of transportation, such as bus, rail, bicycle, and pedestrian, is essential to maximizing mobility opportunities for residents, workers, and visitors.

Bus Service

OCTA provides public bus service for the City of Orange. In addition, the Riverside Transit Agency (RTA) provides long-distance service between The Village at Orange and the Downtown Terminal in Riverside. Table CM-4 identifies local bus routes that connect various activity centers in Orange to each other and to the region.

Table CM-5 shows the different community, station link, and inter- or intra-County routes that serve Orange. Community routes are express bus routes that provide faster connections to activity centers within and outside Orange County. Station link shuttles provide services between the Orange Transportation Center (OTC) and Orange. Both community and station link routes operate only at peak commuter times.

In addition to the fixed-route service, OCTA also offers several types of specialized community transportation services, such as standard service (curb-to-curb service), door-to-door service, subscription service, and same-day taxi service. Some of these services cater to senior citizens and people with disabilities residing in the City.

OCTA has forecast bus ridership to increase by approximately 75 percent by 2030. Some of this ridership increase will be the result of enhanced services, including express bus routes and introduction of bus rapid transit service. Much of the increase will be driven by increased arterial and freeway congestion levels in conjunction with improved local bus service.

Convenient, accessible, frequent, and easy-to-use public transit is a cornerstone element of the proposed land use plan. Planned mixed-use residential and commercial areas and intensified commercial and professional office corridors must be coupled with increases in transit service. Orange's land use plan features mixed-use districts that strategically concentrate population density near alternative transit facilities, such as the OTC in Old Towne, transit hubs at The Outlets at Orange and South Main Street, and the Anaheim Regional Transportation Intermodal Center (ARTIC) station in Anaheim near the City's western boundary at Katella Avenue.

Table CM-4
Local Bus Routes

Route #	Route Type	Route Service	Service Corridors	Key Orange Activity Centers Served
24	Local	Fullerton–Orange	Malvern Ave. / Chapman Ave. / Tustin St..	The Village at Orange Lincoln Park and Ride
42	Local	Orange–Seal Beach	Lincoln Ave. / Los Alamitos Blvd. / Seal Beach Blvd.	The Village at Orange Lincoln Park and Ride
46	Local	Los Alamitos–Orange	Ball Rd. / Taft Ave.	The Village at Orange Lincoln Park and Ride
47	Local	Brea–Newport Beach	Brea Blvd. / Anaheim Blvd. / Fairview St.	Theo Lacy Jail Orangewood Children's Home UCI Medical Center The Block at Orange
50	Local	Long Beach–Orange	Katella Ave.	The Village at Orange Lincoln Park and Ride
53	Local	Brea–Irvine	Main St.	Batavia Industrial Parks Children's Hospital – CHOC St. Joseph's Hospital OCTA Offices
54	Local	Garden Grove–Orange	Chapman Ave.	Orange Civic Center Orange Transportation Center The Plaza UCI Medical Center The Block at Orange Rancho Santiago Community College

Table CM-4
Local Bus Routes

Route #	Route Type	Route Service	Service Corridors	Key Orange Activity Centers Served
56	Local	Garden Grove–Orange	Garden Grove Blvd.	Orange Transportation Center OCTA Offices Children's Hospital–CHOC St. Joseph's Hospital
57	Local	Brea–Newport Beach	State College Blvd. / Bristol St.	The Block at Orange UCI Medical Center Theo Lacy Jail Orangewood Children's Home
59	Local	Brea–Irvine	Kraemer Blvd. / Glassell St. / Grand Ave. / Von Karman Ave.	Orange Transportation Center Chapman University Orange Plaza
71	Local	Yorba Linda–Balboa	Tustin St. / Red Hill Ave. / Newport Blvd.	The Village at Orange Lincoln Park and Ride

Source: Orange County Transportation Authority, 2006

Table CM-5
Community, Station Link, Intra- and Inter-County Bus Routes

Route #	Route Type	Route Service	Service Corridors	Key Orange Activity Centers Served
131	Community	Yorba Linda–Orange	Lakeview Ave./ Riverdale Ave. / Tustin St.	The Village at Orange Lincoln Park and Ride
147	Community	Brea–Santa Ana	Raiit St. / Greenville St. / Fairview St.	UCI Medical Center The Block at Orange OCTA Offices St. Joseph's Hospital Children's Hospital – CHOC
167	Community	Anaheim–Irvine	Santiago Blvd. / Hewes St. / Bryan Ave.	The Village at Orange Lincoln Park and Ride
453	Station Link	Orange Transportation Center –St. Joseph's Hospital	Chapman Ave. / Main St. / La Veta Ave.	Orange Transportation Center Children's Hospital–CHOC St. Joseph's Hospital OCTA Offices

Table CM-5
Community, Station Link, Intra- and Inter-County Bus Routes

Route #	Route Type	Route Service	Service Corridors	Key Orange Activity Centers Served
213	Intra County	Brea–Irvine Express	SR-55	Lincoln Park and Ride The Village at Orange
757	Inter County	Diamond Bar –Santa Ana Express	SR-57	UCI Medical Center The Block at Orange
RTA 149	Inter County	Riverside–Orange (Operated by RTA)	SR-91	The Village at Orange Downtown Riverside

Source: Orange County Transportation Authority and Riverside Transit Agency, 2006.

The City recognizes that ridership of both the bus and rail transit systems will increase, and has designed a land use plan that both enables and accommodates increased transit use. A large part of the City's role in accommodating additional transit use includes providing convenient and attractive transit amenities and streetscape features that improve user comfort and perception of safety, thus encouraging transit use. Transit-oriented Development (TOD) design features will be encouraged in major commercial and employment areas within the City, such as the Town and Country Road corridor, South Main Street, Katella Avenue, Uptown Orange, and Old Towne. Such TOD features may consist of streetscape measures such as bus turn-outs, benches, trash receptacles, shelters from wind and rain, and lighting. TOD features may also be more fundamental to the permitted uses and design within projects, such as incorporating child care centers, convenience stores, or personal services within the retail component of mixed-use projects, or near professional office concentrations. These and other measures help to make the transit system more accessible to a wide range of people.

The City will continue to work with OCTA to pursue expanded community circulators, such as the current Station Link service, that will connect people to rail transit, employment centers, residential areas, and commercial corridors. Additional options, such as jitney services that function as group taxis, will also be explored.

The City will also continue to support OCTA initiatives and services that promote the mobility of Orange's senior, disabled, and youth populations. To accommodate the needs of these groups, the City will continue to work with OCTA to offer para-transit services, and will seek ways to improve mobility for Orange youth through transit.

The City of Orange will continue to cooperate with OCTA and other regional providers to establish new bus routes and stops, and to provide transit amenities. New subdivision plans will be reviewed by OCTA to assess impacts on bus services, and to examine the need to provide bus stops or bays. Orange will also work with OCTA to maintain and, if needed, expand successful transfer stations in Old Towne, the Village at Orange, and The Outlets at Orange.

Rail Transit

Many current passenger and commuter fixed-rail transit options in the City will be expanded in the future, offering significant alternatives to automobile transit for many individuals who commute to or from the City for housing or employment. Rail transit now has a proven track record in Orange, and Metrolink seeks to expand rail services in support of land use transformations near current and future rail stations.

Metrolink

Metrolink operates seven commuter rail lines in Southern California, two of which pass through Orange. The Orange County Line offers direct connections to Union Station in Los Angeles to the north and to Oceanside to the south. The Inland Empire–Orange County Line provides direct connections to Riverside/San Bernardino to the east and San Juan Capistrano to the south. Roughly 30 Metrolink trains pass through the City on a daily basis. The Metrolink station in Orange is located three blocks from The Plaza at the OTC, close to a variety of stores and civic uses in Old Towne.

A much-anticipated rail service improvement is intra-county rail service along the County's core transit corridor. This service, which will be provided by Metrolink on behalf of OCTA, is an attempt to provide intra-county trips with a high degree of reliability. This 30-minute service is planned to operate between the Irvine Transportation Center (and possibly Laguna Niguel) and the Fullerton Transportation Center throughout the day to supplement Metrolink's peak period commuter service.

The City recognizes that plans to increase daily service on the current Metrolink lines, improvements to the OTC, and future expansion of Metrolink services to new destinations would be beneficial to Orange by providing multi-modal transportation options for people living and working in the community, thus reducing auto dependence for business and leisure purposes.

Anaheim Regional Transportation Intermodal Center



The Platinum Triangle, which borders Angel Stadium in the City of Anaheim, proposes a variety of high-density multiple-family high-rise housing, office space, and commercial uses. ARTIC is located adjacent to the proposed development, between Angel Stadium and the Honda Center. ARTIC is a major regional transportation center similar in scale to Union Station in Los Angeles and Ontario International Airport in Ontario. The intermodal center is be a stopping point for Amtrak, Metrolink, buses, and future high-speed rail systems that connect to Ontario

Airport, Las Vegas, and the Bay Area.

The City of Orange will coordinate with the City of Anaheim, OCTA, and others to ensure that the City is able to take full advantage of the regional mobility benefits offered by ARTIC. Potential benefits offered to the City by ARTIC include:

- a dynamic mixed-use commercial and residential center at the westernmost end of Katella Avenue; and
- bicycle and pedestrian connections to the Santa Ana River.

California High Speed Rail Corridor

The California High-Speed Rail Authority was established as a state agency in 1996 to direct the planning, design, construction, and operation of a future high-speed train system extending from Sacramento, San Francisco, and Oakland in the north to Los Angeles and San Diego in the south. This high-speed train is proposed to stop at ARTIC. OCTA estimates that by 2025, the California High Speed Rail Corridor will serve between 150,000 and 230,000 passengers each week.

Sidewalks, Trails and Bikeways

Walking and biking contribute to a healthy community, and play increasingly significant roles as alternatives to the automobile. The City recognizes this by providing and maintaining sidewalks, trails, and bikeways to support pedestrians and cyclists.

Pedestrian Facilities

In addition to providing basic transportation routes, sidewalks and pathways offer the opportunity to create appealing public spaces that reflect community pride and invite people to walk. Proposed mixed-use areas and reinvigorated commercial areas throughout the City will provide new and reinvented spaces for people to walk and shop. Walkability and access are essential components of a circulation system that easily and specifically accommodates pedestrians. Features that contribute to walkability include wide sidewalks, safe street crossings, design elements that encourage cautious driving, and a pleasant and safe walking environment. Sidewalks, walkways, well-designed pedestrian crossings, pathways, and pedestrian short-cuts allow people to get from one destination point to another with ease. Dedicated pedestrian paths can provide access between residential and activity areas, especially if streets do not connect. Access strategies for school children, seniors, and people with disabilities should also be incorporated into street and sidewalk plans. The City supports proactive integration of pedestrian improvements and amenities within the circulation system to improve walkability.

The City will create and implement a pedestrian-oriented streetscape master plan addressing key commercial corridors, including Tustin Street, Chapman Avenue, Main Street, Lincoln Avenue, and Katella Avenue. The master plan will address all functional aspects of the pedestrian environment. It will identify pedestrian links that need improvement and

strengthening, determine new pedestrian links to underserved areas, ensure adequate sidewalk widths to accommodate lighting and street trees, develop sidewalks in the industrial area that create links between bus stops, encourage safe routes to schools and recreation facilities, and minimize barriers to pedestrian and bicycle access.

Recreational Trails and Bikeways

A comprehensive network of recreational trails and bikeways greatly benefits Orange residents and visitors by providing popular modes of transportation for recreation. In addition to recreation activities, the City also supports walking and bicycling as viable commute alternatives to the automobile. The City's plan for recreational trails and bikeways is shown in Figure CM-3.

The plan includes trails maintained by the County and private homeowners associations, and is consistent with the OCTA *Commuter Bikeways Strategic Plan*. It is also consistent with the County's major riding and hiking trails and off-road paved bikeways. As described in the ***Vision for the Future,*** the plan will enable the City to connect parks to activity centers and residential areas using a combination of recreational trails and bikeways that truly allows people to travel from place to place within the City without needing an automobile.

Recreational Trails

As shown in Figure CM-3, over 70 miles of existing recreational trails are located within the City, connecting a large number of neighborhoods and community parks. In addition, 104 miles of proposed future trails are planned throughout Orange on land currently utilized for a variety of purposes, including flood control, railroad rights-of-way, and roadways.

The City will use the annual five-year Capital Improvement Program (CIP) process to prioritize, fund, and build proposed trail segments identified in Figure CM-3. These trails have been and will continue to be constructed and administered through cooperative efforts of the City, County, developers, and private homeowner associations.

Desired cross-sections for recreational trails are identified in Figure CM-4. The City may require construction of portions of proposed trails identified in Figure CM-3 as a condition of development approval for projects located adjacent to the proposed trail alignments. Funding for recreational trails and associated restoration projects comes from a variety of federal, state, and regional sources. Priority for funding of trail improvements will be given to projects that complete loops within the system, provide missing links for regional and local trail systems, or serve as destination links to schools, parks, retail businesses, or regional trails.

Priorities for the recreational trail system include:

- trail connections to the Santiago Oaks Regional Park extension west of Cannon Street, consisting of a connection between Calle Grande and Cannon Street, and the Jamestown trail from the Orange Park Acres equestrian arena to Cannon Street;

- trail connection from Serrano Avenue near Fred Barrera Park to Santiago Oaks Regional Park;
- Mabury Ranch Trail connection from the proposed Cerro Villa Park to the Santiago Oaks Regional Park;
- Serrano Avenue connection near Cannon Street; and
- Additional trail connection into Santiago Oaks Regional Park from Orange Park Acres.

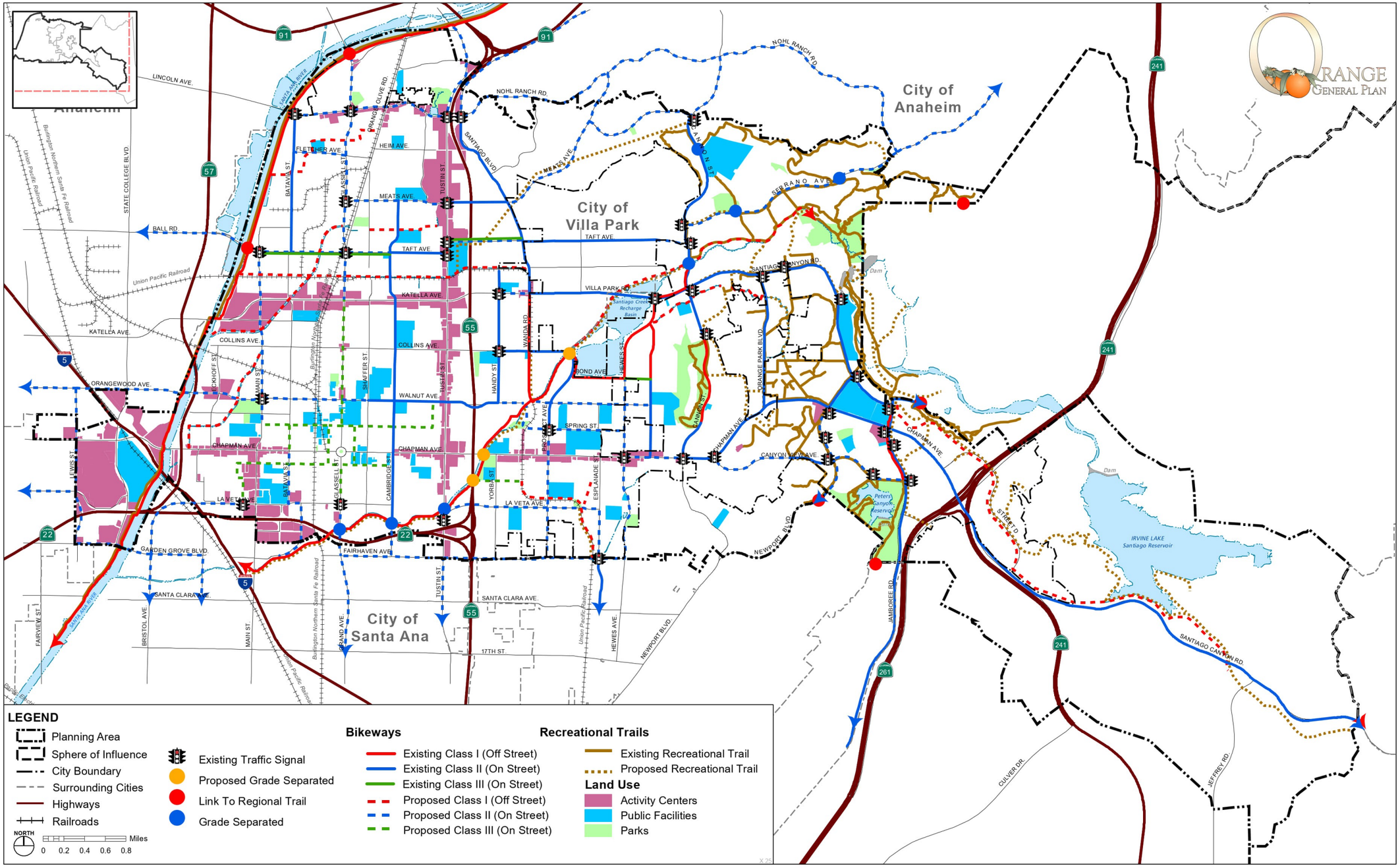
To increase the number of people using non-vehicular means of transportation, the City will encourage putting in place a safe network of crosswalks, grade separations, and walkways to ensure the safety of pedestrians, bicyclists, and equestrians. Where appropriate, traffic calming devices and methods such as median landscaping and provision of bike or transit lanes should be used to slow traffic, improve roadway capacity, and address potential safety issues. The City will continue to work towards improving the overall condition, appearance, and safety of both medians and sidewalks in Orange.

Bikeways

Orange's relatively mild climate permits bicycle riding year-round, and the growing popularity of bicycling has drawn enthusiasts onto the streets and bike trails throughout the City. The plan for recreational trails and bikeways (Figure CM-3) shows the planned system of bikeways within the City. The Orange *Bikeway Master Plan* has established three classes of bicycle routes that adhere to California Department of Transportation (Caltrans) standards:



- **Class I** Off-road bike paths are located on vacated rail lines, water corridors, or areas otherwise separated from streets.
- **Class II** On-road bike lanes are located along arterial roadways that are delineated by painted stripes and other features.
- **Class III** On-road bike routes share use with motor vehicle traffic. They provide a route that is signed but not striped.



Source: City of Orange Community Services Department, 2015.

Figure CM-3 Plan for Recreational Trails and Bikeways

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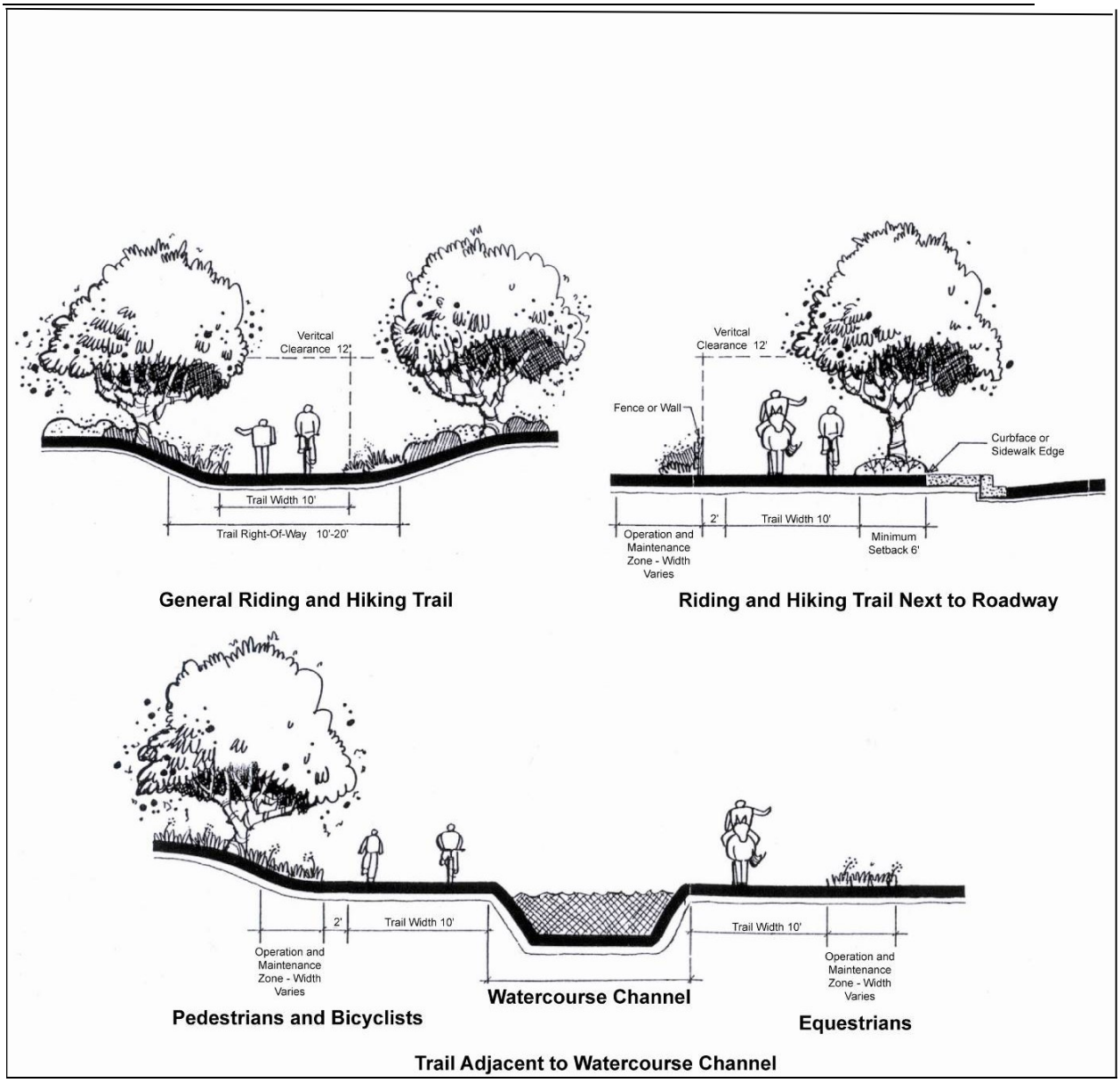


Figure CM-4
Trail Standards

Cross-sections for each type of route are shown in Figure CM-5.

As shown in Figure CM-3, several future bike routes are planned within Orange, including the following high-priority projects:

- Completing Class I bike routes along Santiago Creek and the Tustin Branch Trail
- Establishing Class II bike lanes, including:

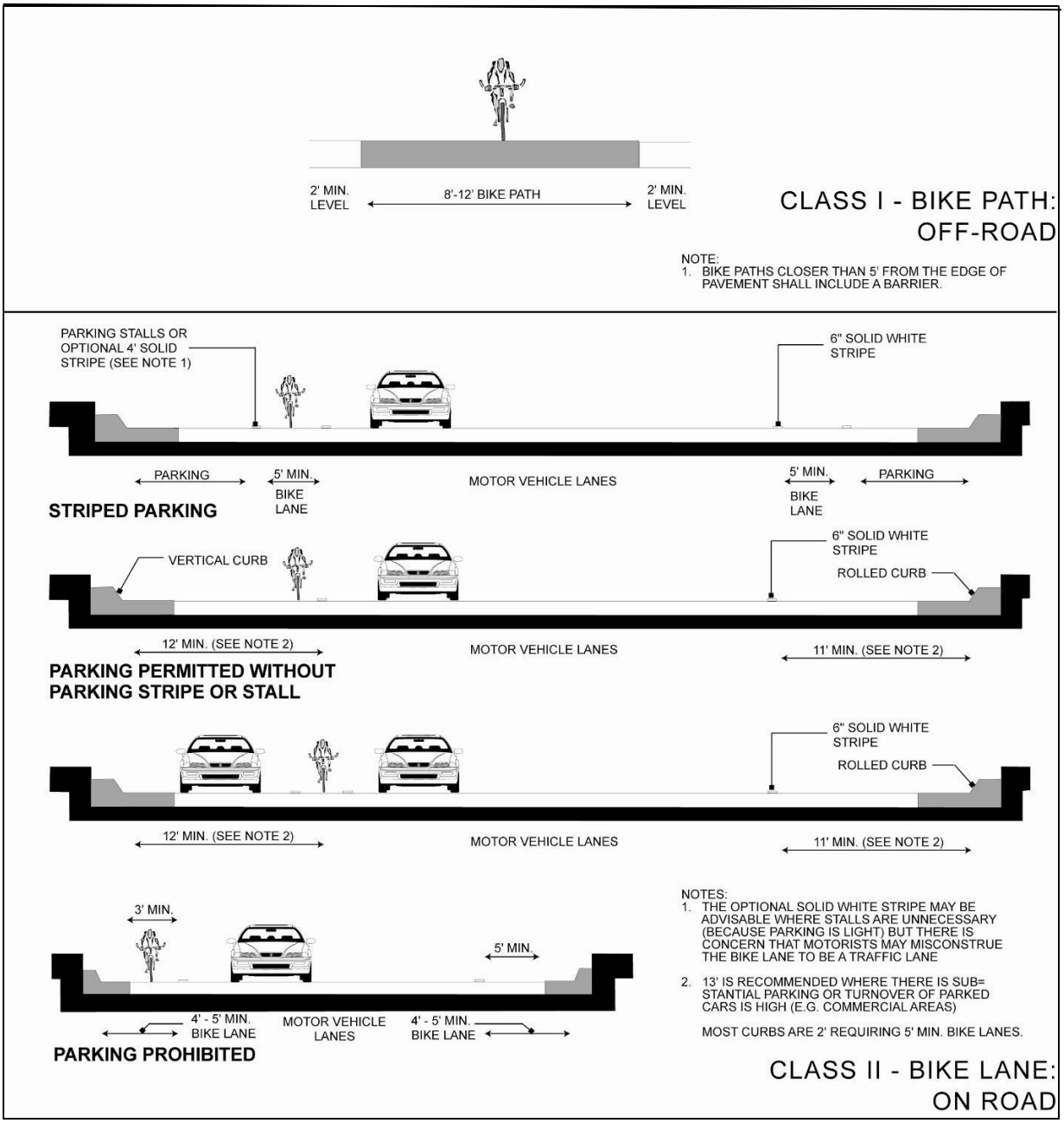


Figure CM-5
Bikeway Standards

  - An east/west corridor on Walnut Avenue from the western City limits to Santiago Boulevard
  - North/south corridors on Main Street from Taft Street to Palm Street, on Batavia Street from Chapman Avenue to La Veta Avenue, and on Parker Street from La Veta Avenue to the City limits
- Establishing Class III routes along Almond Avenue, Feldner Street, and Bedford Street

The City has completed a paved bike trail along Santiago Creek from Tustin Street to the western City limit that continues on to Main Place Mall and the Discovery Science Center as well as two additional segments to the bike trail, which extends the Santiago Creek Trail from Tustin Street to Grijalva Park, and also connect Grijalva Park at Santiago Creek to Collins Avenue along the City-owned portion of the Tustin Branch Trail right-of-way between Walnut and Collins Avenues. The Santiago Creek trail is planned to extend through and beyond the City, connecting the regional Santa Ana River Trail to Santiago Oaks Regional Park and wilderness areas east of Orange. The City will continue to work towards designing a comprehensive bike trail system that is highly accessible and safe for those who wish to use it.

The City has proposed Class II and III routes along many north-south and east-west arterials, all of which connect to pedestrian trails and Class I routes. The City responds to the need to provide safe and efficient bike travel by making every effort to provide bikeways separate from the roadway. When bicyclists must share the road with automobiles, the City will work to improve overall safety.

Currently, two bicycle parking facilities are located in Orange, at the OTC (Metrolink station) and the Old Towne West Metrolink Parking Structure. The City will work to provide greater bike amenities including delineated bike lanes and clear signage along bike trails. The City will also install bicycle racks and safe storage facilities at parking areas for City facilities, as appropriate, and will require privately developed projects to incorporate on-site bicycle facilities in accordance with the City's Zoning Code (Title 17 of the Orange Municipal Code).

Parking Facilities

Although parking is often considered a separate issue from vehicle circulation, it is important to address on-street parking because it has a direct effect on roadway capacity. In order to facilitate improved traffic flow along Orange's major arterials, the City generally plans to permit on-street parking only on streets classified as Primary or Secondary Arterials or Collector Streets.



Off-street parking deficiencies can reduce business activity, and can cause

vehicles to re-circulate on public streets, which increases traffic volumes and congestion by reducing capacity for through traffic. The City's Zoning Code includes parking requirements to ensure that an adequate number of spaces are provided on-site for most uses. The Code also establishes minimum stall dimensions consistent with current standards for other jurisdictions. These regulations apply to all new developments, and may be applied to current uses that are modified or expanded.

Particular concern has been expressed within the community regarding current and expected future parking shortages in Old Towne. Figure CM-6 identifies the location of existing public parking lots in Old Towne. Most Old Towne parking lots have a maximum time limit of three hours, with or without a permit. However, some parking lots, such as the OTC parking located off Chapman Avenue, have no time limit.

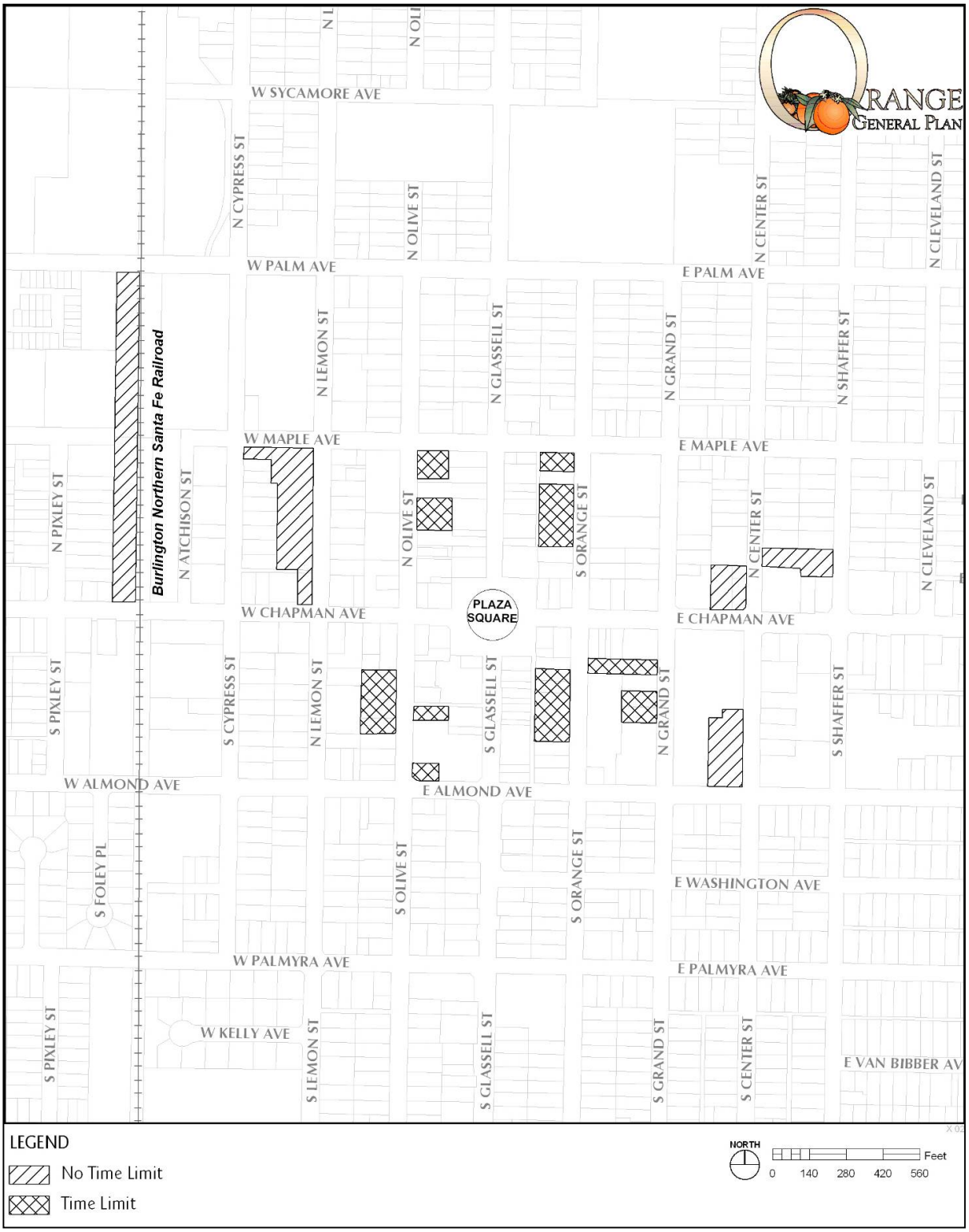
Orange will continue to provide sufficient parking to meet community demands. In Old Towne, the City will study the benefits of creating a parking district and the feasibility of developing underutilized parking lots surrounding The Plaza as public parking facilities. The City may encourage the use of shared parking, consolidated parking facilities, and underground parking or parking structures to provide additional off-street parking to meet future demands in areas throughout the City with higher concentrations of commercial uses.

Circulation System Aesthetics

The City has determined that it is necessary and desirable to improve certain roadways and their rights-of-way to enhance the experience for all users of these corridors. Major commercial corridors have been designated within the Urban Design Element to be improved with pedestrian-scale enhancements. Enhancements could include street trees, sidewalk improvements, lighting, bus shelters, and crosswalks. These improvements not only offer a more aesthetically-pleasing experience for all users in these corridors, they also may offer increased safety and security.

Streetscape improvements offer safety and security to both motorists and non-motorists through increased visual cues, better visibility, and increased activity. Many streetscape enhancements provide not only a comfortable environment for the pedestrian and bicyclist, but also offer drivers visual cues that a non-motorist could be expected in an area. Improvements such as striping, bus shelters, and pedestrian-scaled signage guide the non-motorist towards areas that are most appropriate for their use. These same cues help the motorist to see where the presence of pedestrians and bicyclists is most likely to occur. For example, a well-marked crosswalk guides the pedestrian towards the location in the right-of-way most appropriate for crossing, while also alerting the motorist to slow down and look out for pedestrian traffic.

Visibility offered by pedestrian-scaled lighting benefits non-motorists and motorists alike. Pedestrians and bicyclists are better able to see their way, which increases their feelings of security. Improvements that create a sense of security for pedestrians encourage increased use and activity. This increased activity, in turn, leads to a greater feeling of safety. The City has acknowledged the importance of improving the experience of users on many of its major commercial corridors.



*Note: Possession of City issued parking permit enables holder to exceed specified time limit

GPA 2010-0001 (8/10/10)

Figure CM-6
Public Parking Lots in Old Towne Orange

CIRCULATION & MOBILITY IMPLEMENTATION

The goals, policies, and plans identified in this Element are implemented through a variety of City plans, ordinances, development requirements, capital improvements, and ongoing collaboration with regional agencies and neighboring jurisdictions. Specific implementation measures for this Element are contained in the General Plan Appendix.

Transportation Assessment of Proposed Amendment to the Master Plan of Arterial Highways (MPAH) for Villa Park Road

Prepared for:
Orange County Public Works

March 2024

OC23-0971.01

FEHR PEERS

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Executive Summary

Fehr & Peers completed a transportation assessment to evaluate the roadway designation in the Master Plan of Arterial Highways (MPAH) for Villa Park Road. The study covers Villa Park Road from Wanda Street to Cannon Street, spanning the jurisdictions of the City of Orange, City of Villa Park, and Unincorporated Orange County. This assessment consisted of a review of operations (in the form of LOS and queueing), safety, and user comfort (particularly for non-auto modes). This led to us identifying suggested future lane configurations and bike/ped facilities, which in turn, led to a recommendation regarding the most appropriate MPAH designation.

Villa Park Road between Hewes Street and Cannon Street is a four-lane Primary Arterial and is planned to be a six-lane Major Arterial based on the current MPAH designation. The Orange County Transportation Authority (OCTA) MPAH Guideline (August 2017), designates a Major Arterial highway as a six-lane divided (raised or painted) roadway, with a typical right-of-way width of 120 feet. The right-of-way for Villa Park Road between Hewes Street and Cannon Street is 100 feet, which excludes the off-street trail on the north side of the roadway **(Table 1)**. According to our operations and cross-section analysis, a five-lane Primary Arterial (three westbound lanes and two eastbound lanes) will be sufficient to serve future traffic demand and can fit within the available right-of-way.

Table 1: MPAH Designation

Facility	Extent	Current Designation	Proposed Designation
Villa Park Road	Hewes Street to Cannon Street	Major Arterial (Divided 6-lane)	**Primary Arterial (Divided 5-lane)**
Villa Park Road	Wanda Street to Hewes Street	Primary Arterial (Divided 4-lane)	Primary Arterial (Divided 4-lane)

*Note: Proposed designation that differ from the current MPAH designation is highlighted in **bold**.*
Source: Fehr & Peers, 2023.

1. Introduction

Study Area

Through coordination with Orange County Transportation Authority (OCTA), Orange County Public Works, and the cities of Villa Park and Orange, 15 study intersections and 15 study roadway segments were identified for this assessment **(Figure 1)**. This study evaluated parallel roadway segments and intersections along the corridor to better understand traffic operations in the area and to help guide recommendations along Villa Park Road.

Study Intersections

1. Wanda Road and Villa Park Road (Signalized)
2. Kathleen Lane and Villa Park Road (Side-Street Stop Control [SSSC])[1]
3. Morrow Circle and Villa Park Road (SSSC)
4. Prado Woods Drive and Villa Park Road (SSSC)
5. Kenwick Drive and Villa Park Road (SSSC)
6. Radec Court and Villa Park Road (SSSC)
7. Center Drive and Villa Park Road (Signalized)
8. Park Villa Lane and Villa Park Road (SSSC)
9. Lemon Street and Villa Park Road (Signalized)
10. Hewes Street and Villa Park Road (Signalized)
11. Linda Vista Street and Villa Park Road (SSSC)
12. Cannon Street and Santiago Canyon Road (Signalized)
13. Cannon Street and Taft Avenue (Signalized)
14. Cannon Street and Serrano Avenue (Signalized)
15. Cannon Street and Loma Street (SSSC)

Study Segments

1. Villa Park Road between Wanda Road and Center Drive
2. Villa Park Road between Center Drive and Hewes Street
3. Villa Park Road between Hewes Street and Cannon Street
4. Santiago Canyon Road east of Cannon Street
5. Taft Avenue between Santiago Boulevard and Center Drive
6. Taft Avenue between Center Drive and Cannon Street

[1] In the HCM methodology, Side-Street Stop Controlled (SSSC) intersection is analyzed as Two-Way Stop Control (TWSC).

7. Serrano Avenue east of Cannon Road
8. Santiago Boulevard north of Taft Avenue
9. Wanda Road between Villa Park Road and Taft Avenue
10. Wanda Road south of Villa Park Road
11. Hewes Street south of Villa Park Road
12. Cannon Street north of Serrano Avenue
13. Cannon Street between Taft Avenue and Serrano Avenue
14. Cannon Street between Santiago Canyon Road and Taft Avenue
15. Cannon Street south of Santiago Canyon Road

Analysis Scenarios

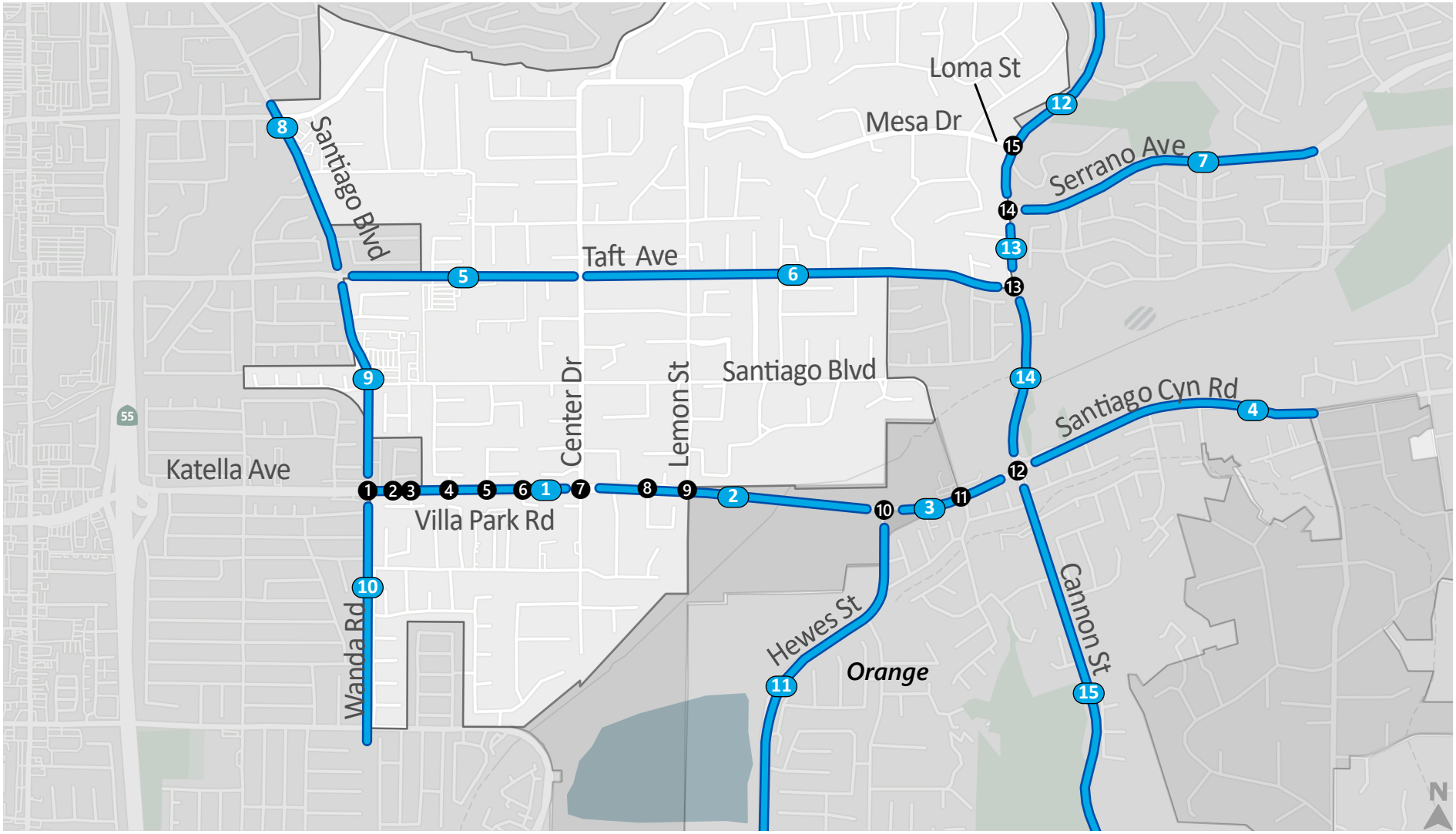
To determine the existing and future needs of the transportation network, Fehr & Peers considered the following scenarios in the analysis:

- Existing (2023) Conditions – Consists of traffic counts collected on April 26, 2023, a typical weekday during an event at Angels Stadium.
- Future Year (2045) Conditions – Consists of traffic forecasts estimated using the County of Orange's travel demand forecasting model, OCTAM.

Report Outline

The remainder of this report contains the following chapters:

- Chapter 2 – Analysis Methodology
- Chapter 3 – Existing (2023) Conditions
- Chapter 4 – Future Year (2045) Conditions
- Chapter 5 – Recommendations
- Chapter 6 – Vehicle Miles Traveled (VMT) Assessment



Study Intersections
Study Segments
City of Villa Park
City of Orange
Unincorporated Orange County

OC23-0971.01

Figure 1: Study Intersections and Segments

Villa Park Road MPAH Amendment

2. Analysis Methodology

This study provides a review of vehicle and multi-modal traffic operations through the corridor.

Roadway LOS Methodology

Study segments were evaluated based on V/C ratios, which were calculated by comparing Average Daily Traffic (ADT) to the theoretical daily capacity. This calculation does not indicate the service level throughout the course of the day, but rather provides a rough estimate of what the service level will be at the primary intersections during peak hours. Roadway capacities for all existing and planned facilities were referenced from the *Guidance for Administration of Orange County Master Plan of Arterial Highways, Orange County Transportation Authority (2017)*, which documents capacities necessary to achieve LOS A through LOS E for various roadway classifications. A threshold of LOS E was utilized for all V/C calculations in conformance with the MPAH. The LOS grades and descriptions for segments are summarized in **Table 2**. Per each jurisdiction's requirements, LOS C is used as the limit of acceptable performance.

Intersection LOS Methodology

Consistent with the County's *Transportation Implementation Manual (2020),* all signalized study intersections within the study area were analyzed utilizing the ICU methodology. ICU methodology reports the volume-to-capacity (V/C) ratio for signalized intersections. The V/C ratio is determined by evaluating the critical movements for each intersection and comparing them to the critical movement capacity of the intersection. For all ICU calculations, a saturation flow rate of 1,700 vehicles per hour per lane (vphpl) and 5% lost time was used.

As the ICU methodology doesn't account for signal timing parameters and several other factors, all signalized intersections were also analyzed using the Transportation Research Board (TRB) *Highway Capacity Manual, 7th Edition* (HCM) methodology. HCM methodology uses signal timing inputs to estimate the average control delay for vehicles at signalized intersections.

All unsignalized intersections were analyzed using HCM methodology, which estimates the worst-case lane delay for side-street stop-controlled intersections. Traffic signal warrants were conducted for unsignalized intersections operating worse than the LOS standards.

After the quantitative V/C or delay estimates are complete, the methodologies assign a qualitative letter grade which represents the operations of the intersection. These grades range from level of service (LOS) A (minimal delay) to LOS F (excessive congestion). LOS E represents at-capacity operations. Descriptions of the LOS letter grades for intersections are provided in **Table 2**.

All ICU and HCM intersection operation analysis was performed using the PTV Vistro 2020 and Synchro 11 software, respectively. Per each jurisdiction's requirements, LOS D is used as the limit of acceptable performance.

Table 2: Intersection LOS Grades

Level Of Service	Description	Signalized Volume-to-Capacity (V/C)	Signalized Delay (Seconds)	Unsignalized Delay (Seconds)
A	Operations with very low delay occurring with favorable progression and/or short cycle length	0.000-0.600	≤ 10.0	≤ 10.0
B	Operations with low delay occurring with good progression and/or short cycle lengths	0.601-0.700	> 10.0 to 20.0	> 10.0 to 15.0
C	Operations with average delays resulting from fair progression and/or longer cycle lengths. Individual cycle failures begin to appear	0.701-0.800	> 20.0 to 35.0	> 15.0 to 25.0
D	Operations with longer delays due to a combination of unfavorable progression, long cycle lengths, or high V/C ratios. Many vehicles stop and individual cycle failures are noticeable	0.801-0.900	> 35.0 to 55.0	> 25.0 to 35.0
E	Operations with high delay values indicating poor progression, long cycle lengths, and high V/C ratios. Individual cycle failures are frequent occurrences	0.901-1.000	> 55.0 to 80.0	> 35.0 to 50.0
F	Operation with delays unacceptable to most drivers occurring due to over saturation, poor progression, or very long cycle lengths	Greater than 1.000	> 80.0	> 50.0

Source: Fehr & Peers, 2023.

Signalized Intersection Queueing Methodology

The HCM methodology was also used to evaluate signalized intersection turn-lane storage capacities. Storage capacities were compared against 95th percentile queue estimates using the Synchro 11 software.

Peak Hour Traffic Signal Warrants

The Peak Hour Traffic Signal Warrant (*California Manual on Uniform Traffic Control Devices (MUTCD) 2014 Edition)* is intended to examine whether a signal is justifiable, but it does not mandate that a signal be installed. Peak hour traffic signal warrants were conducted for all unsignalized intersections operating below acceptable LOS standards.

Multi-modal LOS Methodology

In an effort to provide quantitative data for all user groups, Fehr & Peers developed PersonDelay+, a tool utilizing HCM methodology that provides a more complete picture of how all users are affected by intersection operations. PersonDelay+ reports person delay for all users: vehicle drivers and passengers, pedestrians, and bicyclists for signalized intersections. The data needed is routinely collected as part of the traffic analyses and Complete Streets studies, including capacity, average passenger occupancy, cycle length and effective green time. The LOS grades and descriptions are consistent with the information

provided in **Table 2**. None of the jurisdictions have adopted standards for multi-modal LOS analysis. Therefore, we assumed that LOS D is used as the limit of acceptable performance.

LOS grades below LOS D indicate that on average each person waits over a minute to cross the intersection on foot or by bike. Generally, bike delays are similar to vehicle delays, but that average intersection delay is weighted by measured bicycle counts. Pedestrian delay is a factor of the intersection cycle length and green times that cause the pedestrians to wait for a walk signal.

Collision Review

Vehicle collisions with bicycles and pedestrians along the corridor were reviewed to evaluate potential trends. Data was pulled from the last five years from the Transportation Injury Mapping System (TIMS) database which summarizes reported California crash data from the Statewide Integrated Traffic Records System (SWITRS).

Data Collection

Turning movement and daily traffic counts were collected on Wednesday, April 26th, 2023. Counts were collected on this day to represent peak travel demand in the area. On this day school was in session and a special event (baseball game) was happening at Angels Stadium. This later condition, an event at Angels Stadium or Honda Center, occurs on over a third of the days per calendar year.

Turning movement counts were collected during typical peak commute periods (7:00 AM – 9:00 AM and 4:00 PM to 6:00 PM), and vehicle classifications and bicycle and pedestrian counts were collected at select roadway segments and intersections, respectively. Signal timings were obtained from appropriate jurisdictions.

Future Year Forecasting Methodology

The Orange County Transportation Analysis Model (OCTAM) version 5.0 was utilized to prepare traffic forecasts. OCTAM is consistent with the 2020 SCAG Regional Transportation Plan/Sustainable Communities Strategy (2020 RTP/SCS) with a 2016 base year and a 2045 future year and is the most appropriate model for use in this project because it includes the most recent land use and roadway information for the area. Pending development proposals in the surrounding study area were confirmed with the City of Orange, City of Villa Park, and County staff to account for major pending and approved land use projects **(Table 3).**

Table 3: Pending/Approved Developments

Project Name	Jurisdictions	Location	Quantity	Units
Milan Property	City of Orange	North of Santiago Canyon Road between Cannon Street and Orange Park Boulevard	200	Dwelling Units
Orange Heights	City of Orange	South of Santiago Canyon Road, east of Jamboree Road	1,180	Dwelling Units
Villa Park RV Parking	Orange County	Southeast Corner of Hewes Street and Villa Park Road	69,095	Square Feet

Source: Fehr & Peers, 2023.

Traffic forecasts for study intersections and segments were developed using the Difference Method which is consistent with methodologies delineated in the *National Cooperative Highway Research Program Report (NCHRP) 765* published by the *Transportation Research Board (TRB): Analytical Travel Forecasting Approaches for Project Level Planning and Design (Transportation Research Board, 2014)*. In this study, the Base Year and Future Year model outputs were used to calculate a model growth for the study area, which was then applied to existing traffic volumes to develop 2045 forecasts. Bicycle and pedestrian volumes were assumed to be similar in future conditions.

3. Existing (2023) Conditions

LOS Analysis

The existing turning movements and daily traffic volumes are summarized in **Attachment A**. Existing volumes were utilized along with existing lane configurations to conduct the ICU and HCM analysis. The existing segment, intersection LOS, and multi-modal LOS results are summarized in **Attachment B**, respectively. LOS worksheets are provided in **Attachment C**.

All **roadway segments** operate at LOS C or better except the following:

13. Cannon Street between Taft Avenue and Serrano Avenue (LOS E)
14. Cannon Street between Santiago Canyon Road and Taft Avenue (LOS D)

All **unsignalized intersections** operate at LOS E or F, except Trenton Avenue and Villa Park Road. These intersections have stop control for the side-street, and HCM uses the worst-case movement delay to estimate the intersection LOS. All side-streets have relatively low volumes, which results in only a few vehicles experiencing high delays at the intersections. Since adjacent traffic signals typically meter traffic and provide gaps for stop-controlled movements to be made, gaps in traffic are likely resulting in less delay for the stop-controlled movements than reflected in the LOS analysis.

The following three **signalized intersections** operate at worse than LOS D in the ICU and/or HCM analysis:

12. Cannon Street and Santiago Canyon Road (HCM PM LOS E)
13. Cannon Street and Taft Avenue (ICU PM LOS E and HCM AM LOS E)
14. Cannon Street and Serrano Avenue (ICU PM LOS E)

The following two **signalized intersections** along the Villa Park Road operate at worse than LOS D in the multi-modal LOS analysis. The intersection Wanda Road and Villa Park Road has the highest pedestrian and bike volumes in the corridor (26 at the peak in the AM and 21 at the peak in the PM). Pedestrians on average wait approximately a minute to cross the intersection. The intersection of Hewes Street and Villa Park Road has fewer bikes and peds (5 at the peak in the AM and 9 at the peak in the PM) with the greatest delays for those crossing Villa Park Road.

1. Wanda Road and Villa Park Road (Pedestrian AM and PM LOS E)
10. Hewes Street and Villa Park Road (Bike and Pedestrian AM and PM LOS E)

Queuing Analysis

The queuing analysis was prepared for the intersection of Cannon Street and Villa Park Road since it is the only signalized intersection along Villa Park Road that doesn't meet the County's LOS standard. The intersections queuing analysis is summarized in **Attachment B** and detailed queueing worksheets are

provided in **Attachment D**. The northbound left and southbound left exceed the available storage, and the eastbound left is at capacity.

Signal Warrants

We evaluated the signal warrants for the seven unsignalized intersections which don't meet the County's LOS standards along Villa Park Road. Only the intersection of Linda Vista Street and Villa Park Road meets the peak hour signal warrant[2] (**Attachment E**).

Collision Review

The bicyclist and pedestrian collisions were reviewed from 2018 to 2022, acquired from the Transportation Injury Mapping System (TIMS), which is the most recent data available. There were three reported bike collisions and one pedestrian-involved collision along Villa Park Road between Wanda Road and Cannon Road from 2018 to 2022, which accounts for 29% of the total 14 collisions (**Attachment F**). The three bike collisions all occurred at the Villa Park Road and Wanda Road intersection, while the pedestrian involved collision occurred at Villa Park Road, 225 feet west of Cannon Street. The severity level of the four collisions was "visible injury".

[2] Traffic Signal Warrant Disclaimer: This analysis is intended to examine the general correlation between the planned level of existing facility and the need to install new traffic signals. It estimates existing traffic compared against a sub-set of the standard traffic signal warrants recommended in the Federal Highway Administration *Manual on Uniform Traffic Control Devices* and associated State guidelines. This analysis should not serve as the only basis for deciding whether and when to install a signal. To reach such a decision, the full set of warrants should be investigated based on field-measured, rather than forecast, traffic data and a thorough study of traffic and roadway conditions by an experienced engineer. Furthermore, the decision to install a signal should not be based solely upon the warrants, since the installation of signals can lead to certain types of collisions. The responsible state or local agency should undertake regular monitoring of actual traffic conditions and accident data, and timely re-evaluation of the full set of warrants in order to prioritize and program intersections for signalization.

4. Future Year (2045) Conditions

LOS Analysis

To accurately model the future year roadway network, Fehr & Peers referenced the OCTA 2023 MPAH, 2020 SCAG Regional Transportation Plan and Sustainable Communities Strategy (RTP/SCS), and coordinated with relevant jurisdictions to identify roadway improvements in the study area. The following roadway improvements were assumed in the future year assessment:

- Wanda Road south of Villa Park Road is widened from three to four lanes (MPAH)
- Cannon Street between Santiago Canyon Road and Serrano Avenue is widened in the northbound direction from two to three lanes (City of Orange)

The MPAH identifies Villa Park Road, between Hewes Street and Cannon Street, as six lanes (currently four lanes). The widening was assumed in the modeling and forecasting efforts to analyze unconstrained demand but was not assumed in the intersection and segment operations analysis.

Future year turning movements and daily traffic volumes are provided in **Attachment A** and were utilized along with future year lane configurations to conduct the ICU and HCM analysis. Consistent with standard practice, the HCM analysis assumes routine traffic signal maintenance would occur, and signal timing parameters would be optimized to accommodate future traffic volumes.

The future year roadway segment, intersection LOS, and multi-modal LOS results are summarized in **Attachment B**. LOS worksheets are provided in **Attachment C**.

The following **roadway segments** are forecast to operate below County's LOS C standard:

3. Villa Park Road between Hewes Street and Cannon Street (LOS D)
4. Santiago Canyon Road east of Cannon Street (LOS D)
13. Cannon Street between Taft Avenue and Serrano Avenue (LOS D)

The roadways surrounding Villa Park Road have sufficient capacity to accommodate increased traffic, but Villa Park Road provides the most desirable east/west street in the area and is forecast to have high demand. The segments between Hewes Street and Cannon Street are estimated to not meet the County's LOS standard. There is a significant demand for northbound/southbound traffic on Cannon Street north of Villa Park Road and Hewes Street south of Villa Park Road. This analysis confirms widening Villa Park Road between Hewes Street and Cannon Street is necessary for it to operate within the County's LOS standards.

The side street movements for all **unsignalized intersections** are forecast to operate at LOS E or F or worse. However, all the side-streets have relatively low volumes, which results in only a few vehicles experiencing high delays at the intersections. Since adjacent traffic signals typically meter traffic and

provide gaps for stop-controlled movements to be made, gaps in traffic are likely being underestimated and conditions will be better than the analysis suggests.

The following four **signalized intersections** are forecast to operate at LOS E or F in the ICU and/or HCM analysis:

10. Hewes Street and Villa Park Road (ICU and HCM AM LOS E)
12. Cannon Street and Santiago Canyon Road (ICU AM and PM LOS E)
13. Cannon Street and Taft Avenue (ICU and HCM AM LOS E)
14. Cannon Street and Serrano Avenue (ICU AM LOS F)

The following three **signalized intersections** along the Villa Park Road are forecast to operate at LOS E using a multi-modal LOS analysis. Similar to existing conditions, the pedestrian wait time to cross is on average over a minute at Wanda Street and Hewes Street. The wait times to cross at the intersection of Villa Park Road and Cannon Street is also forecast to be over a minute on average which is worse than existing conditions.

1. Wanda Road and Villa Park Road (Pedestrian AM and PM LOS E)
10. Hewes Street and Villa Park Road (Bike and Pedestrian AM and PM LOS E)
12. Cannon Street and Santiago Canyon Road (Bike and Pedestrian PM LOS E)

Queueing Analysis

The intersections of Hewes Street/Villa Park Road and Cannon Street/Villa Park Road are the only signalized intersections along Villa Park Road that will fail to meet the County's LOS standards. The queuing analysis is summarized in **Attachment B** and detailed queueing worksheets are provided in **Attachment D**. The queues at the following movements exceed available storage capacities:

- Hewes Street and Villa Park Road: westbound left-turn
- Cannon Street and Santiago Canyon Road: eastbound left-turn, northbound left-turn and southbound right-turn

Signal Warrants

Peak hour signal warrants for the unsignalized intersections which operate below County's standards along Villa Park Road are provided in **Attachment E**. Only the intersection of Linda Vista Street and Villa Park Road meets peak hour signal warrant.

5. Recommendations

The recommendations below will provide acceptable operating conditions within the existing right-of-way and built curb-to-curb width.

The future forecasts along Villa Park Road indicate that more capacity is needed between Hewes Street and Cannon Street to effectively serve the future demand. This assessment concluded that an additional westbound through lane on Villa Park Road between Hewes Street and Cannon Street would be the best use of the available right-of-way to increase capacity, meet operational goals, and limit impacts to adjacent properties.

In addition to capacity improvements, we recommend bicycle and pedestrian treatments to improve conditions along the corridor.

Villa Park Road Between Wanda Street and Hewes Street

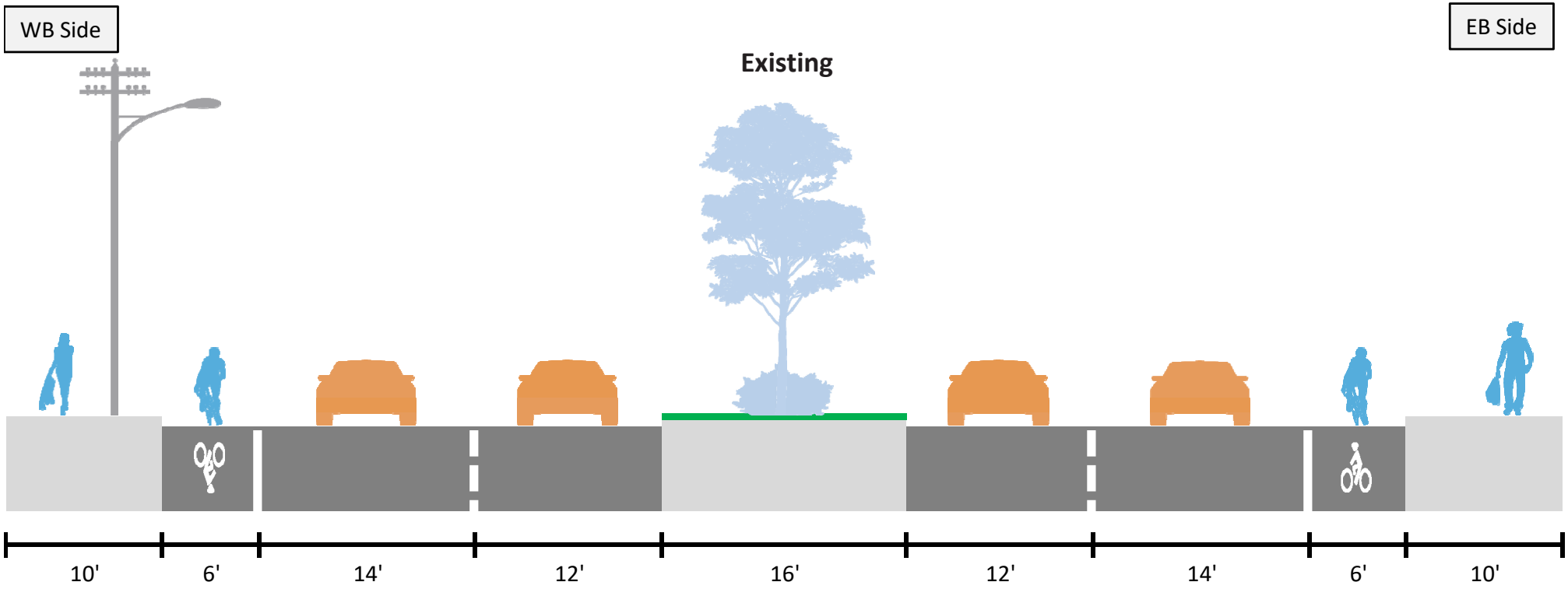
This segment of Villa Park Road has sufficient capacity to meet future traffic demands. While the unsignalized intersections are not forecast to meet the County's standards, they are also not anticipated to meet the warrants for a signal. The analysis does not consider the effects of upstream signals in creating gaps, and the County should monitor the unsignalized intersections for potential signalization if delays become unreasonable.

To improve bicyclist and pedestrian safety and comfort, we recommend restriping the corridor to provide a three-foot buffer between the bike lane and travel lanes, narrowing the two vehicle lanes to 12 feet and 11 feet. The recommended restriping can occur within the available right-of-way and existing curb-to-curb width, without affecting the exiting raised median. This design would provide a six-foot bike lane between Wanda Street and Lemon Street and an eight-foot bike lane between Lemon Street and Hewes Street. The recommended roadway cross-section for Villa Park Road between Wanda Street and Lemon Street is shown on **Figure 2**, and the recommended cross-section for Villa Park Road between Lemon Street and Hewes Street is shown on **Figure 3**.

The buffer will enhance the visibility of the bike lane and increase the separation between motor vehicles and bicyclists. The buffered bike lane also provides space for bicyclists to pass another bicyclist without encroaching into the adjacent motor vehicle lane. According to the guidelines from U.S. Department of Transportation Federal Highway Administration (FHWA), the range of lane widths for urban arterials and collectors ranges from 10 to 12 feet.[3] The Orange County Highway Design Manual Section 1003.2 requires a Class II bike lane on a curbed street to be a minimum of five feet.

[3] Table 3: Range for Lane Width, U.S. Department of Transportation Federal Highway Administration, https://safety.fhwa.dot.gov/geometric/pubs/mitigationstrategies/chapter3/3_lanewidth.cfm

Figure 2: Roadway Cross-section for Villa Park Road Between Wanda Street and Lemon Street



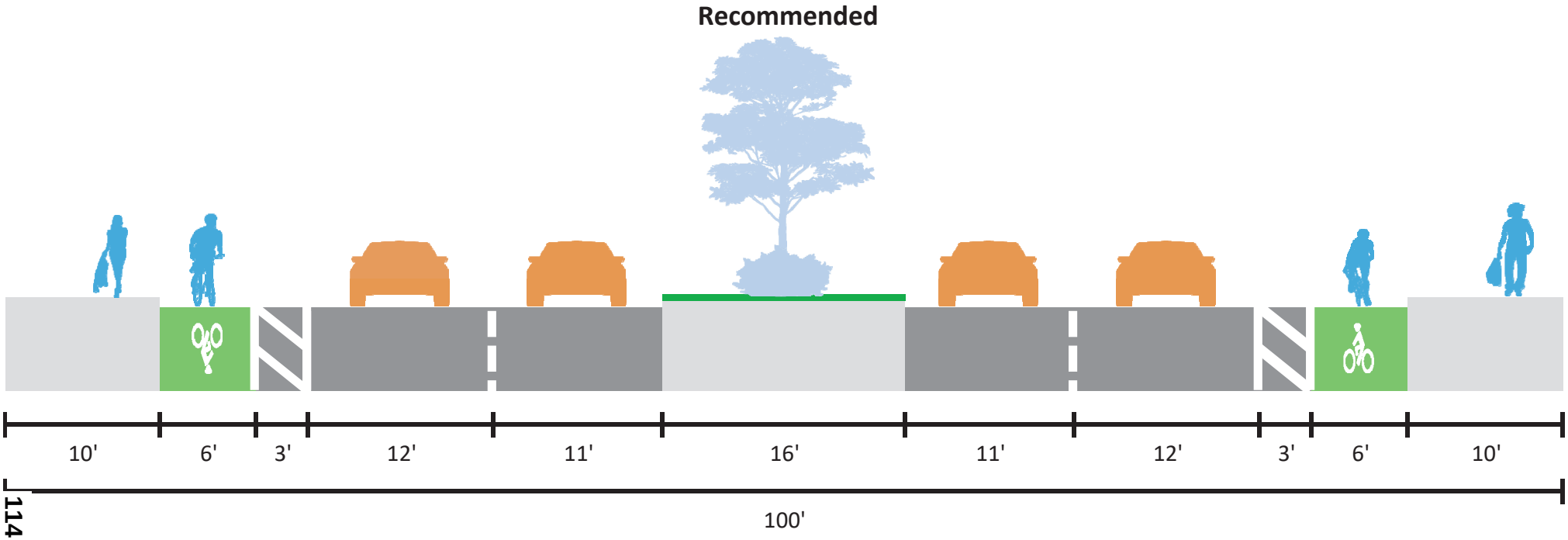
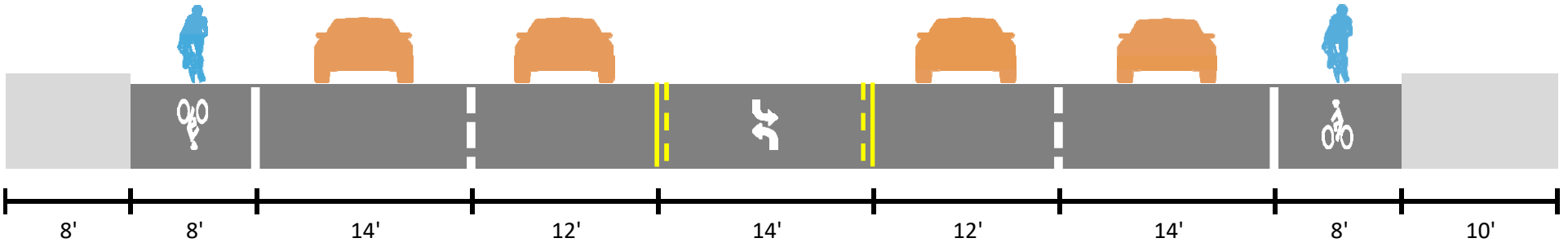


Figure 3: Recommended Roadway Cross-section for Villa Park Road Between Lemon Street and Hewes Street

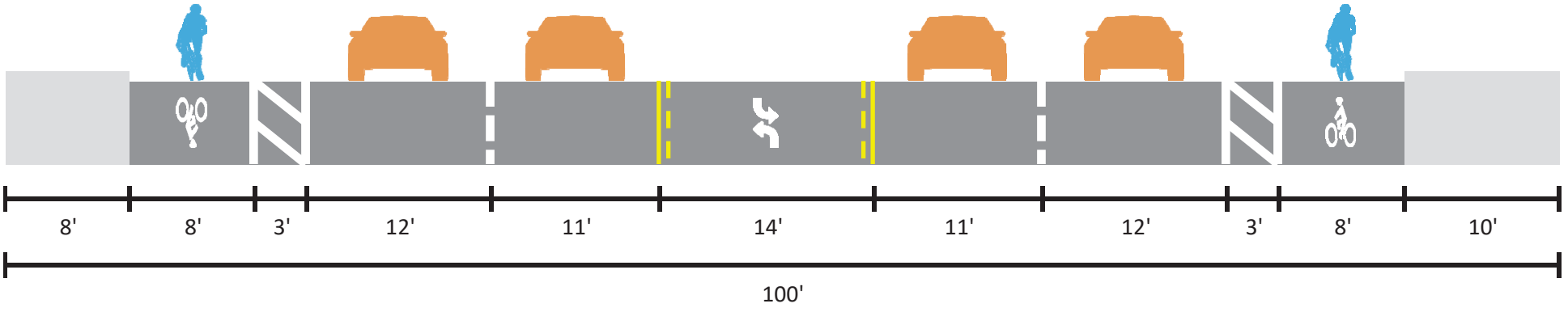
WB Side

EB Side

Existing



Recommended



Villa Park Road Between Hewes Street and Cannon Street

This segment of Villa Park Road will degrade to LOS D, which is below the County's standard. The signalized intersection of Hewes Street and Villa Park Road will not meet the County's standard, and conditions are forecast to worsen at the signalized intersection of Cannon Street and Villa Park Road and unsignalized intersection of Linda Vista Street and Villa Park Road. Vehicle queues at each of these intersections are forecast to extend past available storage, in some cases as far as to adjacent intersections.

To provide conditions that meet the County's operating standards, we recommend the westbound direction of the segment be restriped from two to three lanes between Hewes Street and Cannon Street. This recommendation requires the narrowing of through travel lanes and the center median to 11 feet each to accommodate a third 11-foot westbound vehicle lane. Further, we recommend extending the bike buffer to Cannon Street from Hewes Street by modifying the existing eight-foot Class II bike lane to a five-foot bike lane and a three-foot striped buffer. The recommended cross-section for Villa Park Road between Hewes Street and Cannon Street is shown on **Figure 4**.

In addition to the broader recommendations of a third westbound through lane and bike buffer, these elements should be included in future plans which are shown in **Attachment G**:

Hewes Street and Villa Park Road

- Restripe the westbound approach to add a trap westbound left-turn lane (two total left-turn lanes)
- Extend the westbound left-turn pocket to 450 feet

Linda Vista Street and Villa Park Road

- Signalize the intersection
- Restripe the intersection to add a westbound through lane (three total through lanes)

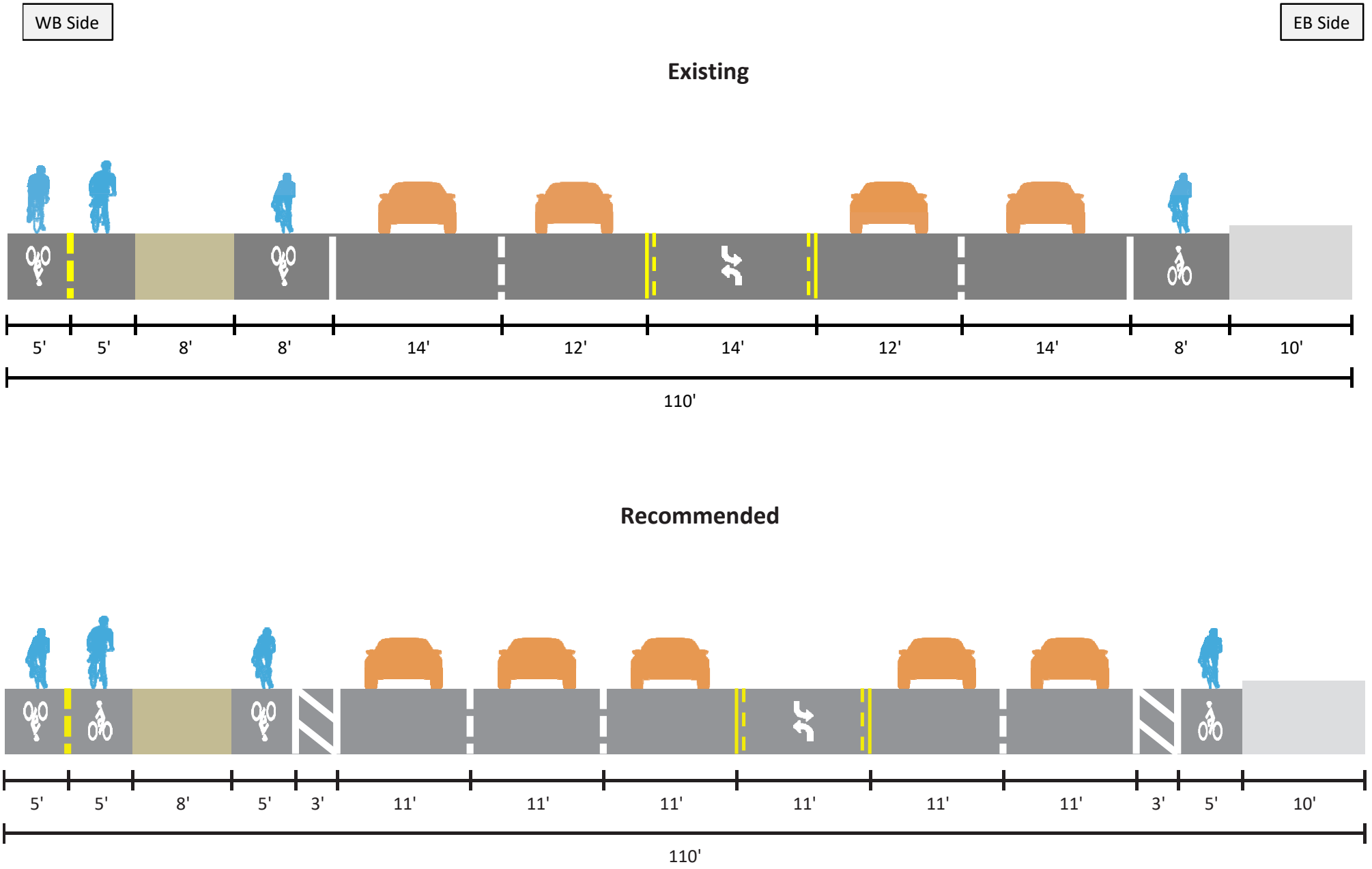
Cannon Street and Santiago Canyon Road

- Stripe a westbound through lane (three total through lanes)
- Add a westbound receiving lane (three total through lanes)
- Restripe one southbound through lane into a trap right-turn lane (two total right-turn lanes)
- Restripe shared southbound through/left lane into a southbound through lane (one total through lane)
- Restripe the existing Class II bike lane between the southbound left-most right-turn lane and the through lane
- Update northbound and southbound signal phasing from split to protected phasing.

The recommended improvement above would meet operational goals and balance multimodal needs along the corridor without requiring roadway widening or additional right-of-way. The recommended cross section provides fewer lanes than what is currently planned in the City of Orange's General Plan Circulation Element. This improvement would not prevent a future project from consideration if future traffic conditions warranted three lanes in each direction.

Fehr & Peers considered other alternatives for the Cannon Street and Santiago Canyon Road intersection but found them to operate inadequately during the PM peak hour. These alternatives were similar to the recommended improvement with a reduction in westbound through lanes and a southbound channelized right turn lane (that would provide a free southbound right movement). A focused alternatives analysis that summarizes average delay results for these alternatives is provided in **Table 8** and was used to screen these alternatives for further consideration.

Figure 4: Recommended Roadway Cross-section for Villa Park Road Between Hewes Street and Cannon Street



LOS Results with Recommended Improvements

The recommended improvements above will provide acceptable daily roadway LOS, peak hour LOS and queues within provided storage. The bicycle and pedestrian delays will also be reduced by the capacity improvements, though not to better than LOS D conditions. **Table 4** summarizes the daily roadway segment LOS results with the improvement. The intersection peak hour LOS and queueing results for the proposed improvements are summarized in **Tables 5, 6,** and **7**, and LOS and queuing worksheets are provided in **Attachments C** and **D**.

Additional directional traffic volume and capacity calculations are provided for Villa Park Road between Hewes Street and Cannon Street. The westbound direction ADT volumes are approximately 1,700 vehicles higher in the existing condition traffic counts and the future forecast westbound ADT is approximately 2,600 vehicles higher than the eastbound direction. As shown, the capacity assessment for the 5-lane cross section as well as the eastbound or westbound isolated direction capacity assessment all result in LOS C or better conditions, further justifying the recommendation for the additional lane in only the westbound direction.

Table 4: Future Year Roadway LOS with Improvements

Segment	Lanes	ADT	Capacity	V/C	LOS
No Improvements					
Villa Park Road between Hewes Street and Cannon Street	4	30,400	37,500	0.81	**D**
With Improvements					
Villa Park Road between Hewes Street and Cannon Street	5	30,400	46,900	0.65	B
Villa Park Road between Hewes Street and Cannon Street (Eastbound)	2	13,900	18,760	0.74	C
Villa Park Road between Hewes Street and Cannon Street (Westbound)	3	16,500	28,140	0.59	A

Note(s):

1. **Bold** symbolizes segment operates below Jurisdictions' LOS standards.
2. Capacities for LOS E were referenced for arterial types from the MPAH.
3. The MPAH does not list capacities for five-lane facilities. The five-lane capacity was estimated by interpolating between the four-lane and 6-lane divided arterial types.

Source: Fehr & Peers, 2023.

Table 5: Future Year Intersection LOS with Improvements

Intersection	Control	Methodology	Peak Hour	Future Year without Improvements		Future Year with Improvements	
				V/C / Delay	LOS	V/C / Delay	LOS
10. Hewes Street and Villa Park Road	Signalized	ICU / HCM	AM PM	**0.955 / 70** 0.889 / 25	**E / E** D / C	0.733 / 41 0.791 / 19	C / D C / B
11. Linda Vista Street and Villa Park Road	TWSC/ Signalized	HCM	AM PM	**>100** **>100**	**F** **F**	0.495 / 14 0.526 / 6	A / B A / A
12. Cannon Street and Santiago Canyon Road	Signalized	ICU / HCM	AM PM	**0.914** / 49 **0.935** / 47	**E** / D **E** / D	0.768 / 36 0.828 / 54	C / D D / D

Note(s):

1. ">100" is reported for highly congested movements where more than 100 seconds is reported.
2. **Bold** symbolizes segment operates below Jurisdictions' LOS standards.

Source: Fehr & Peers, 2023.

Table 6: Future Year Multi-modal LOS with Improvements

Intersection	Mode	Peak Hour	Future Year without Improvements		Future Year with Improvements	
			Delay	LOS	Delay	LOS
1. Wanda Road and Villa Park Road	Bike/ Pedestrian	AM PM	14.2 / 73.0 47.8 / 60.4	B / E D / E	14.2 / 73.0 47.8 / 60.4	B / E D / E
9. Lemon Street and Villa Park Road	Bike/ Pedestrian	AM PM	5.8 / 8.3 6.2 / 9.6	A / A A / A	5.8 / 8.3 6.2 / 9.6	A / A A / A
10. Hewes Street and Villa Park Road	Bike/ Pedestrian	AM PM	62.7 / 63.9 58.7 / 65.6	E / E E / E	62.6 / 63.9 47.1 / 50.6	E / E D / D
12. Cannon Street and Santiago Canyon Road	Bike/ Pedestrian	AM PM	22.8 / 42.2 64.3 / 76.8	B / D E / E	19.8 / 41.0 39.5 / 49.5	B / D D / D

Source: Fehr & Peers, 2023.

Table 7: Future Year 95th Percentile Queues with Improvements

Intersection	Movement	Storage (ft)	Peak Hour	Future Year without Improvements Queue (ft)	Future Year with Improvements Queue (ft)
10. Hewes Street and Villa Park Road	EBT	>1,000	AM	1,025	825
			PM	475	200
	WBL	425[3]	AM	**1,075**	425
			PM	**350**	200
	WBT	550	AM	425	425
			PM	175	150
	NBL	550	AM	450	425
			PM	275	225
	NBR	550	AM	125	125
			PM	225	200
12. Cannon Street and Santiago Canyon Road	EBL	425	AM	175	200
			PM	**675**	<u>425</u>
	EBT	675	AM	375	500
			PM	250	200
	WBL	225	AM	50	50
			PM	50	50
	WBT	>1,000	AM	325	175
			PM	925	500
	WBR	700	AM	0	0
			PM	625	575
	NBL	150	AM	**225**	<u>150</u>
			PM	125	125
	NBT	500	AM	125	125
			PM	350	325
	SBL	550	AM	475	<u>550</u>
			PM	250	350
	SBT	>1,000	AM	475	450
			PM	275	300
	SBR	400	AM	**1,025**	75
			PM	**600**	75

Notes:
1. Queues were calculated using HCM 7th methodologies and are rounded up to the nearest 25-foot increment.
2. "EB" = eastbound, "WB" = westbound, "NB" = northbound, "SB" = southbound, "L" = left, "T" = through, "R" = right.
3. Hewes Street and Villa Park Road WBL turn pocket is proposed to be widened to 450' under Future Year with Improvements conditions.
4. **Bold** symbolizes movement is at or exceeds available storage capacity.
5. <u>Underlined</u> symbolizes movement is at capacity.

Source: Fehr & Peers, 2023.

Table 8 shows the forecasted intersection operations at Intersection 12: Cannon Street and Santiago Canyon Road under the three considered alternatives. Only Alternative 1 provides acceptable conditions during the PM peak hour.

Table 8: Cannon Street/Santiago Canyon Road Future Year Intersection LOS Alternative Results, PM Peak Hour

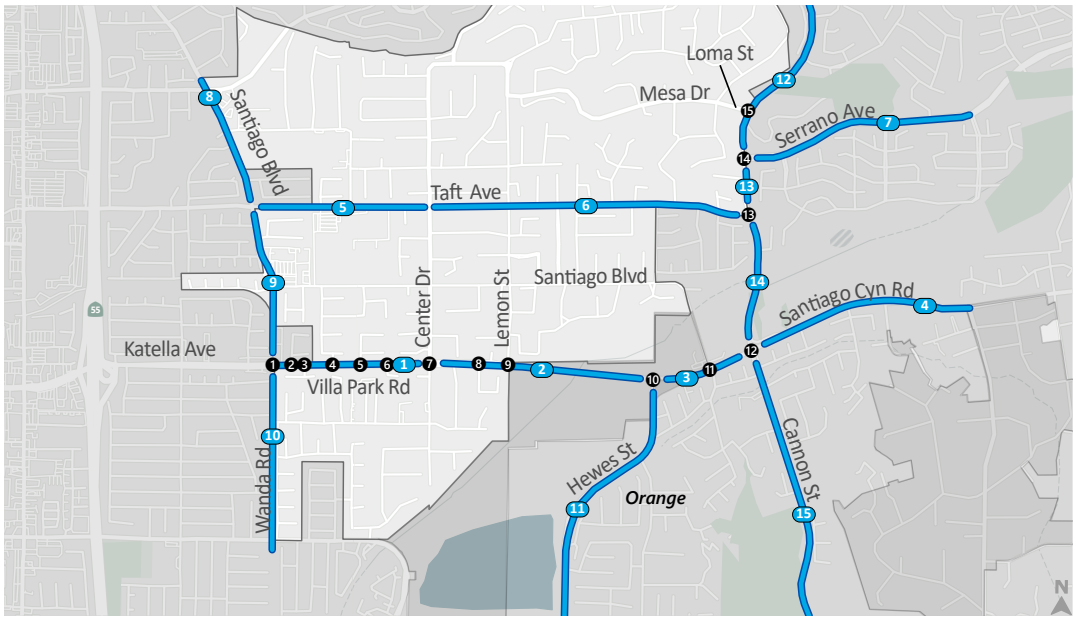
Alternative	Methodology	Future Year with Improvements	
		Delay (sec)	LOS
Alternative 1 (Recommended): Three WB Through Lanes Two SB Right Turn Lanes	HCM	36	D
Alternative 2: Two WB Through Lanes Two SB Right Turn Lanes	HCM	**59**	**E**
Alternative 3: Two WB Through Lanes One SB Right Turn Lane (Channelized)	HCM	**64**	**E**

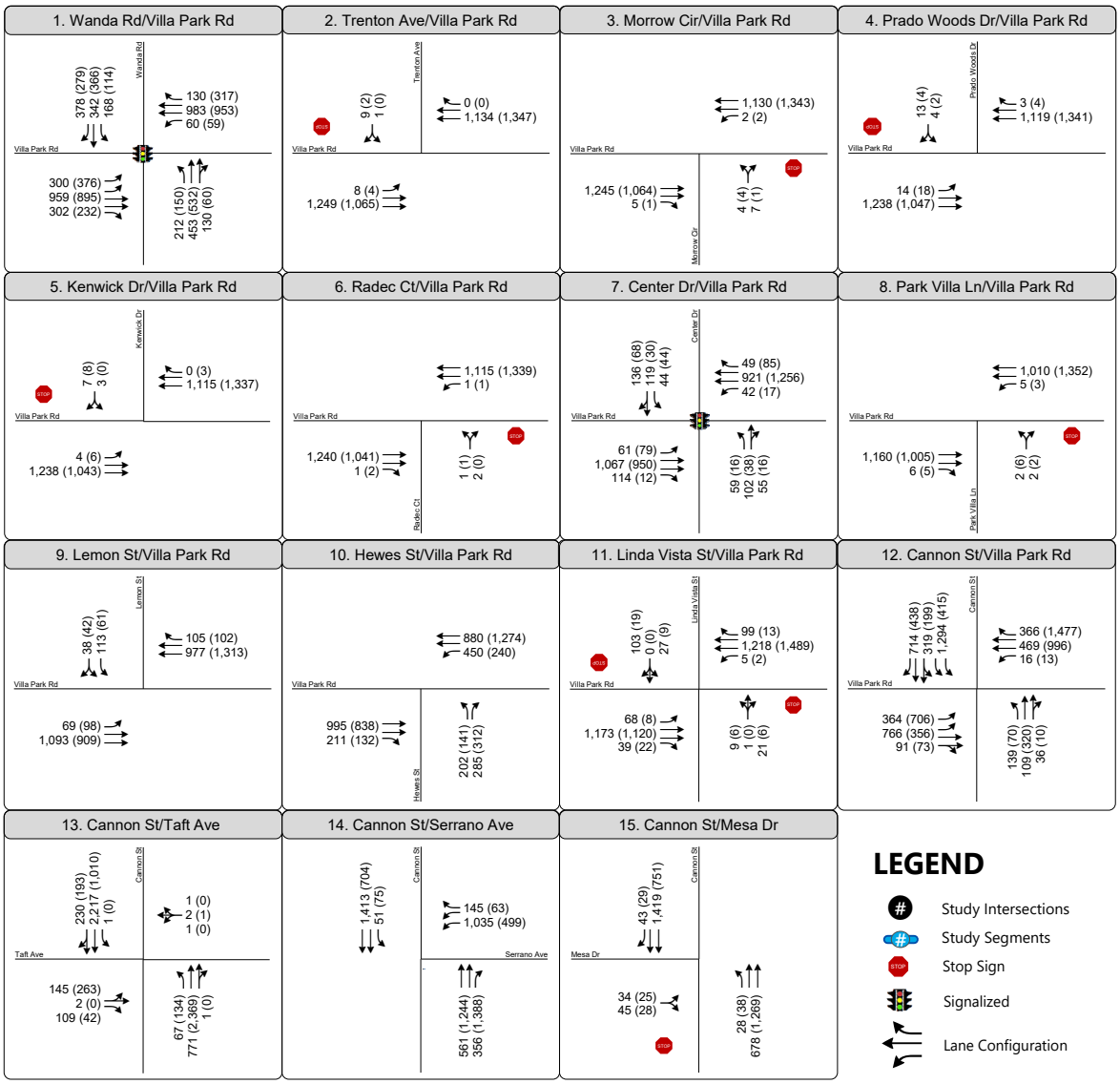
Source: Fehr & Peers, 2024.

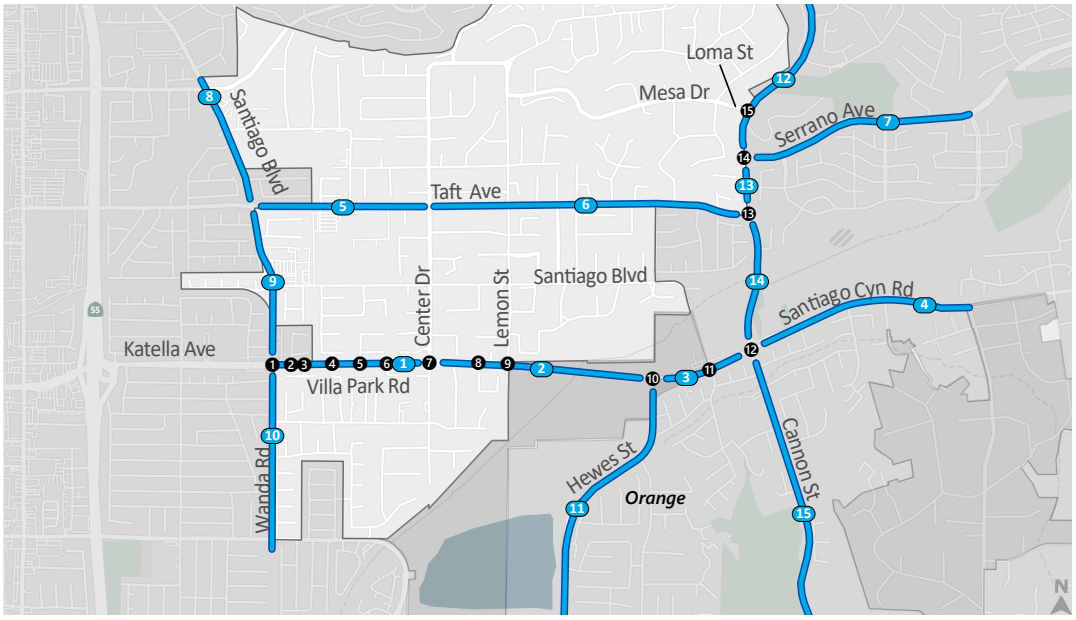
6. Vehicle Miles Traveled (VMT) Assessment

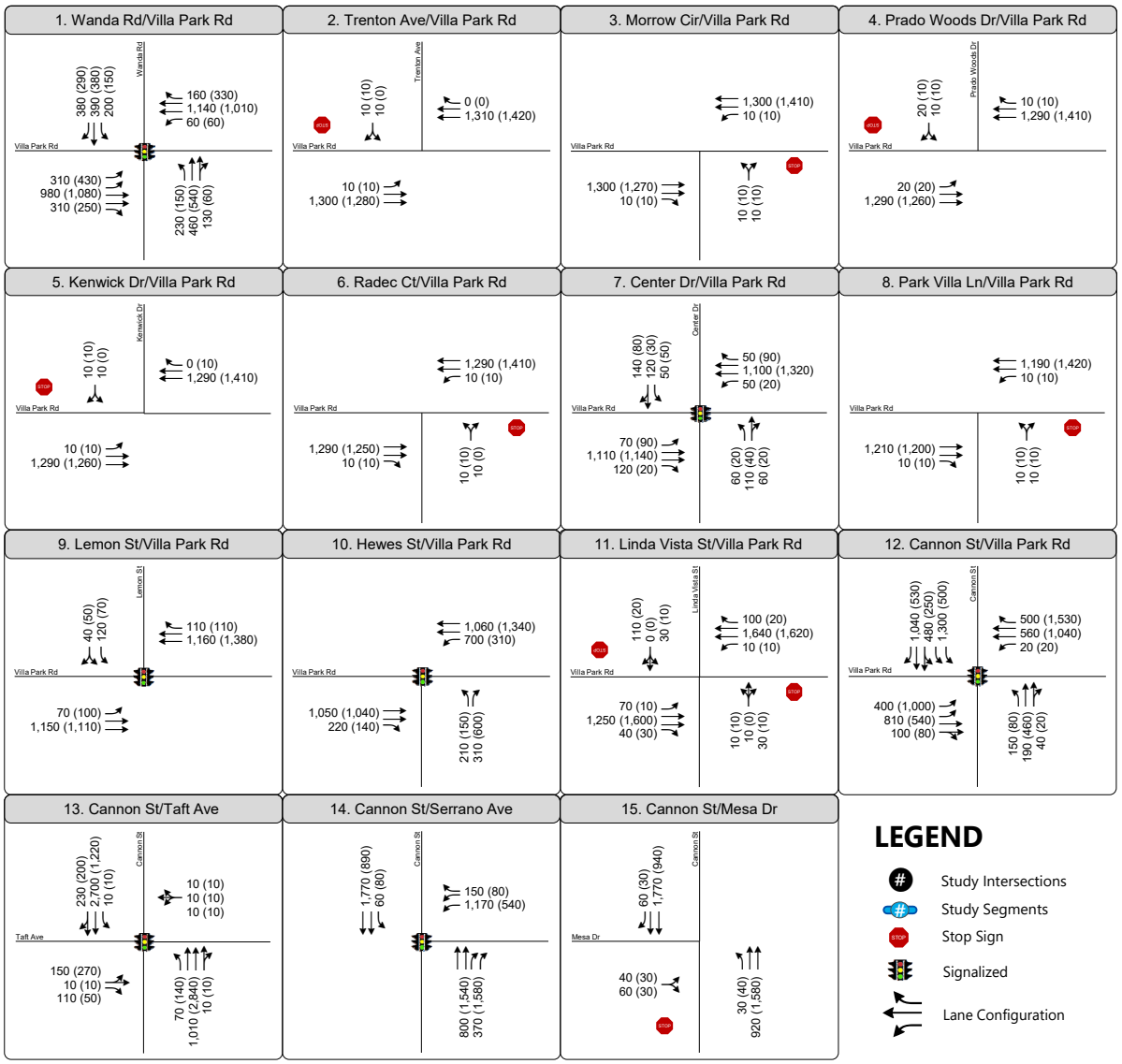
Villa Park Road from Lemon Street to west of Cannon Street is planned to be a six-lane Major Arterial in the current MPAH. According to the operations and cross-section analysis, a five-lane design (three westbound lanes and two eastbound lanes) along Villa Park Road between Hewes Street and Cannon Street is sufficient to serve future traffic demand. The above recommendation won't add additional capacity than what has been assumed in adopted Circulation Plans and EIRs for the County and City of Orange and Villa Park. Since no capacity is being added we can conclude that no VMT will be induced, and that the change to the MPAH designation would result in a less-than-significant transportation impact.

Attachment A: Traffic Volume Data


Loma St
Mesa Dr
Serrano Ave
Santiago Blvd
Taft Ave
Center Dr
Lemon St
Santiago Blvd
Santiago Cyn Rd
Katella Ave
Villa Park Rd
Wanda Rd
Hewes St
Orange
Cannon St
N


1. Wanda Rd/Villa Park Rd
2. Trenton Ave/Villa Park Rd
3. Morrow Cir/Villa Park Rd
4. Prado Woods Dr/Villa Park Rd
5. Kenwick Dr/Villa Park Rd
6. Radec Ct/Villa Park Rd
7. Center Dr/Villa Park Rd
8. Park Villa Ln/Villa Park Rd
9. Lemon St/Villa Park Rd
10. Hewes St/Villa Park Rd
11. Linda Vista St/Villa Park Rd
12. Cannon St/Villa Park Rd
13. Cannon St/Taft Ave
14. Cannon St/Serrano Ave
15. Cannon St/Mesa Dr
LEGEND
Study Intersections
Study Segments
Stop Sign
Signalized
Lane Configuration





Future Year (2045)
Peak Hour Traffic Volumes and Lane Configurations

INTERSECTION TURNING MOVEMENT COUNTS

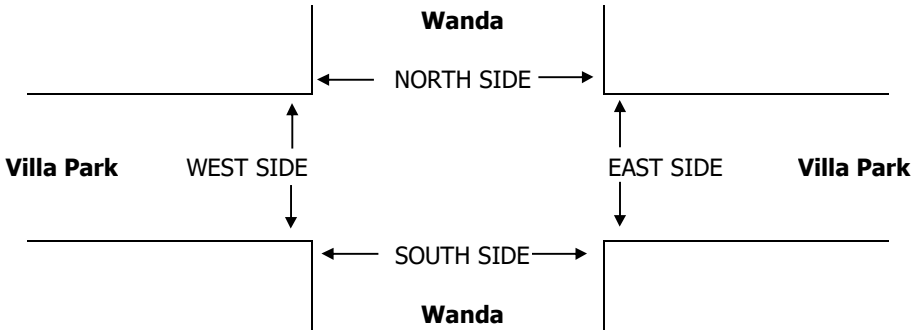
PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE:	LOCATION:	Villa Park - Orange	PROJECT #:	SC3962
Wed, Apr 26, 23	NORTH & SOUTH:	Wanda	LOCATION #:	1
	EAST & WEST:	Villa Park	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N ◄ W E ► S ▼

		NORTHBOUND Wanda			SOUTHBOUND Wanda			EASTBOUND Katella			WESTBOUND Villa Park			
	LANES:	NL 1	NT 2	NR 0	SL 1	ST 1	SR 1	EL 2	ET 2	ER 1	WL 1	WT 2	WR 1	TOTAL
AM	7:00 AM	31	44	14	26	41	60	43	176	37	3	158	20	653
	7:15 AM	30	49	16	31	35	79	55	168	39	9	169	20	700
	7:30 AM	41	90	25	45	78	81	79	267	65	7	263	34	1,075
	7:45 AM	51	127	34	38	105	104	54	270	74	19	251	24	1,151
	8:00 AM	62	145	32	39	88	102	76	221	102	18	253	33	1,171
	8:15 AM	58	91	39	46	71	91	91	201	61	16	216	39	1,020
	8:30 AM	58	74	19	28	70	57	53	147	51	32	194	52	835
	8:45 AM	41	66	14	28	53	64	53	153	47	29	144	50	742
	VOLUMES	372	686	193	281	541	638	504	1,603	476	133	1,648	272	7,347
	APPROACH %	30%	55%	15%	19%	37%	44%	20%	62%	18%	6%	80%	13%	
	APP/DEPART	1,251	/	1,462	1,460	/	1,150	2,583	/	2,077	2,053	/	2,658	0
	BEGIN PEAK HR		7:30 AM											
	VOLUMES	212	453	130	168	342	378	300	959	302	60	983	130	4,417
	APPROACH %	27%	57%	16%	19%	39%	43%	19%	61%	19%	5%	84%	11%	
	PEAK HR FACTOR		0.832			0.899			0.950			0.965		0.943
	APP/DEPART	795	/	883	888	/	704	1,561	/	1,257	1,173	/	1,573	0
PM	4:00 PM	39	107	25	29	74	75	70	201	62	16	206	63	967
	4:15 PM	34	108	17	24	80	64	96	239	67	18	246	65	1,058
	4:30 PM	36	112	19	25	80	58	94	202	78	21	246	59	1,030
	4:45 PM	38	131	17	22	76	60	89	220	77	10	200	67	1,007
	5:00 PM	37	130	16	24	94	64	93	237	70	20	259	76	1,120
	5:15 PM	44	137	18	34	100	89	84	199	43	17	237	93	1,095
	5:30 PM	31	134	9	34	96	66	110	204	42	12	257	81	1,076
	5:45 PM	23	113	22	36	98	64	101	185	45	15	200	65	967
	VOLUMES	282	972	143	228	698	540	737	1,687	484	129	1,851	569	8,320
	APPROACH %	20%	70%	10%	16%	48%	37%	25%	58%	17%	5%	73%	22%	
	APP/DEPART	1,397	/	2,278	1,466	/	1,311	2,908	/	2,058	2,549	/	2,673	0
	BEGIN PEAK HR		4:45 PM											
	VOLUMES	150	532	60	114	366	279	376	860	232	59	953	317	4,298
	APPROACH %	20%	72%	8%	15%	48%	37%	26%	59%	16%	4%	72%	24%	
	PEAK HR FACTOR		0.932			0.851			0.918			0.936		0.959
	APP/DEPART	742	/	1,225	759	/	657	1,468	/	1,034	1,329	/	1,382	0



INTERSECTION TURNING MOVEMENT COUNTS

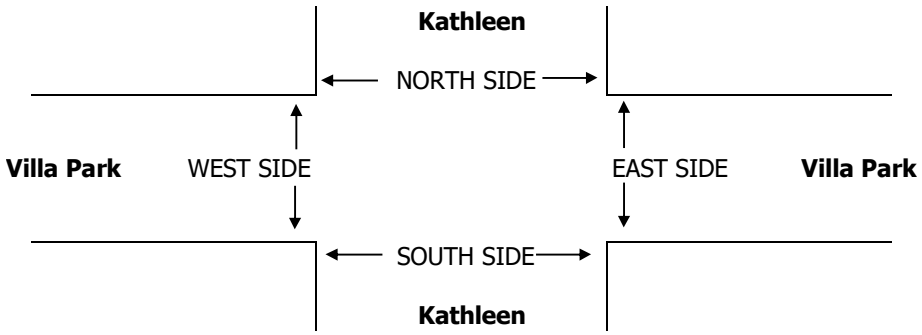
PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE:				
Wed, Apr 26, 23	LOCATION:	Villa Park	PROJECT #:	SC3962
	NORTH & SOUTH:	Kathleen	LOCATION #:	2
	EAST & WEST:	Villa Park	CONTROL:	STOP S

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N ◄ W E ► S ▼

		NORTHBOUND Kathleen			SOUTHBOUND Kathleen			EASTBOUND Katella			WESTBOUND Villa Park			
	LANES:	NL X	NT X	NR X	SL 0	ST X	SR 0	EL 1	ET 2	ER X	WL X	WT 2	WR 0	TOTAL
AM	7:00 AM	0	0	0	0	0	1	1	186	0	0	176	0	364
	7:15 AM	0	0	0	0	0	3	0	219	0	0	195	0	417
	7:30 AM	0	0	0	0	0	4	1	306	0	0	229	0	540
	7:45 AM	0	0	0	0	0	3	3	374	0	0	290	0	670
	8:00 AM	0	0	0	0	0	0	2	277	0	0	282	0	561
	8:15 AM	0	0	0	1	0	2	2	287	0	0	280	0	572
	8:30 AM	0	0	0	0	0	1	0	218	0	0	262	0	481
	8:45 AM	0	0	0	0	0	0	1	188	0	0	217	1	407
	VOLUMES	0	0	0	1	0	14	10	2,055	0	0	1,931	1	4,017
	APPROACH %	0%	0%	0%	7%	0%	93%	0%	99%	0%	0%	100%	0%	
	APP/DEPART	0	/	11	15	/	0	2,070	/	2,056	1,932	/	1,950	0
	BEGIN PEAK HR		7:30 AM											
	VOLUMES	0	0	0	1	0	9	8	1,244	0	0	1,081	0	2,346
	APPROACH %	0%	0%	0%	10%	0%	90%	1%	99%	0%	0%	100%	0%	
	PEAK HR FACTOR		0.000			0.625			0.832			0.932		0.875
	APP/DEPART	0	/	8	10	/	0	1,255	/	1,245	1,081	/	1,093	0
PM	4:00 PM	0	0	0	0	0	1	5	255	0	0	262	0	523
	4:15 PM	0	0	0	0	0	1	0	269	0	0	323	0	593
	4:30 PM	0	0	0	0	0	0	1	268	0	0	302	0	571
	4:45 PM	0	0	0	0	0	0	1	247	0	0	275	0	523
	5:00 PM	0	0	0	0	0	1	1	255	0	0	331	0	588
	5:15 PM	0	0	0	0	0	1	1	264	0	0	360	0	626
	5:30 PM	0	0	0	0	0	4	2	228	0	0	320	0	554
	5:45 PM	0	0	0	0	0	3	1	250	0	0	265	3	522
	VOLUMES	0	0	0	0	0	11	12	2,036	0	0	2,438	3	4,506
	APPROACH %	0%	0%	0%	0%	0%	100%	1%	99%	0%	0%	100%	0%	
	APP/DEPART	0	/	15	11	/	0	2,054	/	2,036	2,441	/	2,455	0
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	VOLUMES	0	0	0	0	0	2	4	1,034	0	0	1,268	0	2,310
	APPROACH %	0%	0%	0%	0%	0%	100%	0%	99%	0%	0%	100%	0%	
	PEAK HR FACTOR		0.000			0.500			0.963			0.881		0.921
	APP/DEPART	0	/	4	2	/	0	1,040	/	1,034	1,268	/	1,272	0



INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

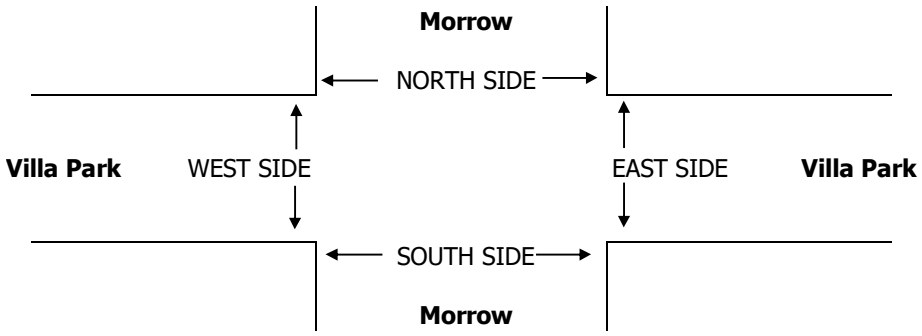
DATE:	LOCATION:	Villa Park	PROJECT #:	SC3962
Wed, Apr 26, 23	NORTH & SOUTH:	Morrow	LOCATION #:	3
	EAST & WEST:	Villa Park	CONTROL:	STOP N

NOTES:

AM / PM / MD / OTHER / OTHER

▲ N ◄ W E ► S ▼

		NORTHBOUND Morrow			SOUTHBOUND Morrow			EASTBOUND Villa Park			WESTBOUND Villa Park			
	LANES:	NL 0	NT X	NR 0	SL X	ST X	SR X	EL X	ET 2	ER 0	WL 1	WT 2	WR X	TOTAL
AM	7:00 AM	3	0	0	0	0	0	0	186	0	1	173	0	363
	7:15 AM	0	0	0	0	0	0	0	218	1	0	195	0	414
	7:30 AM	2	0	2	0	0	0	0	305	1	1	227	0	538
	7:45 AM	0	0	2	0	0	0	0	372	1	0	290	0	665
	8:00 AM	0	0	2	0	0	0	0	276	1	1	282	0	562
	8:15 AM	2	0	1	0	0	0	0	286	2	0	283	0	574
	8:30 AM	1	0	1	0	0	0	0	217	1	0	261	0	481
	8:45 AM	1	0	0	0	0	0	0	186	2	0	217	0	406
	VOLUMES	9	0	8	0	0	0	0	2,046	9	3	1,928	0	4,003
	APPROACH %	53%	0%	47%	0%	0%	0%	0%	100%	0%	0%	100%	0%	
	APP/DEPART	17	/	0	0	/	10	2,055	/	2,056	1,931	/	1,937	0
	BEGIN PEAK HR		7:30 AM											
	VOLUMES	4	0	7	0	0	0	0	1,239	5	2	1,082	0	2,339
	APPROACH %	36%	0%	64%	0%	0%	0%	0%	100%	0%	0%	100%	0%	
	PEAK HR FACTOR		0.688			0.000			0.834			0.934		0.879
	APP/DEPART	11	/	0	0	/	6	1,244	/	1,247	1,084	/	1,086	0
PM	4:00 PM	0	0	1	0	0	0	0	254	1	1	262	0	519
	4:15 PM	1	0	0	0	0	0	0	269	0	0	322	0	592
	4:30 PM	1	0	0	0	0	0	0	268	0	0	301	0	570
	4:45 PM	1	0	1	0	0	0	0	247	0	1	274	0	524
	5:00 PM	1	0	0	1	0	0	0	254	1	0	330	0	587
	5:15 PM	1	0	0	0	0	0	0	264	0	1	359	0	625
	5:30 PM	0	0	0	0	0	0	0	217	1	0	327	0	545
	5:45 PM	1	0	0	0	0	0	0	250	0	0	267	0	518
	VOLUMES	6	0	2	1	0	0	0	2,023	3	3	2,442	0	4,480
	APPROACH %	75%	0%	25%	100%	0%	0%	0%	100%	0%	0%	100%	0%	
	APP/DEPART	8	/	0	1	/	6	2,026	/	2,026	2,445	/	2,448	0
	BEGIN PEAK HR		4:30 PM											
	VOLUMES	4	0	1	1	0	0	0	1,033	1	2	1,264	0	2,306
	APPROACH %	80%	0%	20%	100%	0%	0%	0%	100%	0%	0%	100%	0%	
	PEAK HR FACTOR		0.625			0.250			0.965			0.879		0.922
	APP/DEPART	5	/	0	1	/	3	1,034	/	1,035	1,266	/	1,268	0



INTERSECTION TURNING MOVEMENT COUNTS

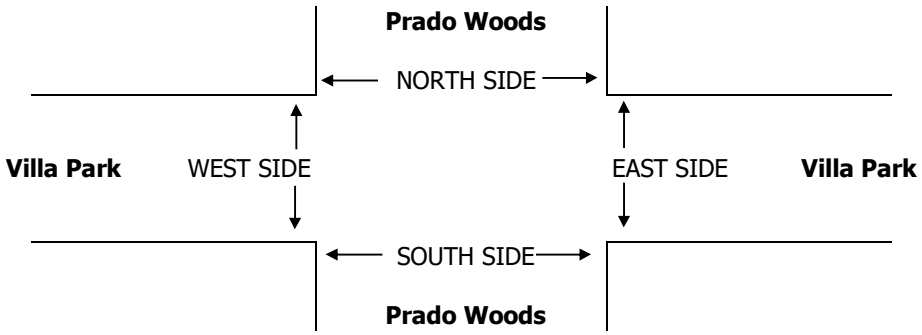
PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE:				
Wed, Apr 26, 23	LOCATION:	Villa Park	PROJECT #:	SC3962
	NORTH & SOUTH:	Prado Woods	LOCATION #:	4
	EAST & WEST:	Villa Park	CONTROL:	STOP S

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N ◄ W E ► S ▼

		NORTHBOUND Prado Woods			SOUTHBOUND Prado Woods			EASTBOUND Villa Park			WESTBOUND Villa Park			
	LANES:	NL X	NT X	NR X	SL 0	ST X	SR 0	EL 1	ET 2	ER X	WL X	WT 2	WR 0	TOTAL
AM	7:00 AM	0	0	0	0	0	2	1	186	0	0	171	0	360
	7:15 AM	0	0	0	1	0	3	2	216	0	0	192	0	414
	7:30 AM	0	0	0	3	0	3	1	307	0	0	225	0	539
	7:45 AM	0	0	0	1	0	4	3	371	0	0	290	3	672
	8:00 AM	0	0	0	0	0	2	4	268	0	0	281	0	555
	8:15 AM	0	0	0	0	0	4	6	281	0	0	277	0	568
	8:30 AM	0	0	0	0	0	2	2	216	0	0	259	0	479
	8:45 AM	0	0	0	0	0	2	3	183	0	0	215	0	403
	VOLUMES	0	0	0	5	0	22	22	2,028	0	0	1,910	3	3,990
	APPROACH %	0%	0%	0%	19%	0%	81%	1%	99%	0%	0%	100%	0%	
	APP/DEPART	0	/	22	27	/	0	2,050	/	2,033	1,913	/	1,935	0
	BEGIN PEAK HR		7:30 AM											
	VOLUMES	0	0	0	4	0	13	14	1,227	0	0	1,073	3	2,334
	APPROACH %	0%	0%	0%	24%	0%	76%	1%	99%	0%	0%	100%	0%	
	PEAK HR FACTOR		0.000			0.708			0.830			0.918		0.868
	APP/DEPART	0	/	15	17	/	0	1,241	/	1,231	1,076	/	1,088	0
PM	4:00 PM	0	0	0	1	0	3	1	254	0	0	266	2	527
	4:15 PM	0	0	0	0	0	0	3	257	0	0	322	1	583
	4:30 PM	0	0	0	0	0	1	7	261	0	0	300	0	569
	4:45 PM	0	0	0	1	0	2	3	245	0	0	273	0	524
	5:00 PM	0	0	0	1	0	0	2	253	0	0	330	1	587
	5:15 PM	0	0	0	0	0	1	6	258	0	0	359	3	627
	5:30 PM	0	0	0	0	0	0	3	214	0	0	327	0	544
	5:45 PM	0	0	0	0	0	1	0	250	0	0	266	1	518
	VOLUMES	0	0	0	3	0	8	25	1,992	0	0	2,443	8	4,479
	APPROACH %	0%	0%	0%	27%	0%	73%	1%	99%	0%	0%	100%	0%	
	APP/DEPART	0	/	33	11	/	0	2,017	/	1,995	2,451	/	2,451	0
	BEGIN PEAK HR		4:30 PM											
	VOLUMES	0	0	0	2	0	4	18	1,017	0	0	1,262	4	2,307
	APPROACH %	0%	0%	0%	33%	0%	67%	2%	98%	0%	0%	100%	0%	
	PEAK HR FACTOR		0.000			0.500			0.965			0.874		0.920
	APP/DEPART	0	/	22	6	/	0	1,035	/	1,019	1,266	/	1,266	0



INTERSECTION TURNING MOVEMENT COUNTS

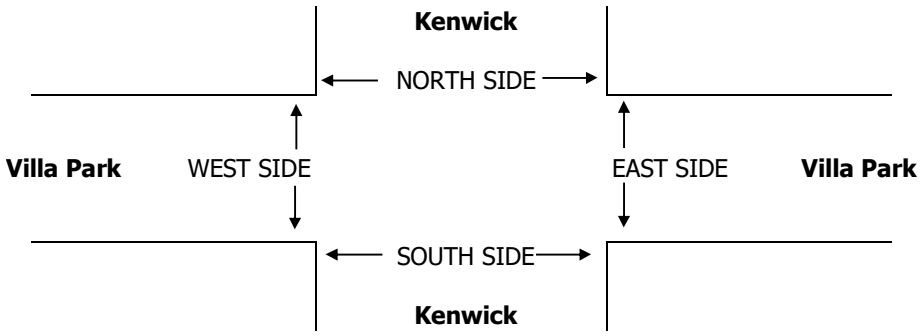
PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE:				
Wed, Apr 26, 23	LOCATION:	Villa Park	PROJECT #:	SC3962
	NORTH & SOUTH:	Kenwick	LOCATION #:	5
	EAST & WEST:	Villa Park	CONTROL:	STOP S

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N ◄ W E ► S ▼

		NORTHBOUND Kenwick			SOUTHBOUND Kenwick			EASTBOUND Villa Park			WESTBOUND Villa Park			
	LANES:	NL X	NT X	NR X	SL 0	ST X	SR 0	EL 1	ET 2	ER X	WL X	WT 2	WR 0	TOTAL
AM	7:00 AM	0	0	0	0	0	1	0	186	0	0	170	0	357
	7:15 AM	0	0	0	0	0	1	0	217	0	0	191	0	409
	7:30 AM	0	0	0	1	0	2	0	304	0	0	223	0	530
	7:45 AM	0	0	0	0	0	2	2	370	0	0	296	0	670
	8:00 AM	0	0	0	2	0	1	0	268	0	0	280	0	551
	8:15 AM	0	0	0	0	0	2	2	279	0	0	275	0	558
	8:30 AM	0	0	0	0	0	0	0	216	0	0	259	0	475
	8:45 AM	0	0	0	0	0	0	1	182	0	0	215	0	398
	VOLUMES	0	0	0	3	0	9	5	2,022	0	0	1,909	0	3,949
	APPROACH %	0%	0%	0%	25%	0%	75%	0%	100%	0%	0%	100%	0%	
	APP/DEPART	0	/	5	12	/	0	2,027	/	2,026	1,910	/	1,918	0
	BEGIN PEAK HR		7:30 AM											
	VOLUMES	0	0	0	3	0	7	4	1,221	0	0	1,074	0	2,309
	APPROACH %	0%	0%	0%	30%	0%	70%	0%	100%	0%	0%	100%	0%	
	PEAK HR FACTOR		0.000			0.833			0.823			0.907		0.862
	APP/DEPART	0	/	4	10	/	0	1,225	/	1,224	1,074	/	1,081	0
PM	4:00 PM	0	0	0	0	0	0	2	253	0	0	268	0	523
	4:15 PM	0	0	0	0	0	0	1	256	0	0	323	0	580
	4:30 PM	0	0	0	0	0	1	1	260	0	0	307	0	569
	4:45 PM	0	0	0	0	0	4	2	244	0	0	269	1	520
	5:00 PM	0	0	0	0	0	1	2	248	0	0	329	1	581
	5:15 PM	0	0	0	0	0	2	1	256	0	0	359	1	619
	5:30 PM	0	0	0	1	0	1	1	213	0	0	326	0	542
	5:45 PM	0	0	0	1	0	1	2	248	0	0	266	1	519
	VOLUMES	0	0	0	2	0	10	12	1,978	0	0	2,447	4	4,455
	APPROACH %	0%	0%	0%	17%	0%	83%	1%	99%	0%	0%	100%	0%	
	APP/DEPART	0	/	16	12	/	0	1,992	/	1,980	2,451	/	2,459	0
	BEGIN PEAK HR		4:30 PM											
	VOLUMES	0	0	0	0	0	8	6	1,008	0	0	1,264	3	2,291
	APPROACH %	0%	0%	0%	0%	0%	100%	1%	99%	0%	0%	100%	0%	
	PEAK HR FACTOR		0.000			0.500			0.973			0.880		0.924
	APP/DEPART	0	/	9	8	/	0	1,016	/	1,008	1,267	/	1,274	0



INTERSECTION TURNING MOVEMENT COUNTS

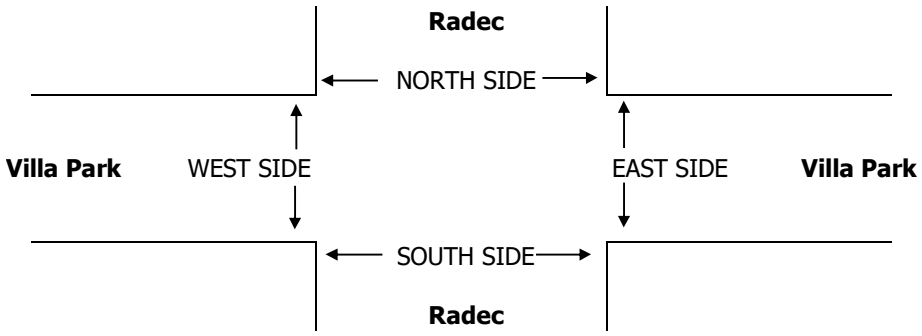
PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE:	LOCATION:	Villa Park	PROJECT #:	SC3962
Wed, Apr 26, 23	NORTH & SOUTH:	Radec	LOCATION #:	6
	EAST & WEST:	Villa Park	CONTROL:	STOP N

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N ◄ W E ► S ▼

		NORTHBOUND Radec			SOUTHBOUND Radec			EASTBOUND Villa Park			WESTBOUND Villa Park			
	LANES:	NL 0	NT X	NR 0	SL X	ST X	SR X	EL X	ET 2	ER 0	WL 1	WT 2	WR X	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	185	1	0	170	0	356
	7:15 AM	0	0	0	0	0	0	0	206	1	0	191	0	398
	7:30 AM	0	0	2	0	0	0	0	303	1	0	222	0	528
	7:45 AM	1	0	0	0	0	0	0	370	0	1	295	0	667
	8:00 AM	0	0	0	0	0	0	0	270	0	0	286	0	556
	8:15 AM	0	0	0	0	0	0	0	279	0	0	275	0	554
	8:30 AM	0	0	0	0	0	0	0	215	0	0	258	0	473
	8:45 AM	1	0	0	0	0	0	0	183	0	0	215	0	399
	VOLUMES	2	0	2	0	0	0	0	2,011	3	1	1,912	0	3,935
	APPROACH %	50%	0%	50%	0%	0%	0%	0%	100%	0%	0%	100%	0%	
	APP/DEPART	4	/	0	0	/	4	2,016	/	2,015	1,915	/	1,916	0
	BEGIN PEAK HR		7:30 AM											
	VOLUMES	1	0	2	0	0	0	0	1,222	1	1	1,078	0	2,307
	APPROACH %	33%	0%	67%	0%	0%	0%	0%	100%	0%	0%	100%	0%	
	PEAK HR FACTOR		0.375			0.000			0.827			0.912		0.865
	APP/DEPART	3	/	0	0	/	2	1,224	/	1,225	1,080	/	1,080	0
PM	4:00 PM	0	0	1	0	0	0	0	252	1	0	268	0	522
	4:15 PM	0	0	0	0	0	0	0	256	0	0	323	0	579
	4:30 PM	0	0	0	0	0	0	0	260	0	0	307	0	567
	4:45 PM	1	0	0	0	0	0	0	242	2	1	279	0	525
	5:00 PM	0	0	0	0	0	0	0	244	0	0	330	0	574
	5:15 PM	0	0	0	0	0	0	0	256	0	0	360	0	616
	5:30 PM	0	0	0	0	0	0	0	214	0	0	326	0	540
	5:45 PM	0	0	0	0	0	0	0	249	0	0	267	0	516
	VOLUMES	1	0	1	0	0	0	0	1,973	3	1	2,460	0	4,439
	APPROACH %	50%	0%	50%	0%	0%	0%	0%	100%	0%	0%	100%	0%	
	APP/DEPART	2	/	0	0	/	4	1,976	/	1,974	2,461	/	2,461	0
	BEGIN PEAK HR		4:30 PM											
	VOLUMES	1	0	0	0	0	0	0	1,002	2	1	1,276	0	2,282
	APPROACH %	100%	0%	0%	0%	0%	0%	0%	100%	0%	0%	100%	0%	
	PEAK HR FACTOR		0.250			0.000			0.965			0.887		0.926
	APP/DEPART	1	/	0	0	/	3	1,004	/	1,002	1,277	/	1,277	0



INTERSECTION TURNING MOVEMENT COUNTS

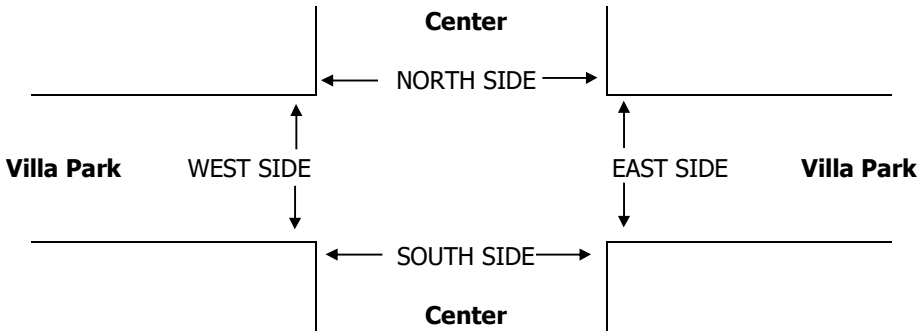
PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE:	LOCATION:	Villa Park	PROJECT #:	SC3962
Wed, Apr 26, 23	NORTH & SOUTH:	Center	LOCATION #:	7
	EAST & WEST:	Villa Park	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N ◄ W E ► S ▼

		NORTHBOUND Center			SOUTHBOUND Center			EASTBOUND Villa Park			WESTBOUND Villa Park			
	LANES:	NL 1	NT 1	NR 0	SL 1	ST 1	SR 0	EL 1	ET 2	ER 0	WL 1	WT 2	WR 0	TOTAL
AM	7:00 AM	1	1	2	6	2	14	11	165	6	4	154	1	367
	7:15 AM	2	5	10	10	8	10	11	192	5	1	180	7	441
	7:30 AM	10	10	6	10	25	15	9	256	31	10	200	10	592
	7:45 AM	27	49	28	6	49	25	23	286	65	23	250	13	844
	8:00 AM	20	29	16	13	23	48	17	245	14	6	220	16	667
	8:15 AM	2	14	5	15	22	48	12	262	4	3	227	10	624
	8:30 AM	5	7	9	18	15	43	14	197	1	4	210	12	535
	8:45 AM	5	2	6	6	6	19	10	170	6	4	190	11	435
	VOLUMES	72	117	82	84	150	222	107	1,773	132	55	1,631	80	4,505
	APPROACH %	27%	43%	30%	18%	33%	49%	5%	88%	7%	3%	92%	5%	
	APP/DEPART	271	/	302	456	/	336	2,012	/	1,940	1,766	/	1,927	0
	BEGIN PEAK HR		7:30 AM											
	VOLUMES	59	102	55	44	119	136	61	1,049	114	42	897	49	2,727
	APPROACH %	27%	47%	25%	15%	40%	45%	5%	86%	9%	4%	91%	5%	
	PEAK HR FACTOR		0.519			0.879			0.818			0.864		0.808
	APP/DEPART	216	/	211	299	/	275	1,224	/	1,148	988	/	1,093	0
PM	4:00 PM	4	7	7	7	11	19	25	233	4	8	245	16	586
	4:15 PM	5	8	9	10	6	18	18	242	3	12	313	10	654
	4:30 PM	1	5	6	10	7	22	17	227	4	4	270	15	588
	4:45 PM	5	9	6	10	8	11	24	222	2	6	277	28	608
	5:00 PM	2	12	3	15	9	19	22	219	3	2	302	22	630
	5:15 PM	8	12	1	9	6	16	16	241	3	5	346	20	683
	5:30 PM	2	11	3	12	9	20	15	197	5	2	300	10	586
	5:45 PM	5	6	2	12	7	20	21	202	4	5	244	9	537
	VOLUMES	32	70	37	85	63	145	158	1,783	28	44	2,297	130	4,872
	APPROACH %	23%	50%	27%	29%	22%	49%	8%	91%	1%	2%	93%	5%	
	APP/DEPART	139	/	358	293	/	134	1,969	/	1,906	2,471	/	2,474	0
	BEGIN PEAK HR		4:30 PM											
	VOLUMES	16	38	16	44	30	68	79	909	12	17	1,195	85	2,509
	APPROACH %	23%	54%	23%	31%	21%	48%	8%	91%	1%	1%	92%	7%	
	PEAK HR FACTOR		0.833			0.826			0.962			0.874		0.918
	APP/DEPART	70	/	202	142	/	59	1,000	/	969	1,297	/	1,279	0



INTERSECTION TURNING MOVEMENT COUNTS

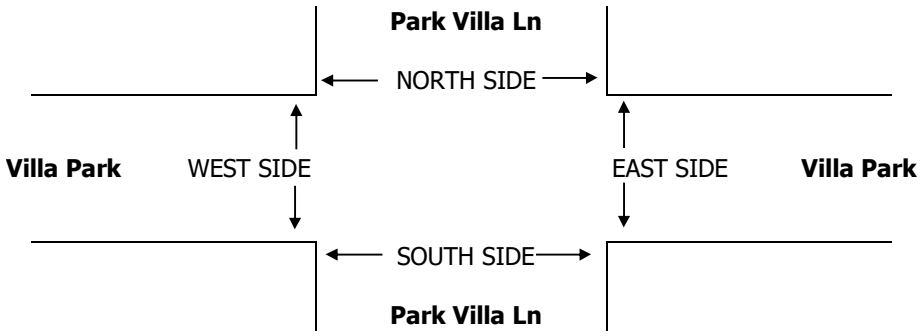
PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE:				
Wed, Apr 26, 23	LOCATION:	Villa Park	PROJECT #:	SC3962
	NORTH & SOUTH:	Park Villa Ln	LOCATION #:	8
	EAST & WEST:	Villa Park	CONTROL:	STOP N

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N ◄ W E ► S ▼

		NORTHBOUND Park Villa Ln			SOUTHBOUND Park Villa Ln			EASTBOUND Villa Park			WESTBOUND Villa Park			
	LANES:	NL 0	NT X	NR 0	SL X	ST X	SR X	EL X	ET 2	ER 0	WL 1	WT 2	WR X	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	170	0	0	159	0	329
	7:15 AM	1	0	0	0	0	0	0	216	0	0	189	0	406
	7:30 AM	0	0	0	0	0	0	0	272	2	0	224	0	498
	7:45 AM	0	0	1	0	0	0	0	305	0	1	286	0	593
	8:00 AM	2	0	0	0	0	0	0	285	0	2	252	0	541
	8:15 AM	0	0	1	0	0	0	0	283	4	2	223	0	513
	8:30 AM	3	0	3	0	0	0	0	224	1	1	231	0	463
	8:45 AM	3	0	3	0	0	0	0	184	0	0	197	0	387
	VOLUMES	9	0	8	0	0	0	0	1,939	7	6	1,761	0	3,730
	APPROACH %	53%	0%	47%	0%	0%	0%	0%	100%	0%	0%	100%	0%	
	APP/DEPART	17	/	0	0	/	10	1,946	/	1,950	1,767	/	1,770	0
	BEGIN PEAK HR		7:30 AM											
	VOLUMES	2	0	2	0	0	0	0	1,145	6	5	985	0	2,145
	APPROACH %	50%	0%	50%	0%	0%	0%	0%	99%	1%	1%	99%	0%	
	PEAK HR FACTOR		0.500			0.000			0.943			0.862		0.904
	APP/DEPART	4	/	0	0	/	8	1,151	/	1,150	990	/	987	0
PM	4:00 PM	1	0	0	0	0	0	0	261	5	2	276	0	545
	4:15 PM	2	0	3	0	0	0	0	255	2	1	337	0	600
	4:30 PM	1	0	0	0	0	0	0	237	3	0	287	0	528
	4:45 PM	2	0	2	0	0	0	0	241	1	2	332	0	580
	5:00 PM	1	0	0	0	0	0	0	238	1	0	321	0	561
	5:15 PM	2	0	0	0	0	0	0	242	0	1	359	0	604
	5:30 PM	0	0	1	0	0	0	0	221	0	0	301	0	523
	5:45 PM	0	0	0	0	0	0	0	208	0	1	260	0	469
	VOLUMES	9	0	6	0	0	0	0	1,903	12	7	2,473	0	4,410
	APPROACH %	60%	0%	40%	0%	0%	0%	0%	99%	1%	0%	100%	0%	
	APP/DEPART	15	/	0	0	/	15	1,915	/	1,913	2,480	/	2,482	0
	BEGIN PEAK HR		4:30 PM											
	VOLUMES	6	0	2	0	0	0	0	958	5	3	1,299	0	2,273
	APPROACH %	75%	0%	25%	0%	0%	0%	0%	99%	1%	0%	100%	0%	
	PEAK HR FACTOR		0.500			0.000			0.995			0.904		0.941
	APP/DEPART	8	/	0	0	/	7	963	/	961	1,302	/	1,305	0



INTERSECTION TURNING MOVEMENT COUNTS

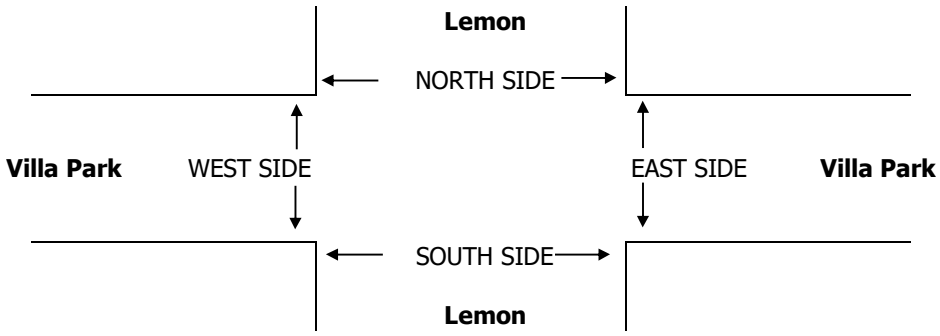
PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE:					
Wed, Apr 26, 23	LOCATION:	Villa Park - Orange	PROJECT #:	SC3962
	NORTH & SOUTH:	Lemon	LOCATION #:	9
	EAST & WEST:	Villa Park	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

		NORTHBOUND Lemon			SOUTHBOUND Lemon			EASTBOUND Villa Park			WESTBOUND Villa Park			
	LANES:	NL X	NT X	NR X	SL 1.5	ST X	SR 0.5	EL 1	ET 2	ER X	WL X	WT 2	WR 0	TOTAL
AM	7:00 AM	0	0	0	12	0	6	13	163	0	0	152	4	350
	7:15 AM	0	0	0	18	0	5	16	203	0	0	184	10	436
	7:30 AM	0	0	0	20	0	4	13	261	0	0	228	11	537
	7:45 AM	0	0	0	31	0	13	14	307	0	0	275	33	673
	8:00 AM	0	0	0	34	0	10	19	251	0	0	242	35	591
	8:15 AM	0	0	0	28	0	11	23	260	0	0	215	26	563
	8:30 AM	0	0	0	19	0	15	27	201	0	0	217	12	491
	8:45 AM	0	0	0	16	0	9	19	164	0	0	188	15	411
	VOLUMES	0	0	0	178	0	73	144	1,810	0	0	1,701	146	4,052
	APPROACH %	0%	0%	0%	71%	0%	29%	7%	93%	0%	0%	92%	8%	
	APP/DEPART	0	/	289	251	/	0	1,954	/	1,988	1,847	/	1,775	0
	BEGIN PEAK HR		7:30 AM											
	VOLUMES	0	0	0	113	0	38	69	1,079	0	0	960	105	2,364
	APPROACH %	0%	0%	0%	75%	0%	25%	6%	94%	0%	0%	90%	10%	
	PEAK HR FACTOR		0.000			0.858			0.894			0.864		0.878
	APP/DEPART	0	/	173	151	/	0	1,148	/	1,192	1,065	/	999	0
PM	4:00 PM	0	0	0	7	0	5	12	225	0	0	278	28	555
	4:15 PM	0	0	0	15	0	10	20	246	0	0	316	28	635
	4:30 PM	0	0	0	14	0	11	25	213	0	0	267	24	554
	4:45 PM	0	0	0	13	0	8	23	232	0	0	323	22	621
	5:00 PM	0	0	0	15	0	15	20	223	0	0	303	22	598
	5:15 PM	0	0	0	18	0	10	26	201	0	0	344	32	631
	5:30 PM	0	0	0	15	0	9	29	208	0	0	292	26	579
	5:45 PM	0	0	0	15	0	6	19	187	0	0	284	33	544
	VOLUMES	0	0	0	112	0	74	174	1,735	0	0	2,407	215	4,717
	APPROACH %	0%	0%	0%	60%	0%	40%	9%	91%	0%	0%	92%	8%	
	APP/DEPART	0	/	386	186	/	0	1,909	/	1,847	2,622	/	2,484	0
	BEGIN PEAK HR		4:45 PM											
	VOLUMES	0	0	0	61	0	42	98	864	0	0	1,262	102	2,429
	APPROACH %	0%	0%	0%	59%	0%	41%	10%	90%	0%	0%	93%	7%	
	PEAK HR FACTOR		0.000			0.858			0.943			0.907		0.962
	APP/DEPART	0	/	198	103	/	0	962	/	925	1,364	/	1,306	0



INTERSECTION TURNING MOVEMENT COUNTS

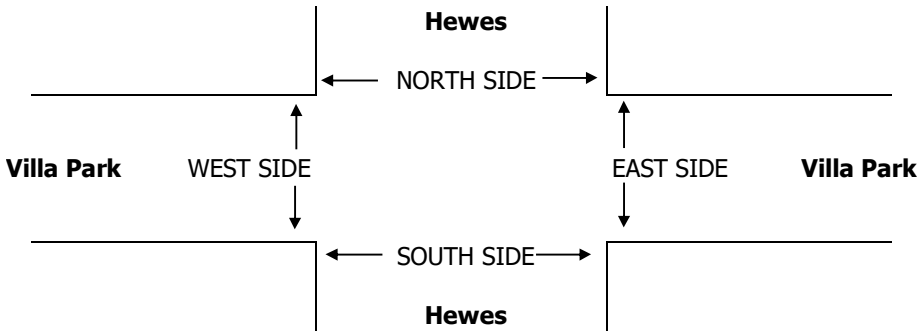
PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE:	LOCATION:	Orange	PROJECT #:	SC3962
Wed, Apr 26, 23	NORTH & SOUTH:	Hewes	LOCATION #:	10
	EAST & WEST:	Villa Park	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N ◄ W E ► S ▼

		NORTHBOUND Hewes			SOUTHBOUND Hewes			EASTBOUND Villa Park			WESTBOUND Villa Park			
	LANES:	NL 1	NT X	NR 1	SL X	ST X	SR X	EL X	ET 2	ER 0	WL 1	WT 2	WR X	TOTAL
AM	7:00 AM	23	0	21	0	0	0	0	142	23	48	139	0	396
	7:15 AM	29	0	40	0	0	0	0	191	32	71	168	0	531
	7:30 AM	44	0	46	0	0	0	0	224	59	85	197	0	655
	7:45 AM	60	0	74	0	0	0	0	257	52	129	267	0	839
	8:00 AM	58	0	75	0	0	0	0	248	47	127	202	0	757
	8:15 AM	40	0	90	0	0	0	0	247	53	109	200	0	739
	8:30 AM	35	0	39	0	0	0	0	180	33	90	204	0	581
	8:45 AM	29	0	39	0	0	0	0	164	22	53	170	0	477
	VOLUMES	318	0	424	0	0	0	0	1,653	321	712	1,547	0	4,975
	APPROACH %	43%	0%	57%	0%	0%	0%	0%	84%	16%	32%	68%	0%	
	APP/DEPART	742	/	0	0	/	1,033	1,974	/	2,077	2,259	/	1,865	0
	BEGIN PEAK HR		7:30 AM											
	VOLUMES	202	0	285	0	0	0	0	976	211	450	866	0	2,990
	APPROACH %	41%	0%	59%	0%	0%	0%	0%	82%	18%	34%	66%	0%	
	PEAK HR FACTOR		0.909			0.000			0.960			0.831		0.891
	APP/DEPART	487	/	0	0	/	661	1,187	/	1,261	1,316	/	1,068	0
PM	4:00 PM	30	0	83	0	0	0	0	208	33	48	281	0	683
	4:15 PM	37	0	62	0	0	0	0	238	32	61	336	0	766
	4:30 PM	40	0	69	0	0	0	0	189	30	59	279	0	666
	4:45 PM	38	0	82	0	0	0	0	176	42	64	292	0	694
	5:00 PM	26	0	99	0	0	0	0	234	28	56	312	0	755
	5:15 PM	33	0	95	0	0	0	0	189	33	61	327	0	738
	5:30 PM	28	0	84	0	0	0	0	190	29	67	294	0	692
	5:45 PM	46	0	71	0	0	0	0	170	34	54	252	0	627
	VOLUMES	278	0	645	0	0	0	0	1,594	261	470	2,373	0	5,621
	APPROACH %	30%	0%	70%	0%	0%	0%	0%	86%	14%	17%	83%	0%	
	APP/DEPART	923	/	0	0	/	731	1,855	/	2,239	2,843	/	2,651	0
	BEGIN PEAK HR		4:15 PM											
	VOLUMES	141	0	312	0	0	0	0	837	132	240	1,219	0	2,881
	APPROACH %	31%	0%	69%	0%	0%	0%	0%	86%	14%	16%	84%	0%	
	PEAK HR FACTOR		0.906			0.000			0.897			0.919		0.940
	APP/DEPART	453	/	0	0	/	372	969	/	1,149	1,459	/	1,360	0



INTERSECTION TURNING MOVEMENT COUNTS

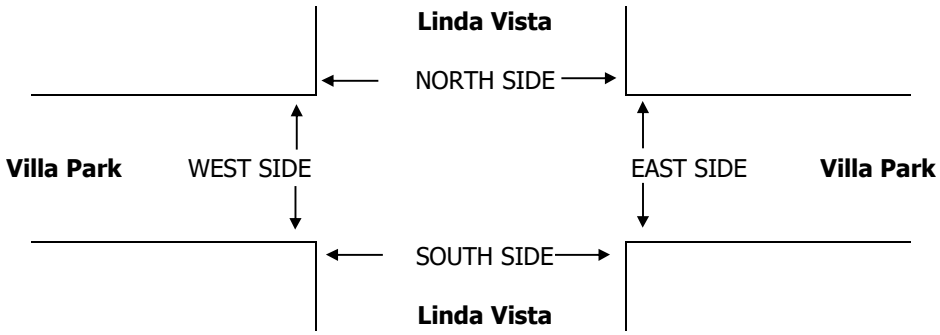
PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE:	LOCATION:	Orange	PROJECT #:	SC3962
Wed, Apr 26, 23	NORTH & SOUTH:	Linda Vista	LOCATION #:	11
	EAST & WEST:	Villa Park	CONTROL:	STOP N/S

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N ◄ W E ► S ▼

		NORTHBOUND Linda Vista			SOUTHBOUND Santiago			EASTBOUND Villa Park			WESTBOUND Villa Park			
	LANES:	NL 0	NT 1	NR 0	SL 0	ST 1	SR 0	EL 1	ET 2	ER 0	WL 1	WT 2	WR 0	TOTAL
AM	7:00 AM	3	0	2	1	0	1	3	165	1	1	182	2	361
	7:15 AM	1	0	5	2	0	6	3	190	6	1	233	5	452
	7:30 AM	2	0	4	5	0	6	7	249	9	1	292	12	587
	7:45 AM	2	0	7	3	0	30	26	307	20	1	309	37	742
	8:00 AM	2	0	3	5	0	37	17	293	6	1	332	16	712
	8:15 AM	3	1	7	14	0	30	18	301	4	2	275	34	689
	8:30 AM	4	0	6	8	0	14	10	229	2	1	262	8	544
	8:45 AM	0	0	2	5	0	12	14	181	6	1	225	6	452
	VOLUMES	17	1	36	43	0	136	98	1,915	54	9	2,110	120	4,539
	APPROACH %	31%	2%	67%	24%	0%	76%	5%	93%	3%	0%	94%	5%	
	APP/DEPART	54	/	219	179	/	63	2,067	/	1,994	2,239	/	2,263	0
	BEGIN PEAK HR		7:30 AM											
	VOLUMES	9	1	21	27	0	103	68	1,150	39	5	1,208	99	2,730
	APPROACH %	29%	3%	68%	21%	0%	79%	5%	91%	3%	0%	92%	8%	
	PEAK HR FACTOR		0.705			0.739			0.890			0.940		0.920
	APP/DEPART	31	/	168	130	/	44	1,257	/	1,198	1,312	/	1,320	0
PM	4:00 PM	0	0	4	6	0	12	2	278	4	1	311	3	621
	4:15 PM	4	0	1	4	0	8	4	278	5	4	349	6	663
	4:30 PM	3	0	3	3	0	13	4	273	3	0	330	6	638
	4:45 PM	3	0	1	3	0	3	1	252	6	0	352	2	623
	5:00 PM	0	0	1	2	0	1	0	323	7	1	394	3	732
	5:15 PM	0	0	1	1	0	2	3	272	6	1	406	2	694
	5:30 PM	4	0	3	1	0	3	0	261	3	3	339	1	618
	5:45 PM	2	0	2	3	0	5	3	254	3	3	315	1	591
	VOLUMES	16	0	16	23	0	47	17	2,191	37	13	2,796	24	5,180
	APPROACH %	50%	0%	50%	33%	0%	67%	1%	98%	2%	0%	99%	1%	
	APP/DEPART	32	/	41	70	/	50	2,245	/	2,230	2,833	/	2,859	0
	BEGIN PEAK HR		4:30 PM											
	VOLUMES	6	0	6	9	0	19	8	1,120	22	2	1,482	13	2,687
	APPROACH %	50%	0%	50%	32%	0%	68%	1%	97%	2%	0%	99%	1%	
	PEAK HR FACTOR		0.500			0.438			0.871			0.915		0.918
	APP/DEPART	12	/	21	28	/	24	1,150	/	1,135	1,497	/	1,507	0



INTERSECTION TURNING MOVEMENT COUNTS

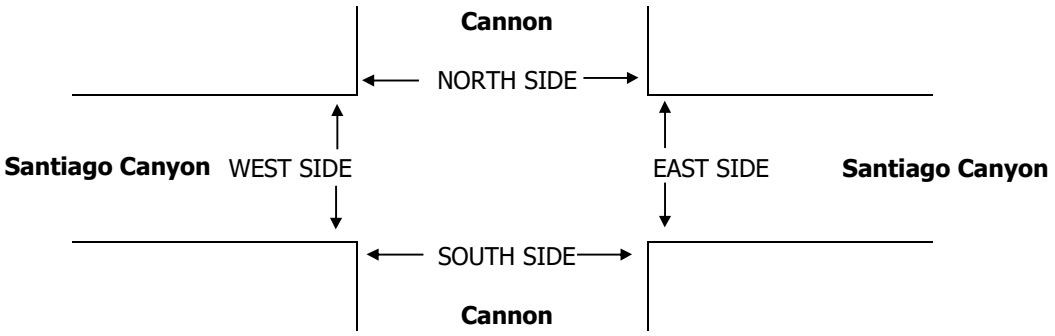
PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE:	LOCATION:	Orange	PROJECT #:	SC3962
Wed, Apr 26, 23	NORTH & SOUTH:	Cannon	LOCATION #:	12
	EAST & WEST:	Santiago Canyon	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N ◄ W E ► S ▼

		NORTHBOUND Cannon			SOUTHBOUND Cannon			EASTBOUND Santiago Canyon			WESTBOUND Santiago Canyon			
	LANES:	NL 1	NT 2	NR 0	SL 2.5	ST 1.5	SR 1	EL 2	ET 2	ER 0	WL 1	WT 2	WR 1	TOTAL
AM	7:00 AM	14	14	2	245	66	117	33	129	6	1	54	37	718
	7:15 AM	16	20	1	338	67	154	58	134	5	1	69	62	925
	7:30 AM	29	23	9	325	86	172	70	160	19	5	104	87	1,089
	7:45 AM	54	25	12	303	87	207	91	182	44	6	96	85	1,192
	8:00 AM	38	36	7	321	54	182	95	194	12	2	129	85	1,155
	8:15 AM	18	25	8	345	50	153	108	198	16	3	140	109	1,173
	8:30 AM	17	19	4	270	72	157	85	148	10	0	97	106	985
	8:45 AM	19	17	3	175	44	117	60	121	7	0	97	84	744
	VOLUMES	205	179	46	2,322	526	1,259	600	1,266	119	18	786	655	7,981
	APPROACH %	48%	42%	11%	57%	13%	31%	30%	64%	6%	1%	54%	45%	
	APP/DEPART	430	/	1,434	4,107	/	663	1,985	/	3,635	1,459	/	2,249	0
	BEGIN PEAK HR		7:30 AM											
	VOLUMES	139	109	36	1,294	277	714	364	734	91	16	469	366	4,609
	APPROACH %	49%	38%	13%	57%	12%	31%	31%	62%	8%	2%	55%	43%	
	PEAK HR FACTOR		0.780			0.957			0.923			0.844		0.967
	APP/DEPART	284	/	839	2,285	/	383	1,189	/	2,065	851	/	1,322	0
PM	4:00 PM	20	53	4	96	27	95	202	61	25	2	200	283	1,068
	4:15 PM	16	63	2	84	28	120	187	76	20	1	223	324	1,144
	4:30 PM	16	55	0	84	33	106	192	67	20	2	218	334	1,127
	4:45 PM	20	81	0	105	44	108	172	73	11	6	226	375	1,221
	5:00 PM	18	81	5	94	30	126	204	97	21	2	254	373	1,305
	5:15 PM	10	65	3	98	42	107	172	82	20	5	292	348	1,244
	5:30 PM	22	93	2	118	44	97	158	86	21	0	224	381	1,246
	5:45 PM	19	86	0	85	42	86	164	73	22	1	214	311	1,103
	VOLUMES	141	577	16	764	290	845	1,451	615	160	19	1,851	2,729	9,458
	APPROACH %	19%	79%	2%	40%	15%	44%	65%	28%	7%	0%	40%	59%	
	APP/DEPART	734	/	4,757	1,899	/	469	2,226	/	1,395	4,599	/	2,837	0
	BEGIN PEAK HR		4:45 PM											
	VOLUMES	70	320	10	415	160	438	706	338	73	13	996	1,477	5,016
	APPROACH %	18%	80%	3%	41%	16%	43%	63%	30%	7%	1%	40%	59%	
	PEAK HR FACTOR		0.855			0.978			0.867			0.964		0.961
	APP/DEPART	400	/	2,503	1,013	/	246	1,117	/	763	2,486	/	1,504	0



INTERSECTION TURNING MOVEMENT COUNTS

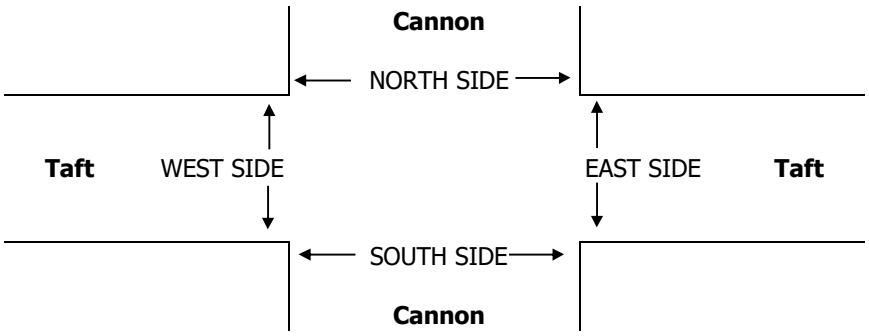
PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE:	LOCATION:	Villa Park - Orange	PROJECT #:	SC3962
Wed, Apr 26, 23	NORTH & SOUTH:	Cannon	LOCATION #:	13
	EAST & WEST:	Taft	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N ◄ W E ► S ▼

		NORTHBOUND Cannon			SOUTHBOUND Cannon			EASTBOUND Taft			WESTBOUND Taft			
	LANES:	NL 1	NT 2	NR 0	SL 1	ST 2	SR 0	EL 0.5	ET 0.5	ER 1	WL 0	WT 1	WR 0	TOTAL
AM	7:00 AM	4	83	0	0	461	28	8	0	12	0	0	0	596
	7:15 AM	3	132	0	0	577	38	13	1	17	0	0	0	781
	7:30 AM	10	155	0	0	600	49	20	0	13	0	0	0	847
	7:45 AM	11	198	1	1	560	57	36	0	27	0	0	1	892
	8:00 AM	22	181	0	0	532	70	45	2	36	1	0	0	889
	8:15 AM	24	212	0	0	504	54	44	0	33	0	2	0	873
	8:30 AM	20	174	0	0	423	44	33	0	26	0	0	0	720
	8:45 AM	24	146	1	0	305	40	28	0	14	0	0	0	558
	VOLUMES	118	1,281	2	1	3,962	380	227	3	178	1	2	1	6,156
	APPROACH %	8%	91%	0%	0%	91%	9%	56%	1%	44%	25%	50%	25%	
	APP/DEPART	1,401	/	1,508	4,343	/	4,141	408	/	6	4	/	501	0
	BEGIN PEAK HR		7:30 AM											
	VOLUMES	67	746	1	1	2,196	230	145	2	109	1	2	1	3,501
	APPROACH %	8%	92%	0%	0%	90%	9%	57%	1%	43%	25%	50%	25%	
	PEAK HR FACTOR		0.862			0.935			0.771			0.500		0.981
	APP/DEPART	814	/	891	2,427	/	2,306	256	/	4	4	/	300	0
PM	4:00 PM	19	536	2	1	220	46	58	0	15	2	0	0	899
	4:15 PM	40	533	0	0	232	51	73	0	12	0	0	0	941
	4:30 PM	24	556	0	0	200	38	75	0	12	0	0	0	905
	4:45 PM	40	567	0	0	271	55	64	0	9	0	0	0	1,006
	5:00 PM	37	606	0	0	234	39	51	0	9	0	0	0	976
	5:15 PM	26	566	0	0	251	51	79	0	12	0	1	0	986
	5:30 PM	31	587	0	0	241	48	69	0	12	0	0	0	988
	5:45 PM	25	521	0	0	183	53	65	1	14	0	0	0	862
	VOLUMES	242	4,472	2	1	1,832	381	534	1	95	2	1	0	7,563
	APPROACH %	5%	95%	0%	0%	83%	17%	85%	0%	15%	67%	33%	0%	
	APP/DEPART	4,716	/	5,006	2,214	/	1,929	630	/	4	3	/	624	0
	BEGIN PEAK HR		4:45 PM											
	VOLUMES	134	2,326	0	0	997	193	263	0	42	0	1	0	3,956
	APPROACH %	5%	95%	0%	0%	84%	16%	86%	0%	14%	0%	100%	0%	
	PEAK HR FACTOR		0.956			0.913			0.838			0.250		0.983
	APP/DEPART	2,460	/	2,589	1,190	/	1,039	305	/	0	1	/	328	0



INTERSECTION TURNING MOVEMENT COUNTS

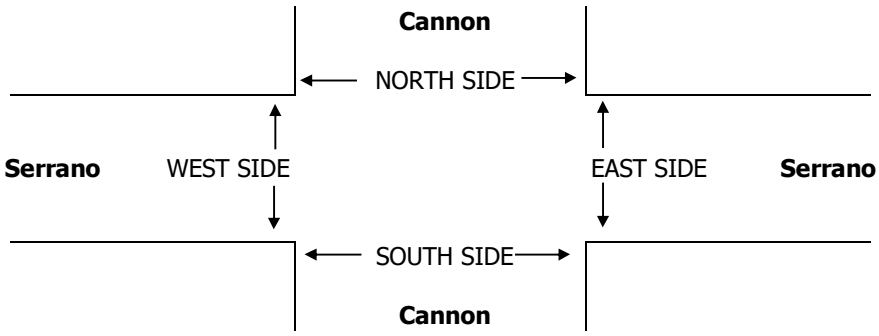
PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE:	LOCATION:	Orange	PROJECT #:	SC3962
Wed, Apr 26, 23	NORTH & SOUTH:	Cannon	LOCATION #:	14
	EAST & WEST:	Serrano	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N ◄ W E ► S ▼

		NORTHBOUND Cannon			SOUTHBOUND Cannon			EASTBOUND Serrano			WESTBOUND Serrano			
	LANES:	NL X	NT 2	NR 1	SL 1	ST 2	SR X	EL X	ET X	ER X	WL 2	WT X	WR 1	TOTAL
AM	7:00 AM	0	56	35	5	243	0	0	0	0	246	0	10	595
	7:15 AM	0	86	59	4	348	0	0	0	0	267	0	17	781
	7:30 AM	0	115	60	9	384	0	0	0	0	276	0	26	870
	7:45 AM	0	134	93	8	351	0	0	0	0	267	0	42	895
	8:00 AM	0	129	96	13	339	0	0	0	0	263	0	50	890
	8:15 AM	0	149	107	21	329	0	0	0	0	229	0	27	862
	8:30 AM	0	130	77	15	252	0	0	0	0	215	0	27	716
	8:45 AM	0	100	74	14	172	0	0	0	0	173	0	12	545
	VOLUMES	0	899	601	89	2,418	0	0	0	0	1,936	0	211	6,154
	APPROACH %	0%	60%	40%	4%	96%	0%	0%	0%	0%	90%	0%	10%	
	APP/DEPART	1,500	/	1,110	2,507	/	4,354	0	/	690	2,147	/	0	0
	BEGIN PEAK HR		7:30 AM											
	VOLUMES	0	527	356	51	1,403	0	0	0	0	1,035	0	145	3,517
	APPROACH %	0%	60%	40%	4%	96%	0%	0%	0%	0%	88%	0%	12%	
	PEAK HR FACTOR		0.862			0.925			0.000			0.942		0.982
	APP/DEPART	883	/	672	1,454	/	2,438	0	/	407	1,180	/	0	0
PM	4:00 PM	0	270	324	21	146	0	0	0	0	121	0	13	895
	4:15 PM	0	272	334	15	150	0	0	0	0	133	0	23	927
	4:30 PM	0	267	364	21	129	0	0	0	0	109	0	15	905
	4:45 PM	0	279	352	26	195	0	0	0	0	131	0	17	1,000
	5:00 PM	0	292	365	14	155	0	0	0	0	118	0	13	957
	5:15 PM	0	295	344	17	167	0	0	0	0	135	0	14	972
	5:30 PM	0	329	327	18	184	0	0	0	0	115	0	19	992
	5:45 PM	0	268	318	22	136	0	0	0	0	100	0	16	860
	VOLUMES	0	2,272	2,728	154	1,262	0	0	0	0	962	0	130	7,508
	APPROACH %	0%	45%	55%	11%	89%	0%	0%	0%	0%	88%	0%	12%	
	APP/DEPART	5,000	/	2,402	1,416	/	2,224	0	/	2,882	1,092	/	0	0
	BEGIN PEAK HR		4:45 PM											
	VOLUMES	0	1,195	1,388	75	701	0	0	0	0	499	0	63	3,921
	APPROACH %	0%	46%	54%	10%	90%	0%	0%	0%	0%	89%	0%	11%	
	PEAK HR FACTOR		0.983			0.878			0.000			0.943		0.980
	APP/DEPART	2,583	/	1,258	776	/	1,200	0	/	1,463	562	/	0	0



INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

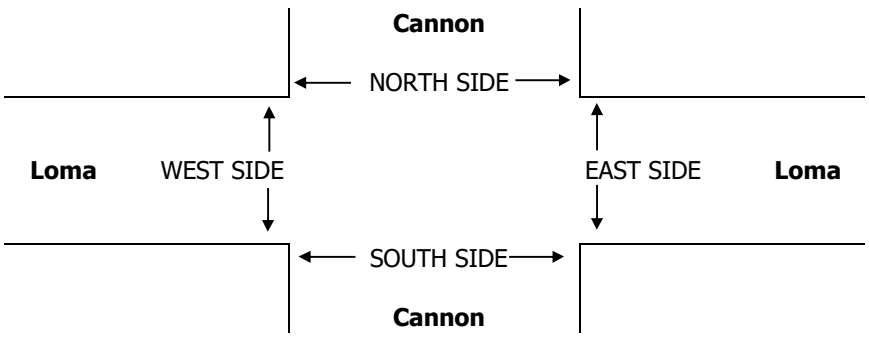
DATE:					
Wed, Apr 26, 23	LOCATION:	Villa Park - Orange	PROJECT #:	SC3962
	NORTH & SOUTH:	Cannon	LOCATION #:	15
	EAST & WEST:	Loma	CONTROL:	STOP E

NOTES:

AM / PM / MD / OTHER / OTHER

▲ N ◄ W E ► S ▼

		NORTHBOUND Cannon			SOUTHBOUND Cannon			EASTBOUND Loma			WESTBOUND Loma			
	LANES:	NL 1	NT 2	NR X	SL X	ST 2	SR 0	EL 0	ET X	ER 0	WL X	WT X	WR X	TOTAL
AM	7:00 AM	3	63	0	0	242	5	5	0	6	0	0	0	324
	7:15 AM	1	90	0	0	347	5	9	0	5	0	0	0	457
	7:30 AM	5	136	0	0	379	8	7	0	14	0	0	0	549
	7:45 AM	11	160	0	0	352	14	10	0	7	0	0	0	554
	8:00 AM	5	174	0	0	350	13	7	0	12	0	0	0	561
	8:15 AM	7	169	0	0	338	8	10	0	12	0	0	0	544
	8:30 AM	11	146	0	0	256	5	10	0	11	0	0	0	439
	8:45 AM	10	102	0	0	188	6	8	0	8	0	0	0	322
	VOLUMES	53	1,040	0	0	2,452	64	66	0	75	0	0	0	3,750
	APPROACH %	5%	95%	0%	0%	97%	3%	47%	0%	53%	0%	0%	0%	
	APP/DEPART	1,093	/	1,106	2,516	/	2,527	141	/	0	0	/	117	0
	BEGIN PEAK HR		7:30 AM											
	VOLUMES	28	639	0	0	1,419	43	34	0	45	0	0	0	2,208
	APPROACH %	4%	96%	0%	0%	97%	3%	43%	0%	57%	0%	0%	0%	
	PEAK HR FACTOR		0.932			0.944			0.898			0.000		0.984
	APP/DEPART	667	/	673	1,462	/	1,464	79	/	0	0	/	71	0
PM	4:00 PM	11	272	0	0	159	4	1	0	7	0	0	0	454
	4:15 PM	8	287	0	0	153	7	7	0	11	0	0	0	473
	4:30 PM	6	276	0	0	150	11	4	0	6	0	0	0	453
	4:45 PM	10	286	0	0	212	6	8	0	7	0	0	0	529
	5:00 PM	10	295	0	0	170	4	6	0	6	0	0	0	491
	5:15 PM	9	300	0	0	174	12	9	0	9	0	0	0	513
	5:30 PM	9	330	0	0	195	7	2	0	6	0	0	0	549
	5:45 PM	12	272	0	0	151	8	1	0	6	0	0	0	450
	VOLUMES	75	2,318	0	0	1,364	59	38	0	58	0	0	0	3,912
	APPROACH %	3%	97%	0%	0%	96%	4%	40%	0%	60%	0%	0%	0%	
	APP/DEPART	2,393	/	2,356	1,423	/	1,430	96	/	0	0	/	126	0
	BEGIN PEAK HR		4:45 PM											
	VOLUMES	38	1,211	0	0	751	29	25	0	28	0	0	0	2,082
	APPROACH %	3%	97%	0%	0%	96%	4%	47%	0%	53%	0%	0%	0%	
	PEAK HR FACTOR		0.921			0.894			0.736			0.000		0.948
	APP/DEPART	1,249	/	1,236	780	/	784	53	/	0	0	/	62	0



Wednesday, April 26, 2023 CITY: Villa Park PROJECT: SC3962

ADT4 Villa Park Rd from N Wanda Rd to Center Dr. Suhsduhg#e|#DlpWG#OOF##who1#:47#586#:;;;

AM Period	EB		WB			PM Period	EB		WB		
0:00	25		12			12:00	166		166		
0:15	12		10			12:15	182		215		
0:30	13		7			12:30	165		232		
0:45	7	57	6	35	92	12:45	149	662	184	797	1459
1:00	5		7			13:00	150		169		
1:15	9		7			13:15	183		194		
1:30	5		4			13:30	149		204		
1:45	5	24	2	20	44	13:45	165	647	175	742	1389
2:00	3		3			14:00	171		220		
2:15	5		3			14:15	159		169		
2:30	5		4			14:30	185		202		
2:45	5	18	7	17	35	14:45	180	695	252	843	1538
3:00	1		5			15:00	186		248		
3:15	4		4			15:15	203		348		
3:30	2		13			15:30	209		288		
3:45	3	10	8	30	40	15:45	238	836	349	1233	2069
4:00	7		9			16:00	255		285		
4:15	6		13			16:15	280		329		
4:30	5		16			16:30	246		326		
4:45	7	25	30	68	93	16:45	259	1040	277	1217	2257
5:00	12		41			17:00	277		355		
5:15	17		52			17:15	251		347		
5:30	27		62			17:30	247		350		
5:45	26	82	93	248	330	17:45	243	1018	280	1332	2350
6:00	51		80			18:00	234		281		
6:15	72		125			18:15	210		250		
6:30	104		139			18:30	224		220		
6:45	142	369	175	519	888	18:45	172	840	147	898	1738
7:00	216		181			19:00	160		176		
7:15	215		198			19:15	168		130		
7:30	337		304			19:30	148		111		
7:45	342	1110	294	977	2087	19:45	156	632	95	512	1144
8:00	292		304			20:00	152		102		
8:15	286		271			20:15	156		82		
8:30	194		278			20:30	139		70		
8:45	195	967	223	1076	2043	20:45	157	604	50	304	908
9:00	154		203			21:00	127		70		
9:15	143		205			21:15	121		52		
9:30	156		205			21:30	106		56		
9:45	151	604	209	822	1426	21:45	97	451	40	218	669
10:00	167		168			22:00	82		29		
10:15	146		218			22:15	47		42		
10:30	144		178			22:30	56		26		
10:45	159	616	197	761	1377	22:45	38	223	21	118	341
11:00	135		187			23:00	29		20		
11:15	184		162			23:15	28		18		
11:30	152		174			23:30	17		14		
11:45	157	628	202	725	1353	23:45	19	93	13	65	158
Total Vol.		4510		5298	**9808**			7741		8279	**16020**

	Daily Totals EB	WB	**Combined**
	12251	13577	**25828**

	AM			**PM**		
Split %	46.0%	54.0%	**38.0%**	48.3%	51.7%	**62.0%**
Peak Hour	7:30	7:30	**7:30**	16:15	17:00	**16:45**
Volume	1257	1173	**2430**	1062	1332	**2363**
P.H.F.	0.92	0.96	**0.95**	0.95	0.94	**0.93**

cs@aimtd.com Tell. 714 253 7888

A32223

24-HOUR ROADWAY SEGMENT COUNTS (WITH FHWA CLASSIFICATION)

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Wednesday, April 26, 2023 **CITY#** Orange

JOB #: SC3962 CLASS5 Villa Park Rd from Center Dr to Hewes St

AM	COMBINED													
TIME	1	2	3	4	5	6	7	8	9	10	11	12	13	TOTAL
0:00	0	28	1	0	1	0	0	0	0	0	0	0	0	30
0:15	0	19	1	0	0	0	0	0	0	0	0	0	0	20
0:30	0	17	0	0	0	0	0	0	0	0	0	0	0	17
0:45	0	9	0	0	0	0	0	0	0	0	0	0	0	9
1:00	0	11	0	0	0	0	0	0	0	0	0	0	0	11
1:15	0	10	0	0	1	0	0	0	0	0	0	0	0	11
1:30	0	9	3	0	0	0	0	0	0	0	0	0	0	12
1:45	0	5	0	0	0	0	0	0	0	0	0	0	0	5
2:00	0	6	1	0	0	0	0	0	0	0	0	0	0	7
2:15	0	6	0	0	0	0	0	0	0	0	0	0	0	6
2:30	0	5	1	0	0	0	0	0	0	0	0	0	0	6
2:45	0	7	2	0	1	0	0	0	0	0	0	0	0	10
3:00	0	7	1	0	0	0	0	0	0	0	0	0	0	8
3:15	0	6	0	0	1	0	0	0	0	0	0	0	0	7
3:30	0	8	0	0	1	0	0	0	0	0	0	0	0	9
3:45	0	10	0	0	0	0	0	0	0	0	0	0	0	10
4:00	0	7	1	0	0	0	0	0	0	0	0	0	0	8
4:15	0	15	1	0	1	0	0	0	0	0	0	0	0	17
4:30	0	17	2	0	1	0	0	0	0	0	0	0	0	20
4:45	0	23	4	0	2	0	0	0	0	0	0	0	0	29
5:00	0	26	4	0	4	0	0	0	0	0	0	0	0	34
5:15	0	52	9	0	1	0	0	0	0	0	0	0	0	62
5:30	0	68	8	0	2	0	0	0	0	0	0	0	0	78
5:45	1	67	10	0	1	2	0	0	1	0	0	0	0	82
6:00	0	93	19	0	2	0	0	0	0	0	0	0	0	114
6:15	0	146	23	0	15	1	0	0	0	0	0	0	0	185
6:30	1	179	33	0	21	1	0	0	1	1	0	0	0	237
6:45	0	209	43	1	25	6	0	0	0	0	1	0	0	285
7:00	0	271	28	1	25	2	0	0	0	0	0	0	0	327
7:15	2	350	37	2	28	0	0	0	0	1	0	0	0	420
7:30	1	443	46	0	31	3	0	0	0	0	0	0	0	524
7:45	0	542	65	0	26	1	0	0	1	0	1	0	0	636
8:00	3	465	62	0	20	3	0	0	0	0	2	0	0	555
8:15	0	469	56	0	13	2	0	0	0	0	0	0	0	540
8:30	2	371	49	1	28	1	0	0	0	0	0	0	0	452
8:45	0	317	36	4	23	1	0	1	1	1	1	0	0	385
9:00	2	266	39	1	12	2	0	0	0	1	0	0	0	323
9:15	0	283	38	4	11	3	0	0	1	0	1	0	0	341
9:30	0	248	49	1	11	2	0	1	0	0	0	0	0	312
9:45	0	251	46	1	8	4	0	0	1	0	1	0	0	312
10:00	0	276	44	1	17	1	0	0	0	0	0	0	0	339
10:15	0	240	46	1	19	2	0	0	0	0	0	0	0	308
10:30	0	202	39	2	10	2	0	0	0	0	0	0	0	255
10:45	1	244	43	0	7	3	0	0	0	0	0	0	0	298
11:00	0	244	47	0	13	2	0	0	0	0	0	0	0	306
11:15	0	253	27	1	16	2	0	0	0	0	0	0	0	299
11:30	0	248	38	1	11	4	0	1	1	0	0	0	0	304
11:45	0	259	45	0	9	1	0	0	0	0	0	0	0	314
TOTAL	13	7,307	1,047	22	418	51	0	3	7	4	7	0	0	8,879

AM PEAK HOUR	7:30 AM
AM PEAK VOLUME	2,255

PM	COMBINED													
Time	1	2	3	4	5	6	7	8	9	10	11	12	13	TOTAL
12:00	2	245	24	0	8	2	0	0	2	0	1	0	0	284
12:15	0	279	37	0	13	3	0	0	2	0	1	0	0	335
12:30	0	285	26	0	13	2	0	0	0	0	0	0	0	326
12:45	1	251	41	1	6	1	0	0	0	0	0	0	0	301
13:00	1	249	33	0	16	4	0	0	0	0	0	0	0	303
13:15	1	295	30	1	10	1	0	0	1	0	0	0	0	339
13:30	0	255	27	0	7	5	0	0	1	0	0	0	0	295
13:45	0	313	20	0	6	2	0	0	0	0	0	0	0	341
14:00	2	281	35	1	12	1	0	0	2	0	0	0	0	334
14:15	3	242	31	0	10	3	0	1	1	0	2	0	0	293
14:30	0	304	39	4	14	3	0	0	0	0	0	0	0	364
14:45	0	351	40	0	15	3	0	0	1	0	0	0	0	410
15:00	0	392	52	1	21	3	0	0	0	0	0	0	0	469
15:15	4	425	45	0	20	3	0	0	0	0	3	0	0	500
15:30	4	501	34	0	26	0	0	0	0	0	0	0	0	565
15:45	0	439	45	0	16	2	0	1	1	0	1	0	0	505
16:00	0	474	55	1	15	4	0	1	2	0	0	0	0	552
16:15	3	567	63	1	7	2	0	0	0	0	0	0	0	643
16:30	4	468	47	1	18	0	0	0	0	0	0	0	0	538
16:45	2	472	56	1	14	2	0	0	1	0	0	0	0	548
17:00	3	542	50	0	5	0	0	0	0	0	0	0	0	600
17:15	2	522	49	1	8	0	0	0	0	0	0	0	0	582
17:30	1	498	31	0	10	0	0	1	0	0	0	0	0	541
17:45	4	443	47	0	6	2	0	0	0	0	0	0	0	502
18:00	2	498	52	0	6	1	0	0	0	0	0	0	0	559
18:15	1	373	31	0	4	1	0	0	0	0	0	0	0	410
18:30	2	377	37	0	3	0	0	1	0	0	0	0	0	420
18:45	2	303	38	0	5	0	0	0	0	0	0	0	0	348
19:00	1	258	28	0	1	0	0	0	0	0	0	0	0	288
19:15	2	245	11	0	4	0	0	0	0	0	0	0	0	262
19:30	1	210	16	0	0	0	0	0	0	0	0	0	0	227
19:45	0	211	23	0	0	0	0	0	0	0	0	0	0	234
20:00	2	167	20	0	1	0	0	0	0	0	0	0	0	190
20:15	0	162	13	0	0	0	0	0	0	0	0	0	0	175
20:30	0	155	17	0	1	0	0	0	0	0	0	0	0	173
20:45	0	165	16	0	1	0	0	0	0	0	0	0	0	182
21:00	1	153	17	0	1	0	0	0	0	0	0	0	0	172
21:15	0	146	16	0	0	0	0	0	0	0	0	0	0	162
21:30	0	135	8	0	1	0	0	0	0	0	0	0	0	144
21:45	0	103	5	0	0	0	0	0	0	0	0	0	0	108
22:00	0	91	8	0	0	0	0	0	0	0	0	0	0	99
22:15	0	77	8	0	0	0	0	0	0	0	0	0	0	85
22:30	2	58	7	0	1	0	0	0	0	0	0	0	0	68
22:45	0	47	9	0	0	0	0	0	0	0	0	0	0	56
23:00	0	42	3	0	0	0	0	0	0	0	0	0	0	45
23:15	0	29	4	0	1	0	0	0	0	0	0	0	0	34
23:30	0	17	3	0	0	0	0	0	0	0	0	0	0	20
23:45	0	28	1	0	1	0	0	0	0	0	0	0	0	30
TOTAL	53	13,143	1,348	13	327	50	0	5	14	0	8	0	0	14,961

PM PEAK HOUR	4:15 PM
PM PEAK VOLUME	2,329

CLASS 1	Class 1 — Motorcycles	**CLASS 8**	3 to 4 Axles, Single Trailer
CLASS 2	Passenger Cars	**CLASS 9**	5 Axles, Single Trailer
CLASS 3	2 Axles, 4-Tire Single Units	**CLASS 10**	6 or More Axles, Single Trailer
CLASS 4	Buses	**CLASS 11**	5 or Less Axles, Multi-Trailers
CLASS 5	2 Axles, 6-Tire Single Units	**CLASS 12**	6 Axles, Multi-Trailers
CLASS 6	3 Axles, Single Unit	**CLASS 13**	7 or More Axles, Multi-Trailers
CLASS 7	4 or More Axles, Single Unit		

TOTAL: AM+PM	66	20,450	2,395	35	745	101	0	8	21	4	15	0	0	23,840
% OF TOTAL	0.3%	85.8%	10.0%	0.1%	3.1%	0.4%	0.0%	0.0%	0.1%	0.0%	0.1%	0.0%	0.0%	100.0%

Wednesday, April 26, 2023 CITY: Orange PROJECT: SC3962

ADT7 Villa Park Rd from Hewes St to Cannon St. Suhsduhg#e|#DlpWG#OOF##who1#:47#586#:;;;

AM Period	EB		WB			PM Period	EB		WB		
0:00	19		11			12:00	140		174		
0:15	13		10			12:15	169		193		
0:30	11		8			12:30	174		199		
0:45	5	48	10	39	87	12:45	138	621	184	750	1371
1:00	4		6			13:00	154		169		
1:15	5		7			13:15	172		182		
1:30	9		2			13:30	144		176		
1:45	3	21	2	17	38	13:45	159	629	181	708	1337
2:00	3		5			14:00	154		210		
2:15	2		3			14:15	164		172		
2:30	5		1			14:30	199		220		
2:45	4	14	8	17	31	14:45	203	720	247	849	1569
3:00	1		4			15:00	219		316		
3:15	1		4			15:15	231		341		
3:30	2		5			15:30	258		377		
3:45	3	7	9	22	29	15:45	251	959	355	1389	2348
4:00	2		3			16:00	291		329		
4:15	4		16			16:15	300		397		
4:30	2		15			16:30	258		338		
4:45	8	16	22	56	72	16:45	258	1107	356	1420	2527
5:00	10		25			17:00	333		368		
5:15	22		42			17:15	284		388		
5:30	25		58			17:30	274		361		
5:45	25	82	59	184	266	17:45	241	1132	306	1423	2555
6:00	44		82			18:00	286		302		
6:15	76		114			18:15	192		223		
6:30	110		148			18:30	244		211		
6:45	137	367	184	528	895	18:45	189	911	186	922	1833
7:00	163		187			19:00	147		140		
7:15	231		239			19:15	161		120		
7:30	270		282			19:30	123		109		
7:45	331	995	396	1104	2099	19:45	147	578	101	470	1048
8:00	323		329			20:00	114		81		
8:15	337		309			20:15	117		72		
8:30	219		294			20:30	122		58		
8:45	203	1082	223	1155	2237	20:45	133	486	73	284	770
9:00	169		202			21:00	119		66		
9:15	138		214			21:15	119		59		
9:30	147		175			21:30	110		55		
9:45	138	592	184	775	1367	21:45	72	420	38	218	638
10:00	161		195			22:00	64		26		
10:15	140		189			22:15	50		40		
10:30	113		166			22:30	45		28		
10:45	128	542	184	734	1276	22:45	34	193	17	111	304
11:00	155		179			23:00	32		17		
11:15	152		181			23:15	19		13		
11:30	157		168			23:30	17		6		
11:45	153	617	199	727	1344	23:45	16	84	17	53	137
Total Vol.		4383		5358	**9741**			7840		8597	**16437**

Daily Totals	EB	WB	**Combined**
	12223	13955	**26178**

	AM			**PM**		
Split %	45.0%	55.0%	**37.2%**	47.7%	52.3%	**62.8%**
Peak Hour	7:30	7:45	**7:30**	16:15	16:45	**16:45**
Volume	1261	1328	**2577**	1149	1473	**2622**
P.H.F.	0.94	0.84	**0.89**	0.86	0.95	**0.94**

cs@aimtd.com Tell. 714 253 7888

A32223

24-HOUR ROADWAY SEGMENT COUNTS (WITH FHWA CLASSIFICATION)

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Wednesday, April 26, 2023 **CITY#** Orange

JOB #: SC3962 CLASS8 Santiago Canyon Rd from Cannon St to Orange Park Blvd

AM	COMBINED														PM	COMBINED													
TIME	1	2	3	4	5	6	7	8	9	10	11	12	13	TOTAL	Time	1	2	3	4	5	6	7	8	9	10	11	12	13	TOTAL
0:00	0	11	1	0	3	0	0	0	0	0	0	0	0	15	12:00	3	241	26	3	6	1	1	0	2	0	1	0	0	284
0:15	0	14	1	0	0	0	0	0	0	0	0	0	0	15	12:15	0	254	35	0	10	1	1	0	2	0	1	0	0	304
0:30	0	12	0	0	0	0	0	0	0	0	0	0	0	12	12:30	1	251	26	1	9	3	1	0	0	0	0	0	0	292
0:45	0	5	1	0	0	0	0	0	0	0	0	0	0	6	12:45	4	283	36	2	7	0	1	0	0	0	0	0	0	333
1:00	0	10	0	0	0	0	0	0	0	0	0	0	0	10	13:00	3	261	32	1	13	2	0	0	0	0	0	0	0	312
1:15	0	3	0	0	1	0	0	0	0	0	0	0	0	4	13:15	3	350	42	1	7	1	0	0	0	0	0	0	0	404
1:30	0	6	0	0	0	0	0	0	0	0	0	0	0	6	13:30	2	291	29	1	8	2	0	0	1	0	0	0	0	334
1:45	0	2	0	0	0	0	0	0	0	0	0	0	0	2	13:45	0	279	26	0	7	1	0	0	1	0	0	0	0	314
2:00	1	4	0	0	0	0	0	0	0	0	0	0	0	5	14:00	1	303	41	1	9	1	0	0	1	0	0	0	0	357
2:15	0	3	0	0	0	0	0	0	0	0	0	0	0	3	14:15	3	289	32	0	7	2	1	1	1	0	2	0	0	338
2:30	0	10	0	0	0	0	0	0	0	0	0	0	0	10	14:30	1	325	46	3	18	1	0	1	0	0	0	0	0	395
2:45	0	6	1	0	0	0	0	0	0	0	0	0	0	7	14:45	2	454	41	1	20	3	1	0	0	0	1	0	0	523
3:00	0	4	0	0	0	0	0	0	0	0	0	0	0	4	15:00	2	596	52	0	19	2	1	0	1	0	0	0	0	673
3:15	0	7	0	0	0	0	0	0	0	0	0	0	0	7	15:15	8	552	51	0	22	2	1	0	0	0	3	0	0	639
3:30	0	8	0	0	0	0	0	0	0	0	0	0	0	8	15:30	7	642	49	0	25	0	0	0	0	0	0	0	0	723
3:45	0	2	0	0	0	0	0	0	0	0	0	0	0	2	15:45	1	582	56	0	23	2	1	0	2	0	1	0	0	668
4:00	0	8	1	0	0	0	0	0	0	0	0	0	0	9	16:00	1	581	42	0	16	3	1	0	2	0	0	0	0	646
4:15	0	23	0	0	0	0	0	0	0	0	0	0	0	23	16:15	6	660	35	0	8	1	0	0	0	0	0	0	0	710
4:30	0	16	1	0	0	0	0	0	0	0	0	0	0	17	16:30	4	652	31	1	12	3	0	0	0	0	2	0	0	705
4:45	0	32	0	0	0	0	0	0	0	0	0	0	0	32	16:45	1	728	35	0	16	1	0	0	2	0	1	0	1	785
5:00	0	28	1	0	0	0	0	0	0	0	0	0	0	29	17:00	5	769	42	0	9	0	0	0	0	0	0	0	0	825
5:15	0	72	1	0	0	0	0	0	0	0	0	0	0	73	17:15	3	776	36	1	11	0	0	0	0	0	0	0	1	828
5:30	1	79	10	0	2	0	0	0	1	0	0	0	0	93	17:30	3	767	30	0	10	1	0	0	0	0	0	0	0	811
5:45	0	71	5	1	1	1	0	0	1	0	0	0	0	80	17:45	5	645	25	0	6	1	1	0	0	0	1	0	0	684
6:00	0	95	8	0	3	0	0	0	0	0	0	0	0	106	18:00	3	589	28	0	4	0	1	0	0	0	0	0	0	625
6:15	4	140	17	0	5	0	1	0	1	0	0	0	0	168	18:15	1	534	25	0	7	1	0	0	0	0	0	0	0	568
6:30	3	239	22	1	8	1	0	0	1	0	1	0	0	276	18:30	2	397	23	0	3	0	0	0	1	0	0	0	0	426
6:45	1	314	33	0	13	3	2	0	1	0	1	0	0	368	18:45	2	348	26	0	5	0	0	0	0	0	0	0	0	381
7:00	0	426	24	0	14	3	0	0	0	0	1	0	0	468	19:00	2	266	18	0	0	0	0	0	0	0	0	0	0	286
7:15	2	536	44	1	19	0	0	0	1	0	1	0	1	605	19:15	3	237	15	0	2	0	0	0	0	0	0	0	0	257
7:30	1	631	35	0	18	3	1	0	0	0	0	0	1	690	19:30	2	218	8	0	0	0	0	0	0	0	0	0	0	228
7:45	1	623	34	1	21	1	0	0	1	0	2	0	0	684	19:45	0	162	8	0	1	0	0	0	0	0	0	0	0	171
8:00	2	677	42	0	12	3	0	0	1	0	2	0	0	739	20:00	2	179	6	0	1	0	0	0	0	0	0	0	0	188
8:15	1	747	40	3	6	5	0	0	0	0	1	0	0	803	20:15	0	166	3	0	0	0	0	0	0	0	0	0	0	169
8:30	3	570	30	0	20	1	1	0	0	0	0	0	0	625	20:30	0	113	6	0	0	0	0	0	0	0	0	0	0	119
8:45	0	424	34	3	14	2	0	0	3	0	0	0	0	480	20:45	0	124	1	0	1	0	0	0	0	0	0	0	0	126
9:00	2	350	38	0	11	2	0	0	0	0	2	0	0	405	21:00	1	132	4	0	1	0	0	0	0	0	0	0	0	138
9:15	2	268	39	6	15	4	0	0	1	0	0	0	0	335	21:15	1	138	0	0	1	0	0	0	0	0	0	0	0	140
9:30	1	267	41	1	8	1	1	0	0	0	0	0	0	320	21:30	1	120	0	0	1	0	0	0	0	0	0	0	0	122
9:45	1	296	27	1	6	1	3	0	1	0	1	0	0	337	21:45	0	78	2	0	0	0	0	0	0	0	0	0	0	80
10:00	0	310	36	1	5	0	1	0	0	0	0	0	0	353	22:00	1	87	4	0	0	0	0	0	0	0	0	0	0	92
10:15	0	271	32	3	7	2	0	0	0	0	0	0	0	315	22:15	0	47	3	0	0	0	0	0	0	0	0	0	0	50
10:30	0	233	32	3	7	0	3	0	0	0	0	0	0	278	22:30	2	41	1	0	0	0	0	0	0	0	0	0	0	44
10:45	1	198	27	0	8	0	1	0	0	0	1	0	0	236	22:45	0	44	2	0	0	0	0	0	0	0	0	0	0	46
11:00	4	216	36	0	3	0	1	0	0	0	0	0	0	260	23:00	0	43	2	0	0	0	0	0	0	0	0	0	0	45
11:15	0	249	36	1	9	1	1	0	0	0	0	0	0	297	23:15	0	26	0	0	0	0	0	0	0	0	0	0	0	26
11:30	2	316	38	0	2	1	0	1	0	0	0	0	0	360	23:30	1	17	0	0	0	0	0	0	0	0	0	0	0	18
11:45	0	319	47	1	8	1	0	0	0	0	0	0	0	376	23:45	0	22	0	0	0	0	0	0	0	0	0	0	0	22
TOTAL	33	9,151	815	27	249	36	16	1	13	0	13	0	2	10,356	TOTAL	93	15,659	1,081	16	325	35	12	2	16	0	13	0	2	17,254

AM PEAK HOUR	7:30 AM
AM PEAK VOLUME	2,916

PM PEAK HOUR	4:45 PM
PM PEAK VOLUME	3,249

CLASS 1	Class 1 — Motorcycles	**CLASS 8**	3 to 4 Axles, Single Trailer
CLASS 2	Passenger Cars	**CLASS 9**	5 Axles, Single Trailer
CLASS 3	2 Axles, 4-Tire Single Units	**CLASS 10**	6 or More Axles, Single Trailer
CLASS 4	Buses	**CLASS 11**	5 or Less Axles, Multi-Trailers
CLASS 5	2 Axles, 6-Tire Single Units	**CLASS 12**	6 Axles, Multi-Trailers
CLASS 6	3 Axles, Single Unit	**CLASS 13**	7 or More Axles, Multi-Trailers
CLASS 7	4 or More Axles, Single Unit		

TOTAL: AM+PM	126	24,810	1,896	43	574	71	28	3	29	0	26	0	4	27,610
% OF TOTAL	0.5%	89.9%	6.9%	0.2%	2.1%	0.3%	0.1%	0.0%	0.1%	0.0%	0.1%	0.0%	0.0%	100.0%

Wednesday, April 26, 2023 CITY: Villa Park - Orange PROJECT: SC3962

ADT15 Taft Ave from Center Dr to Santiago Blvd. Suhsduhg#e|#DlpWG#OOF##who1#:47#586#:;;;

AM Period	EB		WB			PM Period	EB		WB		
0:00	8		1			12:00	71		63		
0:15	4		4			12:15	94		60		
0:30	0		1			12:30	119		85		
0:45	0	12	0	6	18	12:45	162	446	57	265	711
1:00	1		0			13:00	195		107		
1:15	1		0			13:15	146		182		
1:30	1		1			13:30	151		149		
1:45	1	4	0	1	5	13:45	125	617	135	573	1190
2:00	1		2			14:00	78		89		
2:15	0		0			14:15	84		65		
2:30	0		0			14:30	76		66		
2:45	0	1	0	2	3	14:45	80	318	81	301	619
3:00	0		1			15:00	130		88		
3:15	0		4			15:15	180		87		
3:30	0		0			15:30	157		181		
3:45	0	0	0	5	5	15:45	117	584	130	486	1070
4:00	0		2			16:00	99		93		
4:15	0		2			16:15	108		91		
4:30	0		5			16:30	114		94		
4:45	0	0	6	15	15	16:45	161	482	102	380	862
5:00	0		5			17:00	182		103		
5:15	4		8			17:15	160		122		
5:30	4		8			17:30	127		121		
5:45	5	13	10	31	44	17:45	156	625	109	455	1080
6:00	12		13			18:00	119		121		
6:15	34		32			18:15	96		99		
6:30	27		28			18:30	96		86		
6:45	40	113	34	107	220	18:45	89	400	89	395	795
7:00	59		44			19:00	82		90		
7:15	104		76			19:15	50		83		
7:30	135		99			19:30	50		86		
7:45	224	522	127	346	868	19:45	52	234	57	316	550
8:00	309		198			20:00	40		47		
8:15	326		183			20:15	41		31		
8:30	123		147			20:30	26		22		
8:45	101	859	93	621	1480	20:45	24	131	17	117	248
9:00	178		97			21:00	38		17		
9:15	175		126			21:15	28		52		
9:30	82		78			21:30	25		44		
9:45	48	483	56	357	840	21:45	21	112	9	122	234
10:00	46		49			22:00	9		1		
10:15	45		79			22:15	11		8		
10:30	44		62			22:30	13		9		
10:45	71	206	57	247	453	22:45	9	42	4	22	64
11:00	72		70			23:00	9		6		
11:15	55		102			23:15	7		5		
11:30	57		73			23:30	4		2		
11:45	70	254	69	314	568	23:45	4	24	0	13	37
Total Vol.		2467		2052	**4519**			4015		3445	**7460**

	Daily Totals EB	WB	**Combined**
	6482	5497	**11979**

	AM			**PM**		
Split %	54.6%	45.4%	**37.7%**	53.8%	46.2%	**62.3%**
Peak Hour	7:30	7:45	**7:45**	12:45	13:00	**13:00**
Volume	994	655	**1637**	654	573	**1190**
P.H.F.	0.76	0.83	**0.80**	0.84	0.79	**0.91**

cs@aimtd.com Tell. 714 253 7888

Wednesday, April 26, 2023 CITY: Villa Park PROJECT: SC3962

ADT14 Taft Ave from Cannon St to Center Dr. Suhsduhg#e|#DlpWG#OOF##who1#:47#586#:;;;

AM Period	EB		WB			PM Period	EB		WB		
0:00	5		1			12:00	37		54		
0:15	5		2			12:15	49		52		
0:30	1		1			12:30	39		55		
0:45	0	11	0	4	15	12:45	58	183	52	213	396
1:00	2		0			13:00	62		37		
1:15	0		0			13:15	62		61		
1:30	1		0			13:30	57		57		
1:45	0	3	0	0	3	13:45	61	242	40	195	437
2:00	1		2			14:00	60		48		
2:15	0		0			14:15	51		64		
2:30	0		0			14:30	51		55		
2:45	0	1	0	2	3	14:45	39	201	62	229	430
3:00	0		0			15:00	54		67		
3:15	0		3			15:15	57		65		
3:30	0		0			15:30	100		65		
3:45	0	0	1	4	4	15:45	53	264	63	260	524
4:00	0		1			16:00	69		65		
4:15	1		3			16:15	69		79		
4:30	0		4			16:30	77		68		
4:45	0	1	7	15	16	16:45	58	273	96	308	581
5:00	1		4			17:00	51		79		
5:15	2		5			17:15	81		73		
5:30	2		5			17:30	65		77		
5:45	6	11	7	21	32	17:45	71	268	77	306	574
6:00	11		11			18:00	61		61		
6:15	14		20			18:15	53		58		
6:30	13		17			18:30	65		47		
6:45	14	52	29	77	129	18:45	57	236	38	204	440
7:00	20		27			19:00	48		31		
7:15	27		41			19:15	44		32		
7:30	31		59			19:30	51		35		
7:45	68	146	83	210	356	19:45	32	175	35	133	308
8:00	74		81			20:00	44		24		
8:15	69		76			20:15	30		17		
8:30	52		72			20:30	25		10		
8:45	35	230	65	294	524	20:45	20	119	18	69	188
9:00	37		50			21:00	25		12		
9:15	41		49			21:15	23		14		
9:30	43		47			21:30	24		9		
9:45	37	158	52	198	356	21:45	17	89	4	39	128
10:00	32		53			22:00	10		2		
10:15	30		55			22:15	13		6		
10:30	30		49			22:30	9		7		
10:45	31	123	50	207	330	22:45	11	43	4	19	62
11:00	46		48			23:00	7		2		
11:15	30		53			23:15	5		2		
11:30	36		47			23:30	5		0		
11:45	39	151	57	205	356	23:45	4	21	0	4	25
Total Vol.	887		1237		**2124**		2114		1979		**4093**

	Daily Totals		
	EB	WB	**Combined**
	3001	3216	**6217**

	AM			**PM**		
Split %	41.8%	58.2%	**34.2%**	51.6%	48.4%	**65.8%**
Peak Hour	7:45	7:45	**7:45**	15:30	16:45	**16:30**
Volume	263	312	**575**	291	325	**583**
P.H.F.	0.89	0.94	**0.93**	0.73	0.85	**0.95**

cs@aimtd.com Tell. 714 253 7888

A32223

24-HOUR ROADWAY SEGMENT COUNTS (WITH FHWA CLASSIFICATION)

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Wednesday, April 26, 2023 **CITY#** Orange

JOB #: SC3962 CLASS12 Serrano Ave from Cannon St to Orange Park Blvd

AM	COMBINED														PM	COMBINED													
TIME	1	2	3	4	5	6	7	8	9	10	11	12	13	TOTAL	Time	1	2	3	4	5	6	7	8	9	10	11	12	13	TOTAL
0:00	0	21	1	0	0	0	0	0	0	0	0	0	0	22	12:00	0	187	20	1	1	1	1	0	0	0	0	0	0	211
0:15	0	11	0	0	0	0	0	0	0	0	0	0	0	11	12:15	0	216	23	2	7	1	1	0	0	0	0	0	0	250
0:30	0	8	1	0	0	0	0	0	0	0	0	0	0	9	12:30	1	211	29	1	2	1	0	0	0	0	0	0	0	245
0:45	0	5	0	0	0	0	0	0	0	0	0	0	0	5	12:45	1	196	19	1	3	0	0	0	0	0	0	0	0	220
1:00	0	8	0	0	0	0	0	0	0	0	0	0	0	8	13:00	3	189	17	0	5	2	0	0	0	0	0	0	0	216
1:15	0	4	0	0	0	0	0	0	0	0	0	0	0	4	13:15	3	234	24	0	3	0	0	0	0	0	0	0	0	264
1:30	0	7	0	0	0	0	0	0	0	0	0	0	0	7	13:30	1	192	33	0	4	0	1	0	0	0	0	0	0	231
1:45	0	3	0	0	0	0	0	0	0	0	0	0	0	3	13:45	0	191	18	0	6	0	0	0	0	0	0	0	0	215
2:00	0	3	0	0	0	0	0	0	0	0	0	0	0	3	14:00	4	221	29	0	2	0	0	0	0	0	0	0	0	256
2:15	0	4	0	0	0	0	0	0	0	0	0	0	0	4	14:15	0	235	28	0	5	0	0	0	0	0	0	0	0	268
2:30	0	8	1	0	1	0	0	0	0	0	0	0	0	10	14:30	1	233	34	1	7	0	0	0	0	0	0	0	0	276
2:45	0	3	0	0	0	0	0	0	0	0	0	0	0	3	14:45	1	236	28	3	6	0	0	0	0	0	0	0	0	274
3:00	0	3	0	0	0	0	0	0	0	0	0	0	0	3	15:00	0	273	32	0	5	1	0	0	0	0	0	0	0	311
3:15	0	3	3	0	0	0	0	0	0	0	0	0	0	6	15:15	0	316	43	2	3	1	0	0	0	0	0	0	0	365
3:30	0	7	0	0	0	0	0	0	0	0	0	0	0	7	15:30	2	377	44	2	7	0	0	0	0	0	0	0	0	432
3:45	0	4	0	0	0	0	0	0	0	0	0	0	0	4	15:45	1	376	34	5	9	0	0	0	0	0	0	0	0	425
4:00	1	6	1	0	0	0	0	0	0	0	0	0	0	8	16:00	1	438	29	3	8	0	0	0	0	0	0	0	0	479
4:15	0	11	2	0	0	0	0	0	0	0	0	0	0	13	16:15	1	458	41	1	3	1	0	0	0	0	0	0	0	505
4:30	1	17	4	0	0	0	0	0	0	0	0	0	0	22	16:30	3	467	31	0	6	0	0	0	0	0	0	2	0	509
4:45	0	24	1	0	0	0	0	0	0	0	0	0	0	25	16:45	0	477	39	1	7	0	0	0	1	0	1	0	0	526
5:00	0	32	0	0	1	0	0	0	0	0	0	0	0	33	17:00	0	473	33	0	4	0	0	0	0	0	0	0	0	510
5:15	0	39	3	0	2	0	0	0	0	0	0	0	0	44	17:15	0	471	38	0	1	0	0	0	0	0	0	0	0	510
5:30	0	46	2	0	0	0	0	0	0	0	0	0	0	48	17:30	0	448	27	0	4	0	0	0	0	0	0	0	0	479
5:45	0	60	3	0	0	0	0	0	0	0	0	0	0	63	17:45	1	417	35	0	3	0	0	0	0	0	0	0	0	456
6:00	0	94	10	0	0	0	0	0	0	0	0	0	0	104	18:00	0	417	35	0	3	0	0	0	0	0	0	0	0	455
6:15	2	143	6	0	1	0	0	0	0	0	0	0	0	152	18:15	1	371	23	0	5	0	1	0	0	0	0	0	0	401
6:30	1	167	12	3	3	0	0	0	0	0	0	0	0	186	18:30	1	332	12	0	4	0	0	0	0	0	0	0	0	349
6:45	0	198	9	2	3	0	0	0	1	0	0	0	0	213	18:45	2	341	14	0	1	0	0	0	0	0	0	0	0	358
7:00	1	281	8	3	3	0	0	0	0	0	0	0	0	296	19:00	0	225	10	0	1	0	0	0	0	0	0	0	0	236
7:15	1	326	18	1	1	0	0	0	0	0	0	0	0	347	19:15	0	210	11	0	0	0	0	0	0	0	0	0	0	221
7:30	1	342	22	2	3	1	0	0	0	0	0	0	0	371	19:30	0	189	12	0	0	0	0	0	0	0	0	0	0	201
7:45	0	369	32	3	6	0	0	0	0	0	0	0	0	410	19:45	0	191	8	0	3	0	0	0	0	0	0	0	0	202
8:00	3	399	14	2	4	0	0	0	0	0	0	0	0	422	20:00	0	180	4	0	2	0	0	0	0	0	0	0	0	186
8:15	2	350	23	1	7	1	0	0	0	0	0	0	0	384	20:15	0	125	7	0	0	0	0	0	0	0	0	0	0	132
8:30	0	306	19	1	8	0	0	0	0	0	0	0	0	334	20:30	0	125	7	0	2	0	0	0	0	0	0	0	0	134
8:45	1	244	19	2	7	0	0	0	0	0	0	0	0	273	20:45	0	144	8	0	0	0	0	0	0	0	0	0	0	152
9:00	0	197	24	2	4	1	0	0	0	0	0	0	0	228	21:00	0	113	7	0	0	0	0	0	0	0	0	0	0	120
9:15	0	186	23	0	4	0	0	0	0	0	0	0	0	213	21:15	1	121	4	0	0	0	0	0	0	0	0	0	0	126
9:30	2	215	21	0	9	0	0	0	0	0	0	0	0	247	21:30	1	134	1	0	0	0	0	0	0	0	0	0	0	136
9:45	0	190	30	0	6	1	0	0	0	0	0	0	0	227	21:45	0	81	2	0	0	0	0	0	0	0	0	0	0	83
10:00	0	215	30	0	7	0	2	0	0	0	0	0	0	254	22:00	0	59	5	0	0	0	0	0	0	0	0	0	0	64
10:15	1	183	29	0	6	0	0	0	0	0	0	0	0	219	22:15	0	60	4	0	0	0	0	0	0	0	0	0	0	64
10:30	0	169	22	0	6	2	0	0	0	0	0	0	0	199	22:30	0	55	2	0	0	0	0	0	0	0	0	0	0	57
10:45	0	171	27	0	10	0	0	0	0	0	0	0	0	208	22:45	0	32	1	0	0	0	0	0	0	0	0	0	0	33
11:00	3	186	19	0	7	0	1	0	0	0	0	0	0	216	23:00	0	30	2	0	0	0	0	0	0	0	0	0	0	32
11:15	3	194	22	0	9	1	0	0	0	0	0	0	0	229	23:15	0	31	1	0	0	0	0	0	0	0	0	0	0	32
11:30	3	219	26	2	12	0	1	0	0	0	0	0	0	263	23:30	0	13	1	0	0	0	0	0	0	0	0	0	0	14
11:45	0	188	26	2	3	1	0	0	0	0	0	0	0	220	23:45	0	23	2	0	0	0	0	0	0	0	0	0	0	25
TOTAL	26	5,869	513	26	133	8	4	0	1	0	0	0	0	6,580	TOTAL	30	11,134	911	23	132	8	4	0	1	0	3	0	0	12,246

AM PEAK HOUR	7:30 AM	**PM PEAK HOUR**	4:30 PM
AM PEAK VOLUME	1,587	**PM PEAK VOLUME**	2,055

TOTAL: AM+PM	56	17,003	1,424	49	265	16	8	0	2	0	3	0	0	18,826
% OF TOTAL	0.3%	90.3%	7.6%	0.3%	1.4%	0.1%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100.0%

CLASS 1	Class 1 — Motorcycles	**CLASS 8**	3 to 4 Axles, Single Trailer
CLASS 2	Passenger Cars	**CLASS 9**	5 Axles, Single Trailer
CLASS 3	2 Axles, 4-Tire Single Units	**CLASS 10**	6 or More Axles, Single Trailer
CLASS 4	Buses	**CLASS 11**	5 or Less Axles, Multi-Trailers
CLASS 5	2 Axles, 6-Tire Single Units	**CLASS 12**	6 Axles, Multi-Trailers
CLASS 6	3 Axles, Single Unit	**CLASS 13**	7 or More Axles, Multi-Trailers
CLASS 7	4 or More Axles, Single Unit		

A32223

24-HOUR ROADWAY SEGMENT COUNTS (WITH FHWA CLASSIFICATION)

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Wednesday, April 26, 2023 **CITY#** Orange

JOB #: SC3962 CLASS1 Santiago Blvd from Meats Ave to Taft Ave

AM	COMBINED														PM	COMBINED													
TIME	1	2	3	4	5	6	7	8	9	10	11	12	13	TOTAL	Time	1	2	3	4	5	6	7	8	9	10	11	12	13	TOTAL
0:00	0	15	3	0	0	0	0	0	0	0	0	0	0	18	12:00	0	206	38	1	4	1	0	0	1	0	0	0	0	251
0:15	0	12	0	0	0	0	0	0	0	0	0	0	0	12	12:15	0	245	39	1	4	1	0	0	0	0	0	0	0	290
0:30	0	6	0	0	0	0	0	0	0	0	0	0	0	6	12:30	1	241	42	0	1	0	0	1	0	0	0	0	0	286
0:45	0	7	0	0	0	0	0	0	0	0	0	0	0	7	12:45	0	304	63	1	6	1	0	0	0	0	0	0	0	375
1:00	0	8	2	0	0	0	0	0	0	0	0	0	0	10	13:00	1	274	48	1	1	0	0	0	0	0	0	0	0	325
1:15	0	4	1	0	0	0	0	0	0	0	0	0	0	5	13:15	2	302	38	2	7	0	0	0	0	0	0	0	0	351
1:30	0	0	0	0	0	0	0	0	0	0	0	0	0	0	13:30	0	315	34	0	7	0	0	0	0	0	0	0	0	356
1:45	0	9	0	0	1	0	0	0	0	0	0	0	0	10	13:45	0	282	51	0	7	0	0	0	0	0	0	0	0	340
2:00	0	4	0	0	0	0	0	0	0	0	0	0	0	4	14:00	2	219	45	1	3	0	0	0	0	0	0	0	0	270
2:15	0	6	0	0	0	0	0	0	0	0	0	0	0	6	14:15	3	233	44	1	9	1	0	0	0	0	0	0	0	291
2:30	0	4	0	0	0	0	0	0	0	0	0	0	0	4	14:30	1	208	49	1	5	1	0	0	0	0	0	0	0	265
2:45	0	5	2	0	0	0	0	0	0	0	0	0	0	7	14:45	0	272	36	0	9	0	0	0	1	0	0	0	0	318
3:00	0	2	1	0	0	0	0	0	0	0	0	0	0	3	15:00	0	242	55	1	7	0	0	0	0	0	0	0	0	305
3:15	0	2	1	0	0	0	0	0	0	0	0	0	0	3	15:15	0	268	59	0	8	0	0	0	0	0	0	0	0	335
3:30	0	1	0	0	0	0	0	0	0	0	0	0	0	1	15:30	1	346	76	1	8	0	0	0	0	0	0	0	0	432
3:45	0	2	2	0	0	0	0	0	0	0	0	0	0	4	15:45	0	298	57	0	4	0	0	0	0	0	0	0	0	359
4:00	0	3	2	0	0	0	0	0	0	0	0	0	0	5	16:00	0	314	61	1	4	0	0	0	0	0	0	0	0	380
4:15	0	11	3	0	0	0	0	0	0	0	0	0	0	14	16:15	0	318	71	1	6	1	0	0	0	0	0	0	0	397
4:30	0	10	2	0	0	0	0	0	0	0	0	0	0	12	16:30	0	359	56	0	7	1	0	0	0	0	0	0	0	423
4:45	1	15	5	0	1	0	0	0	0	0	0	0	0	22	16:45	0	359	62	0	10	0	0	0	0	0	0	0	0	431
5:00	0	13	7	0	0	0	0	0	0	0	0	0	0	20	17:00	0	379	63	1	5	0	0	0	0	0	0	0	0	448
5:15	0	28	4	1	0	1	0	0	0	0	0	0	0	34	17:15	1	381	63	1	4	1	0	0	0	0	0	0	0	451
5:30	0	31	4	0	0	0	0	0	0	0	0	0	0	35	17:30	0	377	54	0	7	0	0	0	0	0	0	0	0	438
5:45	0	39	17	0	0	0	0	0	0	0	0	0	0	56	17:45	2	379	67	1	5	1	0	0	0	0	0	0	0	455
6:00	0	61	17	1	1	0	0	0	0	0	0	0	0	80	18:00	0	355	43	1	3	0	0	1	0	0	0	0	0	403
6:15	0	70	19	1	2	0	0	0	0	0	0	0	0	92	18:15	2	287	53	0	4	0	0	0	0	0	0	0	0	346
6:30	0	75	27	0	6	0	0	0	1	0	0	0	0	109	18:30	0	265	35	1	3	0	0	1	0	0	0	0	0	305
6:45	0	100	25	0	7	0	0	0	0	0	0	0	0	132	18:45	3	248	37	0	3	0	0	0	0	0	0	0	0	291
7:00	0	152	24	1	6	0	0	0	0	0	0	0	0	183	19:00	1	233	29	1	2	0	0	0	0	0	0	0	0	266
7:15	1	167	30	1	8	0	0	0	0	0	0	0	0	207	19:15	0	207	29	1	2	0	0	0	0	0	0	0	0	239
7:30	0	236	41	0	5	0	0	0	0	0	0	0	0	282	19:30	0	191	27	0	2	0	0	0	0	0	0	0	0	220
7:45	0	295	46	2	2	0	0	0	0	0	0	0	0	345	19:45	0	159	27	0	2	0	0	0	0	0	0	0	0	188
8:00	0	324	58	1	2	1	0	0	0	0	0	0	0	386	20:00	0	131	24	1	2	0	0	0	0	0	0	0	0	158
8:15	0	266	41	0	0	0	0	0	0	0	0	0	0	307	20:15	0	133	16	1	2	0	0	0	0	0	0	0	0	152
8:30	0	243	38	1	5	0	0	0	0	0	0	0	0	287	20:30	0	108	15	1	0	0	0	0	0	0	0	0	0	124
8:45	1	226	38	1	6	2	0	0	0	0	0	0	0	274	20:45	0	103	13	0	0	0	0	0	0	0	0	0	0	116
9:00	2	174	29	1	6	0	0	0	0	0	0	0	0	212	21:00	0	97	13	0	0	0	0	0	0	0	0	0	0	110
9:15	2	232	55	1	5	0	0	0	0	0	0	0	0	295	21:15	0	92	14	1	0	0	0	0	0	0	0	0	0	107
9:30	1	180	36	0	8	0	0	0	1	0	0	0	0	226	21:30	0	87	10	0	0	0	0	0	0	0	0	0	0	97
9:45	1	187	36	0	7	0	0	0	0	0	0	0	0	231	21:45	0	77	7	0	0	0	0	0	0	0	0	0	0	84
10:00	0	171	42	1	1	0	0	0	0	0	0	0	0	215	22:00	0	54	8	0	0	0	0	0	0	0	0	0	0	62
10:15	0	179	42	1	2	0	0	0	0	0	0	0	0	224	22:15	0	62	13	0	0	0	0	0	0	0	0	0	0	75
10:30	0	167	35	0	6	2	0	0	0	0	0	0	0	210	22:30	0	37	4	0	0	0	0	0	0	0	0	0	0	41
10:45	0	183	43	0	1	0	0	0	0	0	0	0	0	227	22:45	0	31	7	0	0	0	0	0	0	0	0	0	0	38
11:00	1	172	46	1	7	0	0	0	0	0	0	0	0	227	23:00	0	29	3	0	0	0	0	0	0	0	0	0	0	32
11:15	1	185	44	1	0	0	0	0	0	0	0	0	0	231	23:15	0	17	4	0	0	0	0	0	0	0	0	0	0	21
11:30	0	175	36	0	7	0	0	0	0	0	0	0	0	218	23:30	1	12	1	0	0	0	0	0	0	0	0	0	0	14
11:45	0	219	37	0	9	0	0	1	0	0	0	0	0	266	23:45	0	19	2	0	0	0	0	0	0	0	0	0	0	21
TOTAL	11	4,676	941	16	111	6	0	1	2	0	0	0	0	5,764	TOTAL	21	10,155	1,705	24	163	9	0	3	2	0	0	0	0	12,082

AM PEAK HOUR	7:45 AM
AM PEAK VOLUME	1,325

PM PEAK HOUR	5:00 PM
PM PEAK VOLUME	1,792

CLASS 1	Class 1 — Motorcycles	**CLASS 8**	3 to 4 Axles, Single Trailer
CLASS 2	Passenger Cars	**CLASS 9**	5 Axles, Single Trailer
CLASS 3	2 Axles, 4-Tire Single Units	**CLASS 10**	6 or More Axles, Single Trailer
CLASS 4	Buses	**CLASS 11**	5 or Less Axles, Multi-Trailers
CLASS 5	2 Axles, 6-Tire Single Units	**CLASS 12**	6 Axles, Multi-Trailers
CLASS 6	3 Axles, Single Unit	**CLASS 13**	7 or More Axles, Multi-Trailers
CLASS 7	4 or More Axles, Single Unit		

TOTAL: AM+PM	32	14,831	2,646	40	274	15	0	4	4	0	0	0	0	17,846
% OF TOTAL	0.2%	83.1%	14.8%	0.2%	1.5%	0.1%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100.0%

Wednesday, April 26, 2023 CITY: Villa Park - Orange PROJECT: SC3962

ADT2 Santiago Blvd from Taft Ave to Villa Park Rd. Suhsduhg#e|#DlpWG#OOF##who1#:47#586#:;;;

AM Period	NB		SB			PM Period	NB		SB		
0:00	13		10			12:00	136		166		
0:15	10		8			12:15	164		168		
0:30	4		5			12:30	242		157		
0:45	7	34	5	28	62	12:45	244	786	186	677	1463
1:00	10		2			13:00	174		167		
1:15	3		4			13:15	229		232		
1:30	0		0			13:30	190		177		
1:45	7	20	2	8	28	13:45	185	778	229	805	1583
2:00	2		4			14:00	163		163		
2:15	5		1			14:15	187		180		
2:30	0		1			14:30	143		157		
2:45	3	10	6	12	22	14:45	162	655	161	661	1316
3:00	2		1			15:00	222		167		
3:15	2		3			15:15	256		169		
3:30	2		1			15:30	250		225		
3:45	4	10	3	8	18	15:45	220	948	205	766	1714
4:00	4		4			16:00	240		178		
4:15	8		16			16:15	269		168		
4:30	9		8			16:30	265		163		
4:45	10	31	11	39	70	16:45	287	1061	158	667	1728
5:00	11		14			17:00	299		182		
5:15	17		20			17:15	314		223		
5:30	14		46			17:30	325		196		
5:45	41	83	39	119	202	17:45	279	1217	198	799	2016
6:00	38		48			18:00	280		171		
6:15	64		69			18:15	231		165		
6:30	57		70			18:30	202		164		
6:45	73	232	90	277	509	18:45	169	882	140	640	1522
7:00	107		127			19:00	178		156		
7:15	124		145			19:15	120		122		
7:30	203		204			19:30	104		136		
7:45	205	639	247	723	1362	19:45	96	498	102	516	1014
8:00	254		229			20:00	90		100		
8:15	221		208			20:15	95		107		
8:30	179		155			20:30	94		77		
8:45	169	823	145	737	1560	20:45	63	342	72	356	698
9:00	166		133			21:00	79		62		
9:15	180		162			21:15	68		73		
9:30	109		157			21:30	57		64		
9:45	137	592	143	595	1187	21:45	61	265	38	237	502
10:00	115		158			22:00	39		36		
10:15	122		138			22:15	41		40		
10:30	129		123			22:30	24		23		
10:45	153	519	138	557	1076	22:45	19	123	16	115	238
11:00	130		148			23:00	16		23		
11:15	126		132			23:15	7		9		
11:30	119		161			23:30	10		11		
11:45	151	526	138	579	1105	23:45	10	43	8	51	94
Total Vol.	3519		3682		**7201**		7598		6290		**13888**

Daily Totals

NB	SB	Combined
11117	9972	**21089**

	AM			PM		
Split %	48.9%	51.1%	**34.1%**	54.7%	45.3%	**65.9%**
Peak Hour	7:30	7:30	**7:30**	16:45	13:00	**17:00**
Volume	883	888	**1771**	1225	805	**2016**
P.H.F.	0.87	0.90	**0.92**	0.97	0.87	**0.94**

cs@aimtd.com Tell. 714 253 7888

Wednesday, April 26, 2023 CITY: Villa Park - Orange PROJECT: SC3962

ADT3 N Wanda Rd from E Garfield Ave to E Collins Ave. Suhsduhg#e|#DlpWG#OOF##who1#:47#586#:;;;

AM Period	NB		SB			PM Period	NB		SB		
0:00	4		17			12:00	125		166		
0:15	8		18			12:15	135		149		
0:30	4		5			12:30	174		183		
0:45	2	18	4	44	62	12:45	189	623	163	661	1284
1:00	7		2			13:00	137		140		
1:15	1		9			13:15	176		180		
1:30	1		1			13:30	130		152		
1:45	3	12	0	12	24	13:45	123	566	202	674	1240
2:00	0		4			14:00	116		134		
2:15	4		6			14:15	127		169		
2:30	2		2			14:30	119		149		
2:45	1	7	1	13	20	14:45	110	472	141	593	1065
3:00	1		4			15:00	141		161		
3:15	1		1			15:15	146		146		
3:30	2		4			15:30	163		190		
3:45	3	7	3	12	19	15:45	115	565	181	678	1243
4:00	5		2			16:00	171		152		
4:15	9		6			16:15	159		165		
4:30	17		4			16:30	167		179		
4:45	13	44	5	17	61	16:45	186	683	163	659	1342
5:00	16		7			17:00	183		184		
5:15	15		8			17:15	199		160		
5:30	28		13			17:30	174		150		
5:45	41	100	12	40	140	17:45	158	714	158	652	1366
6:00	48		35			18:00	150		145		
6:15	61		31			18:15	123		154		
6:30	54		43			18:30	106		142		
6:45	72	235	54	163	398	18:45	111	490	131	572	1062
7:00	89		81			19:00	110		130		
7:15	95		83			19:15	61		107		
7:30	156		150			19:30	61		117		
7:45	212	552	198	512	1064	19:45	63	295	85	439	734
8:00	239		208			20:00	56		82		
8:15	188		148			20:15	59		92		
8:30	151		153			20:30	60		83		
8:45	121	699	129	638	1337	20:45	43	218	79	336	554
9:00	129		116			21:00	43		64		
9:15	139		114			21:15	40		63		
9:30	109		121			21:30	35		61		
9:45	116	493	119	470	963	21:45	33	151	42	230	381
10:00	105		118			22:00	23		41		
10:15	109		114			22:15	17		51		
10:30	119		103			22:30	12		32		
10:45	113	446	128	463	909	22:45	9	61	14	138	199
11:00	120		132			23:00	10		22		
11:15	99		125			23:15	7		11		
11:30	92		137			23:30	5		19		
11:45	124	435	146	540	975	23:45	3	25	10	62	87
Total Vol.		3048		2924	**5972**			4863		5694	**10557**

	Daily Totals		
	NB	SB	**Combined**
	7911	8618	**16529**

	AM			**PM**		
Split %	51.0%	49.0%	**36.1%**	46.1%	53.9%	**63.9%**
Peak Hour	7:30	7:45	**7:30**	16:45	16:15	**16:30**
Volume	795	707	**1499**	742	691	**1421**
P.H.F.	0.83	0.85	**0.84**	0.96	0.94	**0.97**

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Wednesday, April 26, 2023 CITY: Orange PROJECT: SC3962

ADT6 Hewes St from Villa Park Rd to E Spring St. Suhsduhg#e|#DlpWG#OOF##who1#:47#586#:;;;

AM Period	NB		SB			PM Period	NB		SB		
0:00	3		5			12:00	65		55		
0:15	4		3			12:15	56		53		
0:30	0		2			12:30	73		66		
0:45	1	8	5	15	23	12:45	73	267	72	246	513
1:00	0		1			13:00	73		69		
1:15	1		2			13:15	70		81		
1:30	2		1			13:30	67		64		
1:45	0	3	0	4	7	13:45	65	275	96	310	585
2:00	0		3			14:00	59		73		
2:15	1		0			14:15	63		56		
2:30	2		0			14:30	99		84		
2:45	1	4	1	4	8	14:45	97	318	95	308	626
3:00	2		1			15:00	102		96		
3:15	1		1			15:15	115		90		
3:30	2		0			15:30	118		98		
3:45	3	8	3	5	13	15:45	124	459	117	401	860
4:00	1		2			16:00	113		81		
4:15	1		4			16:15	99		93		
4:30	3		2			16:30	109		89		
4:45	6	11	3	11	22	16:45	120	441	106	369	810
5:00	7		2			17:00	125		84		
5:15	3		5			17:15	128		94		
5:30	7		16			17:30	112		96		
5:45	16	33	6	29	62	17:45	117	482	88	362	844
6:00	17		19			18:00	116		67		
6:15	24		27			18:15	92		73		
6:30	45		42			18:30	85		66		
6:45	52	138	56	144	282	18:45	65	358	72	278	636
7:00	44		71			19:00	53		56		
7:15	69		103			19:15	52		47		
7:30	90		144			19:30	49		50		
7:45	134	337	181	499	836	19:45	43	197	41	194	391
8:00	133		174			20:00	38		35		
8:15	130		162			20:15	22		34		
8:30	74		123			20:30	53		30		
8:45	68	405	75	534	939	20:45	34	147	38	137	284
9:00	66		70			21:00	32		25		
9:15	71		74			21:15	41		31		
9:30	41		55			21:30	32		21		
9:45	44	222	58	257	479	21:45	18	123	22	99	222
10:00	38		67			22:00	14		17		
10:15	55		66			22:15	12		11		
10:30	45		55			22:30	8		15		
10:45	51	189	55	243	432	22:45	12	46	7	50	96
11:00	56		47			23:00	7		7		
11:15	49		66			23:15	4		8		
11:30	49		64			23:30	7		4		
11:45	60	214	47	224	438	23:45	5	23	8	27	50
Total Vol.		1572		1969	**3541**			3136		2781	**5917**

	Daily Totals		
	NB	SB	**Combined**
	4708	4750	**9458**

	AM			**PM**		
Split %	44.4%	55.6%	**37.4%**	53.0%	47.0%	**62.6%**
Peak Hour	7:30	7:30	**7:30**	16:45	15:00	**16:45**
Volume	487	661	**1148**	485	401	**865**
P.H.F.	0.91	0.91	**0.91**	0.98	0.86	**0.96**

cs@aimtd.com Tell. 714 253 7888

A32223

24-HOUR ROADWAY SEGMENT COUNTS (WITH FHWA CLASSIFICATION)

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Wednesday, April 26, 2023 **CITY#** Orange

JOB #: SC3962 CLASS13 Cannon St from Serrano Ave to Via Escola

AM	COMBINED														PM	COMBINED													
TIME	1	2	3	4	5	6	7	8	9	10	11	12	13	TOTAL	Time	1	2	3	4	5	6	7	8	9	10	11	12	13	TOTAL
0:00	0	17	1	0	0	0	0	0	0	0	0	0	0	18	12:00	1	196	8	0	2	1	0	0	0	0	0	0	0	208
0:15	0	13	0	0	0	0	0	0	1	0	0	0	0	14	12:15	0	184	14	0	3	0	0	0	0	0	0	0	0	201
0:30	0	8	0	0	0	0	0	0	0	0	0	0	0	8	12:30	1	223	26	1	5	0	0	0	0	0	0	0	0	256
0:45	0	5	0	0	0	0	0	0	0	0	0	0	0	5	12:45	2	224	16	0	1	0	0	0	0	0	0	0	0	243
1:00	0	4	0	0	0	0	0	0	0	0	0	0	0	4	13:00	0	215	14	5	9	0	0	0	0	0	0	0	0	243
1:15	0	3	0	0	0	0	0	0	0	0	0	0	0	3	13:15	1	247	18	1	4	0	0	0	0	0	0	0	0	271
1:30	0	3	0	0	0	0	0	0	0	0	0	0	0	3	13:30	1	204	12	1	3	1	0	0	0	0	0	0	0	222
1:45	0	2	0	0	0	0	0	0	0	0	0	0	0	2	13:45	0	215	27	1	3	0	0	0	0	0	0	0	0	246
2:00	0	7	0	0	0	0	0	0	0	0	0	0	0	7	14:00	1	207	28	3	7	0	0	0	0	0	0	0	0	246
2:15	0	2	0	0	0	0	0	0	0	0	0	0	0	2	14:15	0	217	22	0	5	0	0	0	0	0	0	0	0	244
2:30	0	6	1	0	0	0	0	0	0	0	0	0	0	7	14:30	0	208	36	2	6	0	0	0	0	0	0	0	0	252
2:45	0	6	0	0	0	0	0	0	0	0	0	0	0	6	14:45	1	300	18	0	8	1	0	0	0	0	0	0	0	328
3:00	0	3	0	0	0	0	0	0	0	0	0	0	0	3	15:00	3	354	28	1	2	1	0	0	0	0	1	0	0	390
3:15	0	8	0	0	0	0	0	0	0	0	0	0	0	8	15:15	1	307	15	0	3	1	0	0	0	0	0	0	0	327
3:30	0	5	0	0	0	0	0	0	0	0	0	0	0	5	15:30	3	410	23	0	7	0	0	0	0	0	0	0	0	443
3:45	0	6	0	0	0	0	0	0	0	0	0	0	0	6	15:45	0	422	24	0	9	0	0	0	0	0	0	0	0	455
4:00	1	9	0	0	0	0	0	0	0	0	0	0	0	10	16:00	1	418	21	1	9	0	0	0	0	0	0	0	0	450
4:15	0	18	0	0	0	0	0	0	0	0	0	0	0	18	16:15	5	433	19	0	3	0	0	0	0	0	0	0	0	460
4:30	0	17	0	0	0	0	0	0	0	0	0	0	0	17	16:30	2	409	17	0	4	0	0	0	0	0	0	0	0	432
4:45	0	38	0	0	0	0	0	0	0	0	0	0	0	38	16:45	1	492	22	0	2	0	0	0	0	0	0	0	0	517
5:00	0	34	0	0	2	0	0	0	0	0	0	0	0	36	17:00	3	457	12	0	2	0	0	0	0	0	0	0	0	474
5:15	0	54	2	0	1	0	0	0	0	0	0	0	0	57	17:15	3	469	20	0	1	0	0	0	0	0	0	0	0	493
5:30	0	54	5	0	1	0	0	0	0	0	0	0	0	60	17:30	4	526	14	1	5	0	0	0	0	0	0	0	0	550
5:45	0	61	2	0	1	0	0	0	0	0	0	0	0	64	17:45	2	420	17	1	2	0	0	0	0	0	0	0	0	442
6:00	0	79	1	0	0	0	0	0	0	0	0	0	0	80	18:00	1	400	12	0	0	0	0	0	0	0	0	0	0	413
6:15	2	118	5	0	2	0	0	0	0	0	0	0	0	127	18:15	1	374	14	0	1	0	0	0	0	0	0	0	0	390
6:30	1	193	10	0	3	0	0	0	0	0	0	0	0	207	18:30	1	331	5	1	1	1	0	0	0	0	0	0	0	340
6:45	0	266	14	2	7	0	0	0	0	0	0	0	0	289	18:45	1	270	10	0	0	0	0	0	0	0	0	0	0	281
7:00	0	299	9	1	5	0	0	0	0	0	0	0	0	314	19:00	2	209	9	0	0	0	0	0	0	0	0	0	0	220
7:15	1	423	21	0	9	1	0	0	0	0	0	0	0	455	19:15	0	190	12	0	1	0	0	0	0	0	0	0	0	203
7:30	1	498	25	1	8	1	0	0	0	0	0	0	0	534	19:30	1	161	5	0	1	0	0	0	0	0	0	0	0	168
7:45	1	492	28	3	11	0	0	0	0	0	0	0	0	535	19:45	0	131	5	0	2	0	0	0	0	0	0	0	0	138
8:00	0	492	29	2	8	0	0	0	0	0	0	0	0	531	20:00	0	136	4	0	0	0	0	0	0	0	0	0	0	140
8:15	2	481	30	0	11	2	0	0	0	0	0	0	0	526	20:15	0	143	3	0	1	0	0	0	0	0	0	0	0	147
8:30	3	391	19	1	10	0	0	0	0	0	0	0	0	424	20:30	0	110	6	1	0	0	0	0	0	0	0	0	0	117
8:45	0	275	10	1	10	1	0	0	0	0	1	0	0	298	20:45	1	111	3	0	0	0	0	0	0	0	0	0	0	115
9:00	1	278	26	0	3	1	0	0	0	0	1	0	0	310	21:00	1	114	0	0	0	0	0	0	0	0	0	0	0	115
9:15	0	238	29	3	4	0	0	0	0	0	0	0	0	274	21:15	1	106	1	0	1	0	0	0	0	0	0	0	0	109
9:30	1	223	24	1	3	0	0	0	0	0	0	0	0	252	21:30	1	81	3	0	0	0	0	0	0	0	0	0	0	85
9:45	1	225	14	0	9	0	0	0	0	0	0	0	0	249	21:45	0	56	1	0	0	0	0	0	0	0	0	0	0	57
10:00	0	210	26	0	7	0	0	0	0	0	0	0	0	243	22:00	0	70	1	0	0	0	0	0	0	0	0	0	0	71
10:15	0	197	22	1	8	0	0	0	0	0	0	0	0	228	22:15	0	52	1	0	0	0	0	0	0	0	0	0	0	53
10:30	0	196	22	1	3	0	0	0	0	0	0	0	0	222	22:30	1	46	2	0	0	0	0	0	0	0	0	0	0	49
10:45	1	168	25	0	10	0	0	0	0	0	0	0	0	204	22:45	0	26	0	0	0	0	0	0	0	0	0	0	0	26
11:00	2	198	20	0	4	0	0	0	0	0	0	0	0	224	23:00	0	33	1	0	0	0	0	0	0	0	0	0	0	34
11:15	1	185	31	0	7	0	0	0	0	0	0	0	0	224	23:15	0	22	0	0	0	0	0	0	0	0	0	0	0	22
11:30	1	213	16	3	1	0	0	0	0	0	0	0	0	234	23:30	0	18	0	0	0	0	0	0	0	0	0	0	0	18
11:45	0	214	22	2	3	0	0	0	0	0	0	0	0	241	23:45	0	15	0	0	0	0	0	0	0	0	0	0	0	15
TOTAL	20	6,935	489	22	151	6	0	0	1	0	2	0	0	7,626	TOTAL	48	10,962	569	20	113	6	0	0	0	0	1	0	0	11,719

AM PEAK HOUR	7:30 AM
AM PEAK VOLUME	2,126

PM PEAK HOUR	4:45 PM
PM PEAK VOLUME	2,034

CLASS 1	Class 1 — Motorcycles	**CLASS 8**	3 to 4 Axles, Single Trailer
CLASS 2	Passenger Cars	**CLASS 9**	5 Axles, Single Trailer
CLASS 3	2 Axles, 4-Tire Single Units	**CLASS 10**	6 or More Axles, Single Trailer
CLASS 4	Buses	**CLASS 11**	5 or Less Axles, Multi-Trailers
CLASS 5	2 Axles, 6-Tire Single Units	**CLASS 12**	6 Axles, Multi-Trailers
CLASS 6	3 Axles, Single Unit	**CLASS 13**	7 or More Axles, Multi-Trailers
CLASS 7	4 or More Axles, Single Unit		

TOTAL: AM+PM	68	17,897	1,058	42	264	12	0	0	1	0	3	0	0	19,345
% OF TOTAL	0.4%	92.5%	5.5%	0.2%	1.4%	0.1%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100.0%

Wednesday, April 26, 2023 CITY: Orange PROJECT: SC3962

ADT11 Cannon St from Santiago Taft Ave to Serrano Ave. Suhsduhg#e|#DlpWG#OOF##who1#:47#586#:;;;

AM Period	NB		SB			PM Period	NB		SB		
0:00	22		12			12:00	176		185		
0:15	9		12			12:15	199		212		
0:30	13		4			12:30	211		230		
0:45	5	49	5	33	82	12:45	218	804	197	824	1628
1:00	5		7			13:00	201		208		
1:15	3		4			13:15	256		211		
1:30	6		4			13:30	177		210		
1:45	2	16	3	18	34	13:45	225	859	196	825	1684
2:00	3		5			14:00	238		200		
2:15	2		4			14:15	238		202		
2:30	6		9			14:30	250		214		
2:45	5	16	2	20	36	14:45	314	1040	236	852	1892
3:00	1		5			15:00	403		234		
3:15	1		7			15:15	424		206		
3:30	3		9			15:30	556		241		
3:45	3	8	7	28	36	15:45	518	1901	272	953	2854
4:00	4		14			16:00	594		267		
4:15	7		22			16:15	606		283		
4:30	5		30			16:30	631		238		
4:45	18	34	37	103	137	16:45	631	2462	326	1114	3576
5:00	17		42			17:00	657		273		
5:15	20		75			17:15	639		302		
5:30	25		77			17:30	656		299		
5:45	28	90	93	287	377	17:45	586	2538	236	1110	3648
6:00	38		128			18:00	545		240		
6:15	59		196			18:15	508		193		
6:30	67		304			18:30	416		187		
6:45	90	254	388	1016	1270	18:45	350	1819	201	821	2640
7:00	91		489			19:00	275		119		
7:15	145		615			19:15	246		144		
7:30	175		660			19:30	187		138		
7:45	227	638	618	2382	3020	19:45	184	892	106	507	1399
8:00	225		602			20:00	169		91		
8:15	256		558			20:15	153		76		
8:30	207		467			20:30	144		65		
8:45	174	862	345	1972	2834	20:45	141	607	76	308	915
9:00	169		293			21:00	124		85		
9:15	166		265			21:15	140		71		
9:30	159		292			21:30	145		46		
9:45	160	654	272	1122	1776	21:45	74	483	48	250	733
10:00	160		287			22:00	76		43		
10:15	174		233			22:15	58		49		
10:30	135		242			22:30	56		30		
10:45	146	615	212	974	1589	22:45	33	223	16	138	361
11:00	174		222			23:00	45		15		
11:15	168		247			23:15	33		19		
11:30	217		222			23:30	18		12		
11:45	225	784	194	885	1669	23:45	21	117	17	63	180
Total Vol.		4020		8840	**12860**			13745		7765	**21510**

	Daily Totals		
	NB	SB	**Combined**
	17765	16605	**34370**

	AM			**PM**		
Split %	31.3%	68.7%	**37.4%**	63.9%	36.1%	**62.6%**
Peak Hour	7:45	7:15	**7:30**	16:45	16:45	**16:45**
Volume	915	2495	**3321**	2583	1200	**3783**
P.H.F.	0.89	0.95	**0.98**	0.98	0.92	**0.99**

cs@aimtd.com Tell. 714 253 7888

A32223

24-HOUR ROADWAY SEGMENT COUNTS (WITH FHWA CLASSIFICATION)

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Wednesday, April 26, 2023 **CITY#** Orange

JOB #: SC3962 CLASS10 Cannon St from Santiago Canyon Rd to Taft Ave

AM	COMBINED														PM	COMBINED													
TIME	1	2	3	4	5	6	7	8	9	10	11	12	13	TOTAL	Time	1	2	3	4	5	6	7	8	9	10	11	12	13	TOTAL
0:00	0	31	1	0	0	0	0	0	0	0	0	0	0	32	12:00	1	276	22	1	4	0	2	0	0	0	0	0	0	306
0:15	0	12	1	0	0	0	0	0	0	0	0	0	0	13	12:15	0	314	36	1	10	1	1	0	0	0	0	0	0	363
0:30	0	15	1	0	0	0	0	0	0	0	0	0	0	16	12:30	1	354	33	1	3	1	0	0	0	0	0	0	0	393
0:45	0	12	0	0	0	0	0	0	0	0	0	0	0	12	12:45	3	315	31	3	4	0	0	0	0	0	0	0	0	356
1:00	0	9	0	0	0	0	0	0	0	0	0	0	0	9	13:00	2	281	32	4	8	2	0	0	0	0	0	0	0	329
1:15	0	9	0	0	0	0	0	0	0	0	0	0	0	9	13:15	3	389	40	1	4	0	0	0	0	0	0	0	0	437
1:30	0	8	0	0	0	0	0	0	0	0	0	0	0	8	13:30	2	312	32	1	5	0	1	0	0	0	0	0	0	353
1:45	0	6	0	0	0	0	0	0	0	0	0	0	0	6	13:45	0	345	36	1	7	0	0	0	0	0	0	0	0	389
2:00	0	7	0	0	0	0	0	0	0	0	0	0	0	7	14:00	3	339	36	2	8	0	0	0	0	0	0	0	0	388
2:15	0	7	0	0	0	0	0	0	0	0	0	0	0	7	14:15	0	344	48	1	6	0	0	0	0	0	0	0	0	399
2:30	0	8	0	0	0	0	0	0	0	0	0	0	0	8	14:30	1	367	51	1	17	0	0	0	0	0	0	0	0	437
2:45	0	11	0	0	0	0	0	0	0	0	0	0	0	11	14:45	2	478	35	2	10	1	0	0	0	0	0	0	0	528
3:00	0	5	0	0	0	0	0	0	0	0	0	0	0	5	15:00	2	585	58	1	6	2	1	0	0	0	0	0	0	655
3:15	0	7	0	0	1	0	0	0	0	0	0	0	0	8	15:15	3	617	54	2	7	0	0	0	0	0	0	0	0	683
3:30	0	11	0	0	0	0	0	0	0	0	0	0	0	11	15:30	3	699	54	1	11	0	0	0	0	0	0	0	0	768
3:45	0	10	0	0	0	0	0	0	0	0	0	0	0	10	15:45	1	682	52	3	12	0	0	0	0	0	0	0	0	750
4:00	1	12	1	0	0	0	0	0	0	0	0	0	0	14	16:00	2	701	43	4	5	0	0	0	0	0	0	0	1	756
4:15	0	31	0	0	0	0	0	0	0	0	0	0	0	31	16:15	5	764	32	0	4	1	0	0	0	0	0	0	0	806
4:30	0	29	1	0	0	0	0	0	0	0	0	0	0	30	16:30	3	754	40	1	4	0	0	0	0	0	2	0	0	804
4:45	0	50	0	0	0	0	0	0	0	0	0	0	0	50	16:45	1	839	38	1	2	0	0	0	1	0	1	0	2	885
5:00	1	46	1	0	0	0	0	0	0	0	0	0	0	48	17:00	2	859	43	0	4	0	0	0	0	0	0	0	0	908
5:15	0	90	2	0	1	0	0	0	0	0	0	0	0	93	17:15	3	794	29	0	6	0	0	0	0	0	0	0	0	832
5:30	0	88	10	0	1	0	0	0	1	0	0	0	0	100	17:30	5	847	35	1	3	0	0	0	0	0	0	0	0	891
5:45	1	106	4	0	1	0	0	0	0	0	0	0	0	112	17:45	3	734	31	1	5	0	0	0	0	0	0	0	0	774
6:00	0	137	8	0	1	0	0	0	0	0	0	0	0	146	18:00	1	698	36	0	2	0	0	0	0	0	0	0	0	737
6:15	3	213	10	0	1	0	0	0	1	0	0	0	0	228	18:15	2	626	42	0	4	1	0	0	0	0	0	0	0	675
6:30	2	330	22	3	4	0	0	0	0	0	0	0	0	361	18:30	0	515	29	2	2	0	0	0	0	0	0	0	0	548
6:45	0	417	29	3	7	0	0	0	1	0	0	0	0	457	18:45	1	444	29	0	1	0	0	0	0	0	0	0	0	475
7:00	0	482	21	2	7	0	0	0	0	0	0	0	0	512	19:00	2	350	22	0	0	0	0	0	0	0	0	0	0	374
7:15	4	656	26	3	9	0	0	0	0	0	1	0	0	699	19:15	1	308	24	0	1	0	0	0	0	0	0	0	0	334
7:30	0	721	28	2	11	0	0	0	0	0	1	0	0	763	19:30	1	252	15	0	1	0	0	0	0	0	0	0	0	269
7:45	1	745	40	3	9	0	0	0	0	0	0	0	0	798	19:45	0	254	10	0	1	0	0	0	0	0	0	0	0	265
8:00	2	716	40	4	8	0	0	0	1	0	0	0	2	773	20:00	0	219	4	0	1	0	0	0	0	0	0	0	0	224
8:15	2	734	42	2	7	3	0	0	0	0	0	0	0	790	20:15	0	201	6	0	1	0	0	0	0	0	0	0	0	208
8:30	3	666	29	1	10	0	0	0	0	0	0	0	0	709	20:30	0	176	11	1	0	0	0	0	0	0	0	0	0	188
8:45	1	454	30	4	6	1	0	0	0	0	1	0	0	497	20:45	0	189	5	0	0	0	0	0	0	0	0	0	0	194
9:00	1	391	36	1	7	0	0	0	0	0	1	0	0	437	21:00	1	172	2	0	0	0	0	0	0	0	0	0	0	175
9:15	0	336	38	3	4	0	0	0	0	0	0	0	0	381	21:15	2	193	2	0	1	0	0	0	0	0	0	0	0	198
9:30	2	356	38	0	9	0	0	0	0	0	0	0	0	405	21:30	1	175	2	0	0	0	0	0	0	0	0	0	0	178
9:45	1	342	32	0	12	2	0	0	0	0	0	0	0	389	21:45	0	114	1	0	0	0	0	0	0	0	0	0	0	115
10:00	0	368	40	0	7	0	2	0	0	0	0	0	0	417	22:00	1	105	5	0	0	0	0	0	0	0	0	0	0	111
10:15	0	337	36	1	7	0	0	0	0	0	0	0	0	381	22:15	0	87	6	0	0	0	0	0	0	0	0	0	0	93
10:30	0	280	32	1	7	0	1	0	0	0	0	0	0	321	22:30	2	74	1	0	0	0	0	0	0	0	0	0	0	77
10:45	2	283	31	0	13	1	0	0	0	0	0	0	0	330	22:45	0	47	1	0	0	0	0	0	0	0	0	0	0	48
11:00	5	309	33	0	7	0	1	0	0	0	0	0	0	355	23:00	0	58	1	0	0	0	0	0	0	0	0	0	0	59
11:15	1	311	35	0	9	0	0	0	0	0	0	0	0	356	23:15	0	45	0	0	0	0	0	0	0	0	0	0	0	45
11:30	2	364	32	6	8	1	1	0	0	0	0	0	0	414	23:30	0	29	0	0	0	0	0	0	0	0	0	0	0	29
11:45	0	329	37	2	11	0	1	0	0	0	0	0	0	380	23:45	0	33	1	0	0	0	0	0	0	0	0	0	0	34
TOTAL	35	10,897	767	41	185	8	6	0	4	0	4	0	2	11,949	TOTAL	66	18,353	1,196	37	170	9	5	0	1	0	3	0	3	19,843

AM PEAK HOUR	7:30 AM
AM PEAK VOLUME	3,124

PM PEAK HOUR	4:45 PM
PM PEAK VOLUME	3,516

CLASS 1	Class 1 — Motorcycles	**CLASS 8**	3 to 4 Axles, Single Trailer
CLASS 2	Passenger Cars	**CLASS 9**	5 Axles, Single Trailer
CLASS 3	2 Axles, 4-Tire Single Units	**CLASS 10**	6 or More Axles, Single Trailer
CLASS 4	Buses	**CLASS 11**	5 or Less Axles, Multi-Trailers
CLASS 5	2 Axles, 6-Tire Single Units	**CLASS 12**	6 Axles, Multi-Trailers
CLASS 6	3 Axles, Single Unit	**CLASS 13**	7 or More Axles, Multi-Trailers
CLASS 7	4 or More Axles, Single Unit		

TOTAL: AM+PM	101	29,250	1,963	78	355	17	11	0	5	0	7	0	5	31,792
% OF TOTAL	0.3%	92.0%	6.2%	0.2%	1.1%	0.1%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100.0%

Wednesday, April 26, 2023 CITY: Orange PROJECT: SC3962

ADT9 N Cannon St from Santiago Canyon Rd to E Avenida Palmar. Suhsduhg#e|#DlpWG#OOF##who1#:47#586#:;;;

AM Period	NB		SB			PM Period	NB		SB		
0:00	2		6			12:00	38		42		
0:15	0		4			12:15	58		52		
0:30	4		0			12:30	52		58		
0:45	1	7	0	10	17	12:45	42	190	36	188	378
1:00	0		2			13:00	33		23		
1:15	1		1			13:15	38		40		
1:30	0		0			13:30	34		43		
1:45	0	1	1	4	5	13:45	53	158	38	144	302
2:00	2		1			14:00	47		42		
2:15	1		1			14:15	35		39		
2:30	0		0			14:30	42		43		
2:45	1	4	1	3	7	14:45	41	165	48	172	337
3:00	0		0			15:00	57		32		
3:15	0		0			15:15	72		53		
3:30	2		1			15:30	72		57		
3:45	2	4	0	1	5	15:45	61	262	43	185	447
4:00	2		4			16:00	77		54		
4:15	2		1			16:15	81		49		
4:30	2		2			16:30	71		55		
4:45	8	14	5	12	26	16:45	101	330	61	219	549
5:00	8		1			17:00	104		53		
5:15	6		2			17:15	78		67		
5:30	9		7			17:30	117		65		
5:45	5	28	7	17	45	17:45	105	404	65	250	654
6:00	8		9			18:00	101		42		
6:15	20		25			18:15	64		37		
6:30	28		47			18:30	47		58		
6:45	29	85	48	129	214	18:45	54	266	35	172	438
7:00	30		73			19:00	39		31		
7:15	37		73			19:15	30		42		
7:30	61		110			19:30	22		31		
7:45	91	219	137	393	612	19:45	30	121	26	130	251
8:00	81		67			20:00	29		23		
8:15	51		69			20:15	26		31		
8:30	40		82			20:30	26		29		
8:45	39	211	52	270	481	20:45	19	100	27	110	210
9:00	45		46			21:00	19		19		
9:15	42		39			21:15	16		26		
9:30	36		46			21:30	11		30		
9:45	34	157	35	166	323	21:45	7	53	17	92	145
10:00	34		32			22:00	14		13		
10:15	31		33			22:15	8		22		
10:30	38		42			22:30	12		8		
10:45	40	143	36	143	286	22:45	3	37	9	52	89
11:00	35		35			23:00	6		7		
11:15	33		34			23:15	4		3		
11:30	30		44			23:30	4		6		
11:45	36	134	50	163	297	23:45	3	17	4	20	37
Total Vol.		1007		1311	**2318**			2103		1734	**3837**

	Daily Totals		
	NB	SB	**Combined**
	3110	3045	**6155**

	AM			**PM**		
Split %	43.4%	56.6%	**37.7%**	54.8%	45.2%	**62.3%**
Peak Hour	7:30	7:00	**7:30**	17:00	17:00	**17:00**
Volume	284	393	**667**	404	250	**654**
P.H.F.	0.78	0.72	**0.73**	0.89	0.93	**0.90**

cs@aimtd.com Tell. 714 253 7888

Attachment B: Analysis Results

Attachment B Analysis Results

Existing Roadway Segment LOS

	Segment	Lanes	ADT	Capacity[2]	V/C	LOS
1.	Villa Park Road between Wanda Road and Center Drive	4	25,828	37,500	0.69	B
2.	Villa Park Road between Center Drive and Hewes Street	4	23,840	37,500	0.64	B
3.	Villa Park Road between Hewes Street and Cannon Street	4	26,178	37,500	0.7	B
4.	Santiago Canyon Road east of Cannon Street	4	27,610	37,500	0.74	C
5.	Taft Avenue between Santiago Boulevard and Center Drive	2	11,979	22,000	0.54	A
6.	Taft Avenue between Center Drive and Cannon Street	2	6,217	22,000	0.28	A
7.	Serrano Avenue east of Cannon Road	4	18,826	37,500	0.5	A
8.	Santiago Boulevard north of Taft Avenue	4	17,846	37,500	0.48	A
9.	Wanda Road between Villa Park Road and Taft Avenue	4	21,089	37,500	0.56	A
10.	Wanda Road south of Villa Park Road	3	16,529	25,000	0.66	B
11.	Hewes Street south of Villa Park Road	4	9,458	25,000	0.38	A
12.	Cannon Street north of Serrano Avenue	4	19,345	37,500	0.52	A
13.	Cannon Street between Taft Avenue and Serrano Avenue	4	34,370	37,500	0.92	**E**
14.	Cannon Street between Villa Park Road and Taft Avenue	4	31,792	37,500	0.85	**D**
15.	Cannon Street south of Santiago Canyon Road	4	6,155	25,500	0.25	A

Note(s):

1. **Bold** symbolizes segment operates below OCTA MPAH LOS standards.
2. Capacities for LOS E were referenced for arterial types from the MPAH.

Source: Fehr & Peers, 2023.

Existing Intersection LOS

	Intersection	Control	Methodology	Peak Hour	V/C / Delay	LOS
1.	Wanda Road and Villa Park Road	Signalized	ICU / HCM	AM PM	0.743 / 44 0.731 / 44	C / D C / D
2.	Kathleen Lane and Villa Park Road	TWSC	HCM	AM PM	21 15	C B
3.	Morrow Circle and Villa Park Road	TWSC	HCM	AM PM	**43** **56**	**E** **F**
4.	Prado Woods Drive and Villa Park Road	TWSC	HCM	AM PM	32 **40**	D **E**
5.	Kenwick Drive and Villa Park Road	TWSC	HCM	AM PM	**35** 15	**E** C
6.	Radec Court and Villa Park Road	TWSC	HCM	AM PM	**36** **59**	**E** **F**
7.	Center Drive and Villa Park Road	Signalized	ICU / HCM	AM PM	0.533 / 15 0.511 / 6	A / B A / A
8.	Park Villa Lane and Villa Park Road	TWSC	HCM	AM PM	**39** **48**	**E** **E**
9.	Lemon Street and Villa Park Road	Signalized	ICU / HCM	AM PM	0.390 / 5 0.497 / 5	A / A A / A
10.	Hewes Street and Villa Park Road	Signalized	ICU / HCM	AM PM	0.767 / 46 0.605 / 20	C / D B / B
11.	Linda Vista Street and Villa Park Road	TWSC	HCM	AM PM	**>100** **>100**	**F** **F**
12.	Cannon Street and Santiago Canyon Road	Signalized	ICU / HCM	AM PM	0.694 / 45 0.758 / **61**	B / D C / **E**
13.	Cannon Street and Taft Avenue	Signalized	ICU / HCM	AM PM	0.893 / **63** **0.900** / 55	D / **E** **E** / D
14.	Cannon Street and Serrano Avenue	Signalized	ICU / HCM	AM PM	0.758 / 23 **0.909** / 29	C / C **E** / C
15.	Cannon Street and Loma Street	TWSC	HCM	AM PM	**85** 33	**F** D

Note(s):

1. ">100" is reported for highly congested movements where more than 100 seconds is reported.
2. **Bold** symbolizes segment operates below Jurisdictions' LOS standards.

Source: Fehr & Peers, 2023.

Existing Multimodal LOS

Intersection	Mode	Peak Hour	Delay	LOS
1. Wanda Rod and Villa Park Road	Bike/Pedestrian	AM	52.4/56	D / E
		PM	38.7/60.4	D / E
9. Lemon Street and Villa Park Road	Bike/Pedestrian	AM	5.8/7.7	A / A
		PM	5.7/9.3	A / A
10. Hewes Street and Villa Park Road	Bike/Pedestrian	AM	56.9/56.5	E / E
		PM	60.6/68.1	E / E
12. Cannon Street and Santiago Canyon Road	Bike/Pedestrian	AM	15.6/45.1	B / D
		PM	36.5/49.5	D / D

Source: Fehr & Peers, 2023.

Existing 95th Percentile Queues

Intersection	Movement	Storage (ft)	Peak Hour	Queue (ft)
12. Cannon Street and Santiago Canyon Road	EBL	425	AM	225
			PM	<u>425</u>
	EBT	675	AM	550
			PM	200
	WBL	225	AM	50
			PM	50
	WBT	>1000	AM	275
			PM	750
	WBR	700	AM	0
			PM	500
	NBL	150	AM	**200**
			PM	125
	NBT	500	AM	100
			PM	250
	SBL	550	AM	**600**
			PM	200
	SBT	>1000	AM	450
			PM	200
	SBR	400	AM	300
			PM	150

Notes:
1. Queues were calculated using HCM 7th methodologies and are rounded up to the nearest 25-foot increment.
2. "EB" = eastbound, "WB" = westbound, "NB" = northbound, "SB" = southbound, "L" = left, "T" = through, "R" = right.
3. **Bold** symbolizes movement exceeds available storage capacity.
4. <u>Underlined</u> symbolizes movement is at capacity.

Source: Fehr & Peers, 2023.

Future Year Roadway Segment LOS

	Segment	Lanes	ADT	Capacity[2]	V/C	LOS
1.	Villa Park Road between Wanda Road and Center Drive	4	27,670	37,500	0.74	C
2.	Villa Park Road between Center Drive and Hewes Street	4	25,700	37,500	0.69	B
3.	Villa Park Road between Hewes Street and Cannon Street	4	30,400	37,500	0.81	**D**
4.	Santiago Canyon Road east of Cannon Street	4	30,170	37,500	0.8	**D**
5.	Taft Avenue between Santiago Boulevard and Center Drive	2	11,700	22,000	0.53	A
6.	Taft Avenue between Center Drive and Cannon Street	2	5,530	22,000	0.25	A
7.	Serrano Avenue east of Cannon Road	4	19,270	37,500	0.51	A
8.	Santiago Boulevard north of Taft Avenue	4	18,250	37,500	0.49	A
9.	Wanda Road between Villa Park Road and Taft Avenue	4	21,910	37,500	0.58	A
10.	Wanda Road south of Villa Park Road	4	16,160	25,000	0.65	B
11.	Hewes Street south of Villa Park Road	4	11,770	25,000	0.47	A
12.	Cannon Street north of Serrano Avenue	4	22,660	37,500	0.6	B
13.	Cannon Street between Taft Avenue and Serrano Avenue	5	39,120	46,900	0.83	**D**
14.	Cannon Street between Villa Park Road and Taft Avenue	5	36,890	46,900	0.79	C
15.	Cannon Street south of Santiago Canyon Road	4	7,800	25,500	0.31	A

Note(s):

1. **Bold** symbolizes segment operates below County's LOS standards.
2. Capacities for LOS E were referenced for arterial types from the MPAH.

Source: Fehr & Peers, 2023.

Future Year Intersection LOS

	Intersection	Control	Methodology	Peak Hour	V/C / Delay	LOS
1.	Wanda Rod and Villa Park Road	Signalized	ICU / HCM	AM PM	0.836 / 37 0.774 / 45	D / D C / D
2.	Kathleen Lane and Villa Park Road	TWSC	HCM	AM PM	**64** 15	**F** C
3.	Morrow Circle and Villa Park Road	TWSC	HCM	AM PM	**63** **64**	**F** **F**
4.	Prado Woods Drive and Villa Park Road	TWSC	HCM	AM PM	**50** **79**	**F** **F**
5.	Kenwick Drive and Villa Park Road	TWSC	HCM	AM PM	**63** 15	**F** C
6.	Radec Court and Villa Park Road	TWSC	HCM	AM PM	**60** **>100**	**F** **F**
7.	Center Drive and Villa Park Road	Signalized	ICU / HCM	AM PM	0.585 / 11 0.547 / 7	A / B A / A
8.	Park Villa Lane and Villa Park Road	TWSC	HCM	AM PM	**49** **57**	**E** **F**
9.	Lemon Street and Villa Park Road	Signalized	ICU / HCM	AM PM	0.450 / 5 0.524 / 6	A / A A / A
10.	Hewes Street and Villa Park Road	Signalized	ICU / HCM	AM PM	**0.955 / 70** 0.889 / 25	**E / E** D / C
11.	Linda Vista Street and Villa Park Road	TWSC	HCM	AM PM	**>100** **>100**	**F** **F**
12.	Cannon Street and Santiago Canyon Road	Signalized	ICU / HCM	AM PM	**0.914** / 49 **0.935** / 47	**E** / D **E** / D
13.	Cannon Street and Taft Avenue	Signalized	ICU / HCM	AM PM	**1.064 / >100** 0.783 / 39	**F / F** C / D
14.	Cannon Street and Serrano Avenue	Signalized	ICU / HCM	AM PM	**0.910** / 39 0.696 / 13	**E** / D B / B
15.	Cannon Street and Loma Street	TWSC	HCM	AM PM	**>100** **83**	**F** **F**

Note(s):

1. ">100" is reported for highly congested movements where more than 100 seconds is reported.
2. **Bold** symbolizes segment operates below Jurisdictions' LOS standards.

Source: Fehr & Peers, 2023.

Future Year Multi-modal LOS

Intersection	Mode	Peak Hour	Delay	LOS
1. Wanda Rod and Villa Park Road	Bike/Pedestrian	AM	14.2/73	B / E
		PM	47.8/60.4	D / E
9. Lemon Street and Villa Park Road	Bike/Pedestrian	AM	5.8/8.3	A / A
		PM	6.2/9.6	A / A
10. Hewes Street and Villa Park Road	Bike/Pedestrian	AM	62.7/63.9	E / E
		PM	58.7/65.6	E / E
12. Cannon Street and Santiago Canyon Road	Bike/Pedestrian	AM	22.8/42.2	B / D
		PM	64.3/76.8	E / E

Source: Fehr & Peers, 2023.

Future Year 95th Percentile Queues

Intersection	Movement	Storage (ft)	Peak Hour	Queue (ft)
10. Hewes Street and Villa Park Road	EBT	>1000	AM PM	1,025 475
	WBL	250	AM PM	**1,075** **350**
	WBT	550	AM PM	425 175
	NBL	550	AM PM	450 275
	NBR	550	AM PM	125 225
12. Cannon Street and Santiago Canyon Road	EBL	425	AM PM	175 **675**
	EBT	675	AM PM	375 250
	WBL	225	AM PM	50 50
	WBT	>1000	AM PM	325 925
	WBR	700	AM PM	0 625
	NBL	150	AM PM	**225** 125
	NBT	500	AM PM	125 350
	SBL	550	AM PM	475 250
	SBT	>1000	AM PM	475 275
	SBR	400	AM PM	**1,025** **600**

Notes:
1. Queues were calculated using HCM 7th methodologies and are rounded up to the nearest 25-foot increment.
2. "EB" = eastbound, "WB" = westbound, "NB" = northbound, "SB" = southbound, "L" = left, "T" = through, "R" = right.
3. **Bold** symbolizes movement is at or exceeds available storage capacity.

Source: Fehr & Peers, 2023.

Attachment C: Intersection Level of Service Analysis Worksheets

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	300	959	302	60	983	130	212	453	130	168	342	378
Future Volume (veh/h)	300	959	302	60	983	130	212	453	130	168	342	378
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		0.97	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1634	1634	1634	1634	1634	1634	1687	1687	1687	1687	1687	1687
Adj Flow Rate, veh/h	319	1020	198	64	1046	100	226	482	118	179	364	352
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	5	5	5	5	5	5	1	1	1	1	1	1
Cap, veh/h	471	1406	867	114	1113	699	277	710	172	233	426	592
Arrive On Green	0.16	0.45	0.45	0.07	0.36	0.36	0.17	0.28	0.25	0.15	0.25	0.25
Sat Flow, veh/h	3018	3104	1382	1556	3104	1384	1606	2538	617	1606	1687	1424
Grp Volume(v), veh/h	319	1020	198	64	1046	100	226	303	297	179	364	352
Grp Sat Flow(s),veh/h/ln	1509	1552	1382	1556	1552	1384	1606	1602	1553	1606	1687	1424
Q Serve(g_s), s	13.0	34.8	8.1	5.2	42.4	2.7	17.6	21.8	22.3	13.9	26.7	6.5
Cycle Q Clear(g_c), s	13.0	34.8	8.1	5.2	42.4	2.7	17.6	21.8	22.3	13.9	26.7	6.5
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.40	1.00		1.00
Lane Grp Cap(c), veh/h	471	1406	867	114	1113	699	277	448	434	233	426	592
V/C Ratio(X)	0.68	0.73	0.23	0.56	0.94	0.14	0.82	0.68	0.68	0.77	0.86	0.59
Avail Cap(c_a), veh/h	485	1406	867	227	1113	699	282	448	434	271	432	598
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	51.8	29.0	10.6	58.3	40.3	6.7	51.8	41.6	42.4	53.4	46.3	12.4
Incr Delay (d2), s/veh	2.9	3.3	0.6	1.6	15.9	0.4	15.4	3.3	3.7	8.8	14.5	1.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	5.0	13.1	2.5	2.1	18.1	1.0	8.2	8.9	8.9	6.2	12.8	4.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	54.7	32.3	11.2	59.9	56.3	7.1	67.2	44.9	46.1	62.3	60.9	13.5
LnGrp LOS	D	C	B	E	E	A	E	D	D	E	E	B
Approach Vol, veh/h		1537			1210			826			895	
Approach Delay, s/veh		34.2			52.4			51.4			42.5	
Approach LOS		C			D			D			D	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	22.9	49.0	23.6	34.5	10.5	61.4	20.0	38.1
Change Period (Y+Rc), s	5.5	* 5.5	4.0	5.5	4.0	5.5	4.0	5.5
Max Green Setting (Gmax), s	18.0	* 44	20.0	29.5	16.0	45.5	19.0	30.5
Max Q Clear Time (g_c+I1), s	15.0	44.4	19.6	28.7	7.2	36.8	15.9	24.3
Green Ext Time (p_c), s	0.2	0.0	0.0	0.2	0.0	2.1	0.1	0.7

Intersection Summary	
HCM 6th Ctrl Delay	44.0
HCM 6th LOS	D

Notes

User approved pedestrian interval to be less than phase max green.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection	
Int Delay, s/veh	0.1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	8	1249	1134	0	1	9
Future Vol, veh/h	8	1249	1134	0	1	9
Conflicting Peds, #/hr	5	0	0	5	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	70	-	-	100	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	88	88	88	88	88	88
Heavy Vehicles, %	0	5	5	0	0	0
Mvmt Flow	9	1419	1289	0	1	10

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	1294	0	-	0	2022	650
Stage 1	-	-	-	-	1294	-
Stage 2	-	-	-	-	728	-
Critical Hdwy	4.1	-	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	2.2	-	-	-	3.5	3.3
Pot Cap-1 Maneuver	542	-	-	-	52	417
Stage 1	-	-	-	-	225	-
Stage 2	-	-	-	-	444	-
Platoon blocked, %			-	-	-	
Mov Cap-1 Maneuver	539	-	-	-	51	415
Mov Cap-2 Maneuver	-	-	-	-	51	-
Stage 1	-	-	-	-	220	-
Stage 2	-	-	-	-	442	-

Approach	EB	WB	SB
HCM Control Delay, s	0.1	0	20.6
HCM LOS			C

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	539	-	-	-	242
HCM Lane V/C Ratio	0.017	-	-	-	0.047
HCM Control Delay (s)	11.8	-	-	-	20.6
HCM Lane LOS	B	-	-	-	C
HCM 95th %tile Q(veh)	0.1	-	-	-	0.1

Intersection	
Int Delay, s/veh	0.2

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	1245	5	2	1130	4	7
Future Vol, veh/h	1245	5	2	1130	4	7
Conflicting Peds, #/hr	0	5	5	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	100	100	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	88	88	88	88	88	88
Heavy Vehicles, %	5	0	0	5	0	0
Mvmt Flow	1415	6	2	1284	5	8

Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	1426	0	2066	713
Stage 1	-	-	-	-	1420	-
Stage 2	-	-	-	-	646	-
Critical Hdwy	-	-	4.1	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	483	-	48	379
Stage 1	-	-	-	-	193	-
Stage 2	-	-	-	-	489	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	481	-	48	377
Mov Cap-2 Maneuver	-	-	-	-	48	-
Stage 1	-	-	-	-	192	-
Stage 2	-	-	-	-	487	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0	42.7
HCM LOS			E

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	108	-	-	481	-
HCM Lane V/C Ratio	0.116	-	-	0.005	-
HCM Control Delay (s)	42.7	-	-	12.5	-
HCM Lane LOS	E	-	-	B	-
HCM 95th %tile Q(veh)	0.4	-	-	0	-

Intersection	
Int Delay, s/veh	0.3

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	14	1238	1119	3	4	13
Future Vol, veh/h	14	1238	1119	3	4	13
Conflicting Peds, #/hr	5	0	0	5	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	100	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	87	87	87	87	87	87
Heavy Vehicles, %	0	5	5	0	0	0
Mvmt Flow	16	1423	1286	3	5	15

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	1294	0	-	0	2035	648
Stage 1	-	-	-	-	1291	-
Stage 2	-	-	-	-	744	-
Critical Hdwy	4.1	-	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	2.2	-	-	-	3.5	3.3
Pot Cap-1 Maneuver	542	-	-	-	51	418
Stage 1	-	-	-	-	226	-
Stage 2	-	-	-	-	436	-
Platoon blocked, %			-	-	-	
Mov Cap-1 Maneuver	539	-	-	-	49	416
Mov Cap-2 Maneuver	-	-	-	-	49	-
Stage 1	-	-	-	-	218	-
Stage 2	-	-	-	-	434	-

Approach	EB	WB	SB
HCM Control Delay, s	0.1	0	32.4
HCM LOS			D

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	539	-	-	-	151
HCM Lane V/C Ratio	0.03	-	-	-	0.129
HCM Control Delay (s)	11.9	-	-	-	32.4
HCM Lane LOS	B	-	-	-	D
HCM 95th %tile Q(veh)	0.1	-	-	-	0.4

Intersection	
Int Delay, s/veh	0.1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	4	1238	1115	0	3	7
Future Vol, veh/h	4	1238	1115	0	3	7
Conflicting Peds, #/hr	5	0	0	5	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	100	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	86	86	86	86	86	86
Heavy Vehicles, %	0	5	5	0	0	0
Mvmt Flow	5	1440	1297	0	3	8

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	1302	0	-	0	2032	654
Stage 1	-	-	-	-	1302	-
Stage 2	-	-	-	-	730	-
Critical Hdwy	4.1	-	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	2.2	-	-	-	3.5	3.3
Pot Cap-1 Maneuver	539	-	-	-	51	414
Stage 1	-	-	-	-	223	-
Stage 2	-	-	-	-	443	-
Platoon blocked, %			-	-	-	
Mov Cap-1 Maneuver	536	-	-	-	50	412
Mov Cap-2 Maneuver	-	-	-	-	50	-
Stage 1	-	-	-	-	220	-
Stage 2	-	-	-	-	441	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	35.4
HCM LOS			E

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	536	-	-	-	130
HCM Lane V/C Ratio	0.009	-	-	-	0.089
HCM Control Delay (s)	11.8	-	-	-	35.4
HCM Lane LOS	B	-	-	-	E
HCM 95th %tile Q(veh)	0	-	-	-	0.3

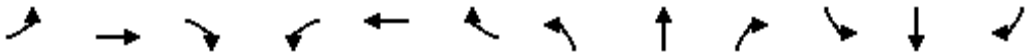
Intersection	
Int Delay, s/veh	0

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	1240	1	1	1115	1	2
Future Vol, veh/h	1240	1	1	1115	1	2
Conflicting Peds, #/hr	0	5	5	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	100	125	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	86	86	86	86	86	86
Heavy Vehicles, %	5	0	0	5	0	0
Mvmt Flow	1442	1	1	1297	0	2

Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	1448	0	2098	726
Stage 1	-	-	-	-	1447	-
Stage 2	-	-	-	-	651	-
Critical Hdwy	-	-	4.1	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	474	-	46	372
Stage 1	-	-	-	-	186	-
Stage 2	-	-	-	-	486	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	472	-	46	370
Mov Cap-2 Maneuver	-	-	-	-	46	-
Stage 1	-	-	-	-	185	-
Stage 2	-	-	-	-	485	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0	36.1
HCM LOS			E

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	119	-	-	472	-
HCM Lane V/C Ratio	0.028	-	-	0.002	-
HCM Control Delay (s)	36.1	-	-	12.6	-
HCM Lane LOS	E	-	-	B	-
HCM 95th %tile Q(veh)	0.1	-	-	0	-



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	61	1067	114	42	921	49	59	102	55	44	119	136
Future Volume (veh/h)	61	1067	114	42	921	49	59	102	55	44	119	136
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.98	1.00		1.00	1.00		0.99	0.99		0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1700	1634	1700	1700	1634	1700	1700	1700	1700	1700	1700	1700
Adj Flow Rate, veh/h	75	1317	119	52	1137	40	73	126	48	54	147	126
Peak Hour Factor	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81	0.81
Percent Heavy Veh, %	0	5	0	0	5	0	0	0	0	0	0	0
Cap, veh/h	138	1672	758	108	1615	747	293	363	138	387	260	223
Arrive On Green	0.09	0.54	0.54	0.07	0.52	0.52	0.31	0.31	0.27	0.31	0.31	0.31
Sat Flow, veh/h	1619	3104	1408	1619	3104	1435	1119	1169	445	1223	836	717
Grp Volume(v), veh/h	75	1317	119	52	1137	40	73	0	174	54	0	273
Grp Sat Flow(s),veh/h/ln	1619	1552	1408	1619	1552	1435	1119	0	1615	1223	0	1553
Q Serve(g_s), s	2.9	22.3	2.8	2.0	18.2	0.9	3.8	0.0	5.5	2.3	0.0	9.6
Cycle Q Clear(g_c), s	2.9	22.3	2.8	2.0	18.2	0.9	13.5	0.0	5.5	7.9	0.0	9.6
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.28	1.00		0.46
Lane Grp Cap(c), veh/h	138	1672	758	108	1615	747	293	0	501	387	0	482
V/C Ratio(X)	0.54	0.79	0.16	0.48	0.70	0.05	0.25	0.00	0.35	0.14	0.00	0.57
Avail Cap(c_a), veh/h	541	2046	928	541	2046	946	635	0	995	761	0	957
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	28.7	12.1	7.6	29.5	11.9	7.8	24.5	0.0	17.7	20.5	0.0	18.9
Incr Delay (d2), s/veh	1.2	1.7	0.1	1.2	0.8	0.0	0.6	0.0	0.6	0.2	0.0	1.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.1	6.2	0.7	0.8	5.0	0.2	1.0	0.0	2.0	0.7	0.0	3.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	30.0	13.8	7.7	30.7	12.7	7.8	25.2	0.0	18.3	20.7	0.0	20.4
LnGrp LOS	C	B	A	C	B	A	C	A	B	C	A	C
Approach Vol, veh/h		1511			1229			247			327	
Approach Delay, s/veh		14.2			13.3			20.3			20.4	
Approach LOS		B			B			C			C	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	5.5	37.6		22.5	6.7	36.4		22.5				
Change Period (Y+Rc), s	3.0	5.5		4.5	3.0	5.5		4.5				
Max Green Setting (Gmax), s	20.0	40.0		38.0	20.0	40.0		38.0				
Max Q Clear Time (g_c+I1), s	4.0	24.3		15.5	4.9	20.2		11.6				
Green Ext Time (p_c), s	0.0	7.8		1.8	0.0	7.7		2.8				

Intersection Summary			
HCM 6th Ctrl Delay		14.9	
HCM 6th LOS		B	

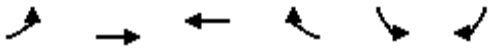
Intersection	
Int Delay, s/veh	0.1

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	1160	6	5	1010	2	2
Future Vol, veh/h	1160	6	5	1010	2	2
Conflicting Peds, #/hr	0	5	5	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	100	140	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	5	0	0	5	0	0
Mvmt Flow	1289	7	6	1122	2	2

Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	1301	0	1867	650
Stage 1	-	-	-	-	1294	-
Stage 2	-	-	-	-	573	-
Critical Hdwy	-	-	4.1	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	539	-	66	417
Stage 1	-	-	-	-	225	-
Stage 2	-	-	-	-	533	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	536	-	65	415
Mov Cap-2 Maneuver	-	-	-	-	65	-
Stage 1	-	-	-	-	224	-
Stage 2	-	-	-	-	527	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0.1	38.5
HCM LOS			E

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	112	-	-	536	-
HCM Lane V/C Ratio	0.04	-	-	0.01	-
HCM Control Delay (s)	38.5	-	-	11.8	-
HCM Lane LOS	E	-	-	B	-
HCM 95th %tile Q(veh)	0.1	-	-	0	-



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	69	1093	977	105	113	38
Future Volume (veh/h)	69	1093	977	105	113	38
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			0.97	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No	No		No	
Adj Sat Flow, veh/h/ln	1700	1634	1634	1700	1700	1700
Adj Flow Rate, veh/h	78	1242	1110	94	86	89
Peak Hour Factor	0.88	0.88	0.88	0.88	0.88	0.88
Percent Heavy Veh, %	0	5	5	0	0	0
Cap, veh/h	171	2206	1786	808	284	253
Arrive On Green	0.11	0.71	0.58	0.58	0.18	0.18
Sat Flow, veh/h	1619	3186	3186	1404	1619	1441
Grp Volume(v), veh/h	78	1242	1110	94	86	89
Grp Sat Flow(s),veh/h/ln	1619	1552	1552	1404	1619	1441
Q Serve(g_s), s	1.7	7.3	8.9	1.1	1.7	2.0
Cycle Q Clear(g_c), s	1.7	7.3	8.9	1.1	1.7	2.0
Prop In Lane	1.00			1.00	1.00	1.00
Lane Grp Cap(c), veh/h	171	2206	1786	808	284	253
V/C Ratio(X)	0.46	0.56	0.62	0.12	0.30	0.35
Avail Cap(c_a), veh/h	940	3556	3556	1608	1610	1433
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	15.8	2.6	5.3	3.6	13.5	13.7
Incr Delay (d2), s/veh	0.7	0.2	0.4	0.1	0.8	1.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.5	0.1	0.7	0.1	0.6	0.6
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	16.5	2.9	5.6	3.7	14.4	14.9
LnGrp LOS	B	A	A	A	B	B
Approach Vol, veh/h		1320	1204		175	
Approach Delay, s/veh		3.7	5.5		14.6	
Approach LOS		A	A		B	

Timer - Assigned Phs	1	2		4		6
Phs Duration (G+Y+Rc), s	5.1	24.0		8.6		29.1
Change Period (Y+Rc), s	3.0	5.5		4.5		5.5
Max Green Setting (Gmax), s	20.0	40.0		35.0		40.0
Max Q Clear Time (g_c+I1), s	3.7	10.9		4.0		9.3
Green Ext Time (p_c), s	0.0	7.6		0.9		10.6

Intersection Summary	
HCM 6th Ctrl Delay	5.2
HCM 6th LOS	A

Notes

User approved volume balancing among the lanes for turning movement.



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	995	211	450	880	202	285
Future Volume (vph)	995	211	450	880	202	285
Ideal Flow (vphpl)	1700	1700	1700	1700	1700	1700
Total Lost time (s)	2.5	2.5	1.0	2.5	1.5	1.5
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Frpb, ped/bikes	1.00	0.98	1.00	1.00	1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.85	1.00	1.00	1.00	0.85
Flt Protected	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	3076	1355	1538	3076	1568	1403
Flt Permitted	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (perm)	3076	1355	1538	3076	1568	1403
Peak-hour factor, PHF	0.89	0.89	0.89	0.89	0.89	0.89
Adj. Flow (vph)	1118	237	506	989	227	320
RTOR Reduction (vph)	0	43	0	0	0	270
Lane Group Flow (vph)	1118	194	506	989	227	50
Confl. Peds. (#/hr)		2			1	
Confl. Bikes (#/hr)		1				
Heavy Vehicles (%)	5%	5%	5%	5%	3%	3%
Turn Type	NA	Perm	Prot	NA	Prot	Prot
Protected Phases	2		1	6	4	4
Permitted Phases		2				
Actuated Green, G (s)	49.7	49.7	34.0	89.7	15.5	15.5
Effective Green, g (s)	54.7	54.7	39.0	94.7	20.5	20.5
Actuated g/C Ratio	0.42	0.42	0.30	0.73	0.16	0.16
Clearance Time (s)	7.5	7.5	6.0	7.5	6.5	6.5
Vehicle Extension (s)	3.0	3.0	3.0	3.0	2.0	2.0
Lane Grp Cap (vph)	1294	570	461	2240	247	221
v/s Ratio Prot	c0.36		c0.33	0.32	c0.14	0.04
v/s Ratio Perm		0.14				
v/c Ratio	0.86	0.34	1.10	0.44	0.92	0.23
Uniform Delay, d1	34.3	25.5	45.5	7.1	53.9	47.8
Progression Factor	1.00	1.00	1.14	0.71	1.00	1.00
Incremental Delay, d2	7.8	1.6	67.1	0.5	35.2	0.2
Delay (s)	42.1	27.1	119.0	5.5	89.2	48.0
Level of Service	D	C	F	A	F	D
Approach Delay (s)	39.5			43.9	65.1	
Approach LOS	D			D	E	

Intersection Summary			
HCM 2000 Control Delay	45.6	HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio	0.89		
Actuated Cycle Length (s)	130.0	Sum of lost time (s)	10.0
Intersection Capacity Utilization	81.1%	ICU Level of Service	D
Analysis Period (min)	15		
c Critical Lane Group			

Intersection	
Int Delay, s/veh	21.4

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	68	1173	39	5	1218	99	9	1	21	27	0	103
Future Vol, veh/h	68	1173	39	5	1218	99	9	1	21	27	0	103
Conflicting Peds, #/hr	5	0	5	5	0	5	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	115	-	100	140	-	100	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	5	5	0	0	5	5	0	0	0	3	3	3
Mvmt Flow	74	1275	42	5	1324	108	10	1	23	29	0	112

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	1437	0	0	1322	0	0	2100	2875	643	2125	2809	667
Stage 1	-	-	-	-	-	-	1428	1428	-	1339	1339	-
Stage 2	-	-	-	-	-	-	672	1447	-	786	1470	-
Critical Hdwy	4.2	-	-	4.1	-	-	7.5	6.5	6.9	7.56	6.56	6.96
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.56	5.56	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.56	5.56	-
Follow-up Hdwy	2.25	-	-	2.2	-	-	3.5	4	3.3	3.53	4.03	3.33
Pot Cap-1 Maneuver	453	-	-	529	-	-	30	17	421	~ 28	18	399
Stage 1	-	-	-	-	-	-	144	203	-	160	218	-
Stage 2	-	-	-	-	-	-	416	198	-	349	188	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	451	-	-	526	-	-	19	14	419	~ 22	15	397
Mov Cap-2 Maneuver	-	-	-	-	-	-	19	14	-	~ 22	15	-
Stage 1	-	-	-	-	-	-	120	169	-	133	215	-
Stage 2	-	-	-	-	-	-	296	195	-	274	156	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0.8	0	157.7	$ 410.4
HCM LOS			F	F

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	52	451	-	-	526	-	-	87
HCM Lane V/C Ratio	0.648	0.164	-	-	0.01	-	-	1.624
HCM Control Delay (s)	157.7	14.5	-	-	11.9	-	-	$ 410.4
HCM Lane LOS	F	B	-	-	B	-	-	F
HCM 95th %tile Q(veh)	2.6	0.6	-	-	0	-	-	11.4

Notes			
~: Volume exceeds capacity	$: Delay exceeds 300s	+: Computation Not Defined	*: All major volume in platoon

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	364	766	91	16	469	366	139	109	36	1294	319	714
Future Volume (veh/h)	364	766	91	16	469	366	139	109	36	1294	319	714
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		1.00	1.00		0.99	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1634	1634	1634	1660	1660	1660	1673	1673	1673	1673	1673	1673
Adj Flow Rate, veh/h	375	790	88	16	484	0	143	112	11	1334	329	636
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	5	5	5	3	3	3	2	2	2	2	2	2
Cap, veh/h	507	863	96	86	621		217	398	39	2151	753	877
Arrive On Green	0.06	0.10	0.10	0.05	0.20	0.00	0.14	0.14	0.10	0.45	0.45	0.45
Sat Flow, veh/h	3018	2811	313	1581	3154	1407	1594	2924	283	4781	1673	1413
Grp Volume(v), veh/h	375	436	442	16	484	0	143	60	63	1334	329	636
Grp Sat Flow(s),veh/h/ln	1509	1552	1572	1581	1577	1407	1594	1590	1618	1594	1673	1413
Q Serve(g_s), s	15.9	36.2	36.2	1.3	18.9	0.0	11.1	4.4	4.6	27.7	17.5	40.4
Cycle Q Clear(g_c), s	15.9	36.2	36.2	1.3	18.9	0.0	11.1	4.4	4.6	27.7	17.5	40.4
Prop In Lane	1.00		0.20	1.00		1.00	1.00		0.18	1.00		1.00
Lane Grp Cap(c), veh/h	507	476	482	86	621		217	216	220	2151	753	877
V/C Ratio(X)	0.74	0.92	0.92	0.19	0.78		0.66	0.28	0.29	0.62	0.44	0.73
Avail Cap(c_a), veh/h	662	476	482	208	692		292	291	296	2151	753	877
HCM Platoon Ratio	0.33	0.33	0.33	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	58.6	56.7	56.8	58.7	49.5	0.0	53.3	50.4	50.8	27.3	24.5	17.1
Incr Delay (d2), s/veh	2.0	21.9	21.8	0.4	4.4	0.0	1.3	0.3	0.3	1.4	1.8	5.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	6.6	17.9	18.1	0.5	7.6	0.0	4.5	1.8	1.9	10.3	7.1	13.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	60.6	78.7	78.5	59.1	53.9	0.0	54.6	50.7	51.1	28.6	26.3	22.3
LnGrp LOS	E	E	E	E	D		D	D	D	C	C	C
Approach Vol, veh/h		1253			500	A		266			2299	
Approach Delay, s/veh		73.2			54.1			52.9			26.6	
Approach LOS		E			D			D			C	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	23.3	27.1		60.7	8.4	42.0		18.9				
Change Period (Y+Rc), s	5.0	5.5		6.0	5.5	* 6.5		5.5				
Max Green Setting (Gmax), s	25.0	24.5		39.0	13.0	* 36		19.5				
Max Q Clear Time (g_c+I1), s	17.9	20.9		42.4	3.3	38.2		13.1				
Green Ext Time (p_c), s	0.4	0.7		0.0	0.0	0.0		0.3				

Intersection Summary			
HCM 6th Ctrl Delay		44.9	
HCM 6th LOS		D	

Notes

User approved volume balancing among the lanes for turning movement.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

HCM 6th Signalized Intersection Summary
12: Cannon St & Santiago Canyon Road Alt2

02/28/2024

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	1000	540	80	20	1040	1530	80	460	20	500	250	530
Future Volume (veh/h)	1000	540	80	20	1040	1530	80	460	20	500	250	530
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		1.00	1.00		0.99	1.00		0.99
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1673	1673	1673	1673	1673	1673	1687	1687	1687	1687	1687	1687
Adj Flow Rate, veh/h	1042	562	75	21	1083	0	83	479	19	521	260	492
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	2	2	2	2	2	2	1	1	1	1	1	1
Cap, veh/h	804	2867	381	93	2573		139	625	25	623	541	1500
Arrive On Green	0.26	1.00	1.00	0.06	0.81	0.00	0.09	0.20	0.16	0.20	0.32	0.32
Sat Flow, veh/h	3092	2815	375	1594	3180	1418	1606	3140	124	3116	1687	2482
Grp Volume(v), veh/h	1042	317	320	21	1083	0	83	244	254	521	260	492
Grp Sat Flow(s),veh/h/ln	1546	1590	1600	1594	1590	1418	1606	1602	1662	1558	1687	1241
Q Serve(g_s), s	31.2	0.0	0.0	1.5	11.8	0.0	6.0	17.3	17.4	19.3	14.9	11.8
Cycle Q Clear(g_c), s	31.2	0.0	0.0	1.5	11.8	0.0	6.0	17.3	17.4	19.3	14.9	11.8
Prop In Lane	1.00		0.23	1.00		1.00	1.00		0.07	1.00		1.00
Lane Grp Cap(c), veh/h	804	1619	1629	93	2573		139	319	331	623	541	1500
V/C Ratio(X)	1.30	0.20	0.20	0.23	0.42		0.60	0.76	0.77	0.84	0.48	0.33
Avail Cap(c_a), veh/h	804	1619	1629	139	2573		139	320	332	623	541	1500
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.81	0.81	0.81	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	44.4	0.0	0.0	53.9	3.3	0.0	52.8	45.4	45.6	46.1	32.7	11.9
Incr Delay (d2), s/veh	140.8	0.2	0.2	0.5	0.5	0.0	4.8	9.5	9.3	9.1	0.2	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	27.1	0.1	0.1	0.6	2.3	0.0	2.6	7.6	7.9	8.0	5.9	3.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	185.2	0.2	0.2	54.3	3.8	0.0	57.6	54.9	54.9	55.2	33.0	12.0
LnGrp LOS	F	A	A	D	A		E	D	D	E	C	B
Approach Vol, veh/h		1679			1104			581			1273	
Approach Delay, s/veh		115.0			4.8			55.3			34.0	
Approach LOS		F			A			E			C	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	34.0	101.1	11.0	39.9	7.5	127.6	26.0	24.9
Change Period (Y+Rc), s	5.0	* 6.5	5.0	6.0	5.0	6.5	6.0	* 5.5
Max Green Setting (Gmax), s	29.0	* 31	6.0	33.0	6.0	52.5	20.0	* 20
Max Q Clear Time (g_c+I1), s	33.2	13.8	8.0	16.9	3.5	2.0	21.3	19.4
Green Ext Time (p_c), s	0.0	4.3	0.0	1.9	0.0	2.1	0.0	0.0

Intersection Summary	
HCM 6th Ctrl Delay	59.0
HCM 6th LOS	E

Notes

User approved pedestrian interval to be less than phase max green.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

HCM 6th Signalized Intersection Summary
12: Cannon St & Santiago Canyon Road Alt3

02/28/2024

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	1000	540	80	20	1040	1530	80	460	20	500	250	530
Future Volume (veh/h)	1000	540	80	20	1040	1530	80	460	20	500	250	530
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		1.00	1.00		0.99	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1673	1673	1673	1673	1673	1673	1687	1687	1687	1687	1687	1687
Adj Flow Rate, veh/h	1042	562	75	21	1083	0	83	479	19	521	260	0
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	2	2	2	2	2	2	1	1	1	1	1	1
Cap, veh/h	804	2867	381	93	2573		139	625	25	623	1028	
Arrive On Green	0.26	1.00	1.00	0.06	0.81	0.00	0.09	0.20	0.16	0.20	0.32	0.00
Sat Flow, veh/h	3092	2815	375	1594	3180	1418	1606	3140	124	3116	3205	1429
Grp Volume(v), veh/h	1042	317	320	21	1083	0	83	244	254	521	260	0
Grp Sat Flow(s),veh/h/ln	1546	1590	1600	1594	1590	1418	1606	1602	1662	1558	1602	1429
Q Serve(g_s), s	31.2	0.0	0.0	1.5	11.8	0.0	6.0	17.3	17.4	19.3	7.2	0.0
Cycle Q Clear(g_c), s	31.2	0.0	0.0	1.5	11.8	0.0	6.0	17.3	17.4	19.3	7.2	0.0
Prop In Lane	1.00		0.23	1.00		1.00	1.00		0.07	1.00		1.00
Lane Grp Cap(c), veh/h	804	1619	1629	93	2573		139	319	331	623	1028	
V/C Ratio(X)	1.30	0.20	0.20	0.23	0.42		0.60	0.76	0.77	0.84	0.25	
Avail Cap(c_a), veh/h	804	1619	1629	139	2573		139	320	332	623	1028	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	0.81	0.81	0.81	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	44.4	0.0	0.0	53.9	3.3	0.0	52.8	45.4	45.6	46.1	30.1	0.0
Incr Delay (d2), s/veh	140.8	0.2	0.2	0.5	0.5	0.0	4.8	9.5	9.3	9.1	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	27.1	0.1	0.1	0.6	2.3	0.0	2.6	7.6	7.9	8.0	2.7	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	185.2	0.2	0.2	54.3	3.8	0.0	57.6	54.9	54.9	55.2	30.2	0.0
LnGrp LOS	F	A	A	D	A		E	D	D	E	C	
Approach Vol, veh/h		1679			1104			581			781	
Approach Delay, s/veh		115.0			4.8			55.3			46.9	
Approach LOS		F			A			E			D	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	34.0	101.1	11.0	39.9	7.5	127.6	26.0	24.9
Change Period (Y+Rc), s	5.0	* 6.5	5.0	6.0	5.0	6.5	6.0	* 5.5
Max Green Setting (Gmax), s	29.0	* 31	6.0	33.0	6.0	52.5	20.0	* 20
Max Q Clear Time (g_c+I1), s	33.2	13.8	8.0	9.2	3.5	2.0	21.3	19.4
Green Ext Time (p_c), s	0.0	4.3	0.0	0.9	0.0	2.1	0.0	0.0

Intersection Summary	
HCM 6th Ctrl Delay	64.4
HCM 6th LOS	E

Notes

User approved pedestrian interval to be less than phase max green.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [WBR, SBR] is excluded from calculations of the approach delay and intersection delay.



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	145	2	109	1	2	1	67	771	1	1	2217	230
Future Volume (veh/h)	145	2	109	1	2	1	67	771	1	1	2217	230
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.97	0.99		1.00	1.00		1.00	1.00		0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1700	1700	1700	1700	1700	1700	1700	1673	1700	1700	1673	1700
Adj Flow Rate, veh/h	148	2	97	1	2	1	68	787	1	1	2262	232
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	0	0	0	0	0	0	0	2	0	0	2	0
Cap, veh/h	113	1	302	26	47	16	121	2565	3	44	2152	216
Arrive On Green	0.11	0.14	0.14	0.11	0.14	0.14	0.07	0.79	0.79	0.03	0.74	0.74
Sat Flow, veh/h	530	7	1404	0	345	115	1619	3258	4	1619	2910	292
Grp Volume(v), veh/h	150	0	97	4	0	0	68	384	404	1	1215	1279
Grp Sat Flow(s),veh/h/ln	538	0	1404	461	0	0	1619	1590	1673	1619	1590	1612
Q Serve(g_s), s	0.0	0.0	10.2	0.0	0.0	0.0	7.1	11.9	11.9	0.1	129.4	129.4
Cycle Q Clear(g_c), s	20.0	0.0	10.2	20.0	0.0	0.0	7.1	11.9	11.9	0.1	129.4	129.4
Prop In Lane	0.99		1.00	0.25		0.25	1.00		0.00	1.00		0.18
Lane Grp Cap(c), veh/h	102	0	302	78	0	0	121	1252	1317	44	1176	1192
V/C Ratio(X)	1.47	0.00	0.32	0.05	0.00	0.00	0.56	0.31	0.31	0.02	1.03	1.07
Avail Cap(c_a), veh/h	102	0	302	78	0	0	132	1252	1317	88	1176	1192
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	79.8	0.0	58.1	66.6	0.0	0.0	78.2	5.2	5.2	82.9	22.8	22.8
Incr Delay (d2), s/veh	255.2	0.0	0.6	0.3	0.0	0.0	2.0	0.6	0.6	0.1	35.2	47.9
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	12.0	0.0	3.7	0.2	0.0	0.0	3.0	3.7	3.8	0.0	52.6	57.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	335.0	0.0	58.7	66.8	0.0	0.0	80.2	5.9	5.8	83.0	58.0	70.7
LnGrp LOS	F	A	E	E	A	A	F	A	A	F	F	F
Approach Vol, veh/h		247			4			856			2495	
Approach Delay, s/veh		226.5			66.8			11.7			64.5	
Approach LOS		F			E			B			E	

Timer - Assigned Phs	1	2		4	5	6		8
Phs Duration (G+Y+Rc), s	5.2	144.8		25.0	13.8	136.2		25.0
Change Period (Y+Rc), s	5.0	6.0		5.0	5.0	6.0		5.0
Max Green Setting (Gmax), s	5.0	134.0		20.0	10.0	129.0		20.0
Max Q Clear Time (g_c+I1), s	2.1	13.9		22.0	9.1	131.4		22.0
Green Ext Time (p_c), s	0.0	1.3		0.0	0.0	0.0		0.0

Intersection Summary		
HCM 6th Ctrl Delay		63.1
HCM 6th LOS		E

Notes

User approved pedestrian interval to be less than phase max green.



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	1035	145	561	356	51	1413
Future Volume (veh/h)	1035	145	561	356	51	1413
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1673	1673	1673	1673	1673	1673
Adj Flow Rate, veh/h	1056	106	572	308	52	1442
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	1181	690	1527	1247	142	1806
Arrive On Green	0.38	0.40	0.48	0.48	0.09	0.57
Sat Flow, veh/h	3092	1418	3263	1418	1594	3263
Grp Volume(v), veh/h	1056	106	572	308	52	1442
Grp Sat Flow(s),veh/h/ln	1546	1418	1590	1418	1594	1590
Q Serve(g_s), s	32.1	4.1	11.4	3.4	3.1	35.8
Cycle Q Clear(g_c), s	32.1	4.1	11.4	3.4	3.1	35.8
Prop In Lane	1.00	1.00		1.00	1.00	
Lane Grp Cap(c), veh/h	1181	690	1527	1247	142	1806
V/C Ratio(X)	0.89	0.15	0.37	0.25	0.37	0.80
Avail Cap(c_a), veh/h	1527	849	1527	1247	228	1806
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	29.0	14.2	16.5	0.9	42.9	17.1
Incr Delay (d2), s/veh	5.0	0.0	0.7	0.5	0.6	3.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	11.9	4.5	3.9	4.4	1.2	12.1
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	34.0	14.3	17.2	1.4	43.5	20.9
LnGrp LOS	C	B	B	A	D	C
Approach Vol, veh/h	1162		880			1494
Approach Delay, s/veh	32.2		11.7			21.6
Approach LOS	C		B			C

Timer - Assigned Phs	1	2		4		6
Phs Duration (G+Y+Rc), s	9.6	49.6		40.8		59.2
Change Period (Y+Rc), s	5.0	6.0		5.3		6.0
Max Green Setting (Gmax), s	10.6	27.0		46.7		42.0
Max Q Clear Time (g_c+I1), s	5.1	13.4		34.1		37.8
Green Ext Time (p_c), s	0.0	2.5		1.4		2.5

Intersection Summary	
HCM 6th Ctrl Delay	22.6
HCM 6th LOS	C

Intersection	
Int Delay, s/veh	3.1

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	34	45	28	678	1419	43
Future Vol, veh/h	34	45	28	678	1419	43
Conflicting Peds, #/hr	0	0	5	0	0	5
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	200	-	-	100
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	98	98	98	98	98	98
Heavy Vehicles, %	0	0	0	2	2	0
Mvmt Flow	35	46	29	692	1448	44

Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	1857	729	1497	0	-	0
Stage 1	1453	-	-	-	-	-
Stage 2	404	-	-	-	-	-
Critical Hdwy	6.8	6.9	4.1	-	-	-
Critical Hdwy Stg 1	5.8	-	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	67	370	454	-	-	-
Stage 1	185	-	-	-	-	-
Stage 2	649	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	62	368	452	-	-	-
Mov Cap-2 Maneuver	62	-	-	-	-	-
Stage 1	172	-	-	-	-	-
Stage 2	646	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	84.5	0.5	0
HCM LOS	F		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	452	-	118	-	-
HCM Lane V/C Ratio	0.063	-	0.683	-	-
HCM Control Delay (s)	13.5	-	84.5	-	-
HCM Lane LOS	B	-	F	-	-
HCM 95th %tile Q(veh)	0.2	-	3.6	-	-

Intersection	
Int Delay, s/veh	0

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	0	0	839	0	0	2327
Future Vol, veh/h	0	0	839	0	0	2327
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	150	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	1	0	0	1
Mvmt Flow	0	0	912	0	0	2529

Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	2177	456	0	0	912	0
Stage 1	912	-	-	-	-	-
Stage 2	1265	-	-	-	-	-
Critical Hdwy	6.8	6.9	-	-	4.1	-
Critical Hdwy Stg 1	5.8	-	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	41	557	-	-	755	-
Stage 1	357	-	-	-	-	-
Stage 2	233	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	41	557	-	-	755	-
Mov Cap-2 Maneuver	41	-	-	-	-	-
Stage 1	357	-	-	-	-	-
Stage 2	233	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	0	0	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	-	755	-
HCM Lane V/C Ratio	-	-	-	-	-
HCM Control Delay (s)	-	-	0	0	-
HCM Lane LOS	-	-	A	A	-
HCM 95th %tile Q(veh)	-	-	-	0	-

Intersection	
Int Delay, s/veh	0

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	0	0	917	2448	0
Future Vol, veh/h	0	0	0	917	2448	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	150	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	1	1	0
Mvmt Flow	0	0	0	997	2661	0

Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	3160	1331	2661	0	-	0
Stage 1	2661	-	-	-	-	-
Stage 2	499	-	-	-	-	-
Critical Hdwy	6.8	6.9	4.1	-	-	-
Critical Hdwy Stg 1	5.8	-	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	8	147	160	-	-	-
Stage 1	40	-	-	-	-	-
Stage 2	581	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	8	147	160	-	-	-
Mov Cap-2 Maneuver	8	-	-	-	-	-
Stage 1	40	-	-	-	-	-
Stage 2	581	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	0	0	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	160	-	-	-	-
HCM Lane V/C Ratio	-	-	-	-	-
HCM Control Delay (s)	0	-	0	-	-
HCM Lane LOS	A	-	A	-	-
HCM 95th %tile Q(veh)	0	-	-	-	-

Intersection	
Int Delay, s/veh	0

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	0	0	706	0	0	1464
Future Vol, veh/h	0	0	706	0	0	1464
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	125	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	2	0	0	2
Mvmt Flow	0	0	767	0	0	1591

Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	-	384	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.9	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	620	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	620	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	0	0	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBT
Capacity (veh/h)	-	-	-	-
HCM Lane V/C Ratio	-	-	-	-
HCM Control Delay (s)	-	-	0	-
HCM Lane LOS	-	-	A	-
HCM 95th %tile Q(veh)	-	-	-	-

MPAH Amendment Study - Villa Park Rd

Vistro File: N:\...\Villa Park_v6.vistro
Report File: N:\...\01_EXAM.pdf

Scenario 1 Existing AM Peak Hour
7/21/2023

Intersection Analysis Summary

ID	Intersection Name	Control Type	Method	Worst Mvmt	V/C	Delay (s/veh)	LOS
1	Villa Park Rd & Wanda Rd	Signalized	ICU 2	WB Thru	0.743	-	C
7	Villa Park Rd & Center Dr	Signalized	ICU 2	EB Thru	0.553	-	A
9	Villa Park Rd & Lemon St	Signalized	ICU 2	EB Thru	0.390	-	A
10	Villa Park Rd & Hewes St	Signalized	ICU 2	EB Thru	0.767	-	C
12	Santiago Canyon Rd & Cannon St	Signalized	ICU 2	SB Right	0.694	-	B
13	Taft Ave & Cannon St	Signalized	ICU 2	SB Right	0.893	-	D
14	Serrano Ave & Cannon St	Signalized	ICU 2	SB Thru	0.758	-	C

V/C, Delay, LOS: For two-way stop, these values are taken from the movement with the worst (highest) delay value. For all other control types, they are taken for the whole intersection.

Intersection Level Of Service Report
Intersection 1: Villa Park Rd & Wanda Rd

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	C
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.743

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	0	2	0	1	1	0	1
Entry Pocket Length [ft]	100.00	100.00	100.00	175.00	100.00	100.00	225.00	100.00	300.00	150.00	100.00	175.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			Yes			Yes			Yes		

Volumes

Name												
Base Volume Input [veh/h]	212	453	130	168	342	378	300	959	302	60	983	130
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	212	453	130	168	342	378	300	959	302	60	983	130
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	53	113	33	42	86	95	75	240	76	15	246	33
Total Analysis Volume [veh/h]	212	453	130	168	342	378	300	959	302	60	983	130
Pedestrian Volume [ped/h]	3			0			3			10		
Bicycle Volume [bicycles/h]	0			0			2			8		

Intersection Settings

Cycle Length [s]	130
Lost time [s]	7.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Overlap	Protecte	Permiss	Overlap	Protecte	Permiss	Overlap
Signal Group	3	8	0	7	4	4	1	6	6	5	2	2
Auxiliary Signal Groups						1,4			3,6			2,7
Lead / Lag	Lead	-	-	Lead	-	-	Lag	-	-	Lead	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.12	0.17	0.17	0.10	0.20	0.13	0.09	0.28	0.05	0.04	0.29	0.00
Intersection LOS	C											
Intersection V/C	0.743											

Intersection Level Of Service Report
Intersection 7: Villa Park Rd & Center Dr

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	A
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.553

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	0	1	0	1	1	0	1
Entry Pocket Length [ft]	150.00	100.00	100.00	140.00	100.00	100.00	160.00	100.00	100.00	150.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			Yes			Yes			Yes		

Volumes

Name												
Base Volume Input [veh/h]	59	102	55	44	119	136	61	1067	114	42	921	49
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	59	102	55	44	119	136	61	1067	114	42	921	49
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	15	26	14	11	30	34	15	267	29	11	230	12
Total Analysis Volume [veh/h]	59	102	55	44	119	136	61	1067	114	42	921	49
Pedestrian Volume [ped/h]	1			4			9			1		
Bicycle Volume [bicycles/h]	3			0			1			0		

Intersection Settings

Cycle Length [s]	111
Lost time [s]	6.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	0	4	0	0	4	0	5	2	0	1	6	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	Lead	-	-	Lead	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.03	0.09	0.09	0.03	0.15	0.15	0.04	0.31	0.07	0.02	0.27	0.03
Intersection LOS	A											
Intersection V/C	0.553											

Intersection Level Of Service Report
Intersection 9: Villa Park Rd & Lemon St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	A
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.390

Intersection Setup

Name						
Approach	Southbound		Eastbound		Westbound	
Lane Configuration						
Turning Movement	Left	Right	Left	Thru	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	1	0	0	1
Entry Pocket Length [ft]	115.00	100.00	180.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00		30.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	Yes		Yes		No	

Volumes

Name						
Base Volume Input [veh/h]	113	38	69	1093	977	105
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	113	38	69	1093	977	105
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	28	10	17	273	244	26
Total Analysis Volume [veh/h]	113	38	69	1093	977	105
Pedestrian Volume [ped/h]	5		5		0	
Bicycle Volume [bicycles/h]	2		2		0	

Intersection Settings

Cycle Length [s]	108
Lost time [s]	5.00

Phasing & Timing

Control Type	Permissive	Permissive	Protected	Permissive	Permissive	Permissive
Signal Group	4	0	1	6	2	0
Auxiliary Signal Groups						
Lead / Lag	Lag	-	Lead	-	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.03	0.04	0.04	0.32	0.29	0.06
Intersection LOS	A					
Intersection V/C	0.390					

Intersection Level Of Service Report
Intersection 10: Villa Park Rd & Hewes St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	C
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.767

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0	0	0	1	1	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	215.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			No			Yes			No		

Volumes

Name												
Base Volume Input [veh/h]	202	0	285	0	1	0	0	995	211	450	880	0
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	202	0	285	0	1	0	0	995	211	450	880	0
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	51	0	71	0	0	0	0	249	53	113	220	0
Total Analysis Volume [veh/h]	202	0	285	0	1	0	0	995	211	450	880	0
Pedestrian Volume [ped/h]	2			0			1			0		
Bicycle Volume [bicycles/h]	1			0			0			0		

Intersection Settings

Cycle Length [s]	130
Lost time [s]	7.00

Phasing & Timing

Control Type	Split	Permiss	Split	Split	Split	Split	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	4	0	0	0	8	0	0	2	0	1	6	0
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	-	-	-	-	-	-	Lead	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.12	0.00	0.17	0.00	0.00	0.00	0.00	0.29	0.12	0.26	0.26	0.00
Intersection LOS	C											
Intersection V/C	0.767											

Intersection Level Of Service Report
Intersection 12: Santiago Canyon Rd & Cannon St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	B
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.694

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	2	0	1	2	0	0	1	0	1
Entry Pocket Length [ft]	80.00	100.00	100.00	375.00	100.00	250.00	325.00	100.00	100.00	150.00	100.00	450.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			No			Yes			No		

Volumes

Name												
Base Volume Input [veh/h]	139	109	36	1294	319	714	364	766	91	16	469	366
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	139	109	36	1294	319	714	364	766	91	16	469	366
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	35	27	9	324	80	179	91	192	23	4	117	92
Total Analysis Volume [veh/h]	139	109	36	1294	319	714	364	766	91	16	469	366
Pedestrian Volume [ped/h]	0			0			6			0		
Bicycle Volume [bicycles/h]	3			0			0			1		

Intersection Settings

Cycle Length [s]	130
Lost time [s]	7.00

Phasing & Timing

Control Type	Split	Split	Split	Split	Split	Overlap	Protecte	Permiss	Permiss	Protecte	Permiss	Unsigna
Signal Group	0	8	0	0	4	4	1	6	0	5	2	0
Auxiliary Signal Groups						1,4						
Lead / Lag	-	-	-	-	-	-	Lead	-	-	Lag	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.08	0.04	0.04	0.25	0.19	0.31	0.11	0.25	0.25	0.01	0.14	0.00
Intersection LOS	B											
Intersection V/C	0.694											

Intersection Level Of Service Report
Intersection 13: Taft Ave & Cannon St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	D
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.893

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	0	0	0	1	0	0	0
Entry Pocket Length [ft]	225.00	100.00	100.00	150.00	100.00	100.00	100.00	100.00	200.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			No			Yes			Yes		

Volumes

Name												
Base Volume Input [veh/h]	67	771	1	1	2217	230	145	2	109	1	2	1
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	67	771	1	1	2217	230	145	2	109	1	2	1
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	17	193	0	0	554	58	36	1	27	0	1	0
Total Analysis Volume [veh/h]	67	771	1	1	2217	230	145	2	109	1	2	1
Pedestrian Volume [ped/h]	5			0			5			0		
Bicycle Volume [bicycles/h]	2			0			2			0		

Intersection Settings

Cycle Length [s]	175
Lost time [s]	9.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Permiss	Permiss	Overlap	Permiss	Permiss	Permiss
Signal Group	5	2	0	1	6	0	0	4	4	0	4	0
Auxiliary Signal Groups									4,5			
Lead / Lag	Lead	-	-	Lead	-	-	-	-	-	-	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.04	0.23	0.23	0.00	0.72	0.72	0.09	0.09	0.02	0.00	0.00	0.00
Intersection LOS	D											
Intersection V/C	0.893											

Intersection Level Of Service Report
Intersection 14: Serrano Ave & Cannon St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	C
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.758

Intersection Setup

Name						
Approach	Northbound		Southbound		Westbound	
Lane Configuration						
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	1	1	0	1	0
Entry Pocket Length [ft]	100.00	150.00	275.00	100.00	350.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00		30.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	No		Yes		Yes	

Volumes

Name						
Base Volume Input [veh/h]	561	356	51	1413	1035	145
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	561	356	51	1413	1035	145
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	140	89	13	353	259	36
Total Analysis Volume [veh/h]	561	356	51	1413	1035	145
Pedestrian Volume [ped/h]	0		0		0	
Bicycle Volume [bicycles/h]	0		0		0	

Intersection Settings

Cycle Length [s]	100
Lost time [s]	5.00

Phasing & Timing

Control Type	Permissive	Overlap	Protected	Permissive	Split	Overlap
Signal Group	2	2	1	6	4	4
Auxiliary Signal Groups		2,4				1,4
Lead / Lag	-	-	Lead	-	Lag	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.17	0.00	0.03	0.42	0.30	0.06
Intersection LOS	C					
Intersection V/C	0.758					

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	376	895	232	59	953	317	150	532	60	114	366	279
Future Volume (veh/h)	376	895	232	59	953	317	150	532	60	114	366	279
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		1.00	1.00		0.98	1.00		0.99
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1673	1673	1673	1673	1673	1673	1673	1673	1673	1673	1673	1673
Adj Flow Rate, veh/h	392	932	140	61	993	293	156	554	57	119	381	240
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	488	1274	729	245	1295	732	195	810	83	170	441	601
Arrive On Green	0.16	0.40	0.40	0.15	0.41	0.41	0.12	0.28	0.25	0.11	0.26	0.26
Sat Flow, veh/h	3092	3180	1399	1594	3180	1418	1594	2904	298	1594	1673	1400
Grp Volume(v), veh/h	392	932	140	61	993	293	156	303	308	119	381	240
Grp Sat Flow(s),veh/h/ln	1546	1590	1399	1594	1590	1418	1594	1590	1612	1594	1673	1400
Q Serve(g_s), s	17.1	34.8	4.1	4.7	37.7	17.6	13.3	23.7	23.9	10.1	30.4	16.6
Cycle Q Clear(g_c), s	17.1	34.8	4.1	4.7	37.7	17.6	13.3	23.7	23.9	10.1	30.4	16.6
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.18	1.00		1.00
Lane Grp Cap(c), veh/h	488	1274	729	245	1295	732	195	444	450	170	441	601
V/C Ratio(X)	0.80	0.73	0.19	0.25	0.77	0.40	0.80	0.68	0.69	0.70	0.86	0.40
Avail Cap(c_a), veh/h	629	1274	729	245	1295	732	195	444	450	249	512	660
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	56.9	35.6	7.1	52.2	35.8	20.7	59.8	44.9	45.3	60.4	49.1	27.7
Incr Delay (d2), s/veh	4.4	3.7	0.6	0.2	4.4	1.6	19.4	3.5	3.6	1.9	11.5	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	6.9	13.8	1.5	1.9	15.0	6.0	6.4	9.7	9.9	4.2	14.0	5.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	61.3	39.3	7.7	52.4	40.2	22.3	79.2	48.5	48.9	62.3	60.7	27.9
LnGrp LOS	E	D	A	D	D	C	E	D	D	E	E	C
Approach Vol, veh/h		1464			1347			767			740	
Approach Delay, s/veh		42.2			36.8			54.9			50.3	
Approach LOS		D			D			D			D	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	23.6	59.3	18.0	39.1	23.9	59.0	16.0	41.1
Change Period (Y+Rc), s	4.0	5.5	4.0	5.5	5.5	* 5.5	4.0	5.5
Max Green Setting (Gmax), s	26.0	41.5	14.0	39.5	14.0	* 54	19.0	34.5
Max Q Clear Time (g_c+I1), s	19.1	39.7	15.3	32.4	6.7	36.8	12.1	25.9
Green Ext Time (p_c), s	0.5	0.7	0.0	1.2	0.0	2.3	0.1	0.9

Intersection Summary	
HCM 6th Ctrl Delay	44.2
HCM 6th LOS	D

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection	
Int Delay, s/veh	0

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	4	1065	1347	0	0	2
Future Vol, veh/h	4	1065	1347	0	0	2
Conflicting Peds, #/hr	1	0	0	1	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	70	-	-	100	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	2	2	0	0	0
Mvmt Flow	4	1158	1464	0	0	2

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	1465	0	-	0	2052	733
Stage 1	-	-	-	-	1465	-
Stage 2	-	-	-	-	587	-
Critical Hdwy	4.1	-	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	2.2	-	-	-	3.5	3.3
Pot Cap-1 Maneuver	467	-	-	-	49	368
Stage 1	-	-	-	-	182	-
Stage 2	-	-	-	-	524	-
Platoon blocked, %			-	-	-	
Mov Cap-1 Maneuver	467	-	-	-	48	368
Mov Cap-2 Maneuver	-	-	-	-	48	-
Stage 1	-	-	-	-	180	-
Stage 2	-	-	-	-	523	-

Approach	EB	WB	SB
HCM Control Delay, s	0	0	14.8
HCM LOS			B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	467	-	-	-	368
HCM Lane V/C Ratio	0.009	-	-	-	0.006
HCM Control Delay (s)	12.8	-	-	-	14.8
HCM Lane LOS	B	-	-	-	B
HCM 95th %tile Q(veh)	0	-	-	-	0

Intersection	
Int Delay, s/veh	0.1

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	1064	1	2	1343	4	1
Future Vol, veh/h	1064	1	2	1343	4	1
Conflicting Peds, #/hr	0	1	1	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	100	100	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	0	0	2	0	0
Mvmt Flow	1157	1	2	1460	4	1

Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	1159	0	1892	580
Stage 1	-	-	-	-	1158	-
Stage 2	-	-	-	-	734	-
Critical Hdwy	-	-	4.1	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	610	-	63	463
Stage 1	-	-	-	-	265	-
Stage 2	-	-	-	-	441	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	609	-	63	463
Mov Cap-2 Maneuver	-	-	-	-	63	-
Stage 1	-	-	-	-	265	-
Stage 2	-	-	-	-	440	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0	56
HCM LOS			F

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	76	-	-	609	-
HCM Lane V/C Ratio	0.072	-	-	0.004	-
HCM Control Delay (s)	56	-	-	10.9	-
HCM Lane LOS	F	-	-	B	-
HCM 95th %tile Q(veh)	0.2	-	-	0	-

Intersection	
Int Delay, s/veh	0.2

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	18	1047	1341	4	2	4
Future Vol, veh/h	18	1047	1341	4	2	4
Conflicting Peds, #/hr	1	0	0	1	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	100	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	2	2	0	0	0
Mvmt Flow	20	1138	1458	4	2	4

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	1463	0	-	0	2068	730
Stage 1	-	-	-	-	1459	-
Stage 2	-	-	-	-	609	-
Critical Hdwy	4.1	-	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	2.2	-	-	-	3.5	3.3
Pot Cap-1 Maneuver	468	-	-	-	48	369
Stage 1	-	-	-	-	183	-
Stage 2	-	-	-	-	511	-
Platoon blocked, %			-	-	-	
Mov Cap-1 Maneuver	468	-	-	-	46	369
Mov Cap-2 Maneuver	-	-	-	-	46	-
Stage 1	-	-	-	-	175	-
Stage 2	-	-	-	-	510	-

Approach	EB	WB	SB
HCM Control Delay, s	0.2	0	39.8
HCM LOS			E

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	468	-	-	-	110
HCM Lane V/C Ratio	0.042	-	-	-	0.059
HCM Control Delay (s)	13	-	-	-	39.8
HCM Lane LOS	B	-	-	-	E
HCM 95th %tile Q(veh)	0.1	-	-	-	0.2

Intersection	
Int Delay, s/veh	0.1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	6	1043	1337	3	0	8
Future Vol, veh/h	6	1043	1337	3	0	8
Conflicting Peds, #/hr	1	0	0	1	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	100	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	2	2	0	0	0
Mvmt Flow	7	1134	1453	3	0	9

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	1457	0	-	0	2035	728
Stage 1	-	-	-	-	1454	-
Stage 2	-	-	-	-	581	-
Critical Hdwy	4.1	-	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	2.2	-	-	-	3.5	3.3
Pot Cap-1 Maneuver	470	-	-	-	51	370
Stage 1	-	-	-	-	185	-
Stage 2	-	-	-	-	528	-
Platoon blocked, %			-	-	-	
Mov Cap-1 Maneuver	470	-	-	-	50	370
Mov Cap-2 Maneuver	-	-	-	-	50	-
Stage 1	-	-	-	-	182	-
Stage 2	-	-	-	-	527	-

Approach	EB	WB	SB
HCM Control Delay, s	0.1	0	15
HCM LOS			C

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	470	-	-	-	370
HCM Lane V/C Ratio	0.014	-	-	-	0.024
HCM Control Delay (s)	12.8	-	-	-	15
HCM Lane LOS	B	-	-	-	C
HCM 95th %tile Q(veh)	0	-	-	-	0.1

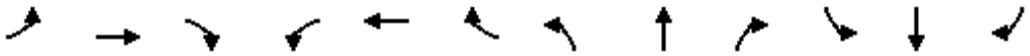
Intersection	
Int Delay, s/veh	0

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	1041	2	1	1339	1	0
Future Vol, veh/h	1041	2	1	1339	1	0
Conflicting Peds, #/hr	0	1	1	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	100	125	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	93	93	93	93	93	93
Heavy Vehicles, %	2	0	0	2	0	0
Mvmt Flow	1119	2	1	1440	1	0

Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	1122	0	1842	561
Stage 1	-	-	-	-	1120	-
Stage 2	-	-	-	-	722	-
Critical Hdwy	-	-	4.1	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	630	-	68	476
Stage 1	-	-	-	-	278	-
Stage 2	-	-	-	-	447	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	629	-	68	476
Mov Cap-2 Maneuver	-	-	-	-	68	-
Stage 1	-	-	-	-	278	-
Stage 2	-	-	-	-	446	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0	58.8
HCM LOS			F

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	68	-	-	629	-
HCM Lane V/C Ratio	0.016	-	-	0.002	-
HCM Control Delay (s)	58.8	-	-	10.7	-
HCM Lane LOS	F	-	-	B	-
HCM 95th %tile Q(veh)	0	-	-	0	-



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	79	950	12	17	1256	85	16	38	16	44	30	68
Future Volume (veh/h)	79	950	12	17	1256	85	16	38	16	44	30	68
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.98	1.00		1.00	1.00		1.00	1.00		0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1700	1673	1700	1700	1673	1700	1700	1700	1700	1700	1700	1700
Adj Flow Rate, veh/h	86	1033	11	18	1365	78	17	41	3	48	33	11
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Percent Heavy Veh, %	0	2	0	0	2	0	0	0	0	0	0	0
Cap, veh/h	170	2158	957	90	2000	906	326	225	16	327	175	58
Arrive On Green	0.11	0.68	0.68	0.06	0.63	0.63	0.14	0.14	0.09	0.14	0.14	0.14
Sat Flow, veh/h	1619	3180	1410	1619	3180	1440	1384	1565	115	1384	1213	404
Grp Volume(v), veh/h	86	1033	11	18	1365	78	17	0	44	48	0	44
Grp Sat Flow(s),veh/h/ln	1619	1590	1410	1619	1590	1440	1384	0	1679	1384	0	1618
Q Serve(g_s), s	2.3	7.0	0.1	0.5	12.6	1.0	0.5	0.0	1.0	1.4	0.0	1.1
Cycle Q Clear(g_c), s	2.3	7.0	0.1	0.5	12.6	1.0	1.6	0.0	1.0	2.5	0.0	1.1
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.07	1.00		0.25
Lane Grp Cap(c), veh/h	170	2158	957	90	2000	906	326	0	242	327	0	233
V/C Ratio(X)	0.51	0.48	0.01	0.20	0.68	0.09	0.05	0.00	0.18	0.15	0.00	0.19
Avail Cap(c_a), veh/h	785	3040	1347	785	3040	1376	1363	0	1501	1365	0	1446
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	19.1	3.5	2.3	20.4	5.4	3.3	17.7	0.0	17.1	18.1	0.0	17.0
Incr Delay (d2), s/veh	0.9	0.2	0.0	0.4	0.4	0.0	0.1	0.0	0.5	0.3	0.0	0.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.8	0.6	0.0	0.2	1.7	0.1	0.1	0.0	0.4	0.4	0.0	0.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	20.0	3.6	2.4	20.8	5.9	3.3	17.8	0.0	17.6	18.4	0.0	17.6
LnGrp LOS	B	A	A	C	A	A	B	A	B	B	A	B
Approach Vol, veh/h		1130			1461			61			92	
Approach Delay, s/veh		4.9			5.9			17.6			18.0	
Approach LOS		A			A			B			B	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	3.6	33.0		8.6	5.8	30.7		8.6				
Change Period (Y+Rc), s	3.0	5.5		4.5	3.0	5.5		4.5				
Max Green Setting (Gmax), s	20.0	40.0		38.0	20.0	40.0		38.0				
Max Q Clear Time (g_c+I1), s	2.5	9.0		3.6	4.3	14.6		4.5				
Green Ext Time (p_c), s	0.0	8.1		0.4	0.0	10.6		0.5				

Intersection Summary			
HCM 6th Ctrl Delay		6.1	
HCM 6th LOS		A	

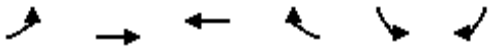
Intersection	
Int Delay, s/veh	0.2

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	1005	5	3	1352	6	2
Future Vol, veh/h	1005	5	3	1352	6	2
Conflicting Peds, #/hr	0	1	1	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	100	140	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	2	0	0	2	0	0
Mvmt Flow	1069	5	3	1438	6	2

Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	1075	0	1795	536
Stage 1	-	-	-	-	1070	-
Stage 2	-	-	-	-	725	-
Critical Hdwy	-	-	4.1	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	656	-	73	494
Stage 1	-	-	-	-	295	-
Stage 2	-	-	-	-	446	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	655	-	73	494
Mov Cap-2 Maneuver	-	-	-	-	73	-
Stage 1	-	-	-	-	295	-
Stage 2	-	-	-	-	444	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0	47.6
HCM LOS			E

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	93	-	-	655	-
HCM Lane V/C Ratio	0.092	-	-	0.005	-
HCM Control Delay (s)	47.6	-	-	10.5	-
HCM Lane LOS	E	-	-	B	-
HCM 95th %tile Q(veh)	0.3	-	-	0	-



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	98	909	1313	102	61	42
Future Volume (veh/h)	98	909	1313	102	61	42
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			0.98	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No	No		No	
Adj Sat Flow, veh/h/ln	1700	1673	1673	1700	1700	1700
Adj Flow Rate, veh/h	102	947	1368	91	54	55
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	0	2	2	0	0	0
Cap, veh/h	193	2464	2003	888	203	181
Arrive On Green	0.12	0.77	0.63	0.63	0.13	0.13
Sat Flow, veh/h	1619	3263	3263	1409	1619	1441
Grp Volume(v), veh/h	102	947	1368	91	54	55
Grp Sat Flow(s),veh/h/ln	1619	1590	1590	1409	1619	1441
Q Serve(g_s), s	2.6	4.1	12.1	1.1	1.3	1.5
Cycle Q Clear(g_c), s	2.6	4.1	12.1	1.1	1.3	1.5
Prop In Lane	1.00			1.00	1.00	1.00
Lane Grp Cap(c), veh/h	193	2464	2003	888	203	181
V/C Ratio(X)	0.53	0.38	0.68	0.10	0.27	0.30
Avail Cap(c_a), veh/h	821	3182	3182	1410	1407	1252
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	17.9	1.6	5.2	3.2	17.1	17.2
Incr Delay (d2), s/veh	0.8	0.1	0.4	0.1	1.0	1.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.8	0.0	0.9	0.1	0.5	0.5
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	18.7	1.7	5.6	3.2	18.1	18.5
LnGrp LOS	B	A	A	A	B	B
Approach Vol, veh/h		1049	1459		109	
Approach Delay, s/veh		3.3	5.5		18.3	
Approach LOS		A	A		B	

Timer - Assigned Phs	1	2		4		6
Phs Duration (G+Y+Rc), s	6.3	29.5		7.4		35.7
Change Period (Y+Rc), s	3.0	5.5		4.5		5.5
Max Green Setting (Gmax), s	20.0	40.0		35.0		40.0
Max Q Clear Time (g_c+I1), s	4.6	14.1		3.5		6.1
Green Ext Time (p_c), s	0.1	9.9		0.5		7.4

Intersection Summary	
HCM 6th Ctrl Delay	5.1
HCM 6th LOS	A

Notes

User approved volume balancing among the lanes for turning movement.

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	838	132	240	1274	141	312
Future Volume (vph)	838	132	240	1274	141	312
Ideal Flow (vphpl)	1700	1700	1700	1700	1700	1700
Total Lost time (s)	2.5	2.5	1.0	2.5	1.5	1.5
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Frpb, ped/bikes	1.00	0.99	1.00	1.00	1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.85	1.00	1.00	1.00	0.85
Flt Protected	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	3167	1398	1583	3167	1568	1403
Flt Permitted	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (perm)	3167	1398	1583	3167	1568	1403
Peak-hour factor, PHF	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	891	140	255	1355	150	332
RTOR Reduction (vph)	0	23	0	0	0	285
Lane Group Flow (vph)	891	117	255	1355	150	47
Confl. Bikes (#/hr)		3				
Heavy Vehicles (%)	2%	2%	2%	2%	3%	3%
Turn Type	NA	Perm	Prot	NA	Prot	Prot
Protected Phases	2		1	6	4	4
Permitted Phases		2				
Actuated Green, G (s)	81.3	81.3	23.7	111.0	15.0	15.0
Effective Green, g (s)	86.3	86.3	28.7	116.0	20.0	20.0
Actuated g/C Ratio	0.62	0.62	0.20	0.83	0.14	0.14
Clearance Time (s)	7.5	7.5	6.0	7.5	6.5	6.5
Vehicle Extension (s)	3.0	3.0	3.0	3.0	2.0	2.0
Lane Grp Cap (vph)	1952	861	324	2624	224	200
v/s Ratio Prot	0.28		c0.16	c0.43	c0.10	0.03
v/s Ratio Perm		0.08				
v/c Ratio	0.46	0.14	0.79	0.52	0.67	0.24
Uniform Delay, d1	14.3	11.2	52.8	3.6	56.9	53.2
Progression Factor	1.00	1.00	0.86	1.00	1.00	1.00
Incremental Delay, d2	0.8	0.3	8.0	0.5	5.8	0.2
Delay (s)	15.1	11.6	53.3	4.1	62.6	53.5
Level of Service	B	B	D	A	E	D
Approach Delay (s)	14.6			11.9	56.3	
Approach LOS	B			B	E	

Intersection Summary			
HCM 2000 Control Delay	19.6	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.62		
Actuated Cycle Length (s)	140.0	Sum of lost time (s)	10.0
Intersection Capacity Utilization	59.5%	ICU Level of Service	B
Analysis Period (min)	15		

c Critical Lane Group

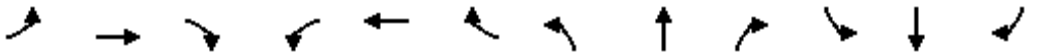
Intersection	
Int Delay, s/veh	1.7

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	8	1120	22	2	1489	13	6	0	6	9	0	19
Future Vol, veh/h	8	1120	22	2	1489	13	6	0	6	9	0	19
Conflicting Peds, #/hr	1	0	1	1	0	1	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	115	-	100	140	-	100	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	0	0	2	2	0	0	0	3	3	3
Mvmt Flow	9	1217	24	2	1618	14	7	0	7	10	0	21

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	1633	0	0	1242	0	0	2049	2873	610	2250	2883	810
Stage 1	-	-	-	-	-	-	1236	1236	-	1623	1623	-
Stage 2	-	-	-	-	-	-	813	1637	-	627	1260	-
Critical Hdwy	4.14	-	-	4.1	-	-	7.5	6.5	6.9	7.56	6.56	6.96
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.56	5.56	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.56	5.56	-
Follow-up Hdwy	2.22	-	-	2.2	-	-	3.5	4	3.3	3.53	4.03	3.33
Pot Cap-1 Maneuver	393	-	-	568	-	-	33	17	442	22	16	321
Stage 1	-	-	-	-	-	-	190	250	-	106	158	-
Stage 2	-	-	-	-	-	-	343	160	-	436	238	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	393	-	-	567	-	-	30	17	442	21	16	321
Mov Cap-2 Maneuver	-	-	-	-	-	-	30	17	-	21	16	-
Stage 1	-	-	-	-	-	-	185	244	-	103	157	-
Stage 2	-	-	-	-	-	-	320	159	-	420	232	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0.1	0	87.8	125.1
HCM LOS			F	F

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	56	393	-	-	567	-	-	57
HCM Lane V/C Ratio	0.233	0.022	-	-	0.004	-	-	0.534
HCM Control Delay (s)	87.8	14.4	-	-	11.4	-	-	125.1
HCM Lane LOS	F	B	-	-	B	-	-	F
HCM 95th %tile Q(veh)	0.8	0.1	-	-	0	-	-	2.1



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	706	356	73	13	996	1477	70	320	10	415	199	438
Future Volume (veh/h)	706	356	73	13	996	1477	70	320	10	415	199	438
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		1.00	1.00		0.99	1.00		0.99
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1673	1673	1673	1673	1673	1673	1687	1687	1687	1687	1687	1687
Adj Flow Rate, veh/h	735	371	68	14	1038	0	73	333	9	376	286	351
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	2	2	2	2	2	2	1	1	1	1	1	1
Cap, veh/h	844	1128	205	268	1016		218	432	12	713	749	714
Arrive On Green	0.09	0.14	0.14	0.17	0.32	0.00	0.14	0.14	0.10	0.22	0.22	0.22
Sat Flow, veh/h	3092	2681	486	1594	3180	1418	1606	3186	86	3213	3373	1409
Grp Volume(v), veh/h	735	218	221	14	1038	0	73	167	175	376	286	351
Grp Sat Flow(s),veh/h/ln	1546	1590	1578	1594	1590	1418	1606	1602	1670	1606	1687	1409
Q Serve(g_s), s	32.9	17.4	17.7	1.0	44.7	0.0	5.8	14.1	14.2	14.4	10.1	23.1
Cycle Q Clear(g_c), s	32.9	17.4	17.7	1.0	44.7	0.0	5.8	14.1	14.2	14.4	10.1	23.1
Prop In Lane	1.00		0.31	1.00		1.00	1.00		0.05	1.00		1.00
Lane Grp Cap(c), veh/h	844	669	664	268	1016		218	217	227	713	749	714
V/C Ratio(X)	0.87	0.33	0.33	0.05	1.02		0.33	0.77	0.77	0.53	0.38	0.49
Avail Cap(c_a), veh/h	881	669	664	268	1016		218	217	227	872	916	784
HCM Platoon Ratio	0.33	0.33	0.33	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	61.3	42.4	42.5	48.9	47.6	0.0	54.8	58.4	58.5	48.0	46.3	23.1
Incr Delay (d2), s/veh	8.7	1.3	1.3	0.0	33.9	0.0	0.3	13.9	13.7	0.2	0.1	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	14.6	7.6	7.7	0.4	21.7	0.0	2.3	6.5	6.8	5.7	4.2	7.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	70.0	43.7	43.9	48.9	81.6	0.0	55.1	72.3	72.3	48.2	46.4	23.2
LnGrp LOS	E	D	D	D	F		E	E	E	D	D	C
Approach Vol, veh/h		1174			1052	A		415			1013	
Approach Delay, s/veh		60.2			81.1			69.3			39.1	
Approach LOS		E			F			E			D	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	40.3	46.6		33.1	24.9	62.0		20.0				
Change Period (Y+Rc), s	5.0	5.5		6.0	5.5	* 6.5		5.5				
Max Green Setting (Gmax), s	37.0	32.5		34.0	13.0	* 56		14.5				
Max Q Clear Time (g_c+I1), s	34.9	46.7		25.1	3.0	19.7		16.2				
Green Ext Time (p_c), s	0.4	0.0		1.9	0.0	1.4		0.0				

Intersection Summary			
HCM 6th Ctrl Delay		61.4	
HCM 6th LOS		E	

Notes

User approved volume balancing among the lanes for turning movement.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	263	0	42	0	1	0	134	2369	0	0	1010	193
Future Volume (veh/h)	263	0	42	0	1	0	134	2369	0	0	1010	193
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.98	1.00		1.00	1.00		1.00	1.00		0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1700	1700	1700	1700	1700	1700	1700	1687	1700	1700	1687	1700
Adj Flow Rate, veh/h	268	0	14	0	1	0	137	2417	0	0	1031	188
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	0	0	0	0	0	0	0	1	0	0	1	0
Cap, veh/h	339	0	457	0	345	0	195	2192	0	92	1693	308
Arrive On Green	0.18	0.00	0.20	0.00	0.20	0.00	0.12	0.68	0.00	0.00	1.00	1.00
Sat Flow, veh/h	1435	0	1418	0	1700	0	1619	3289	0	1619	2697	491
Grp Volume(v), veh/h	268	0	14	0	1	0	137	2417	0	0	612	607
Grp Sat Flow(s),veh/h/ln	1435	0	1418	0	1700	0	1619	1602	0	1619	1602	1585
Q Serve(g_s), s	26.8	0.0	1.0	0.0	0.1	0.0	12.2	102.6	0.0	0.0	0.0	0.0
Cycle Q Clear(g_c), s	26.9	0.0	1.0	0.0	0.1	0.0	12.2	102.6	0.0	0.0	0.0	0.0
Prop In Lane	1.00		1.00	0.00		0.00	1.00		0.00	1.00		0.31
Lane Grp Cap(c), veh/h	305	0	457	0	345	0	195	2192	0	92	1006	995
V/C Ratio(X)	0.88	0.00	0.03	0.00	0.00	0.00	0.70	1.10	0.00	0.00	0.61	0.61
Avail Cap(c_a), veh/h	305	0	457	0	345	0	257	2192	0	103	1006	995
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	2.00	2.00	2.00
Upstream Filter(I)	1.00	0.00	1.00	0.00	1.00	0.00	1.00	1.00	0.00	0.00	1.00	1.00
Uniform Delay (d), s/veh	60.7	0.0	35.0	0.0	47.7	0.0	63.4	23.7	0.0	0.0	0.0	0.0
Incr Delay (d2), s/veh	23.9	0.0	0.0	0.0	0.0	0.0	3.0	53.8	0.0	0.0	2.7	2.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	12.6	0.0	0.4	0.0	0.0	0.0	5.1	49.6	0.0	0.0	0.8	0.8
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	84.5	0.0	35.0	0.0	47.7	0.0	66.5	77.5	0.0	0.0	2.7	2.8
LnGrp LOS	F	A	D	A	D	A	E	F	A	A	A	A
Approach Vol, veh/h		282			1			2554			1219	
Approach Delay, s/veh		82.1			47.7			76.9			2.8	
Approach LOS		F			D			E			A	

Timer - Assigned Phs	1	2		4	5	6		8
Phs Duration (G+Y+Rc), s	10.0	108.0		32.0	19.2	98.8		32.0
Change Period (Y+Rc), s	6.0	* 6		5.0	5.0	6.0		5.0
Max Green Setting (Gmax), s	5.0	* 1E2		27.0	20.0	87.0		27.0
Max Q Clear Time (g_c+I1), s	0.0	104.6		28.9	14.2	2.0		2.1
Green Ext Time (p_c), s	0.0	0.0		0.0	0.1	2.4		0.0

Intersection Summary		
HCM 6th Ctrl Delay		54.9
HCM 6th LOS		D

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	499	63	1244	1388	75	704
Future Volume (veh/h)	499	63	1244	1388	75	704
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1687	1687	1687	1687	1687	1687
Adj Flow Rate, veh/h	509	19	1269	1351	77	718
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	1	1	1	1	1	1
Cap, veh/h	592	393	2167	1265	135	2436
Arrive On Green	0.19	0.19	0.90	0.92	0.08	0.76
Sat Flow, veh/h	3116	1429	3289	1429	1606	3289
Grp Volume(v), veh/h	509	19	1269	1351	77	718
Grp Sat Flow(s),veh/h/ln	1558	1429	1602	1429	1606	1602
Q Serve(g_s), s	23.7	1.5	12.7	104.2	6.9	10.4
Cycle Q Clear(g_c), s	23.7	1.5	12.7	104.2	6.9	10.4
Prop In Lane	1.00	1.00		1.00	1.00	
Lane Grp Cap(c), veh/h	592	393	2167	1265	135	2436
V/C Ratio(X)	0.86	0.05	0.59	1.07	0.57	0.29
Avail Cap(c_a), veh/h	592	393	2167	1265	203	2436
HCM Platoon Ratio	1.00	1.00	1.33	1.33	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	58.8	39.9	3.1	1.7	66.1	5.6
Incr Delay (d2), s/veh	11.7	0.0	1.2	45.7	1.4	0.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	10.2	1.4	2.6	51.5	2.9	3.1
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	70.5	39.9	4.3	47.4	67.5	5.9
LnGrp LOS	E	D	A	F	E	A
Approach Vol, veh/h	528		2620			795
Approach Delay, s/veh	69.4		26.5			11.8
Approach LOS	E		C			B

Timer - Assigned Phs	1	2		4		6
Phs Duration (G+Y+Rc), s	13.6	106.4		30.0		120.0
Change Period (Y+Rc), s	5.0	6.0		5.3		6.0
Max Green Setting (Gmax), s	15.0	94.0		24.7		114.0
Max Q Clear Time (g_c+I1), s	8.9	106.2		25.7		12.4
Green Ext Time (p_c), s	0.0	0.0		0.0		3.1

Intersection Summary	
HCM 6th Ctrl Delay	29.3
HCM 6th LOS	C

Notes

User approved pedestrian interval to be less than phase max green.

Intersection	
Int Delay, s/veh	1

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	25	28	38	1269	751	29
Future Vol, veh/h	25	28	38	1269	751	29
Conflicting Peds, #/hr	0	0	1	0	0	1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	200	-	-	100
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	0	0	1	1	0
Mvmt Flow	26	29	40	1336	791	31

Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	1540	397	823	0	-	0
Stage 1	792	-	-	-	-	-
Stage 2	748	-	-	-	-	-
Critical Hdwy	6.8	6.9	4.1	-	-	-
Critical Hdwy Stg 1	5.8	-	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	108	608	816	-	-	-
Stage 1	412	-	-	-	-	-
Stage 2	434	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	102	607	815	-	-	-
Mov Cap-2 Maneuver	102	-	-	-	-	-
Stage 1	391	-	-	-	-	-
Stage 2	434	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	33.3	0.3	0
HCM LOS	D		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	815	-	182	-	-
HCM Lane V/C Ratio	0.049	-	0.307	-	-
HCM Control Delay (s)	9.6	-	33.3	-	-
HCM Lane LOS	A	-	D	-	-
HCM 95th %tile Q(veh)	0.2	-	1.2	-	-

Intersection	
Int Delay, s/veh	0

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	0	0	2503	0	0	1052
Future Vol, veh/h	0	0	2503	0	0	1052
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	150	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	0	0	2721	0	0	1143

Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	3293	1361	0	0	2721	0
Stage 1	2721	-	-	-	-	-
Stage 2	572	-	-	-	-	-
Critical Hdwy	6.8	6.9	-	-	4.1	-
Critical Hdwy Stg 1	5.8	-	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	7	141	-	-	151	-
Stage 1	37	-	-	-	-	-
Stage 2	534	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	7	141	-	-	151	-
Mov Cap-2 Maneuver	7	-	-	-	-	-
Stage 1	37	-	-	-	-	-
Stage 2	534	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	0	0	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	-	151	-
HCM Lane V/C Ratio	-	-	-	-	-
HCM Control Delay (s)	-	-	0	0	-
HCM Lane LOS	-	-	A	A	-
HCM 95th %tile Q(veh)	-	-	-	0	-

Intersection	
Int Delay, s/veh	0

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	0	0	2632	1203	0
Future Vol, veh/h	0	0	0	2632	1203	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	150	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	0	0	0	2861	1308	0

Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	2739	654	1308	0	-	0
Stage 1	1308	-	-	-	-	-
Stage 2	1431	-	-	-	-	-
Critical Hdwy	6.8	6.9	4.1	-	-	-
Critical Hdwy Stg 1	5.8	-	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	17	414	536	-	-	-
Stage 1	221	-	-	-	-	-
Stage 2	190	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	17	414	536	-	-	-
Mov Cap-2 Maneuver	17	-	-	-	-	-
Stage 1	221	-	-	-	-	-
Stage 2	190	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	0	0	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	536	-	-	-	-
HCM Lane V/C Ratio	-	-	-	-	-
HCM Control Delay (s)	0	-	0	-	-
HCM Lane LOS	A	-	A	-	-
HCM 95th %tile Q(veh)	0	-	-	-	-

Intersection	
Int Delay, s/veh	0

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	0	0	1307	0	0	779
Future Vol, veh/h	0	0	1307	0	0	779
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	125	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	0	0	1421	0	0	847

Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	-	711	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.9	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	380	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	380	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	0	0	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBT
Capacity (veh/h)	-	-	-	-
HCM Lane V/C Ratio	-	-	-	-
HCM Control Delay (s)	-	-	0	-
HCM Lane LOS	-	-	A	-
HCM 95th %tile Q(veh)	-	-	-	-

MPAH Amendment Study - Villa Park Rd

Vistro File: N:\...\Villa Park_v6.vistro
Report File: N:\...\02_EXPM.pdf

Scenario 2 Exisitng PM Peak Hour
7/21/2023

Intersection Analysis Summary

ID	Intersection Name	Control Type	Method	Worst Mvmt	V/C	Delay (s/veh)	LOS
1	Villa Park Rd & Wanda Rd	Signalized	ICU 2	WB Thru	0.731	-	C
7	Villa Park Rd & Center Dr	Signalized	ICU 2	WB Thru	0.511	-	A
9	Villa Park Rd & Lemon St	Signalized	ICU 2	WB Thru	0.497	-	A
10	Villa Park Rd & Hewes St	Signalized	ICU 2	WB Thru	0.605	-	B
12	Santiago Canyon Rd & Cannon St	Signalized	ICU 2	WB Thru	0.758	-	C
13	Taft Ave & Cannon St	Signalized	ICU 2	NB Thru	0.900	-	E
14	Serrano Ave & Cannon St	Signalized	ICU 2	NB Right	0.909	-	E

V/C, Delay, LOS: For two-way stop, these values are taken from the movement with the worst (highest) delay value. For all other control types, they are taken for the whole intersection.

Intersection Level Of Service Report
Intersection 1: Villa Park Rd & Wanda Rd

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	C
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.731

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	0	2	0	1	1	0	1
Entry Pocket Length [ft]	100.00	100.00	100.00	175.00	100.00	100.00	225.00	100.00	300.00	150.00	100.00	175.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			Yes			Yes			Yes		

Volumes

Name												
Base Volume Input [veh/h]	150	532	60	114	366	279	376	895	232	59	953	317
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	150	532	60	114	366	279	376	895	232	59	953	317
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	38	133	15	29	92	70	94	224	58	15	238	79
Total Analysis Volume [veh/h]	150	532	60	114	366	279	376	895	232	59	953	317
Pedestrian Volume [ped/h]	0			0			0			1		
Bicycle Volume [bicycles/h]	1			0			1			11		

Intersection Settings

Cycle Length [s]	140
Lost time [s]	7.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Overlap	Protecte	Permiss	Overlap	Protecte	Permiss	Overlap
Signal Group	3	8	0	7	4	4	1	6	6	5	2	2
Auxiliary Signal Groups						1,4			3,6			2,7
Lead / Lag	Lead	-	-	Lead	-	-	Lead	-	-	Lag	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.09	0.17	0.17	0.07	0.22	0.05	0.11	0.26	0.05	0.03	0.28	0.12
Intersection LOS	C											
Intersection V/C	0.731											

Intersection Level Of Service Report
Intersection 7: Villa Park Rd & Center Dr

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	A
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.511

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	0	1	0	1	1	0	1
Entry Pocket Length [ft]	150.00	100.00	100.00	140.00	100.00	100.00	160.00	100.00	100.00	150.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			Yes			Yes			Yes		

Volumes

Name												
Base Volume Input [veh/h]	16	38	16	44	30	68	79	950	12	17	1256	85
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	16	38	16	44	30	68	79	950	12	17	1256	85
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	4	10	4	11	8	17	20	238	3	4	314	21
Total Analysis Volume [veh/h]	16	38	16	44	30	68	79	950	12	17	1256	85
Pedestrian Volume [ped/h]	0			1			0			0		
Bicycle Volume [bicycles/h]	3			0			5			0		

Intersection Settings

Cycle Length [s]	111
Lost time [s]	6.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	0	4	0	0	4	0	5	2	0	1	6	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	Lead	-	-	Lead	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.01	0.03	0.03	0.03	0.06	0.06	0.05	0.28	0.01	0.01	0.37	0.05
Intersection LOS	A											
Intersection V/C	0.511											

Intersection Level Of Service Report
Intersection 9: Villa Park Rd & Lemon St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	A
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.497

Intersection Setup

Name						
Approach	Southbound		Eastbound		Westbound	
Lane Configuration						
Turning Movement	Left	Right	Left	Thru	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	1	0	0	1
Entry Pocket Length [ft]	115.00	100.00	180.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00		30.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	Yes		Yes		No	

Volumes

Name						
Base Volume Input [veh/h]	61	42	98	909	1313	102
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	61	42	98	909	1313	102
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	15	11	25	227	328	26
Total Analysis Volume [veh/h]	61	42	98	909	1313	102
Pedestrian Volume [ped/h]	1		1		0	
Bicycle Volume [bicycles/h]	2		2		0	

Intersection Settings

Cycle Length [s]	108
Lost time [s]	5.00

Phasing & Timing

Control Type	Permissive	Permissive	Protected	Permissive	Permissive	Permissive
Signal Group	4	0	1	6	2	0
Auxiliary Signal Groups						
Lead / Lag	Lag	-	Lead	-	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.02	0.03	0.06	0.27	0.39	0.06
Intersection LOS	A					
Intersection V/C	0.497					

Intersection Level Of Service Report
Intersection 10: Villa Park Rd & Hewes St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	B
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.605

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0	0	0	1	1	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	215.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			No			Yes			No		

Volumes

Name												
Base Volume Input [veh/h]	141	0	312	0	6	0	0	838	132	240	1274	0
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	141	0	312	0	6	0	0	838	132	240	1274	0
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	35	0	78	0	2	0	0	210	33	60	319	0
Total Analysis Volume [veh/h]	141	0	312	0	6	0	0	838	132	240	1274	0
Pedestrian Volume [ped/h]	0			0			0			0		
Bicycle Volume [bicycles/h]	3			0			6			0		

Intersection Settings

Cycle Length [s]	140
Lost time [s]	7.00

Phasing & Timing

Control Type	Split	Permiss	Split	Split	Split	Split	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	4	0	0	0	8	0	0	2	0	1	6	0
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	-	-	-	-	-	-	Lead	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.08	0.00	0.18	0.00	0.00	0.00	0.00	0.25	0.08	0.14	0.37	0.00
Intersection LOS	B											
Intersection V/C	0.605											

Intersection Level Of Service Report
Intersection 12: Santiago Canyon Rd & Cannon St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	C
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.758

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	2	0	1	2	0	0	1	0	1
Entry Pocket Length [ft]	80.00	100.00	100.00	375.00	100.00	250.00	325.00	100.00	100.00	150.00	100.00	450.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			No			Yes			No		

Volumes

Name												
Base Volume Input [veh/h]	70	320	10	415	199	438	706	356	73	13	996	1477
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	70	320	10	415	199	438	706	356	73	13	996	1477
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	18	80	3	104	50	110	177	89	18	3	249	369
Total Analysis Volume [veh/h]	70	320	10	415	199	438	706	356	73	13	996	1477
Pedestrian Volume [ped/h]	1			0			1			0		
Bicycle Volume [bicycles/h]	3			3			1			1		

Intersection Settings

Cycle Length [s]	140
Lost time [s]	7.00

Phasing & Timing

Control Type	Split	Split	Split	Split	Split	Overlap	Protecte	Permiss	Permiss	Protecte	Permiss	Unsigna
Signal Group	0	8	0	0	4	4	1	6	0	5	2	0
Auxiliary Signal Groups						1,4						
Lead / Lag	-	-	-	-	-	-	Lead	-	-	Lead	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.04	0.10	0.10	0.12	0.06	0.05	0.21	0.13	0.13	0.01	0.29	0.00
Intersection LOS	C											
Intersection V/C	0.758											

Intersection Level Of Service Report
Intersection 13: Taft Ave & Cannon St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	E
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.900

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	0	0	0	1	0	0	0
Entry Pocket Length [ft]	225.00	100.00	100.00	150.00	100.00	100.00	100.00	100.00	200.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			No			Yes			Yes		

Volumes

Name												
Base Volume Input [veh/h]	134	2369	0	0	1010	193	263	0	42	0	1	0
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	134	2369	0	0	1010	193	263	0	42	0	1	0
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	34	592	0	0	253	48	66	0	11	0	0	0
Total Analysis Volume [veh/h]	134	2369	0	0	1010	193	263	0	42	0	1	0
Pedestrian Volume [ped/h]	1			0			1			0		
Bicycle Volume [bicycles/h]	2			0			2			0		

Intersection Settings

Cycle Length [s]	150
Lost time [s]	8.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Permiss	Permiss	Overlap	Permiss	Permiss	Permiss
Signal Group	5	2	0	1	6	0	0	4	4	0	4	0
Auxiliary Signal Groups									4,5			
Lead / Lag	Lead	-	-	Lag	-	-	-	-	-	-	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.08	0.70	0.00	0.00	0.35	0.35	0.15	0.00	0.00	0.00	0.00	0.00
Intersection LOS	E											
Intersection V/C	0.900											

Intersection Level Of Service Report
Intersection 14: Serrano Ave & Cannon St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	E
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.909

Intersection Setup

Name						
Approach	Northbound		Southbound		Westbound	
Lane Configuration						
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	1	1	0	1	0
Entry Pocket Length [ft]	100.00	150.00	275.00	100.00	350.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00		30.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	No		Yes		Yes	

Volumes

Name						
Base Volume Input [veh/h]	1244	1388	75	704	499	63
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	1244	1388	75	704	499	63
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	311	347	19	176	125	16
Total Analysis Volume [veh/h]	1244	1388	75	704	499	63
Pedestrian Volume [ped/h]	0		0		0	
Bicycle Volume [bicycles/h]	0		0		0	

Intersection Settings

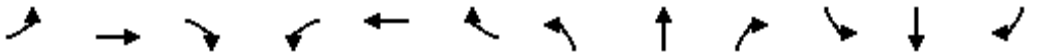
Cycle Length [s]	150
Lost time [s]	8.00

Phasing & Timing

Control Type	Permissive	Overlap	Protected	Permissive	Split	Overlap
Signal Group	2	2	1	6	4	4
Auxiliary Signal Groups		2,4				1,4
Lead / Lag	-	-	Lead	-	Lag	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.37	0.67	0.04	0.21	0.15	0.00
Intersection LOS	E					
Intersection V/C	0.909					



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	310	980	310	60	1140	160	230	460	130	200	390	380
Future Volume (veh/h)	310	980	310	60	1140	160	230	460	130	200	390	380
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		0.97	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1634	1634	1634	1634	1634	1634	1687	1687	1687	1687	1687	1687
Adj Flow Rate, veh/h	326	1032	255	63	1200	112	242	484	117	211	411	360
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	5	5	5	5	5	5	1	1	1	1	1	1
Cap, veh/h	323	2154	1173	108	2037	1138	245	710	170	264	471	560
Arrive On Green	0.11	0.69	0.69	0.07	0.66	0.66	0.15	0.28	0.25	0.16	0.28	0.28
Sat Flow, veh/h	3018	3104	1383	1556	3104	1384	1606	2545	611	1606	1687	1425
Grp Volume(v), veh/h	326	1032	255	63	1200	112	242	303	298	211	411	360
Grp Sat Flow(s),veh/h/ln	1509	1552	1383	1556	1552	1384	1606	1602	1554	1606	1687	1425
Q Serve(g_s), s	13.9	19.8	1.6	5.1	28.2	2.0	19.5	21.9	22.3	16.4	30.2	28.4
Cycle Q Clear(g_c), s	13.9	19.8	1.6	5.1	28.2	2.0	19.5	21.9	22.3	16.4	30.2	28.4
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.39	1.00		1.00
Lane Grp Cap(c), veh/h	323	2154	1173	108	2037	1138	245	447	433	264	471	560
V/C Ratio(X)	1.01	0.48	0.22	0.58	0.59	0.10	0.99	0.68	0.69	0.80	0.87	0.64
Avail Cap(c_a), veh/h	323	2154	1173	108	2037	1138	245	447	433	271	477	566
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	58.0	9.1	6.0	58.7	12.5	2.2	55.0	41.7	42.5	52.3	44.7	38.1
Incr Delay (d2), s/veh	52.7	0.8	0.4	5.4	1.3	0.2	54.2	3.4	3.8	14.1	15.4	1.9
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	7.6	6.2	2.1	2.1	9.2	0.4	11.5	8.9	8.9	7.6	14.5	9.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	110.7	9.9	6.4	64.1	13.8	2.4	109.2	45.1	46.3	66.3	60.1	39.9
LnGrp LOS	F	A	A	E	B	A	F	D	D	E	E	D
Approach Vol, veh/h		1613			1375			843			982	
Approach Delay, s/veh		29.7			15.1			63.9			54.0	
Approach LOS		C			B			E			D	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	15.0	88.7	22.5	38.0	10.0	93.7	22.4	38.0
Change Period (Y+Rc), s	4.0	5.5	5.5	* 5.5	4.0	5.5	4.0	5.5
Max Green Setting (Gmax), s	11.0	50.0	17.0	* 33	6.0	55.0	19.0	31.0
Max Q Clear Time (g_c+I1), s	15.9	30.2	21.5	32.2	7.1	21.8	18.4	24.3
Green Ext Time (p_c), s	0.0	3.2	0.0	0.3	0.0	2.8	0.0	0.8

Intersection Summary	
HCM 6th Ctrl Delay	36.5
HCM 6th LOS	D

Notes

User approved pedestrian interval to be less than phase max green.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection	
Int Delay, s/veh	0.5

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	10	1300	1310	0	10	10
Future Vol, veh/h	10	1300	1310	0	10	10
Conflicting Peds, #/hr	5	0	0	5	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	70	-	-	100	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	5	5	0	0	0
Mvmt Flow	11	1368	1379	0	11	11

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	1384	0	-	0	2090	695
Stage 1	-	-	-	-	1384	-
Stage 2	-	-	-	-	706	-
Critical Hdwy	4.1	-	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	2.2	-	-	-	3.5	3.3
Pot Cap-1 Maneuver	501	-	-	-	46	389
Stage 1	-	-	-	-	201	-
Stage 2	-	-	-	-	456	-
Platoon blocked, %			-	-	-	
Mov Cap-1 Maneuver	499	-	-	-	45	387
Mov Cap-2 Maneuver	-	-	-	-	45	-
Stage 1	-	-	-	-	196	-
Stage 2	-	-	-	-	454	-

Approach	EB	WB	SB
HCM Control Delay, s	0.1	0	64.4
HCM LOS			F

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	499	-	-	-	81
HCM Lane V/C Ratio	0.021	-	-	-	0.26
HCM Control Delay (s)	12.4	-	-	-	64.4
HCM Lane LOS	B	-	-	-	F
HCM 95th %tile Q(veh)	0.1	-	-	-	0.9

Intersection	
Int Delay, s/veh	0.5

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	1300	10	10	1300	10	10
Future Vol, veh/h	1300	10	10	1300	10	10
Conflicting Peds, #/hr	0	5	5	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	100	100	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	5	0	0	5	0	0
Mvmt Flow	1368	11	11	1368	11	11

Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	1384	0	2079	689
Stage 1	-	-	-	-	1373	-
Stage 2	-	-	-	-	706	-
Critical Hdwy	-	-	4.1	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	501	-	47	393
Stage 1	-	-	-	-	204	-
Stage 2	-	-	-	-	456	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	499	-	46	391
Mov Cap-2 Maneuver	-	-	-	-	46	-
Stage 1	-	-	-	-	203	-
Stage 2	-	-	-	-	446	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0.1	63.4
HCM LOS			F

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	82	-	-	499	-
HCM Lane V/C Ratio	0.257	-	-	0.021	-
HCM Control Delay (s)	63.4	-	-	12.4	-
HCM Lane LOS	F	-	-	B	-
HCM 95th %tile Q(veh)	0.9	-	-	0.1	-

Intersection	
Int Delay, s/veh	0.7

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	20	1290	1290	10	10	20
Future Vol, veh/h	20	1290	1290	10	10	20
Conflicting Peds, #/hr	5	0	0	5	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	100	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	5	5	0	0	0
Mvmt Flow	21	1358	1358	11	11	21

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	1374	0	-	0	2084	684
Stage 1	-	-	-	-	1363	-
Stage 2	-	-	-	-	721	-
Critical Hdwy	4.1	-	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	2.2	-	-	-	3.5	3.3
Pot Cap-1 Maneuver	506	-	-	-	47	396
Stage 1	-	-	-	-	207	-
Stage 2	-	-	-	-	448	-
Platoon blocked, %			-	-	-	
Mov Cap-1 Maneuver	504	-	-	-	45	394
Mov Cap-2 Maneuver	-	-	-	-	45	-
Stage 1	-	-	-	-	197	-
Stage 2	-	-	-	-	446	-

Approach	EB	WB	SB
HCM Control Delay, s	0.2	0	50.4
HCM LOS			F

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	504	-	-	-	110
HCM Lane V/C Ratio	0.042	-	-	-	0.287
HCM Control Delay (s)	12.5	-	-	-	50.4
HCM Lane LOS	B	-	-	-	F
HCM 95th %tile Q(veh)	0.1	-	-	-	1.1

Intersection	
Int Delay, s/veh	0.5

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	10	1290	1290	0	10	10
Future Vol, veh/h	10	1290	1290	0	10	10
Conflicting Peds, #/hr	5	0	0	5	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	100	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	5	5	0	0	0
Mvmt Flow	11	1358	1358	0	11	11

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	1363	0	-	0	2064	684
Stage 1	-	-	-	-	1363	-
Stage 2	-	-	-	-	701	-
Critical Hdwy	4.1	-	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	2.2	-	-	-	3.5	3.3
Pot Cap-1 Maneuver	511	-	-	-	48	396
Stage 1	-	-	-	-	207	-
Stage 2	-	-	-	-	459	-
Platoon blocked, %			-	-	-	
Mov Cap-1 Maneuver	509	-	-	-	46	394
Mov Cap-2 Maneuver	-	-	-	-	46	-
Stage 1	-	-	-	-	201	-
Stage 2	-	-	-	-	457	-

Approach	EB	WB	SB
HCM Control Delay, s	0.1	0	63.4
HCM LOS			F

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	509	-	-	-	82
HCM Lane V/C Ratio	0.021	-	-	-	0.257
HCM Control Delay (s)	12.2	-	-	-	63.4
HCM Lane LOS	B	-	-	-	F
HCM 95th %tile Q(veh)	0.1	-	-	-	0.9

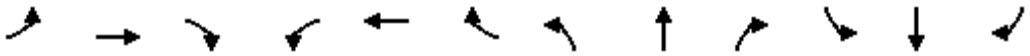
Intersection	
Int Delay, s/veh	0.5

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	1290	10	10	1290	10	10
Future Vol, veh/h	1290	10	10	1290	10	10
Conflicting Peds, #/hr	0	5	5	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	100	125	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	5	0	0	5	0	0
Mvmt Flow	1358	11	11	1358	0	11

Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	1374	0	2064	684
Stage 1	-	-	-	-	1363	-
Stage 2	-	-	-	-	701	-
Critical Hdwy	-	-	4.1	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	506	-	48	396
Stage 1	-	-	-	-	207	-
Stage 2	-	-	-	-	459	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	504	-	47	394
Mov Cap-2 Maneuver	-	-	-	-	47	-
Stage 1	-	-	-	-	206	-
Stage 2	-	-	-	-	449	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0.1	59.5
HCM LOS			F

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	86	-	-	504	-
HCM Lane V/C Ratio	0.239	-	-	0.021	-
HCM Control Delay (s)	59.5	-	-	12.3	-
HCM Lane LOS	F	-	-	B	-
HCM 95th %tile Q(veh)	0.9	-	-	0.1	-



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	70	1110	120	50	1100	50	60	110	60	50	120	140
Future Volume (veh/h)	70	1110	120	50	1100	50	60	110	60	50	120	140
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.98	1.00		1.00	0.99		0.99	0.99		0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1700	1634	1700	1700	1634	1700	1700	1700	1700	1700	1700	1700
Adj Flow Rate, veh/h	74	1168	104	53	1158	33	63	116	43	53	126	105
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	0	5	0	0	5	0	0	0	0	0	0	0
Cap, veh/h	143	1676	760	116	1624	751	322	343	127	391	246	205
Arrive On Green	0.09	0.54	0.54	0.07	0.52	0.52	0.29	0.29	0.25	0.29	0.29	0.29
Sat Flow, veh/h	1619	3104	1408	1619	3104	1435	1161	1179	437	1239	848	706
Grp Volume(v), veh/h	74	1168	104	53	1158	33	63	0	159	53	0	231
Grp Sat Flow(s),veh/h/ln	1619	1552	1408	1619	1552	1435	1161	0	1616	1239	0	1554
Q Serve(g_s), s	2.5	15.6	2.1	1.8	16.0	0.6	2.7	0.0	4.4	2.0	0.0	7.0
Cycle Q Clear(g_c), s	2.5	15.6	2.1	1.8	16.0	0.6	9.7	0.0	4.4	6.4	0.0	7.0
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.27	1.00		0.45
Lane Grp Cap(c), veh/h	143	1676	760	116	1624	751	322	0	470	391	0	452
V/C Ratio(X)	0.52	0.70	0.14	0.46	0.71	0.04	0.20	0.00	0.34	0.14	0.00	0.51
Avail Cap(c_a), veh/h	630	2382	1080	630	2382	1101	818	0	1160	920	0	1116
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	24.5	9.5	6.4	25.1	10.2	6.5	20.7	0.0	16.0	18.2	0.0	16.6
Incr Delay (d2), s/veh	1.1	0.5	0.1	1.0	0.6	0.0	0.4	0.0	0.6	0.2	0.0	1.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.9	3.7	0.5	0.6	3.9	0.1	0.7	0.0	1.6	0.5	0.0	2.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	25.6	10.1	6.5	26.1	10.8	6.6	21.1	0.0	16.6	18.5	0.0	17.9
LnGrp LOS	C	B	A	C	B	A	C	A	B	B	A	B
Approach Vol, veh/h		1346			1244			222			284	
Approach Delay, s/veh		10.7			11.3			17.9			18.0	
Approach LOS		B			B			B			B	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	5.1	32.7		18.5	6.1	31.7		18.5				
Change Period (Y+Rc), s	3.0	5.5		4.5	3.0	5.5		4.5				
Max Green Setting (Gmax), s	20.0	40.0		38.0	20.0	40.0		38.0				
Max Q Clear Time (g_c+I1), s	3.8	17.6		11.7	4.5	18.0		9.0				
Green Ext Time (p_c), s	0.0	8.0		1.7	0.0	8.3		2.4				

Intersection Summary			
HCM 6th Ctrl Delay		12.1	
HCM 6th LOS		B	

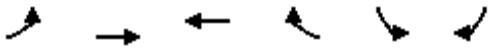
Intersection	
Int Delay, s/veh	0.4

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	1210	10	10	1190	10	10
Future Vol, veh/h	1210	10	10	1190	10	10
Conflicting Peds, #/hr	0	5	5	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	100	140	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	5	0	0	5	0	0
Mvmt Flow	1274	11	11	1253	11	11

Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	1290	0	1928	642
Stage 1	-	-	-	-	1279	-
Stage 2	-	-	-	-	649	-
Critical Hdwy	-	-	4.1	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	544	-	60	422
Stage 1	-	-	-	-	229	-
Stage 2	-	-	-	-	487	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	541	-	59	420
Mov Cap-2 Maneuver	-	-	-	-	59	-
Stage 1	-	-	-	-	228	-
Stage 2	-	-	-	-	477	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0.1	48.7
HCM LOS			E

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	103	-	-	541	-
HCM Lane V/C Ratio	0.204	-	-	0.019	-
HCM Control Delay (s)	48.7	-	-	11.8	-
HCM Lane LOS	E	-	-	B	-
HCM 95th %tile Q(veh)	0.7	-	-	0.1	-



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	70	1150	1160	110	120	40
Future Volume (veh/h)	70	1150	1160	110	120	40
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			0.97	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No	No		No	
Adj Sat Flow, veh/h/ln	1700	1634	1634	1700	1700	1700
Adj Flow Rate, veh/h	74	1211	1221	96	84	87
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	0	5	5	0	0	0
Cap, veh/h	161	2256	1862	842	271	241
Arrive On Green	0.10	0.73	0.60	0.60	0.17	0.17
Sat Flow, veh/h	1619	3186	3186	1404	1619	1441
Grp Volume(v), veh/h	74	1211	1221	96	84	87
Grp Sat Flow(s),veh/h/ln	1619	1552	1552	1404	1619	1441
Q Serve(g_s), s	1.7	7.1	10.5	1.2	1.8	2.2
Cycle Q Clear(g_c), s	1.7	7.1	10.5	1.2	1.8	2.2
Prop In Lane	1.00			1.00	1.00	1.00
Lane Grp Cap(c), veh/h	161	2256	1862	842	271	241
V/C Ratio(X)	0.46	0.54	0.66	0.11	0.31	0.36
Avail Cap(c_a), veh/h	876	3312	3312	1498	1499	1334
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	17.2	2.5	5.3	3.5	14.8	14.9
Incr Delay (d2), s/veh	0.8	0.2	0.4	0.1	0.9	1.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.6	0.1	0.8	0.1	0.6	0.7
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	17.9	2.7	5.7	3.5	15.7	16.2
LnGrp LOS	B	A	A	A	B	B
Approach Vol, veh/h		1285	1317		171	
Approach Delay, s/veh		3.6	5.6		16.0	
Approach LOS		A	A		B	

Timer - Assigned Phs	1	2		4		6
Phs Duration (G+Y+Rc), s	5.1	26.6		8.8		31.7
Change Period (Y+Rc), s	3.0	5.5		4.5		5.5
Max Green Setting (Gmax), s	20.0	40.0		35.0		40.0
Max Q Clear Time (g_c+I1), s	3.7	12.5		4.2		9.1
Green Ext Time (p_c), s	0.0	8.6		0.8		10.2

Intersection Summary	
HCM 6th Ctrl Delay	5.3
HCM 6th LOS	A

Notes

User approved volume balancing among the lanes for turning movement.



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	1050	220	700	1060	210	310
Future Volume (vph)	1050	220	700	1060	210	310
Ideal Flow (vphpl)	1700	1700	1700	1700	1700	1700
Total Lost time (s)	2.5	2.5	1.0	2.5	1.5	1.5
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Frpb, ped/bikes	1.00	0.98	1.00	1.00	1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.85	1.00	1.00	1.00	0.85
Flt Protected	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	3076	1354	1538	3076	1568	1403
Flt Permitted	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (perm)	3076	1354	1538	3076	1568	1403
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	1105	232	737	1116	221	326
RTOR Reduction (vph)	0	39	0	0	0	291
Lane Group Flow (vph)	1105	193	737	1116	221	35
Confl. Peds. (#/hr)		2			1	
Confl. Bikes (#/hr)		1				
Heavy Vehicles (%)	5%	5%	5%	5%	3%	3%
Turn Type	NA	Perm	Prot	NA	Prot	Prot
Protected Phases	2		1	6	4	4
Permitted Phases		2				
Actuated Green, G (s)	46.7	46.7	56.0	108.7	10.5	10.5
Effective Green, g (s)	51.7	51.7	61.0	113.7	15.5	15.5
Actuated g/C Ratio	0.36	0.36	0.42	0.78	0.11	0.11
Clearance Time (s)	7.5	7.5	6.0	7.5	6.5	6.5
Vehicle Extension (s)	3.0	3.0	3.0	3.0	2.0	2.0
Lane Grp Cap (vph)	1096	482	647	2412	167	149
v/s Ratio Prot	c0.36		c0.48	0.36	c0.14	0.02
v/s Ratio Perm		0.14				
v/c Ratio	1.01	0.40	1.14	0.46	1.32	0.23
Uniform Delay, d1	46.6	35.0	42.0	5.3	64.8	59.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	29.2	2.5	80.3	0.6	181.0	0.3
Delay (s)	75.9	37.5	122.3	5.9	245.7	59.6
Level of Service	E	D	F	A	F	E
Approach Delay (s)	69.2			52.2	134.8	
Approach LOS	E			D	F	

Intersection Summary			
HCM 2000 Control Delay	70.4	HCM 2000 Level of Service	E
HCM 2000 Volume to Capacity ratio	1.03		
Actuated Cycle Length (s)	145.0	Sum of lost time (s)	10.0
Intersection Capacity Utilization	98.8%	ICU Level of Service	F
Analysis Period (min)	15		
c Critical Lane Group			

Intersection	
Int Delay, s/veh	20.3

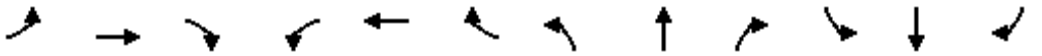
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	70	1250	40	10	1640	100	10	10	30	30	0	110
Future Vol, veh/h	70	1250	40	10	1640	100	10	10	30	30	0	110
Conflicting Peds, #/hr	5	0	5	5	0	5	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	115	-	100	140	-	100	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	5	5	0	0	5	5	0	0	0	3	3	3
Mvmt Flow	74	1316	42	11	1726	105	11	11	32	32	0	116

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	1836	0	0	1363	0	0	2354	3327	663	2565	3264	868
Stage 1	-	-	-	-	-	-	1469	1469	-	1753	1753	-
Stage 2	-	-	-	-	-	-	885	1858	-	812	1511	-
Critical Hdwy	4.2	-	-	4.1	-	-	7.5	6.5	6.9	7.56	6.56	6.96
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.56	5.56	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.56	5.56	-
Follow-up Hdwy	2.25	-	-	2.2	-	-	3.5	4	3.3	3.53	4.03	3.33
Pot Cap-1 Maneuver	316	-	-	511	-	-	19	~ 8	409	~ 13	9	294
Stage 1	-	-	-	-	-	-	136	194	-	88	136	-
Stage 2	-	-	-	-	-	-	310	125	-	337	180	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	314	-	-	509	-	-	~ 9	~ 6	407	-	7	293
Mov Cap-2 Maneuver	-	-	-	-	-	-	~ 9	~ 6	-	-	7	-
Stage 1	-	-	-	-	-	-	103	147	-	67	132	-
Stage 2	-	-	-	-	-	-	183	122	-	221	137	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	1	0.1	$ 1309.1	
HCM LOS			F	-

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	18	314	-	-	509	-	-	-
HCM Lane V/C Ratio	2.924	0.235	-	-	0.021	-	-	-
HCM Control Delay (s)	$ 1309.1	19.9	-	-	12.2	-	-	-
HCM Lane LOS	F	C	-	-	B	-	-	-
HCM 95th %tile Q(veh)	7.1	0.9	-	-	0.1	-	-	-

Notes			
~: Volume exceeds capacity	$: Delay exceeds 300s	+: Computation Not Defined	*: All major volume in platoon



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	400	810	100	20	560	500	150	190	40	1300	480	1040
Future Volume (veh/h)	400	810	100	20	560	500	150	190	40	1300	480	1040
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		1.00	1.00		0.98	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1634	1634	1634	1660	1660	1660	1673	1673	1673	1673	1673	1673
Adj Flow Rate, veh/h	412	835	96	21	577	0	155	196	23	1366	459	1013
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	5	5	5	3	3	3	2	2	2	2	2	2
Cap, veh/h	596	994	114	106	724		167	301	35	2003	701	880
Arrive On Green	0.20	0.35	0.31	0.07	0.23	0.00	0.10	0.10	0.05	0.42	0.42	0.42
Sat Flow, veh/h	3018	2800	322	1581	3154	1407	1594	2865	332	4781	1673	1412
Grp Volume(v), veh/h	412	463	468	21	577	0	155	108	111	1366	459	1013
Grp Sat Flow(s),veh/h/ln	1509	1552	1570	1581	1577	1407	1594	1590	1607	1594	1673	1412
Q Serve(g_s), s	12.7	27.4	27.5	1.3	17.2	0.0	9.6	6.5	6.7	23.2	22.0	41.9
Cycle Q Clear(g_c), s	12.7	27.4	27.5	1.3	17.2	0.0	9.6	6.5	6.7	23.2	22.0	41.9
Prop In Lane	1.00		0.21	1.00		1.00	1.00		0.21	1.00		1.00
Lane Grp Cap(c), veh/h	596	551	557	106	724		167	167	169	2003	701	880
V/C Ratio(X)	0.69	0.84	0.84	0.20	0.80		0.93	0.64	0.66	0.68	0.65	1.15
Avail Cap(c_a), veh/h	860	594	601	150	724		167	167	169	2003	701	880
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	37.3	29.6	30.1	44.1	36.3	0.0	44.4	43.0	43.5	23.6	23.3	18.9
Incr Delay (d2), s/veh	0.5	9.0	9.0	0.3	5.7	0.0	47.8	6.5	7.4	1.9	4.7	81.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	4.5	10.7	11.0	0.5	6.8	0.0	5.9	2.8	3.0	8.4	8.9	36.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	37.8	38.7	39.1	44.4	42.1	0.0	92.2	49.5	50.9	25.5	28.0	99.9
LnGrp LOS	D	D	D	D	D		F	D	D	C	C	F
Approach Vol, veh/h		1343			598	A		374			2838	
Approach Delay, s/veh		38.6			42.2			67.6			52.5	
Approach LOS		D			D			E			D	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	21.2	24.0		43.8	7.7	37.5		11.0				
Change Period (Y+Rc), s	5.0	5.5		6.0	5.5	* 6.5		5.5				
Max Green Setting (Gmax), s	25.0	14.8		32.7	5.0	* 34		5.5				
Max Q Clear Time (g_c+I1), s	15.7	19.2		43.9	3.3	29.5		11.6				
Green Ext Time (p_c), s	0.6	0.0		0.0	0.0	1.5		0.0				

Intersection Summary			
HCM 6th Ctrl Delay		48.7	
HCM 6th LOS		D	

Notes

User approved pedestrian interval to be less than phase max green.

User approved volume balancing among the lanes for turning movement.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	150	10	110	10	10	10	70	1010	10	10	2700	230
Future Volume (veh/h)	150	10	110	10	10	10	70	1010	10	10	2700	230
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.98	1.00		1.00	1.00		1.00	1.00		0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1700	1700	1700	1700	1700	1700	1700	1673	1700	1700	1673	1700
Adj Flow Rate, veh/h	153	10	69	10	10	3	71	1031	10	10	2755	233
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	0	0	0	0	0	0	0	2	0	0	2	0
Cap, veh/h	168	8	348	30	24	4	111	3359	33	74	2087	174
Arrive On Green	0.15	0.18	0.18	0.15	0.18	0.18	0.07	0.72	0.73	0.05	0.70	0.70
Sat Flow, veh/h	729	48	1408	0	139	21	1619	4666	45	1619	2965	247
Grp Volume(v), veh/h	163	0	69	23	0	0	71	673	368	10	1456	1532
Grp Sat Flow(s),veh/h/ln	776	0	1408	160	0	0	1619	1523	1665	1619	1590	1622
Q Serve(g_s), s	0.0	0.0	6.8	0.0	0.0	0.0	7.5	13.9	13.9	1.0	123.2	123.2
Cycle Q Clear(g_c), s	27.0	0.0	6.8	27.0	0.0	0.0	7.5	13.9	13.9	1.0	123.2	123.2
Prop In Lane	0.94		1.00	0.43		0.13	1.00		0.03	1.00		0.15
Lane Grp Cap(c), veh/h	160	0	348	54	0	0	111	2193	1199	74	1119	1142
V/C Ratio(X)	1.02	0.00	0.20	0.42	0.00	0.00	0.64	0.31	0.31	0.14	1.30	1.34
Avail Cap(c_a), veh/h	160	0	348	54	0	0	111	2193	1199	83	1119	1142
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	75.7	0.0	52.3	62.8	0.0	0.0	79.4	8.8	8.8	80.2	25.9	25.9
Incr Delay (d2), s/veh	76.9	0.0	0.3	5.2	0.0	0.0	9.2	0.4	0.7	0.3	141.9	159.9
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	10.8	0.0	2.5	1.0	0.0	0.0	3.4	4.4	5.0	0.4	85.1	92.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	152.5	0.0	52.6	68.0	0.0	0.0	88.6	9.2	9.5	80.5	167.8	185.8
LnGrp LOS	F	A	D	E	A	A	F	A	A	F	F	F
Approach Vol, veh/h		232			23			1112			2998	
Approach Delay, s/veh		122.8			68.0			14.3			176.7	
Approach LOS		F			E			B			F	

Timer - Assigned Phs	1	2		4	5	6		8
Phs Duration (G+Y+Rc), s	40.0	133.0		32.0	13.0	130.0		32.0
Change Period (Y+Rc), s	6.0	* 6		5.0	5.0	6.0		5.0
Max Green Setting (Gmax), s	5.0	* 1.3E2		27.0	8.0	124.0		27.0
Max Q Clear Time (g_c+I1), s	13.0	15.9		29.0	9.5	125.2		29.0
Green Ext Time (p_c), s	0.0	2.1		0.0	0.0	0.0		0.0

Intersection Summary	
HCM 6th Ctrl Delay	131.9
HCM 6th LOS	F

Notes

User approved pedestrian interval to be less than phase max green.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	1170	150	800	370	60	1770
Future Volume (veh/h)	1170	150	800	370	60	1770
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1673	1673	1673	1673	1673	1673
Adj Flow Rate, veh/h	1194	102	816	325	61	1806
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	1187	689	1498	2167	142	1800
Arrive On Green	0.38	0.40	0.47	0.47	0.09	0.57
Sat Flow, veh/h	3092	1418	3263	2496	1594	3263
Grp Volume(v), veh/h	1194	102	816	325	61	1806
Grp Sat Flow(s),veh/h/ln	1546	1418	1590	1248	1594	1590
Q Serve(g_s), s	38.4	4.0	18.3	2.0	3.6	56.6
Cycle Q Clear(g_c), s	38.4	4.0	18.3	2.0	3.6	56.6
Prop In Lane	1.00	1.00		1.00	1.00	
Lane Grp Cap(c), veh/h	1187	689	1498	2167	142	1800
V/C Ratio(X)	1.01	0.15	0.54	0.15	0.43	1.00
Avail Cap(c_a), veh/h	1187	689	1498	2167	159	1800
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	30.8	14.2	18.8	1.0	43.1	21.7
Incr Delay (d2), s/veh	27.5	0.0	1.4	0.1	0.8	22.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	17.8	4.3	6.4	2.1	1.4	23.1
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	58.3	14.3	20.2	1.1	43.9	43.8
LnGrp LOS	F	B	C	A	D	F
Approach Vol, veh/h	1296		1141			1867
Approach Delay, s/veh	54.8		14.8			43.8
Approach LOS	D		B			D

Timer - Assigned Phs	1	2		4		6
Phs Duration (G+Y+Rc), s	9.9	49.1		41.0		59.0
Change Period (Y+Rc), s	5.0	6.0		5.3		6.0
Max Green Setting (Gmax), s	6.3	42.0		35.7		53.0
Max Q Clear Time (g_c+I1), s	5.6	20.3		40.4		58.6
Green Ext Time (p_c), s	0.0	4.2		0.0		0.0

Intersection Summary	
HCM 6th Ctrl Delay	39.4
HCM 6th LOS	D

Intersection	
Int Delay, s/veh	16.7

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	40	60	30	920	1770	60
Future Vol, veh/h	40	60	30	920	1770	60
Conflicting Peds, #/hr	0	0	5	0	0	5
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	200	-	-	100
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	98	98	98	98	98	98
Heavy Vehicles, %	0	0	0	2	2	0
Mvmt Flow	41	61	31	939	1806	61

Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	2343	908	1872	0	-	0
Stage 1	1811	-	-	-	-	-
Stage 2	532	-	-	-	-	-
Critical Hdwy	6.8	6.9	4.1	-	-	-
Critical Hdwy Stg 1	5.8	-	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	~ 31	282	326	-	-	-
Stage 1	118	-	-	-	-	-
Stage 2	559	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	~ 28	281	324	-	-	-
Mov Cap-2 Maneuver	~ 28	-	-	-	-	-
Stage 1	106	-	-	-	-	-
Stage 2	556	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	$ 474.9	0.5	0
HCM LOS	F		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	324	-	61	-	-
HCM Lane V/C Ratio	0.094	-	1.673	-	-
HCM Control Delay (s)	17.3	-	$ 474.9	-	-
HCM Lane LOS	C	-	F	-	-
HCM 95th %tile Q(veh)	0.3	-	9.3	-	-

Notes			
~: Volume exceeds capacity	$: Delay exceeds 300s	+: Computation Not Defined	*: All major volume in platoon

Intersection	
Int Delay, s/veh	0

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	0	0	1090	0	0	2820
Future Vol, veh/h	0	0	1090	0	0	2820
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	150	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	1	0	0	1
Mvmt Flow	0	0	1185	0	0	3065

Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	2718	593	0	0	1185	0
Stage 1	1185	-	-	-	-	-
Stage 2	1533	-	-	-	-	-
Critical Hdwy	6.25	7.1	-	-	5.3	-
Critical Hdwy Stg 1	6.6	-	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-	-
Follow-up Hdwy	3.65	3.9	-	-	3.1	-
Pot Cap-1 Maneuver	26	388	-	-	324	-
Stage 1	193	-	-	-	-	-
Stage 2	164	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	26	388	-	-	324	-
Mov Cap-2 Maneuver	26	-	-	-	-	-
Stage 1	193	-	-	-	-	-
Stage 2	164	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	0	0	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	-	324	-
HCM Lane V/C Ratio	-	-	-	-	-
HCM Control Delay (s)	-	-	0	0	-
HCM Lane LOS	-	-	A	A	-
HCM 95th %tile Q(veh)	-	-	-	0	-

Intersection	
Int Delay, s/veh	0

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	0	0	1170	2940	0
Future Vol, veh/h	0	0	0	1170	2940	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	150	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	1	1	0
Mvmt Flow	0	0	0	1272	3196	0

Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	3705	1598	3196	0	-	0
Stage 1	3196	-	-	-	-	-
Stage 2	509	-	-	-	-	-
Critical Hdwy	6.25	6.9	4.1	-	-	-
Critical Hdwy Stg 1	5.8	-	-	-	-	-
Critical Hdwy Stg 2	6	-	-	-	-	-
Follow-up Hdwy	3.65	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	6	97	98	-	-	-
Stage 1	19	-	-	-	-	-
Stage 2	541	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	6	97	98	-	-	-
Mov Cap-2 Maneuver	6	-	-	-	-	-
Stage 1	19	-	-	-	-	-
Stage 2	541	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	0	0	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	98	-	-	-	-
HCM Lane V/C Ratio	-	-	-	-	-
HCM Control Delay (s)	0	-	0	-	-
HCM Lane LOS	A	-	A	-	-
HCM 95th %tile Q(veh)	0	-	-	-	-

Intersection	
Int Delay, s/veh	0

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	0	0	950	0	0	1830
Future Vol, veh/h	0	0	950	0	0	1830
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	125	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	2	0	0	2
Mvmt Flow	0	0	1033	0	0	1989

Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	-	517	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.9	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	509	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	509	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	0	0	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBT
Capacity (veh/h)	-	-	-	-
HCM Lane V/C Ratio	-	-	-	-
HCM Control Delay (s)	-	-	0	-
HCM Lane LOS	-	-	A	-
HCM 95th %tile Q(veh)	-	-	-	-

MPAH Amendment Study - Villa Park Rd

Vistro File: N:\...\Villa Park_v6.vistro — Scenario 3 Future AM Peak Hour

Report File: N:\...\03_FYNoImprovementAM.pdf — 7/21/2023

Intersection Analysis Summary

ID	Intersection Name	Control Type	Method	Worst Mvmt	V/C	Delay (s/veh)	LOS
1	Villa Park Rd & Wanda Rd	Signalized	ICU 2	WB Thru	0.836	-	D
7	Villa Park Rd & Center Dr	Signalized	ICU 2	EB Thru	0.585	-	A
9	Villa Park Rd & Lemon St	Signalized	ICU 2	WB Thru	0.450	-	A
10	Villa Park Rd & Hewes St	Signalized	ICU 2	WB Left	0.955	-	E
12	Santiago Canyon Rd & Cannon St	Signalized	ICU 2	SB Right	0.914	-	E
13	Taft Ave & Cannon St	Signalized	ICU 2	SB Thru	1.064	-	F
14	Serrano Ave & Cannon St	Signalized	ICU 2	SB Thru	0.910	-	E

V/C, Delay, LOS: For two-way stop, these values are taken from the movement with the worst (highest) delay value. For all other control types, they are taken for the whole intersection.

Intersection Level Of Service Report
Intersection 1: Villa Park Rd & Wanda Rd

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	D
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.836

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	0	2	0	1	1	0	1
Entry Pocket Length [ft]	100.00	100.00	100.00	175.00	100.00	100.00	225.00	100.00	300.00	150.00	100.00	175.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			Yes			Yes			Yes		

Volumes

Name												
Base Volume Input [veh/h]	230	460	130	200	390	380	310	980	310	60	1140	160
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	230	460	130	200	390	380	310	980	310	60	1140	160
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	58	115	33	50	98	95	78	245	78	15	285	40
Total Analysis Volume [veh/h]	230	460	130	200	390	380	310	980	310	60	1140	160
Pedestrian Volume [ped/h]	3			0			3			10		
Bicycle Volume [bicycles/h]	0			0			2			8		

Intersection Settings

Cycle Length [s]	130
Lost time [s]	7.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Overlap	Protecte	Permiss	Overlap	Protecte	Permiss	Overlap
Signal Group	3	8	0	7	4	4	1	6	6	5	2	2
Auxiliary Signal Groups						1,4			3,6			2,7
Lead / Lag	Lead	-	-	Lead	-	-	Lag	-	-	Lead	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.14	0.17	0.17	0.12	0.23	0.13	0.09	0.29	0.05	0.04	0.34	0.00
Intersection LOS	D											
Intersection V/C	0.836											

Intersection Level Of Service Report
Intersection 7: Villa Park Rd & Center Dr

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	A
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.585

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	0	1	0	1	1	0	1
Entry Pocket Length [ft]	150.00	100.00	100.00	140.00	100.00	100.00	160.00	100.00	100.00	150.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			Yes			Yes			Yes		

Volumes

Name												
Base Volume Input [veh/h]	60	110	60	50	120	140	70	1110	120	50	1100	50
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	60	110	60	50	120	140	70	1110	120	50	1100	50
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	15	28	15	13	30	35	18	278	30	13	275	13
Total Analysis Volume [veh/h]	60	110	60	50	120	140	70	1110	120	50	1100	50
Pedestrian Volume [ped/h]	1			4			9			1		
Bicycle Volume [bicycles/h]	3			0			1			0		

Intersection Settings

Cycle Length [s]	111
Lost time [s]	6.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	0	4	0	0	4	0	5	2	0	1	6	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	Lead	-	-	Lead	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.04	0.10	0.10	0.03	0.15	0.15	0.04	0.33	0.07	0.03	0.32	0.03
Intersection LOS	A											
Intersection V/C	0.585											

Intersection Level Of Service Report
Intersection 9: Villa Park Rd & Lemon St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	A
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.450

Intersection Setup

Name						
Approach	Southbound		Eastbound		Westbound	
Lane Configuration						
Turning Movement	Left	Right	Left	Thru	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	1	0	0	1
Entry Pocket Length [ft]	115.00	100.00	180.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00		30.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	Yes		Yes		No	

Volumes

Name						
Base Volume Input [veh/h]	120	40	70	1150	1160	110
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	120	40	70	1150	1160	110
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	30	10	18	288	290	28
Total Analysis Volume [veh/h]	120	40	70	1150	1160	110
Pedestrian Volume [ped/h]	5		5		0	
Bicycle Volume [bicycles/h]	2		2		0	

Intersection Settings

Cycle Length [s]	108
Lost time [s]	5.00

Phasing & Timing

Control Type	Permissive	Permissive	Protected	Permissive	Permissive	Permissive
Signal Group	4	0	1	6	2	0
Auxiliary Signal Groups						
Lead / Lag	Lag	-	Lead	-	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.04	0.05	0.04	0.34	0.34	0.06
Intersection LOS	A					
Intersection V/C	0.450					

Intersection Level Of Service Report
Intersection 10: Villa Park Rd & Hewes St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	E
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.955

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0	0	0	1	1	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	215.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			No			Yes			No		

Volumes

Name												
Base Volume Input [veh/h]	210	0	310	0	1	0	0	1050	220	700	1060	0
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	210	0	310	0	1	0	0	1050	220	700	1060	0
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	53	0	78	0	0	0	0	263	55	175	265	0
Total Analysis Volume [veh/h]	210	0	310	0	1	0	0	1050	220	700	1060	0
Pedestrian Volume [ped/h]	2			0			1			0		
Bicycle Volume [bicycles/h]	1			0			0			0		

Intersection Settings

Cycle Length [s]	130
Lost time [s]	7.00

Phasing & Timing

Control Type	Split	Permiss	Split	Split	Split	Split	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	4	0	0	0	8	0	0	2	0	1	6	0
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	-	-	-	-	-	-	Lead	-	-

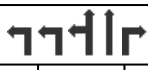
Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.12	0.00	0.18	0.00	0.00	0.00	0.00	0.31	0.13	0.41	0.31	0.00
Intersection LOS	E											
Intersection V/C	0.955											

Intersection Level Of Service Report
Intersection 12: Santiago Canyon Rd & Cannon St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	E
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.914

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	2	0	1	2	0	0	1	0	1
Entry Pocket Length [ft]	80.00	100.00	100.00	375.00	100.00	250.00	325.00	100.00	100.00	150.00	100.00	450.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			No			Yes			No		

Volumes

Name												
Base Volume Input [veh/h]	150	190	40	1300	480	1040	400	810	100	20	560	500
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	150	190	40	1300	480	1040	400	810	100	20	560	500
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	38	48	10	325	120	260	100	203	25	5	140	125
Total Analysis Volume [veh/h]	150	190	40	1300	480	1040	400	810	100	20	560	500
Pedestrian Volume [ped/h]	0			0			6			0		
Bicycle Volume [bicycles/h]	3			0			0			1		

Intersection Settings

Cycle Length [s]	130
Lost time [s]	7.00

Phasing & Timing

Control Type	Split	Split	Split	Split	Split	Overlap	Protecte	Permiss	Permiss	Protecte	Permiss	Unsigna
Signal Group	0	8	0	0	4	4	1	6	0	5	2	0
Auxiliary Signal Groups						1,4						
Lead / Lag	-	-	-	-	-	-	Lead	-	-	Lag	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.09	0.07	0.07	0.25	0.26	0.49	0.12	0.27	0.27	0.01	0.16	0.00
Intersection LOS	E											
Intersection V/C	0.914											

Intersection Level Of Service Report
Intersection 13: Taft Ave & Cannon St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	F
Analysis Period:	1 hour	Volume to Capacity (v/c):	1.064

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	0	0	0	1	0	0	0
Entry Pocket Length [ft]	225.00	100.00	100.00	150.00	100.00	100.00	100.00	100.00	200.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			No			Yes			Yes		

Volumes

Name												
Base Volume Input [veh/h]	70	1010	10	10	2700	230	150	10	110	10	10	10
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	70	1010	10	10	2700	230	150	10	110	10	10	10
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	18	253	3	3	675	58	38	3	28	3	3	3
Total Analysis Volume [veh/h]	70	1010	10	10	2700	230	150	10	110	10	10	10
Pedestrian Volume [ped/h]	5			0			5			0		
Bicycle Volume [bicycles/h]	2			0			2			0		

Intersection Settings

Cycle Length [s]	175
Lost time [s]	9.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Permiss	Permiss	Overlap	Permiss	Permiss	Permiss
Signal Group	5	2	0	1	6	0	0	4	4	0	4	0
Auxiliary Signal Groups									4,5			
Lead / Lag	Lead	-	-	Lag	-	-	-	-	-	-	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.04	0.20	0.20	0.01	0.86	0.86	0.09	0.09	0.02	0.01	0.02	0.02
Intersection LOS	F											
Intersection V/C	1.064											

Intersection Level Of Service Report
Intersection 14: Serrano Ave & Cannon St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	E
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.910

Intersection Setup

Name						
Approach	Northbound		Southbound		Westbound	
Lane Configuration						
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	1	1	0	1	0
Entry Pocket Length [ft]	100.00	150.00	275.00	100.00	350.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00		30.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	No		Yes		Yes	

Volumes

Name						
Base Volume Input [veh/h]	800	370	60	1770	1170	150
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	800	370	60	1770	1170	150
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	200	93	15	443	293	38
Total Analysis Volume [veh/h]	800	370	60	1770	1170	150
Pedestrian Volume [ped/h]	0		0		0	
Bicycle Volume [bicycles/h]	0		0		0	

Intersection Settings

Cycle Length [s]	100
Lost time [s]	5.00

Phasing & Timing

Control Type	Permissive	Overlap	Protected	Permissive	Split	Overlap
Signal Group	2	2	1	6	4	4
Auxiliary Signal Groups		2,4				1,4
Lead / Lag	-	-	Lead	-	Lag	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.24	0.00	0.04	0.52	0.34	0.05
Intersection LOS	E					
Intersection V/C	0.910					

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	430	1080	250	60	1010	330	150	540	60	150	380	290
Future Volume (veh/h)	430	1080	250	60	1010	330	150	540	60	150	380	290
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		1.00	1.00		0.98	1.00		0.99
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1673	1673	1673	1673	1673	1673	1673	1673	1673	1673	1673	1673
Adj Flow Rate, veh/h	448	1125	180	62	1052	298	156	562	57	156	396	263
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	556	1467	824	146	1186	713	206	785	79	204	450	639
Arrive On Green	0.18	0.46	0.46	0.09	0.37	0.37	0.13	0.27	0.24	0.13	0.27	0.27
Sat Flow, veh/h	3092	3180	1400	1594	3180	1418	1594	2908	294	1594	1673	1400
Grp Volume(v), veh/h	448	1125	180	62	1052	298	156	307	312	156	396	263
Grp Sat Flow(s),veh/h/ln	1546	1590	1400	1594	1590	1418	1594	1590	1612	1594	1673	1400
Q Serve(g_s), s	19.5	41.3	5.5	5.1	43.4	9.7	13.2	24.4	24.6	13.2	31.7	3.6
Cycle Q Clear(g_c), s	19.5	41.3	5.5	5.1	43.4	9.7	13.2	24.4	24.6	13.2	31.7	3.6
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.18	1.00		1.00
Lane Grp Cap(c), veh/h	556	1467	824	146	1186	713	206	429	435	204	450	639
V/C Ratio(X)	0.81	0.77	0.22	0.43	0.89	0.42	0.76	0.71	0.72	0.77	0.88	0.41
Avail Cap(c_a), veh/h	556	1467	824	146	1186	713	206	449	455	204	470	656
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	55.1	31.4	6.2	60.1	41.1	8.6	58.8	46.2	46.6	59.0	49.1	12.6
Incr Delay (d2), s/veh	7.9	3.9	0.6	0.7	10.0	1.8	13.4	4.2	4.3	14.4	16.2	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	8.1	16.1	2.1	2.1	18.2	2.9	6.0	10.0	10.3	6.1	15.2	3.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	62.9	35.3	6.8	60.9	51.1	10.4	72.2	50.4	50.9	73.5	65.2	12.7
LnGrp LOS	E	D	A	E	D	B	E	D	D	E	E	B
Approach Vol, veh/h		1753			1412			775			815	
Approach Delay, s/veh		39.4			43.0			55.0			49.9	
Approach LOS		D			D			E			D	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	26.7	54.5	19.0	39.8	13.7	67.5	19.0	39.8
Change Period (Y+Rc), s	4.0	5.5	4.0	5.5	4.0	5.5	4.0	5.5
Max Green Setting (Gmax), s	21.0	49.0	15.0	36.0	8.0	62.0	15.0	36.0
Max Q Clear Time (g_c+I1), s	21.5	45.4	15.2	33.7	7.1	43.3	15.2	26.6
Green Ext Time (p_c), s	0.0	1.3	0.0	0.6	0.0	2.9	0.0	0.9

Intersection Summary	
HCM 6th Ctrl Delay	44.8
HCM 6th LOS	D

Intersection	
Int Delay, s/veh	0.1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	10	1280	1420	0	0	10
Future Vol, veh/h	10	1280	1420	0	0	10
Conflicting Peds, #/hr	1	0	0	1	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	70	-	-	100	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	2	2	0	0	0
Mvmt Flow	11	1347	1495	0	0	11

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	1496	0	-	0	2192	749
Stage 1	-	-	-	-	1496	-
Stage 2	-	-	-	-	696	-
Critical Hdwy	4.1	-	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	2.2	-	-	-	3.5	3.3
Pot Cap-1 Maneuver	454	-	-	-	40	359
Stage 1	-	-	-	-	175	-
Stage 2	-	-	-	-	461	-
Platoon blocked, %			-	-	-	
Mov Cap-1 Maneuver	454	-	-	-	39	359
Mov Cap-2 Maneuver	-	-	-	-	39	-
Stage 1	-	-	-	-	171	-
Stage 2	-	-	-	-	461	-

Approach	EB	WB	SB
HCM Control Delay, s	0.1	0	15.3
HCM LOS			C

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	454	-	-	-	359
HCM Lane V/C Ratio	0.023	-	-	-	0.029
HCM Control Delay (s)	13.1	-	-	-	15.3
HCM Lane LOS	B	-	-	-	C
HCM 95th %tile Q(veh)	0.1	-	-	-	0.1

Intersection	
Int Delay, s/veh	0.5

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	1270	10	10	1410	10	10
Future Vol, veh/h	1270	10	10	1410	10	10
Conflicting Peds, #/hr	0	1	1	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	100	100	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	0	0	2	0	0
Mvmt Flow	1337	11	11	1484	11	11

Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	1349	0	2102	670
Stage 1	-	-	-	-	1338	-
Stage 2	-	-	-	-	764	-
Critical Hdwy	-	-	4.1	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	517	-	46	404
Stage 1	-	-	-	-	213	-
Stage 2	-	-	-	-	426	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	517	-	45	404
Mov Cap-2 Maneuver	-	-	-	-	45	-
Stage 1	-	-	-	-	213	-
Stage 2	-	-	-	-	417	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0.1	64.4
HCM LOS			F

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	81	-	-	517	-
HCM Lane V/C Ratio	0.26	-	-	0.02	-
HCM Control Delay (s)	64.4	-	-	12.1	-
HCM Lane LOS	F	-	-	B	-
HCM 95th %tile Q(veh)	0.9	-	-	0.1	-

Intersection	
Int Delay, s/veh	0.7

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	20	1260	1410	10	10	10
Future Vol, veh/h	20	1260	1410	10	10	10
Conflicting Peds, #/hr	1	0	0	1	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	100	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	2	2	0	0	0
Mvmt Flow	21	1326	1484	11	11	11

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	1496	0	-	0	2190	743
Stage 1	-	-	-	-	1485	-
Stage 2	-	-	-	-	705	-
Critical Hdwy	4.1	-	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	2.2	-	-	-	3.5	3.3
Pot Cap-1 Maneuver	454	-	-	-	40	362
Stage 1	-	-	-	-	178	-
Stage 2	-	-	-	-	456	-
Platoon blocked, %			-	-	-	
Mov Cap-1 Maneuver	454	-	-	-	38	362
Mov Cap-2 Maneuver	-	-	-	-	38	-
Stage 1	-	-	-	-	170	-
Stage 2	-	-	-	-	456	-

Approach	EB	WB	SB
HCM Control Delay, s	0.2	0	78.6
HCM LOS			F

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	454	-	-	-	69
HCM Lane V/C Ratio	0.046	-	-	-	0.305
HCM Control Delay (s)	13.3	-	-	-	78.6
HCM Lane LOS	B	-	-	-	F
HCM 95th %tile Q(veh)	0.1	-	-	-	1.1

Intersection	
Int Delay, s/veh	0.1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	10	1260	1410	10	0	10
Future Vol, veh/h	10	1260	1410	10	0	10
Conflicting Peds, #/hr	1	0	0	1	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	150	-	-	100	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	2	2	0	0	0
Mvmt Flow	11	1326	1484	11	0	11

Major/Minor	Major1		Major2		Minor2	
Conflicting Flow All	1496	0	-	0	2170	743
Stage 1	-	-	-	-	1485	-
Stage 2	-	-	-	-	685	-
Critical Hdwy	4.1	-	-	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	2.2	-	-	-	3.5	3.3
Pot Cap-1 Maneuver	454	-	-	-	41	362
Stage 1	-	-	-	-	178	-
Stage 2	-	-	-	-	467	-
Platoon blocked, %			-	-	-	
Mov Cap-1 Maneuver	454	-	-	-	40	362
Mov Cap-2 Maneuver	-	-	-	-	40	-
Stage 1	-	-	-	-	174	-
Stage 2	-	-	-	-	467	-

Approach	EB	WB	SB
HCM Control Delay, s	0.1	0	15.2
HCM LOS			C

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	454	-	-	-	362
HCM Lane V/C Ratio	0.023	-	-	-	0.029
HCM Control Delay (s)	13.1	-	-	-	15.2
HCM Lane LOS	B	-	-	-	C
HCM 95th %tile Q(veh)	0.1	-	-	-	0.1

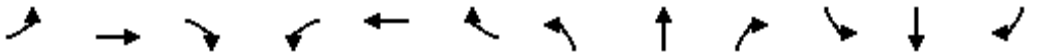
Intersection	
Int Delay, s/veh	0.4

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	1250	10	10	1410	10	0
Future Vol, veh/h	1250	10	10	1410	10	0
Conflicting Peds, #/hr	0	1	1	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	100	125	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	0	0	2	0	0
Mvmt Flow	1316	11	11	1484	11	0

Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	1328	0	2081	659
Stage 1	-	-	-	-	1317	-
Stage 2	-	-	-	-	764	-
Critical Hdwy	-	-	4.1	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	527	-	47	411
Stage 1	-	-	-	-	219	-
Stage 2	-	-	-	-	426	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	526	-	46	411
Mov Cap-2 Maneuver	-	-	-	-	46	-
Stage 1	-	-	-	-	219	-
Stage 2	-	-	-	-	417	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0.1	105.1
HCM LOS			F

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	46	-	-	526	-
HCM Lane V/C Ratio	0.229	-	-	0.02	-
HCM Control Delay (s)	105.1	-	-	12	-
HCM Lane LOS	F	-	-	B	-
HCM 95th %tile Q(veh)	0.8	-	-	0.1	-



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	90	1140	20	20	1320	90	20	40	20	50	30	80
Future Volume (veh/h)	90	1140	20	20	1320	90	20	40	20	50	30	80
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.98	1.00		1.00	1.00		1.00	1.00		0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1700	1673	1700	1700	1673	1700	1700	1700	1700	1700	1700	1700
Adj Flow Rate, veh/h	95	1200	16	21	1389	75	21	42	3	53	32	13
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	0	2	0	0	2	0	0	0	0	0	0	0
Cap, veh/h	180	2145	951	93	1973	893	328	230	16	329	168	68
Arrive On Green	0.11	0.67	0.67	0.06	0.62	0.62	0.15	0.15	0.09	0.15	0.15	0.15
Sat Flow, veh/h	1619	3180	1409	1619	3180	1440	1383	1568	112	1383	1142	464
Grp Volume(v), veh/h	95	1200	16	21	1389	75	21	0	45	53	0	45
Grp Sat Flow(s),veh/h/ln	1619	1590	1409	1619	1590	1440	1383	0	1680	1383	0	1606
Q Serve(g_s), s	2.5	8.9	0.2	0.6	13.3	0.9	0.6	0.0	1.1	1.6	0.0	1.1
Cycle Q Clear(g_c), s	2.5	8.9	0.2	0.6	13.3	0.9	1.7	0.0	1.1	2.7	0.0	1.1
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.07	1.00		0.29
Lane Grp Cap(c), veh/h	180	2145	951	93	1973	893	328	0	246	329	0	236
V/C Ratio(X)	0.53	0.56	0.02	0.23	0.70	0.08	0.06	0.00	0.18	0.16	0.00	0.19
Avail Cap(c_a), veh/h	211	2749	1218	175	2678	1213	1052	0	1126	1053	0	1076
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00
Uniform Delay (d), s/veh	19.0	3.9	2.4	20.4	5.8	3.4	17.7	0.0	17.0	18.1	0.0	17.0
Incr Delay (d2), s/veh	0.9	0.2	0.0	0.5	0.5	0.0	0.1	0.0	0.5	0.3	0.0	0.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.8	0.8	0.0	0.2	1.9	0.1	0.2	0.0	0.4	0.5	0.0	0.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	19.9	4.1	2.4	20.9	6.3	3.5	17.9	0.0	17.5	18.5	0.0	17.5
LnGrp LOS	B	A	A	C	A	A	B	A	B	B	A	B
Approach Vol, veh/h		1311			1485			66			98	
Approach Delay, s/veh		5.2			6.4			17.6			18.0	
Approach LOS		A			A			B			B	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	3.7	32.9		8.8	6.1	30.4		8.8				
Change Period (Y+Rc), s	3.0	5.5		4.5	3.0	5.5		4.5				
Max Green Setting (Gmax), s	3.0	36.0		28.0	4.0	35.0		28.0				
Max Q Clear Time (g_c+I1), s	2.6	10.9		3.7	4.5	15.3		4.7				
Green Ext Time (p_c), s	0.0	9.2		0.3	0.0	9.6		0.5				

Intersection Summary			
HCM 6th Ctrl Delay		6.5	
HCM 6th LOS		A	

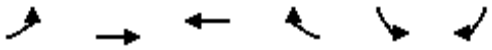
Intersection	
Int Delay, s/veh	0.5

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	1200	10	10	1420	10	10
Future Vol, veh/h	1200	10	10	1420	10	10
Conflicting Peds, #/hr	0	1	1	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	100	140	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	0	0	2	0	0
Mvmt Flow	1263	11	11	1495	11	11

Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	1275	0	2034	633
Stage 1	-	-	-	-	1264	-
Stage 2	-	-	-	-	770	-
Critical Hdwy	-	-	4.1	-	6.8	6.9
Critical Hdwy Stg 1	-	-	-	-	5.8	-
Critical Hdwy Stg 2	-	-	-	-	5.8	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	551	-	51	427
Stage 1	-	-	-	-	233	-
Stage 2	-	-	-	-	423	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	550	-	50	427
Mov Cap-2 Maneuver	-	-	-	-	50	-
Stage 1	-	-	-	-	233	-
Stage 2	-	-	-	-	415	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0.1	56.8
HCM LOS			F

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	90	-	-	550	-
HCM Lane V/C Ratio	0.234	-	-	0.019	-
HCM Control Delay (s)	56.8	-	-	11.7	-
HCM Lane LOS	F	-	-	B	-
HCM 95th %tile Q(veh)	0.8	-	-	0.1	-



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Volume (veh/h)	100	1110	1380	110	70	50
Future Volume (veh/h)	100	1110	1380	110	70	50
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00			0.98	1.00	1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No	No		No	
Adj Sat Flow, veh/h/ln	1700	1673	1673	1700	1700	1700
Adj Flow Rate, veh/h	104	1156	1438	91	62	63
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	0	2	2	0	0	0
Cap, veh/h	192	2456	2000	886	213	190
Arrive On Green	0.12	0.77	0.63	0.63	0.13	0.13
Sat Flow, veh/h	1619	3263	3263	1409	1619	1441
Grp Volume(v), veh/h	104	1156	1438	91	62	63
Grp Sat Flow(s),veh/h/ln	1619	1590	1590	1409	1619	1441
Q Serve(g_s), s	2.7	5.8	13.7	1.1	1.5	1.8
Cycle Q Clear(g_c), s	2.7	5.8	13.7	1.1	1.5	1.8
Prop In Lane	1.00			1.00	1.00	1.00
Lane Grp Cap(c), veh/h	192	2456	2000	886	213	190
V/C Ratio(X)	0.54	0.47	0.72	0.10	0.29	0.33
Avail Cap(c_a), veh/h	213	3207	2710	1201	1102	981
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	18.6	1.8	5.6	3.3	17.6	17.7
Incr Delay (d2), s/veh	0.9	0.1	0.6	0.1	1.1	1.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.9	0.0	1.3	0.1	0.6	0.6
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	19.5	2.0	6.2	3.3	18.6	19.1
LnGrp LOS	B	A	A	A	B	B
Approach Vol, veh/h		1260	1529		125	
Approach Delay, s/veh		3.4	6.1		18.9	
Approach LOS		A	A		B	

Timer - Assigned Phs	1	2		4		6
Phs Duration (G+Y+Rc), s	6.4	30.5		7.9		36.9
Change Period (Y+Rc), s	3.0	5.5		4.5		5.5
Max Green Setting (Gmax), s	4.0	35.0		28.0		42.0
Max Q Clear Time (g_c+I1), s	4.7	15.7		3.8		7.8
Green Ext Time (p_c), s	0.0	9.3		0.5		9.8

Intersection Summary	
HCM 6th Ctrl Delay	5.5
HCM 6th LOS	A

Notes

User approved volume balancing among the lanes for turning movement.

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	1040	140	310	1340	150	600
Future Volume (vph)	1040	140	310	1340	150	600
Ideal Flow (vphpl)	1700	1700	1700	1700	1700	1700
Total Lost time (s)	2.5	2.5	1.0	2.5	1.5	1.5
Lane Util. Factor	0.95	1.00	1.00	0.95	1.00	1.00
Frpb, ped/bikes	1.00	0.99	1.00	1.00	1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.85	1.00	1.00	1.00	0.85
Flt Protected	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	3167	1398	1583	3167	1568	1403
Flt Permitted	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (perm)	3167	1398	1583	3167	1568	1403
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	1095	147	326	1411	158	632
RTOR Reduction (vph)	0	19	0	0	0	555
Lane Group Flow (vph)	1095	128	326	1411	158	77
Confl. Bikes (#/hr)		3				
Heavy Vehicles (%)	2%	2%	2%	2%	3%	3%
Turn Type	NA	Perm	Prot	NA	Prot	Prot
Protected Phases	2		1	6	4	4
Permitted Phases		2				
Actuated Green, G (s)	70.5	70.5	33.0	109.5	11.5	11.5
Effective Green, g (s)	75.5	75.5	38.0	114.5	16.5	16.5
Actuated g/C Ratio	0.56	0.56	0.28	0.85	0.12	0.12
Clearance Time (s)	7.5	7.5	6.0	7.5	6.5	6.5
Vehicle Extension (s)	3.0	3.0	3.0	3.0	2.0	2.0
Lane Grp Cap (vph)	1771	781	445	2686	191	171
v/s Ratio Prot	c0.35		c0.21	0.45	c0.10	0.06
v/s Ratio Perm		0.09				
v/c Ratio	0.62	0.16	0.73	0.53	0.83	0.45
Uniform Delay, d1	20.0	14.4	43.9	2.8	57.9	55.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	1.6	0.5	6.1	0.7	23.4	0.7
Delay (s)	21.7	14.9	50.0	3.5	81.3	55.7
Level of Service	C	B	D	A	F	E
Approach Delay (s)	20.9			12.3	60.9	
Approach LOS	C			B	E	

Intersection Summary			
HCM 2000 Control Delay	25.3	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.69		
Actuated Cycle Length (s)	135.0	Sum of lost time (s)	10.0
Intersection Capacity Utilization	80.3%	ICU Level of Service	D
Analysis Period (min)	15		

c Critical Lane Group

Intersection	
Int Delay, s/veh	5.7

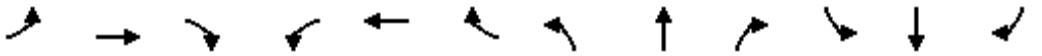
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	10	1600	30	10	1620	20	10	0	10	10	0	20
Future Vol, veh/h	10	1600	30	10	1620	20	10	0	10	10	0	20
Conflicting Peds, #/hr	1	0	1	1	0	1	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	115	-	100	140	-	100	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	95	95	95	95	95	95	95	95	95	95	95	95
Heavy Vehicles, %	2	2	0	0	2	2	0	0	0	3	3	3
Mvmt Flow	11	1684	32	11	1705	21	11	0	11	11	0	21

Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	1727	0	0	1717	0	0	2582	3456	843	2592	3467	854
Stage 1	-	-	-	-	-	-	1707	1707	-	1728	1728	-
Stage 2	-	-	-	-	-	-	875	1749	-	864	1739	-
Critical Hdwy	4.14	-	-	4.1	-	-	7.5	6.5	6.9	7.56	6.56	6.96
Critical Hdwy Stg 1	-	-	-	-	-	-	6.5	5.5	-	6.56	5.56	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.5	5.5	-	6.56	5.56	-
Follow-up Hdwy	2.22	-	-	2.2	-	-	3.5	4	3.3	3.53	4.03	3.33
Pot Cap-1 Maneuver	362	-	-	374	-	-	13	7	311	12	6	300
Stage 1	-	-	-	-	-	-	97	148	-	91	140	-
Stage 2	-	-	-	-	-	-	315	141	-	313	138	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	362	-	-	374	-	-	12	7	311	11	6	300
Mov Cap-2 Maneuver	-	-	-	-	-	-	12	7	-	11	6	-
Stage 1	-	-	-	-	-	-	94	143	-	88	136	-
Stage 2	-	-	-	-	-	-	284	137	-	293	134	-

Approach	EB	WB	NB	SB
HCM Control Delay, s	0.1	0.1	$ 397.1	$ 356.1
HCM LOS			F	F

Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1
Capacity (veh/h)	23	362	-	-	374	-	-	31
HCM Lane V/C Ratio	0.915	0.029	-	-	0.028	-	-	1.019
HCM Control Delay (s)	$ 397.1	15.2	-	-	14.9	-	-	$ 356.1
HCM Lane LOS	F	C	-	-	B	-	-	F
HCM 95th %tile Q(veh)	2.7	0.1	-	-	0.1	-	-	3.5

Notes			
~: Volume exceeds capacity	$: Delay exceeds 300s	+: Computation Not Defined	*: All major volume in platoon



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	1000	540	80	20	1040	1530	80	460	20	500	250	530
Future Volume (veh/h)	1000	540	80	20	1040	1530	80	460	20	500	250	530
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		1.00	1.00		0.99	1.00		0.99
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1673	1673	1673	1673	1673	1673	1687	1687	1687	1687	1687	1687
Adj Flow Rate, veh/h	1042	562	77	21	1083	0	83	479	19	456	351	466
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	2	2	2	2	2	2	1	1	1	1	1	1
Cap, veh/h	1078	2115	289	79	1424		285	557	22	632	663	796
Arrive On Green	0.35	0.75	0.74	0.05	0.45	0.00	0.18	0.18	0.15	0.20	0.20	0.20
Sat Flow, veh/h	3092	2805	383	1594	3180	1418	1606	3140	124	3213	3373	1409
Grp Volume(v), veh/h	1042	318	321	21	1083	0	83	244	254	456	351	466
Grp Sat Flow(s),veh/h/ln	1546	1590	1598	1594	1590	1418	1606	1602	1662	1606	1687	1409
Q Serve(g_s), s	49.7	9.2	9.4	1.9	42.8	0.0	6.7	22.2	22.3	19.9	14.0	29.5
Cycle Q Clear(g_c), s	49.7	9.2	9.4	1.9	42.8	0.0	6.7	22.2	22.3	19.9	14.0	29.5
Prop In Lane	1.00		0.24	1.00		1.00	1.00		0.07	1.00		1.00
Lane Grp Cap(c), veh/h	1078	1199	1205	79	1424		285	284	295	632	663	796
V/C Ratio(X)	0.97	0.27	0.27	0.27	0.76		0.29	0.86	0.86	0.72	0.53	0.59
Avail Cap(c_a), veh/h	1078	1199	1205	101	1424		285	284	295	632	663	796
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	48.0	5.7	5.8	68.7	34.7	0.0	53.5	59.9	60.1	56.4	54.0	21.7
Incr Delay (d2), s/veh	19.6	0.5	0.5	0.7	3.9	0.0	0.2	21.3	21.2	3.5	0.4	0.8
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	21.3	2.7	2.8	0.8	16.5	0.0	2.7	10.6	11.0	8.3	5.9	10.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	67.6	6.2	6.3	69.3	38.5	0.0	53.7	81.2	81.2	59.9	54.4	22.4
LnGrp LOS	E	A	A	E	D		D	F	F	E	D	C
Approach Vol, veh/h		1681			1104	A		581			1273	
Approach Delay, s/veh		44.3			39.1			77.3			44.7	
Approach LOS		D			D			E			D	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	55.0	71.0		31.0	7.9	118.1		28.0				
Change Period (Y+Rc), s	5.0	* 6.5		6.0	5.0	6.5		5.5				
Max Green Setting (Gmax), s	50.0	* 31		25.0	5.0	74.5		22.5				
Max Q Clear Time (g_c+I1), s	51.7	44.8		31.5	3.9	11.4		24.3				
Green Ext Time (p_c), s	0.0	0.0		0.0	0.0	2.1		0.0				

Intersection Summary			
HCM 6th Ctrl Delay		47.3	
HCM 6th LOS		D	

Notes

User approved pedestrian interval to be less than phase max green.

User approved volume balancing among the lanes for turning movement.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	270	10	50	10	10	10	140	2840	10	10	1220	200
Future Volume (veh/h)	270	10	50	10	10	10	140	2840	10	10	1220	200
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.98	1.00		1.00	1.00		1.00	1.00		0.98
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1700	1700	1700	1700	1700	1700	1700	1687	1700	1700	1687	1700
Adj Flow Rate, veh/h	276	10	20	10	10	3	143	2898	10	10	1245	196
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	0	0	0	0	0	0	0	1	0	0	1	0
Cap, veh/h	225	6	521	34	28	4	203	3117	11	62	1594	249
Arrive On Green	0.22	0.24	0.24	0.22	0.24	0.24	0.13	0.66	0.66	0.08	1.00	1.00
Sat Flow, veh/h	732	27	1419	0	114	17	1619	4737	16	1619	2767	433
Grp Volume(v), veh/h	286	0	20	23	0	0	143	1877	1031	10	717	724
Grp Sat Flow(s),veh/h/ln	758	0	1419	131	0	0	1619	1535	1684	1619	1602	1597
Q Serve(g_s), s	0.0	0.0	0.0	0.0	0.0	0.0	12.7	80.7	81.1	0.9	0.0	0.0
Cycle Q Clear(g_c), s	32.9	0.0	0.0	32.9	0.0	0.0	12.7	80.7	81.1	0.9	0.0	0.0
Prop In Lane	0.97		1.00	0.43		0.13	1.00		0.01	1.00		0.27
Lane Grp Cap(c), veh/h	213	0	521	63	0	0	203	2020	1108	62	923	920
V/C Ratio(X)	1.34	0.00	0.04	0.36	0.00	0.00	0.70	0.93	0.93	0.16	0.78	0.79
Avail Cap(c_a), veh/h	213	0	521	63	0	0	214	2020	1108	97	923	920
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	2.00	2.00	2.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	0.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	60.8	0.0	30.6	47.5	0.0	0.0	62.9	22.6	22.6	67.1	0.0	0.0
Incr Delay (d2), s/veh	181.1	0.0	0.0	3.5	0.0	0.0	7.8	9.1	14.8	0.5	6.4	6.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	19.2	0.0	0.5	0.8	0.0	0.0	5.6	28.4	33.0	0.4	1.6	1.7
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	241.8	0.0	30.7	50.9	0.0	0.0	70.8	31.7	37.4	67.5	6.4	6.7
LnGrp LOS	F	A	C	D	A	A	E	C	D	E	A	A
Approach Vol, veh/h		306			23			3051			1451	
Approach Delay, s/veh		228.0			50.9			35.5			7.0	
Approach LOS		F			D			D			A	

Timer - Assigned Phs	1	2		4	5	6		8
Phs Duration (G+Y+Rc), s	6.7	105.3		38.0	21.0	91.0		38.0
Change Period (Y+Rc), s	5.0	6.0		5.0	6.0	* 6		5.0
Max Green Setting (Gmax), s	5.0	96.0		33.0	16.0	* 85		33.0
Max Q Clear Time (g_c+I1), s	2.9	83.1		34.9	14.7	2.0		34.9
Green Ext Time (p_c), s	0.0	7.2		0.0	0.0	3.1		0.0

Intersection Summary	
HCM 6th Ctrl Delay	39.2
HCM 6th LOS	D

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	540	80	1540	1580	80	890
Future Volume (veh/h)	540	80	1540	1580	80	890
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1687	1687	1687	1687	1687	1687
Adj Flow Rate, veh/h	551	63	1571	1557	82	908
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	1	1	1	1	1	1
Cap, veh/h	661	512	1880	2057	232	2364
Arrive On Green	0.21	0.21	0.98	1.00	0.14	0.74
Sat Flow, veh/h	3116	1429	3289	2516	1606	3289
Grp Volume(v), veh/h	551	63	1571	1557	82	908
Grp Sat Flow(s),veh/h/ln	1558	1429	1602	1258	1606	1602
Q Serve(g_s), s	25.4	0.0	8.2	90.8	6.9	15.6
Cycle Q Clear(g_c), s	25.4	0.0	8.2	90.8	6.9	15.6
Prop In Lane	1.00	1.00		1.00	1.00	
Lane Grp Cap(c), veh/h	661	512	1880	2057	232	2364
V/C Ratio(X)	0.83	0.12	0.84	0.76	0.35	0.38
Avail Cap(c_a), veh/h	883	613	1880	2057	232	2364
HCM Platoon Ratio	1.00	1.00	1.67	1.67	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	56.5	32.3	0.7	0.0	57.9	7.2
Incr Delay (d2), s/veh	4.0	0.0	4.6	2.7	0.3	0.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	10.2	2.6	1.7	0.8	2.8	4.8
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	60.5	32.4	5.3	2.7	58.2	7.7
LnGrp LOS	E	C	A	A	E	A
Approach Vol, veh/h	614		3128			990
Approach Delay, s/veh	57.6		4.0			11.9
Approach LOS	E		A			B

Timer - Assigned Phs	1	2		4		6
Phs Duration (G+Y+Rc), s	23.7	93.0		33.3		116.7
Change Period (Y+Rc), s	6.0	* 6		5.3		6.0
Max Green Setting (Gmax), s	8.0	* 87		38.7		100.0
Max Q Clear Time (g_c+I1), s	10.2	92.8		27.4		17.6
Green Ext Time (p_c), s	0.0	0.0		0.7		4.2

Intersection Summary	
HCM 6th Ctrl Delay	12.6
HCM 6th LOS	B

Notes

User approved pedestrian interval to be less than phase max green.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Intersection	
Int Delay, s/veh	2.1

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	30	30	40	1580	940	30
Future Vol, veh/h	30	30	40	1580	940	30
Conflicting Peds, #/hr	0	0	1	0	0	1
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	200	-	-	100
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	0	0	1	1	0
Mvmt Flow	32	32	42	1663	989	32

Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	1906	496	1022	0	-	0
Stage 1	990	-	-	-	-	-
Stage 2	916	-	-	-	-	-
Critical Hdwy	6.8	6.9	4.1	-	-	-
Critical Hdwy Stg 1	5.8	-	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	62	525	687	-	-	-
Stage 1	325	-	-	-	-	-
Stage 2	355	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	58	525	686	-	-	-
Mov Cap-2 Maneuver	58	-	-	-	-	-
Stage 1	305	-	-	-	-	-
Stage 2	355	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	82.7	0.3	0
HCM LOS	F		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	686	-	104	-	-
HCM Lane V/C Ratio	0.061	-	0.607	-	-
HCM Control Delay (s)	10.6	-	82.7	-	-
HCM Lane LOS	B	-	F	-	-
HCM 95th %tile Q(veh)	0.2	-	2.9	-	-

Intersection	
Int Delay, s/veh	0

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	0	0	2990	0	0	1280
Future Vol, veh/h	0	0	2990	0	0	1280
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	150	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	0	0	3250	0	0	1391

Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	3946	1625	0	0	3250	0
Stage 1	3250	-	-	-	-	-
Stage 2	696	-	-	-	-	-
Critical Hdwy	6.25	7.1	-	-	5.3	-
Critical Hdwy Stg 1	6.6	-	-	-	-	-
Critical Hdwy Stg 2	5.8	-	-	-	-	-
Follow-up Hdwy	3.65	3.9	-	-	3.1	-
Pot Cap-1 Maneuver	4	80	-	-	29	-
Stage 1	9	-	-	-	-	-
Stage 2	448	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	4	80	-	-	29	-
Mov Cap-2 Maneuver	4	-	-	-	-	-
Stage 1	9	-	-	-	-	-
Stage 2	448	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	0	0	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBL	SBT
Capacity (veh/h)	-	-	-	29	-
HCM Lane V/C Ratio	-	-	-	-	-
HCM Control Delay (s)	-	-	0	0	-
HCM Lane LOS	-	-	A	A	-
HCM 95th %tile Q(veh)	-	-	-	0	-

Intersection	
Int Delay, s/veh	0

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	0	0	0	3120	1430	0
Future Vol, veh/h	0	0	0	3120	1430	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	150	-	-	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	0	0	0	3391	1554	0

Major/Minor	Minor2		Major1		Major2	
Conflicting Flow All	2910	777	1554	0	-	0
Stage 1	1554	-	-	-	-	-
Stage 2	1356	-	-	-	-	-
Critical Hdwy	6.25	6.9	4.1	-	-	-
Critical Hdwy Stg 1	5.8	-	-	-	-	-
Critical Hdwy Stg 2	6	-	-	-	-	-
Follow-up Hdwy	3.65	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	20	344	432	-	-	-
Stage 1	160	-	-	-	-	-
Stage 2	189	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	20	344	432	-	-	-
Mov Cap-2 Maneuver	20	-	-	-	-	-
Stage 1	160	-	-	-	-	-
Stage 2	189	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	0	0	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	432	-	-	-	-
HCM Lane V/C Ratio	-	-	-	-	-
HCM Control Delay (s)	0	-	0	-	-
HCM Lane LOS	A	-	A	-	-
HCM 95th %tile Q(veh)	0	-	-	-	-

Intersection	
Int Delay, s/veh	0

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	0	0	1620	0	0	970
Future Vol, veh/h	0	0	1620	0	0	970
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	125	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	0	0	1761	0	0	1054

Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	-	881	0	0	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.9	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.3	-	-	-	-
Pot Cap-1 Maneuver	0	294	-	-	0	-
Stage 1	0	-	-	-	0	-
Stage 2	0	-	-	-	0	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	-	294	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	0	0	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBT	NBR	WBLn1	SBT
Capacity (veh/h)	-	-	-	-
HCM Lane V/C Ratio	-	-	-	-
HCM Control Delay (s)	-	-	0	-
HCM Lane LOS	-	-	A	-
HCM 95th %tile Q(veh)	-	-	-	-

MPAH Amendment Study - Villa Park Rd

Vistro File: N:\...\Villa Park_v6.vistro — Scenario 4 Future PM Peak Hour

Report File: N:\...\04_FYNoImprovementPM.pdf — 7/21/2023

Intersection Analysis Summary

ID	Intersection Name	Control Type	Method	Worst Mvmt	V/C	Delay (s/veh)	LOS
1	Villa Park Rd & Wanda Rd	Signalized	ICU 2	EB Thru	0.774	-	C
7	Villa Park Rd & Center Dr	Signalized	ICU 2	WB Thru	0.547	-	A
9	Villa Park Rd & Lemon St	Signalized	ICU 2	WB Thru	0.524	-	A
10	Villa Park Rd & Hewes St	Signalized	ICU 2	WB Thru	0.889	-	D
12	Santiago Canyon Rd & Cannon St	Signalized	ICU 2	WB Thru	0.935	-	E
13	Taft Ave & Cannon St	Signalized	ICU 2	NB Thru	0.783	-	C
14	Serrano Ave & Cannon St	Signalized	ICU 2	NB Thru	0.696	-	B

V/C, Delay, LOS: For two-way stop, these values are taken from the movement with the worst (highest) delay value. For all other control types, they are taken for the whole intersection.

Intersection Level Of Service Report
Intersection 1: Villa Park Rd & Wanda Rd

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	C
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.774

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	0	2	0	1	1	0	1
Entry Pocket Length [ft]	100.00	100.00	100.00	175.00	100.00	100.00	225.00	100.00	300.00	150.00	100.00	175.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			Yes			Yes			Yes		

Volumes

Name												
Base Volume Input [veh/h]	150	540	60	150	380	290	430	1080	250	60	1010	330
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	150	540	60	150	380	290	430	1080	250	60	1010	330
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	38	135	15	38	95	73	108	270	63	15	253	83
Total Analysis Volume [veh/h]	150	540	60	150	380	290	430	1080	250	60	1010	330
Pedestrian Volume [ped/h]	0			0			0			1		
Bicycle Volume [bicycles/h]	1			0			1			11		

Intersection Settings

Cycle Length [s]	140
Lost time [s]	7.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Overlap	Protecte	Permiss	Overlap	Protecte	Permiss	Overlap
Signal Group	3	8	0	7	4	4	1	6	6	5	2	2
Auxiliary Signal Groups						1,4			3,6			2,7
Lead / Lag	Lead	-	-	Lead	-	-	Lead	-	-	Lag	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.09	0.18	0.18	0.09	0.22	0.04	0.13	0.32	0.06	0.04	0.30	0.11
Intersection LOS	C											
Intersection V/C	0.774											

Intersection Level Of Service Report
Intersection 7: Villa Park Rd & Center Dr

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	A
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.547

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	0	1	0	1	1	0	1
Entry Pocket Length [ft]	150.00	100.00	100.00	140.00	100.00	100.00	160.00	100.00	100.00	150.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			Yes			Yes			Yes		

Volumes

Name												
Base Volume Input [veh/h]	20	40	20	50	30	80	90	1140	20	20	1320	90
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	20	40	20	50	30	80	90	1140	20	20	1320	90
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	5	10	5	13	8	20	23	285	5	5	330	23
Total Analysis Volume [veh/h]	20	40	20	50	30	80	90	1140	20	20	1320	90
Pedestrian Volume [ped/h]	0			1			0			0		
Bicycle Volume [bicycles/h]	3			0			5			0		

Intersection Settings

Cycle Length [s]	111
Lost time [s]	6.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	0	4	0	0	4	0	5	2	0	1	6	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	Lead	-	-	Lead	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.01	0.04	0.04	0.03	0.06	0.06	0.05	0.34	0.01	0.01	0.39	0.05
Intersection LOS	A											
Intersection V/C	0.547											

Intersection Level Of Service Report
Intersection 9: Villa Park Rd & Lemon St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	A
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.524

Intersection Setup

Name						
Approach	Southbound		Eastbound		Westbound	
Lane Configuration						
Turning Movement	Left	Right	Left	Thru	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	1	0	0	1
Entry Pocket Length [ft]	115.00	100.00	180.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00		30.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	Yes		Yes		No	

Volumes

Name						
Base Volume Input [veh/h]	70	50	100	1110	1380	110
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	70	50	100	1110	1380	110
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	18	13	25	278	345	28
Total Analysis Volume [veh/h]	70	50	100	1110	1380	110
Pedestrian Volume [ped/h]	1		1		0	
Bicycle Volume [bicycles/h]	2		2		0	

Intersection Settings

Cycle Length [s]	108
Lost time [s]	5.00

Phasing & Timing

Control Type	Permissive	Permissive	Protected	Permissive	Permissive	Permissive
Signal Group	4	0	1	6	2	0
Auxiliary Signal Groups						
Lead / Lag	Lag	-	Lead	-	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.02	0.04	0.06	0.33	0.41	0.06
Intersection LOS	A					
Intersection V/C	0.524					

Intersection Level Of Service Report
Intersection 10: Villa Park Rd & Hewes St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	D
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.889

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0	0	0	1	1	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	215.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			No			Yes			No		

Volumes

Name												
Base Volume Input [veh/h]	150	0	600	0	6	0	0	1040	140	310	1340	0
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	150	0	600	0	6	0	0	1040	140	310	1340	0
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	38	0	150	0	2	0	0	260	35	78	335	0
Total Analysis Volume [veh/h]	150	0	600	0	6	0	0	1040	140	310	1340	0
Pedestrian Volume [ped/h]	0			0			0			0		
Bicycle Volume [bicycles/h]	3			0			6			0		

Intersection Settings

Cycle Length [s]	140
Lost time [s]	7.00

Phasing & Timing

Control Type	Split	Permiss	Split	Split	Split	Split	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	4	0	0	0	8	0	0	2	0	1	6	0
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	-	-	-	-	-	-	Lead	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.09	0.00	0.35	0.00	0.00	0.00	0.00	0.31	0.08	0.18	0.39	0.00
Intersection LOS	D											
Intersection V/C	0.889											

Intersection Level Of Service Report
Intersection 12: Santiago Canyon Rd & Cannon St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	E
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.935

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	2	0	1	2	0	0	1	0	1
Entry Pocket Length [ft]	80.00	100.00	100.00	375.00	100.00	250.00	325.00	100.00	100.00	150.00	100.00	450.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			No			Yes			No		

Volumes

Name												
Base Volume Input [veh/h]	80	460	20	500	250	530	1000	540	80	20	1040	1530
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	80	460	20	500	250	530	1000	540	80	20	1040	1530
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	20	115	5	125	63	133	250	135	20	5	260	383
Total Analysis Volume [veh/h]	80	460	20	500	250	530	1000	540	80	20	1040	1530
Pedestrian Volume [ped/h]	1			0			1			0		
Bicycle Volume [bicycles/h]	3			3			1			1		

Intersection Settings

Cycle Length [s]	140
Lost time [s]	7.00

Phasing & Timing

Control Type	Split	Split	Split	Split	Split	Overlap	Protecte	Permiss	Permiss	Protecte	Permiss	Unsigna
Signal Group	0	8	0	0	4	4	1	6	0	5	2	0
Auxiliary Signal Groups						1,4						
Lead / Lag	-	-	-	-	-	-	Lead	-	-	Lead	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.05	0.14	0.14	0.15	0.07	0.02	0.29	0.18	0.18	0.01	0.31	0.00
Intersection LOS	E											
Intersection V/C	0.935											

Intersection Level Of Service Report
Intersection 13: Taft Ave & Cannon St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	C
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.783

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	1	0	0	0	0	1	0	0	0
Entry Pocket Length [ft]	225.00	100.00	100.00	150.00	100.00	100.00	100.00	100.00	200.00	100.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			No			Yes			Yes		

Volumes

Name												
Base Volume Input [veh/h]	140	2840	10	10	1220	200	270	10	50	10	10	10
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	140	2840	10	10	1220	200	270	10	50	10	10	10
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	35	710	3	3	305	50	68	3	13	3	3	3
Total Analysis Volume [veh/h]	140	2840	10	10	1220	200	270	10	50	10	10	10
Pedestrian Volume [ped/h]	1			0			1			0		
Bicycle Volume [bicycles/h]	2			0			2			0		

Intersection Settings

Cycle Length [s]	150
Lost time [s]	8.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Permiss	Permiss	Overlap	Permiss	Permiss	Permiss
Signal Group	5	2	0	1	6	0	0	4	4	0	4	0
Auxiliary Signal Groups									4,5			
Lead / Lag	Lag	-	-	Lead	-	-	-	-	-	-	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.08	0.56	0.56	0.01	0.42	0.42	0.16	0.16	0.00	0.01	0.02	0.02
Intersection LOS	C											
Intersection V/C	0.783											

Intersection Level Of Service Report
Intersection 14: Serrano Ave & Cannon St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	B
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.696

Intersection Setup

Name						
Approach	Northbound		Southbound		Westbound	
Lane Configuration						
Turning Movement	Thru	Right	Left	Thru	Left	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	1	1	0	1	0
Entry Pocket Length [ft]	100.00	150.00	275.00	100.00	350.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00		30.00		30.00	
Grade [%]	0.00		0.00		0.00	
Crosswalk	No		Yes		Yes	

Volumes

Name						
Base Volume Input [veh/h]	1540	1580	80	890	540	80
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0
Total Hourly Volume [veh/h]	1540	1580	80	890	540	80
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	385	395	20	223	135	20
Total Analysis Volume [veh/h]	1540	1580	80	890	540	80
Pedestrian Volume [ped/h]	0		0		0	
Bicycle Volume [bicycles/h]	0		0		0	

Intersection Settings

Cycle Length [s]	150
Lost time [s]	8.00

Phasing & Timing

Control Type	Permissive	Overlap	Protected	Permissive	Split	Overlap
Signal Group	2	2	1	6	4	4
Auxiliary Signal Groups		2,4				1,4
Lead / Lag	-	-	Lag	-	Lag	-

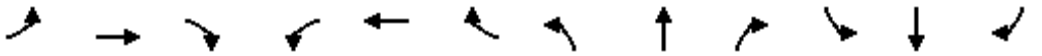
Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.45	0.31	0.05	0.26	0.16	0.00
Intersection LOS	B					
Intersection V/C	0.696					



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	1050	220	700	1060	210	310
Future Volume (vph)	1050	220	700	1060	210	310
Ideal Flow (vphpl)	1700	1700	1700	1700	1700	1700
Total Lost time (s)	2.5	2.5	1.0	2.5	1.5	1.5
Lane Util. Factor	0.95	1.00	0.97	0.95	1.00	1.00
Frpb, ped/bikes	1.00	0.98	1.00	1.00	1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.85	1.00	1.00	1.00	0.85
Flt Protected	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	3076	1354	2984	3076	1568	1403
Flt Permitted	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (perm)	3076	1354	2984	3076	1568	1403
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	1105	232	737	1116	221	326
RTOR Reduction (vph)	0	36	0	0	0	287
Lane Group Flow (vph)	1105	196	737	1116	221	39
Confl. Peds. (#/hr)		2			1	
Confl. Bikes (#/hr)		1				
Heavy Vehicles (%)	5%	5%	5%	5%	3%	3%
Turn Type	NA	Perm	Prot	NA	Prot	Prot
Protected Phases	2		1	6	4	4
Permitted Phases		2				
Actuated Green, G (s)	64.5	64.5	36.2	106.7	12.5	12.5
Effective Green, g (s)	69.5	69.5	41.2	111.7	17.5	17.5
Actuated g/C Ratio	0.48	0.48	0.28	0.77	0.12	0.12
Clearance Time (s)	7.5	7.5	6.0	7.5	6.5	6.5
Vehicle Extension (s)	3.0	3.0	3.0	3.0	2.0	2.0
Lane Grp Cap (vph)	1474	648	847	2369	189	169
v/s Ratio Prot	c0.36		c0.25	0.36	c0.14	0.03
v/s Ratio Perm		0.14				
v/c Ratio	0.75	0.30	0.87	0.47	1.17	0.23
Uniform Delay, d1	30.7	23.0	49.4	6.0	63.8	57.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	3.5	1.2	9.7	0.7	118.5	0.3
Delay (s)	34.2	24.2	59.0	6.7	182.2	57.9
Level of Service	C	C	E	A	F	E
Approach Delay (s)	32.5			27.5	108.2	
Approach LOS	C			C	F	

Intersection Summary			
HCM 2000 Control Delay	41.1	HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio	0.78		
Actuated Cycle Length (s)	145.0	Sum of lost time (s)	10.0
Intersection Capacity Utilization	77.8%	ICU Level of Service	D
Analysis Period (min)	15		
c Critical Lane Group			



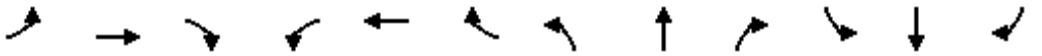
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	70	1250	40	10	1640	100	10	10	30	30	0	110
Future Volume (veh/h)	70	1250	40	10	1640	100	10	10	30	30	0	110
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.97	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1634	1634	1700	1700	1634	1634	1700	1700	1700	1660	1660	1660
Adj Flow Rate, veh/h	74	1316	42	11	1726	105	11	11	32	32	0	116
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	5	5	0	0	5	5	0	0	0	3	3	3
Cap, veh/h	91	1847	835	110	2661	162	64	57	113	70	11	135
Arrive On Green	0.06	0.60	0.60	0.09	0.82	0.82	0.12	0.12	0.12	0.12	0.00	0.12
Sat Flow, veh/h	1556	3104	1404	1619	4298	261	167	468	924	215	90	1107
Grp Volume(v), veh/h	74	1316	42	11	1193	638	54	0	0	148	0	0
Grp Sat Flow(s),veh/h/ln	1556	1552	1404	1619	1487	1585	1559	0	0	1412	0	0
Q Serve(g_s), s	4.7	29.8	1.2	0.6	15.2	15.3	0.0	0.0	0.0	6.8	0.0	0.0
Cycle Q Clear(g_c), s	4.7	29.8	1.2	0.6	15.2	15.3	3.2	0.0	0.0	10.3	0.0	0.0
Prop In Lane	1.00		1.00	1.00		0.16	0.20		0.59	0.22		0.78
Lane Grp Cap(c), veh/h	91	1847	835	110	1841	982	234	0	0	216	0	0
V/C Ratio(X)	0.81	0.71	0.05	0.10	0.65	0.65	0.23	0.00	0.00	0.69	0.00	0.00
Avail Cap(c_a), veh/h	171	1847	835	110	1841	982	282	0	0	262	0	0
HCM Platoon Ratio	1.00	1.00	1.00	1.33	1.33	1.33	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	0.71	0.71	0.71	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	46.5	14.2	8.5	42.7	4.7	4.7	39.9	0.0	0.0	43.0	0.0	0.0
Incr Delay (d2), s/veh	15.3	2.4	0.1	0.3	1.3	2.4	0.2	0.0	0.0	3.5	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.1	9.1	0.3	0.2	2.6	3.1	1.2	0.0	0.0	3.8	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	61.8	16.6	8.6	42.9	6.0	7.1	40.1	0.0	0.0	46.5	0.0	0.0
LnGrp LOS	E	B	A	D	A	A	D	A	A	D	A	A
Approach Vol, veh/h		1432			1842			54			148	
Approach Delay, s/veh		18.7			6.6			40.1			46.5	
Approach LOS		B			A			D			D	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	14.3	67.0		18.7	11.9	69.4		18.7				
Change Period (Y+Rc), s	7.5	* 7.5		6.5	6.0	7.5		6.5				
Max Green Setting (Gmax), s	5.0	* 60		15.5	11.0	53.5		15.5				
Max Q Clear Time (g_c+I1), s	2.6	31.8		5.2	6.7	17.3		12.3				
Green Ext Time (p_c), s	0.0	10.4		0.1	0.0	16.2		0.1				

Intersection Summary			
HCM 6th Ctrl Delay		13.8	
HCM 6th LOS		B	

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	400	810	100	20	560	500	150	190	40	1300	480	1040
Future Volume (veh/h)	400	810	100	20	560	500	150	190	40	1300	480	1040
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		1.00	1.00		0.99	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1634	1634	1634	1660	1660	1660	1673	1673	1673	1673	1673	1673
Adj Flow Rate, veh/h	412	835	95	21	577	0	155	196	23	1340	495	916
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	5	5	5	3	3	3	2	2	2	2	2	2
Cap, veh/h	574	863	98	106	775		239	377	44	1440	757	1601
Arrive On Green	0.19	0.31	0.31	0.07	0.17	0.00	0.15	0.13	0.08	0.47	0.45	0.45
Sat Flow, veh/h	3018	2804	319	1581	4532	1407	1594	2866	332	3092	1673	2486
Grp Volume(v), veh/h	412	462	468	21	577	0	155	108	111	1340	495	916
Grp Sat Flow(s),veh/h/ln	1509	1552	1571	1581	1511	1407	1594	1590	1608	1546	1673	1243
Q Serve(g_s), s	12.8	29.4	29.4	1.3	12.1	0.0	9.2	6.3	6.5	40.9	23.0	7.0
Cycle Q Clear(g_c), s	12.8	29.4	29.4	1.3	12.1	0.0	9.2	6.3	6.5	40.9	23.0	7.0
Prop In Lane	1.00		0.20	1.00		1.00	1.00		0.21	1.00		1.00
Lane Grp Cap(c), veh/h	574	478	484	106	775		239	209	211	1440	757	1601
V/C Ratio(X)	0.72	0.97	0.97	0.20	0.74		0.65	0.51	0.53	0.93	0.65	0.57
Avail Cap(c_a), veh/h	574	478	484	150	775		349	209	211	1500	757	1601
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	38.0	34.1	34.1	44.1	39.4	0.0	40.0	40.5	41.0	25.2	21.3	2.7
Incr Delay (d2), s/veh	3.7	32.5	32.3	0.3	3.5	0.0	1.1	1.0	1.2	10.1	4.4	1.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	4.8	14.5	14.6	0.5	4.5	0.0	3.6	2.5	2.6	15.4	9.1	1.6
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	41.7	66.7	66.4	44.4	42.9	0.0	41.1	41.4	42.3	35.3	25.7	4.2
LnGrp LOS	D	E	E	D	D		D	D	D	D	C	A
Approach Vol, veh/h		1342			598	A		374			2751	
Approach Delay, s/veh		58.9			42.9			41.5			23.2	
Approach LOS		E			D			D			C	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	21.5	18.0	16.1	47.2	7.2	32.3	49.1	14.2
Change Period (Y+Rc), s	6.5	* 5.5	5.0	6.0	5.0	6.5	5.0	* 6
Max Green Setting (Gmax), s	15.0	* 13	18.0	33.0	5.0	21.5	46.0	* 5.5
Max Q Clear Time (g_c+I1), s	15.8	14.1	11.2	25.0	3.3	31.4	42.9	8.5
Green Ext Time (p_c), s	0.0	0.0	0.1	2.9	0.0	0.0	1.2	0.0

Intersection Summary	
HCM 6th Ctrl Delay	36.4
HCM 6th LOS	D

Notes

User approved pedestrian interval to be less than phase max green.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

MPAH Amendment Study - Villa Park Rd

Vistro File: N:\...\Villa Park_v6.vistro
Scenario 17 Future AM Peak Hour - Recommendations

Report File: N:\...\05_FYBAM.pdf
7/21/2023

Intersection Analysis Summary

ID	Intersection Name	Control Type	Method	Worst Mvmt	V/C	Delay (s/veh)	LOS
10	Villa Park Rd & Hewes St	Signalized	ICU 2	WB Thru	0.733	-	C
11	Villa Park Rd & Linda Vista St	Signalized	ICU 2	EB Thru	0.495	-	A
12	Santiago Canyon Rd & Cannon St	Signalized	ICU 2	SB Left	0.768	-	C

V/C, Delay, LOS: For two-way stop, these values are taken from the movement with the worst (highest) delay value. For all other control types, they are taken for the whole intersection.

Intersection Level Of Service Report
Intersection 10: Villa Park Rd & Hewes St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	C
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.733

Intersection Setup

Name											
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0	0	0	1	1	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	215.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			No			Yes			No		

Volumes

Name												
Base Volume Input [veh/h]	210	0	310	0	1	0	0	1050	220	700	1060	0
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	210	0	310	0	1	0	0	1050	220	700	1060	0
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	53	0	78	0	0	0	0	263	55	175	265	0
Total Analysis Volume [veh/h]	210	0	310	0	1	0	0	1050	220	700	1060	0
Pedestrian Volume [ped/h]	2			0			1			0		
Bicycle Volume [bicycles/h]	1			0			0			0		

Intersection Settings

Cycle Length [s]	145
Lost time [s]	7.00

Phasing & Timing

Control Type	Split	Permiss	Split	Split	Split	Split	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	4	0	0	0	8	0	0	2	0	1	6	0
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	-	-	-	-	-	-	Lead	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.12	0.00	0.18	0.00	0.00	0.00	0.00	0.31	0.13	0.21	0.31	0.00
Intersection LOS	C											
Intersection V/C	0.733											

Intersection Level Of Service Report
Intersection 11: Villa Park Rd & Linda Vista St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	A
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.495

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0	1	0	1	1	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	115.00	100.00	100.00	140.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			Yes			No			No		

Volumes

Name												
Base Volume Input [veh/h]	10	10	30	30	0	110	70	1250	40	10	1640	100
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	10	10	30	30	0	110	70	1250	40	10	1640	100
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	3	3	8	8	0	28	18	313	10	3	410	25
Total Analysis Volume [veh/h]	10	10	30	30	0	110	70	1250	40	10	1640	100
Pedestrian Volume [ped/h]	0			0			0			0		
Bicycle Volume [bicycles/h]	0			0			0			0		

Intersection Settings

Cycle Length [s]	100
Lost time [s]	5.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	0	4	0	0	8	0	5	2	0	1	6	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	Lead	-	-	Lag	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.01	0.03	0.03	0.02	0.00	0.08	0.04	0.37	0.02	0.01	0.34	0.34
Intersection LOS	A											
Intersection V/C	0.495											

Intersection Level Of Service Report
Intersection 12: Santiago Canyon Rd & Cannon St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	C
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.768

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	2	0	1	2	0	0	1	0	1
Entry Pocket Length [ft]	80.00	100.00	100.00	375.00	100.00	250.00	325.00	100.00	100.00	150.00	100.00	450.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			No			Yes			No		

Volumes

Name												
Base Volume Input [veh/h]	150	190	40	1300	480	1040	400	810	100	20	560	500
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	150	190	40	1300	480	1040	400	810	100	20	560	500
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	38	48	10	325	120	260	100	203	25	5	140	125
Total Analysis Volume [veh/h]	150	190	40	1300	480	1040	400	810	100	20	560	500
Pedestrian Volume [ped/h]	0			0			6			0		
Bicycle Volume [bicycles/h]	3			0			0			1		

Intersection Settings

Cycle Length [s]	100
Lost time [s]	5.00

Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Unsigna
Signal Group	3	8	0	7	4	4	1	6	0	5	2	0
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	Lead	-	-	Lag	-	-	Lead	-	-

Movement, Approach, & Intersection Results

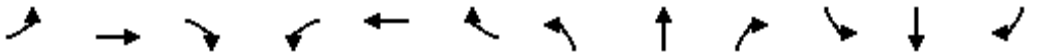
V/C, Movement V/C Ratio	0.09	0.07	0.07	0.38	0.28	0.31	0.12	0.27	0.27	0.01	0.11	0.00
Intersection LOS	C											
Intersection V/C	0.768											



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Volume (vph)	1040	140	310	1340	150	600
Future Volume (vph)	1040	140	310	1340	150	600
Ideal Flow (vphpl)	1700	1700	1700	1700	1700	1700
Total Lost time (s)	2.5	2.5	1.0	2.5	1.5	1.5
Lane Util. Factor	0.95	1.00	0.97	0.95	1.00	1.00
Frpb, ped/bikes	1.00	0.99	1.00	1.00	1.00	1.00
Flpb, ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.85	1.00	1.00	1.00	0.85
Flt Protected	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (prot)	3167	1398	3072	3167	1568	1403
Flt Permitted	1.00	1.00	0.95	1.00	0.95	1.00
Satd. Flow (perm)	3167	1398	3072	3167	1568	1403
Peak-hour factor, PHF	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	1095	147	326	1411	158	632
RTOR Reduction (vph)	0	20	0	0	0	550
Lane Group Flow (vph)	1095	127	326	1411	158	82
Confl. Bikes (#/hr)		3				
Heavy Vehicles (%)	2%	2%	2%	2%	3%	3%
Turn Type	NA	Perm	Prot	NA	Prot	Prot
Protected Phases	2		1	6	4	4
Permitted Phases		2				
Actuated Green, G (s)	68.4	68.4	8.0	82.4	8.6	8.6
Effective Green, g (s)	73.4	73.4	13.0	87.4	13.6	13.6
Actuated g/C Ratio	0.70	0.70	0.12	0.83	0.13	0.13
Clearance Time (s)	7.5	7.5	6.0	7.5	6.5	6.5
Vehicle Extension (s)	3.0	3.0	3.0	3.0	2.0	2.0
Lane Grp Cap (vph)	2213	977	380	2636	203	181
v/s Ratio Prot	0.35		c0.11	c0.45	c0.10	0.06
v/s Ratio Perm		0.09				
v/c Ratio	0.49	0.13	0.86	0.54	0.78	0.45
Uniform Delay, d1	7.3	5.2	45.1	2.7	44.2	42.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.8	0.3	17.1	0.8	15.6	0.7
Delay (s)	8.1	5.5	62.2	3.4	59.9	42.9
Level of Service	A	A	E	A	E	D
Approach Delay (s)	7.8			14.5	46.3	
Approach LOS	A			B	D	

Intersection Summary			
HCM 2000 Control Delay	18.9	HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.63		
Actuated Cycle Length (s)	105.0	Sum of lost time (s)	10.0
Intersection Capacity Utilization	80.3%	ICU Level of Service	D
Analysis Period (min)	15		

c Critical Lane Group



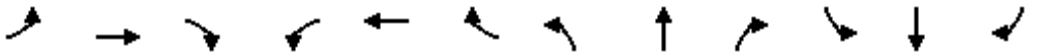
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	10	1600	30	10	1620	20	10	0	10	10	0	20
Future Volume (veh/h)	10	1600	30	10	1620	20	10	0	10	10	0	20
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.98	1.00		0.98	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1673	1673	1700	1700	1673	1673	1700	1700	1700	1660	1660	1660
Adj Flow Rate, veh/h	11	1684	32	11	1705	21	11	0	11	11	0	21
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	0	0	2	2	0	0	0	3	3	3
Cap, veh/h	232	2435	1080	21	2887	36	74	12	41	58	9	51
Arrive On Green	0.15	0.77	0.77	0.03	1.00	1.00	0.05	0.00	0.05	0.05	0.00	0.05
Sat Flow, veh/h	1594	3180	1410	1619	4650	57	529	216	745	321	170	938
Grp Volume(v), veh/h	11	1684	32	11	1117	609	22	0	0	32	0	0
Grp Sat Flow(s),veh/h/ln	1594	1590	1410	1619	1523	1662	1490	0	0	1430	0	0
Q Serve(g_s), s	0.7	31.6	0.7	0.8	0.0	0.0	0.0	0.0	0.0	0.4	0.0	0.0
Cycle Q Clear(g_c), s	0.7	31.6	0.7	0.8	0.0	0.0	1.6	0.0	0.0	2.5	0.0	0.0
Prop In Lane	1.00		1.00	1.00		0.03	0.50		0.50	0.34		0.66
Lane Grp Cap(c), veh/h	232	2435	1080	21	1891	1032	125	0	0	117	0	0
V/C Ratio(X)	0.05	0.69	0.03	0.53	0.59	0.59	0.18	0.00	0.00	0.27	0.00	0.00
Avail Cap(c_a), veh/h	232	2435	1080	67	1891	1032	291	0	0	280	0	0
HCM Platoon Ratio	1.00	1.00	1.00	2.00	2.00	2.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	0.53	0.53	0.53	1.00	0.00	0.00	1.00	0.00	0.00
Uniform Delay (d), s/veh	44.1	7.0	3.4	58.1	0.0	0.0	54.4	0.0	0.0	54.9	0.0	0.0
Incr Delay (d2), s/veh	0.1	1.6	0.1	10.7	0.7	1.3	0.2	0.0	0.0	0.5	0.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.3	7.8	0.1	0.4	0.2	0.4	0.6	0.0	0.0	1.0	0.0	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	44.2	8.6	3.4	68.8	0.7	1.3	54.7	0.0	0.0	55.3	0.0	0.0
LnGrp LOS	D	A	A	E	A	A	D	A	A	E	A	A
Approach Vol, veh/h		1727			1737			22			32	
Approach Delay, s/veh		8.8			1.4			54.7			55.3	
Approach LOS		A			A			D			E	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	7.5	99.4		13.1	24.9	82.0		13.1				
Change Period (Y+Rc), s	6.0	7.5		6.5	7.5	* 7.5		6.5				
Max Green Setting (Gmax), s	5.0	74.5		20.5	5.0	* 75		20.5				
Max Q Clear Time (g_c+I1), s	2.8	33.6		3.6	2.7	2.0		4.5				
Green Ext Time (p_c), s	0.0	17.3		0.0	0.0	17.2		0.1				

Intersection Summary			
HCM 6th Ctrl Delay		5.8	
HCM 6th LOS		A	

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	1000	540	80	20	1040	1530	80	460	20	500	250	530
Future Volume (veh/h)	1000	540	80	20	1040	1530	80	460	20	500	250	530
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		0.99	1.00		1.00	1.00		0.99	1.00		0.99
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1673	1673	1673	1673	1673	1673	1687	1687	1687	1687	1687	1687
Adj Flow Rate, veh/h	1042	562	75	21	1083	0	83	479	19	521	260	492
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	2	2	2	2	2	2	1	1	1	1	1	1
Cap, veh/h	1145	1595	212	93	1129		153	497	20	493	388	1552
Arrive On Green	0.37	0.57	0.56	0.06	0.25	0.00	0.09	0.16	0.12	0.16	0.23	0.23
Sat Flow, veh/h	3092	2815	375	1594	4569	1418	1606	3140	124	3116	1687	2480
Grp Volume(v), veh/h	1042	317	320	21	1083	0	83	244	254	521	260	492
Grp Sat Flow(s),veh/h/ln	1546	1590	1600	1594	1523	1418	1606	1602	1662	1558	1687	1240
Q Serve(g_s), s	38.4	12.9	13.0	1.5	28.1	0.0	5.9	18.1	18.2	19.0	16.8	11.3
Cycle Q Clear(g_c), s	38.4	12.9	13.0	1.5	28.1	0.0	5.9	18.1	18.2	19.0	16.8	11.3
Prop In Lane	1.00		0.23	1.00		1.00	1.00		0.07	1.00		1.00
Lane Grp Cap(c), veh/h	1145	901	906	93	1129		153	254	263	493	388	1552
V/C Ratio(X)	0.91	0.35	0.35	0.23	0.96		0.54	0.96	0.97	1.06	0.67	0.32
Avail Cap(c_a), veh/h	1397	901	906	126	1129		153	254	263	493	388	1552
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	35.9	14.1	14.1	53.9	44.6	0.0	51.8	50.1	50.3	50.5	42.1	10.8
Incr Delay (d2), s/veh	7.2	1.1	1.1	0.5	18.6	0.0	2.2	45.5	45.4	56.1	3.6	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	14.7	4.5	4.6	0.6	12.0	0.0	2.4	10.3	10.7	11.0	7.2	2.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	43.1	15.2	15.2	54.3	63.2	0.0	54.1	95.6	95.8	106.6	45.7	10.8
LnGrp LOS	D	B	B	D	E		D	F	F	F	D	B
Approach Vol, veh/h		1679			1104	A		581			1273	
Approach Delay, s/veh		32.5			63.0			89.8			57.1	
Approach LOS		C			E			F			E	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	47.2	31.8	12.0	29.0	7.5	71.5	21.0	20.0
Change Period (Y+Rc), s	5.0	* 6.5	5.0	6.0	5.0	6.5	6.0	* 5.5
Max Green Setting (Gmax), s	52.0	* 18	7.0	22.0	5.0	63.5	15.0	* 15
Max Q Clear Time (g_c+I1), s	40.4	30.1	7.9	18.8	3.5	15.0	21.0	20.2
Green Ext Time (p_c), s	1.8	0.0	0.0	0.8	0.0	2.1	0.0	0.0

Intersection Summary	
HCM 6th Ctrl Delay	53.7
HCM 6th LOS	D

Notes

User approved pedestrian interval to be less than phase max green.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [WBR] is excluded from calculations of the approach delay and intersection delay.

MPAH Amendment Study - Villa Park Rd

Vistro File: N:\...\Villa Park_v6.vistro
Scenario 18 Future PM Peak Hour - Recommendation

Report File: N:\...\05_FYBPM.pdf
7/21/2023

Intersection Analysis Summary

ID	Intersection Name	Control Type	Method	Worst Mvmt	V/C	Delay (s/veh)	LOS
10	Villa Park Rd & Hewes St	Signalized	ICU 2	WB Thru	0.791	-	C
11	Villa Park Rd & Linda Vista St	Signalized	ICU 2	EB Thru	0.526	-	A
12	Santiago Canyon Rd & Cannon St	Signalized	ICU 2	EB Left	0.828	-	D

V/C, Delay, LOS: For two-way stop, these values are taken from the movement with the worst (highest) delay value. For all other control types, they are taken for the whole intersection.

Intersection Level Of Service Report
Intersection 10: Villa Park Rd & Hewes St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	C
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.791

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0	0	0	1	1	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	100.00	215.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			No			Yes			No		

Volumes

Name												
Base Volume Input [veh/h]	150	0	600	0	6	0	0	1040	140	310	1340	0
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	150	0	600	0	6	0	0	1040	140	310	1340	0
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	38	0	150	0	2	0	0	260	35	78	335	0
Total Analysis Volume [veh/h]	150	0	600	0	6	0	0	1040	140	310	1340	0
Pedestrian Volume [ped/h]	0			0			0			0		
Bicycle Volume [bicycles/h]	3			0			6			0		

Intersection Settings

Cycle Length [s]	105
Lost time [s]	5.00

Phasing & Timing

Control Type	Split	Permiss	Split	Split	Split	Split	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	4	0	0	0	8	0	0	2	0	1	6	0
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	-	-	-	-	-	-	Lead	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.09	0.00	0.35	0.00	0.00	0.00	0.00	0.31	0.08	0.09	0.39	0.00
Intersection LOS	C											
Intersection V/C	0.791											

Intersection Level Of Service Report
Intersection 11: Villa Park Rd & Linda Vista St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	A
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.526

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	0	0	0	0	0	0	1	0	1	1	0	0
Entry Pocket Length [ft]	100.00	100.00	100.00	100.00	100.00	100.00	115.00	100.00	100.00	140.00	100.00	100.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			Yes			No			No		

Volumes

Name												
Base Volume Input [veh/h]	10	0	10	10	0	20	10	1600	30	10	1620	20
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	10	0	10	10	0	20	10	1600	30	10	1620	20
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	3	0	3	3	0	5	3	400	8	3	405	5
Total Analysis Volume [veh/h]	10	0	10	10	0	20	10	1600	30	10	1620	20
Pedestrian Volume [ped/h]	0			0			0			0		
Bicycle Volume [bicycles/h]	0			0			0			0		

Intersection Settings

Cycle Length [s]	120
Lost time [s]	6.00

Phasing & Timing

Control Type	Permiss	Permiss	Permiss	Permiss	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss
Signal Group	0	4	0	0	8	0	5	2	0	1	6	0
Auxiliary Signal Groups												
Lead / Lag	-	-	-	-	-	-	Lag	-	-	Lead	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.01	0.00	0.01	0.01	0.00	0.02	0.01	0.47	0.02	0.01	0.32	0.32
Intersection LOS	A											
Intersection V/C	0.526											

Intersection Level Of Service Report
Intersection 12: Santiago Canyon Rd & Cannon St

Control Type:	Signalized	Delay (sec / veh):	-
Analysis Method:	ICU 2	Level Of Service:	D
Analysis Period:	1 hour	Volume to Capacity (v/c):	0.828

Intersection Setup

Name												
Approach	Northbound			Southbound			Eastbound			Westbound		
Lane Configuration												
Turning Movement	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Lane Width [ft]	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00	12.00
No. of Lanes in Entry Pocket	1	0	0	2	0	1	2	0	0	1	0	1
Entry Pocket Length [ft]	80.00	100.00	100.00	375.00	100.00	250.00	325.00	100.00	100.00	150.00	100.00	450.00
No. of Lanes in Exit Pocket	0	0	0	0	0	0	0	0	0	0	0	0
Exit Pocket Length [ft]	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Speed [mph]	30.00			30.00			30.00			30.00		
Grade [%]	0.00			0.00			0.00			0.00		
Crosswalk	Yes			No			Yes			No		

Volumes

Name												
Base Volume Input [veh/h]	80	460	20	500	250	530	1000	540	80	20	1040	1530
Base Volume Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Heavy Vehicles Percentage [%]	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00	2.00
Growth Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
In-Process Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Site-Generated Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Diverted Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Pass-by Trips [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Existing Site Adjustment Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Other Volume [veh/h]	0	0	0	0	0	0	0	0	0	0	0	0
Total Hourly Volume [veh/h]	80	460	20	500	250	530	1000	540	80	20	1040	1530
Peak Hour Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Other Adjustment Factor	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000	1.0000
Total 15-Minute Volume [veh/h]	20	115	5	125	63	133	250	135	20	5	260	383
Total Analysis Volume [veh/h]	80	460	20	500	250	530	1000	540	80	20	1040	1530
Pedestrian Volume [ped/h]	1			0			1			0		
Bicycle Volume [bicycles/h]	3			3			1			1		

Intersection Settings

Cycle Length [s]	120
Lost time [s]	6.00

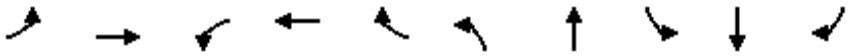
Phasing & Timing

Control Type	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Permiss	Protecte	Permiss	Unsigna
Signal Group	3	8	0	7	4	4	1	6	0	5	2	0
Auxiliary Signal Groups												
Lead / Lag	Lead	-	-	Lag	-	-	Lead	-	-	Lead	-	-

Movement, Approach, & Intersection Results

V/C, Movement V/C Ratio	0.05	0.14	0.14	0.15	0.15	0.16	0.29	0.18	0.18	0.01	0.20	0.00
Intersection LOS	D											
Intersection V/C	0.828											

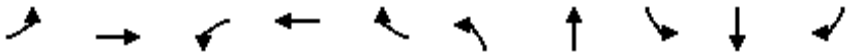
Attachment D: Intersection Queueing Analysis Worksheets



Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	375	884	16	484	377	143	149	894	769	736
v/c Ratio	0.66	0.83	0.11	0.75	0.27	0.60	0.31	0.82	0.69	0.79
Control Delay	46.1	53.4	53.0	56.5	0.5	61.4	39.3	43.7	38.1	14.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	46.1	53.4	53.0	56.5	0.5	61.4	39.3	43.7	38.1	14.5
Queue Length 50th (ft)	168	395	13	200	0	113	46	409	326	161
Queue Length 95th (ft)	m214	#526	35	262	0	180	77	#600	441	288
Internal Link Dist (ft)		700		272			165		561	
Turn Bay Length (ft)	325		150			80		375		250
Base Capacity (vph)	654	1059	206	687	1403	289	581	1095	1120	970
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.57	0.83	0.08	0.70	0.27	0.49	0.26	0.82	0.69	0.76

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
m Volume for 95th percentile queue is metered by upstream signal.



Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	735	447	14	1038	1539	73	343	315	324	456
v/c Ratio	0.88	0.25	0.10	0.92	1.09	0.34	0.79	0.62	0.62	0.59
Control Delay	50.7	11.3	57.8	56.4	56.3	59.7	72.4	57.4	57.1	11.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	50.7	11.3	57.8	56.4	56.3	59.7	72.4	57.4	57.1	11.1
Queue Length 50th (ft)	325	91	12	476	~231	61	160	157	161	110
Queue Length 95th (ft)	408	189	34	#741	#495	113	#227	194	198	137
Internal Link Dist (ft)		700		272			165		561	
Turn Bay Length (ft)	325		150			80		375		250
Base Capacity (vph)	875	1809	193	1133	1417	217	433	746	771	791
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.84	0.25	0.07	0.92	1.09	0.34	0.79	0.42	0.42	0.58

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

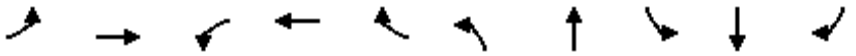


Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Group Flow (vph)	1105	232	737	1116	221	326
v/c Ratio	0.94	0.42	1.14	0.45	1.32	0.74
Control Delay	55.7	29.6	119.0	7.1	228.7	16.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	55.7	29.6	119.0	7.1	228.7	16.9
Queue Length 50th (ft)	475	106	~810	86	~269	0
Queue Length 95th (ft)	#1009	#307	#1059	405	#441	105
Internal Link Dist (ft)	2348			454	338	
Turn Bay Length (ft)		100	215			
Base Capacity (vph)	1181	556	647	2496	167	441
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.94	0.42	1.14	0.45	1.32	0.74

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	412	938	21	577	515	155	237	911	924	1072
v/c Ratio	0.48	0.70	0.14	0.95	0.37	0.93	0.70	0.91	0.90	1.08
Control Delay	32.0	26.5	44.0	67.7	0.7	100.6	51.3	44.4	42.7	66.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	32.0	26.5	44.0	67.7	0.7	100.6	51.3	44.4	42.7	66.3
Queue Length 50th (ft)	111	223	13	193	0	100	71	316	318	~361
Queue Length 95th (ft)	158	354	36	#302	0	#223	#119	#455	#455	#1009
Internal Link Dist (ft)		700		272			165		561	
Turn Bay Length (ft)	325		150			80		375		250
Base Capacity (vph)	850	1347	148	605	1403	166	340	1002	1030	990
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.48	0.70	0.14	0.95	0.37	0.93	0.70	0.91	0.90	1.08

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

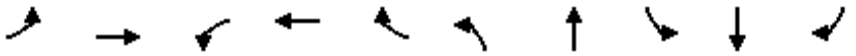
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Group Flow (vph)	1095	147	326	1411	158	632
v/c Ratio	0.62	0.18	0.73	0.53	0.83	0.87
Control Delay	23.1	12.2	53.4	3.6	90.1	17.9
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	23.1	12.2	53.4	3.6	90.1	17.9
Queue Length 50th (ft)	329	42	259	135	137	0
Queue Length 95th (ft)	464	92	337	163	#261	#205
Internal Link Dist (ft)	2348			454	338	
Turn Bay Length (ft)		100	215			
Base Capacity (vph)	1772	799	551	2686	191	726
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.62	0.18	0.59	0.53	0.83	0.87

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	1042	646	21	1083	1594	83	500	386	395	552
v/c Ratio	0.97	0.38	0.21	1.43	1.12	0.29	0.89	0.76	0.75	0.61
Control Delay	69.6	20.2	72.0	239.3	72.4	56.9	78.2	40.8	40.2	20.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	69.6	20.2	72.0	239.3	72.4	56.9	78.2	40.8	40.2	20.2
Queue Length 50th (ft)	517	194	20	~770	~359	71	253	176	180	452
Queue Length 95th (ft)	#665	243	50	#909	#623	125	#350	m247	m251	593
Internal Link Dist (ft)		700		272			165		561	
Turn Bay Length (ft)	325		150			80		375		250
Base Capacity (vph)	1071	1719	100	760	1417	283	565	540	559	906
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.97	0.38	0.21	1.43	1.12	0.29	0.88	0.71	0.71	0.61

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

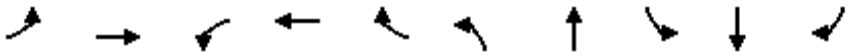


Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Group Flow (vph)	1105	232	737	1116	221	326
v/c Ratio	0.71	0.32	0.87	0.45	1.17	0.72
Control Delay	32.1	19.3	61.2	7.8	172.0	15.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	32.1	19.3	61.2	7.8	172.0	15.4
Queue Length 50th (ft)	368	77	339	99	~248	0
Queue Length 95th (ft)	#823	220	422	420	#420	103
Internal Link Dist (ft)	2348			454	338	
Turn Bay Length (ft)		100	215			
Base Capacity (vph)	1558	720	864	2454	189	455
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.71	0.32	0.85	0.45	1.17	0.72

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	412	938	21	577	515	155	237	1340	495	1072
v/c Ratio	0.74	0.96	0.14	0.75	0.37	0.56	0.62	0.92	0.71	0.62
Control Delay	47.5	55.4	44.0	46.3	0.7	44.9	46.6	35.9	32.3	4.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	47.5	55.4	44.0	46.3	0.7	44.9	46.6	35.9	32.3	4.6
Queue Length 50th (ft)	128	281	13	129	0	91	71	387	259	36
Queue Length 95th (ft)	181	#500	36	171	0	149	#119	#540	#447	75
Internal Link Dist (ft)		700		272			165		561	
Turn Bay Length (ft)	325		150			80		375		250
Base Capacity (vph)	566	977	148	770	1403	346	385	1489	697	1734
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.73	0.96	0.14	0.75	0.37	0.45	0.62	0.90	0.71	0.62

Intersection Summary

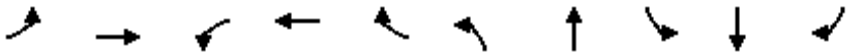
95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	EBT	EBR	WBL	WBT	NBL	NBR
Lane Group Flow (vph)	1095	147	326	1411	158	632
v/c Ratio	0.49	0.15	0.86	0.54	0.78	0.86
Control Delay	8.2	3.3	67.3	3.5	70.4	17.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	8.2	3.3	67.3	3.5	70.4	17.2
Queue Length 50th (ft)	156	16	112	108	104	0
Queue Length 95th (ft)	199	35	#188	136	#210	#191
Internal Link Dist (ft)	2348			454	338	
Turn Bay Length (ft)		100	215			
Base Capacity (vph)	2213	996	380	2636	203	731
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.49	0.15	0.86	0.54	0.78	0.86

Intersection Summary

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.



Lane Group	EBL	EBT	WBL	WBT	WBR	NBL	NBT	SBL	SBT	SBR
Lane Group Flow (vph)	1042	646	21	1083	1594	83	500	521	260	552
v/c Ratio	0.84	0.34	0.17	1.04	1.12	0.56	0.99	1.06	0.64	0.32
Control Delay	38.8	12.6	55.0	83.6	72.6	67.1	88.0	106.0	50.2	5.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	38.8	12.6	55.0	83.6	72.6	67.1	88.0	106.0	50.2	5.4
Queue Length 50th (ft)	362	107	15	~343	~285	62	204	~228	187	57
Queue Length 95th (ft)	421	177	42	#488	#551	117	#321	#339	283	74
Internal Link Dist (ft)		700		272			165		561	
Turn Bay Length (ft)	325		150			80		375		250
Base Capacity (vph)	1387	1880	125	1043	1417	151	505	491	407	1859
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.75	0.34	0.17	1.04	1.12	0.55	0.99	1.06	0.64	0.30

Intersection Summary

~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

Attachment E: Traffic Signal Warrants

FEHR PEERS

Intersection	2	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Kathleen Ln	Peak Hour	AM

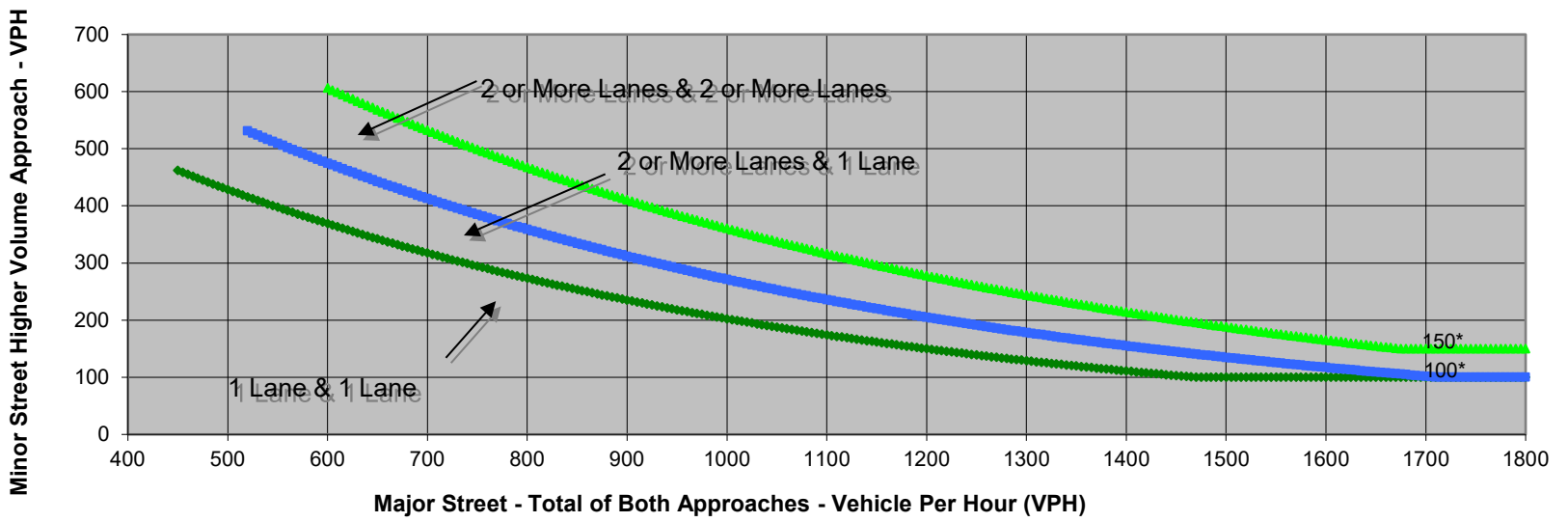
Turn Movement Volumes

	NB	SB	EB	WB
Left	0	1	8	0
Through	0	0	1249	1134
Right	0	9	0	0
Total	0	10	1,257	1,134

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	E Trenton Ave	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,391**	**10**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	3	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Morrow Cir	Peak Hour	AM

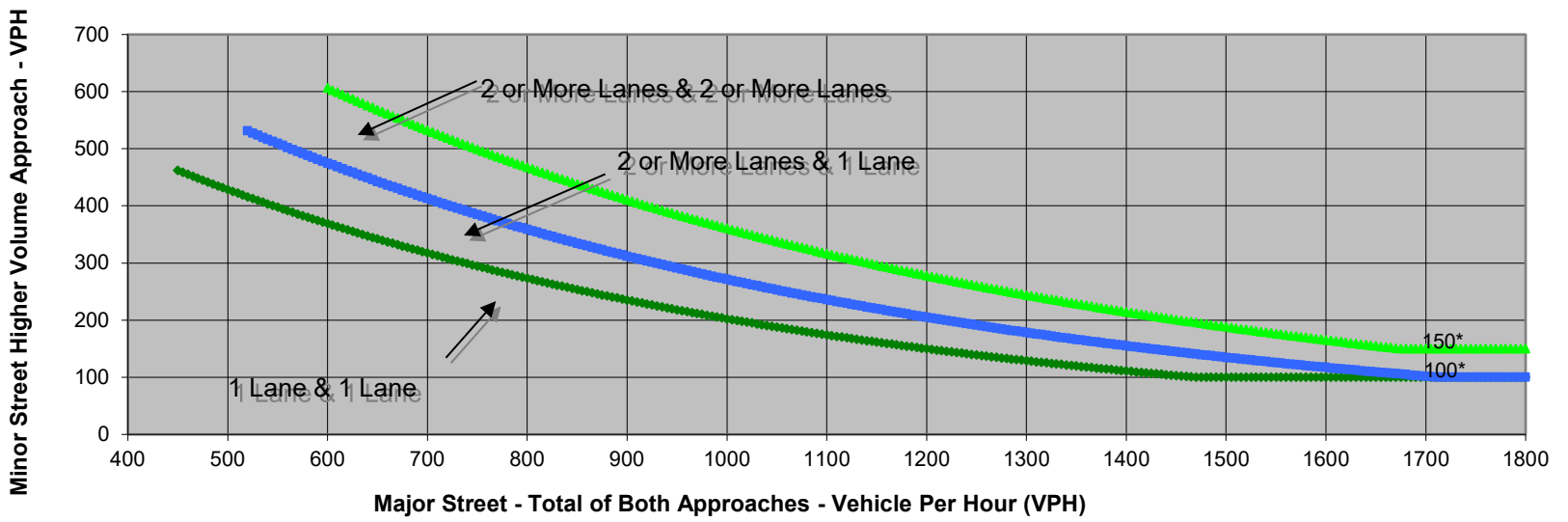
Turn Movement Volumes

	NB	SB	EB	WB
Left	4	0	0	2
Through	0	0	1245	1130
Right	7	0	5	0
Total	11	0	1,250	1,132

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	Morrow Cir	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,382**	**11**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	4	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Prado Woods Dr	Peak Hour	AM

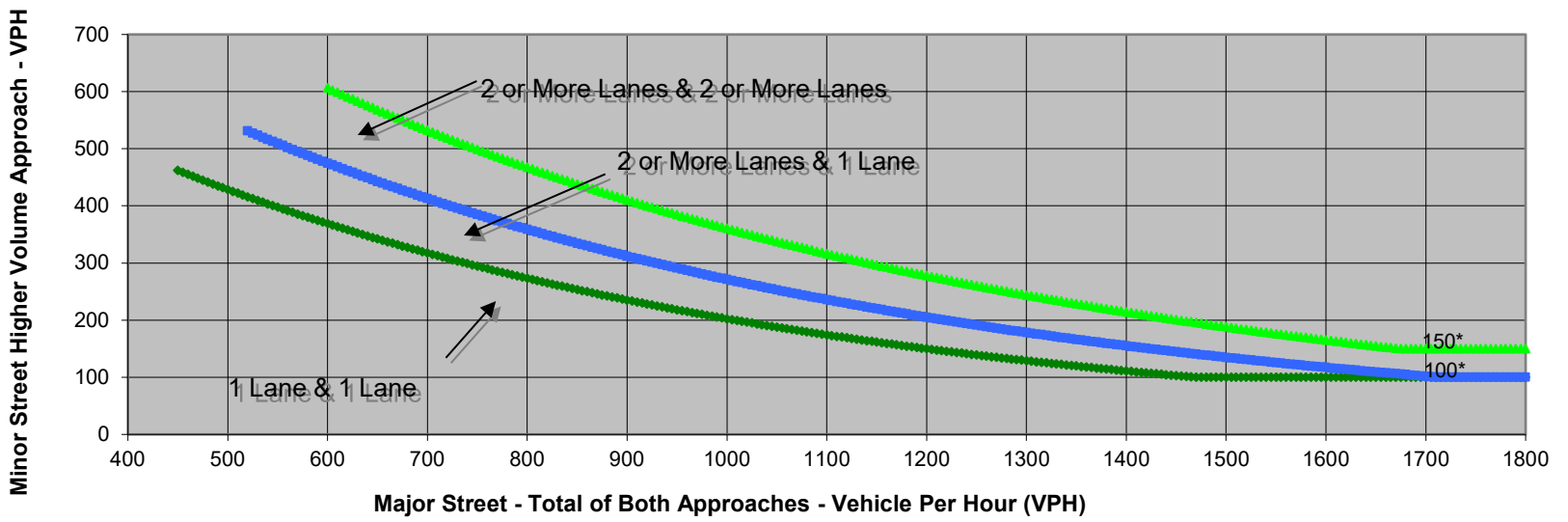
Turn Movement Volumes

	NB	SB	EB	WB
Left	0	4	14	0
Through	0	0	1238	1119
Right	0	13	0	3
Total	0	17	1,252	1,122

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	Prado Woods Dr	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,374**	**17**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	5	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Kenwick Dr	Peak Hour	AM

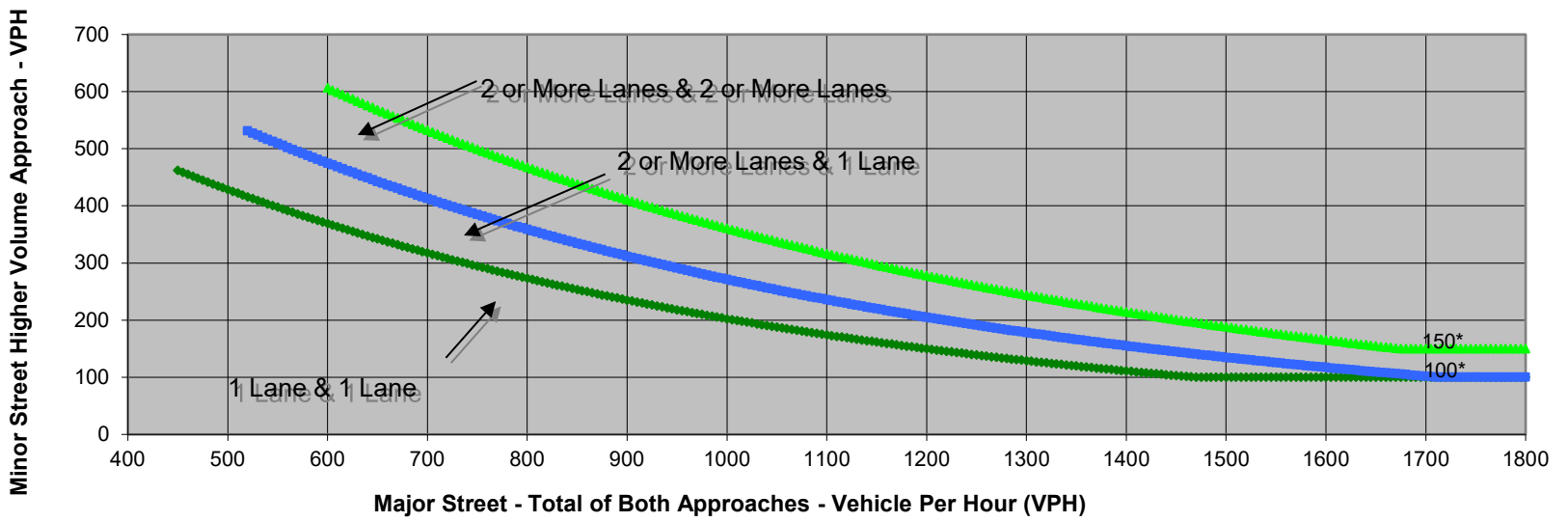
Turn Movement Volumes

	NB	SB	EB	WB
Left	0	3	4	0
Through	0	0	1238	1115
Right	0	7	0	0
Total	0	10	1,242	1,115

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	Kenwick Dr	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,357**	**10**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	6	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Radec Ct	Peak Hour	AM

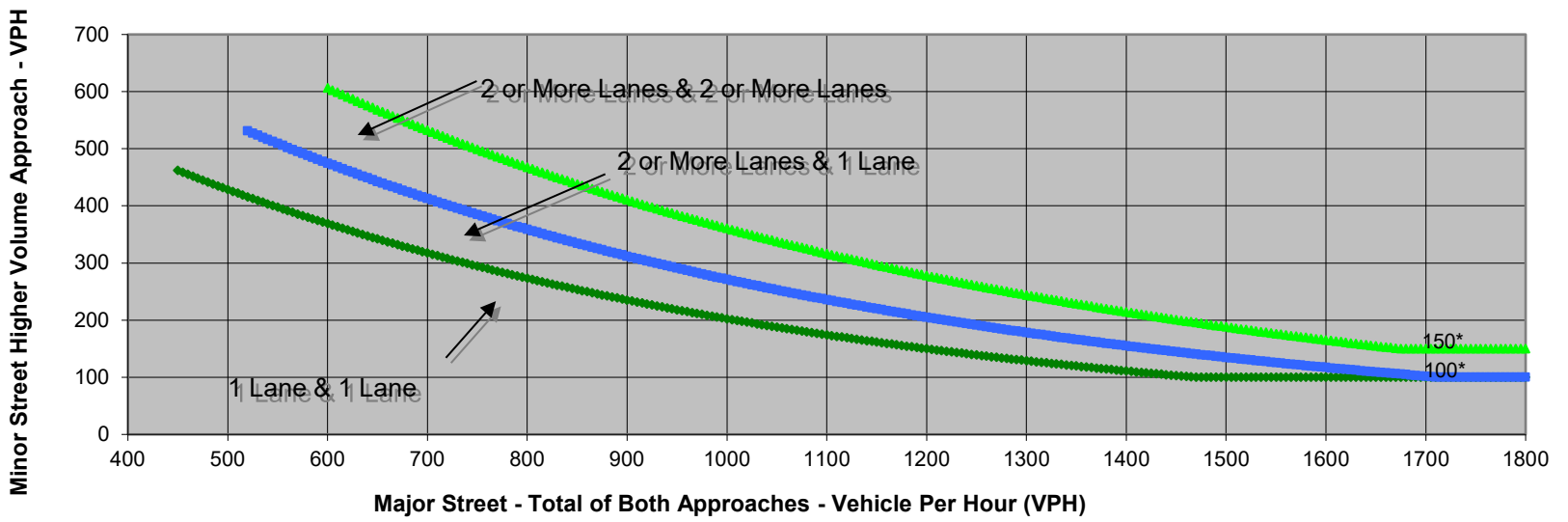
Turn Movement Volumes

	NB	SB	EB	WB
Left	1	0	0	1
Through	0	0	1240	1115
Right	2	0	1	0
Total	3	0	1,241	1,116

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	Radec Ct	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,357**	**3**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	8	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Park Villa Ln	Peak Hour	AM

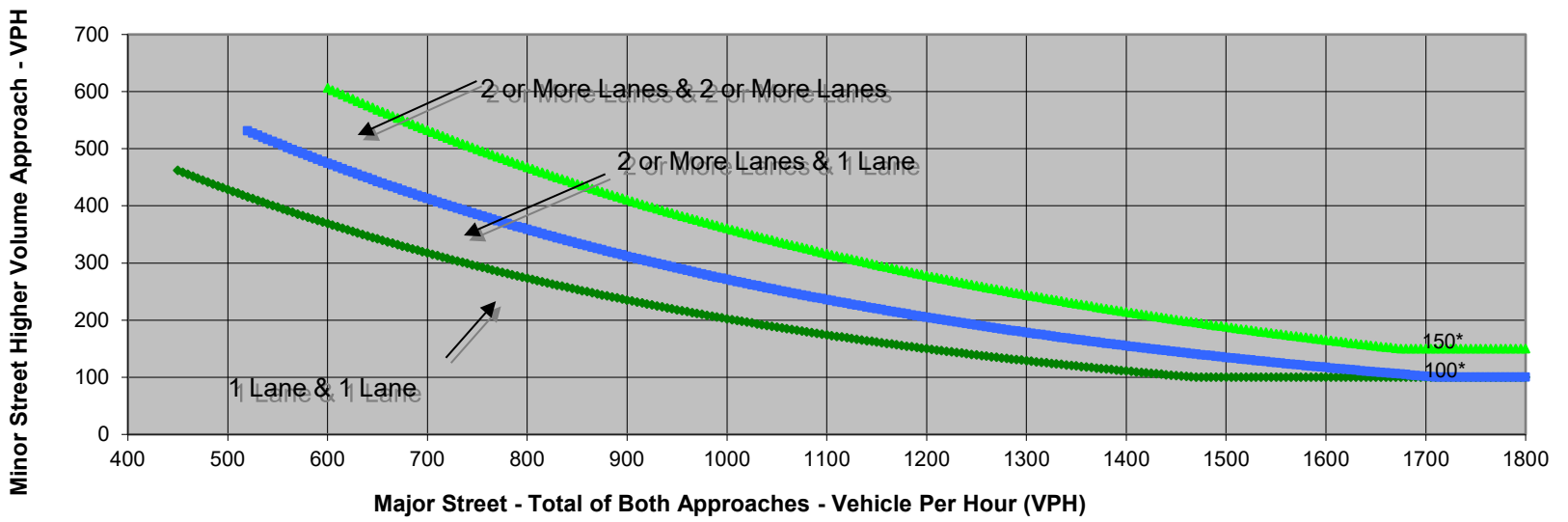
Turn Movement Volumes

	NB	SB	EB	WB
Left	2	0	0	5
Through	0	0	1160	1010
Right	2	0	6	0
Total	4	0	1,166	1,015

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	Park Villa Ln	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,181**	**4**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	11	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	N Linda Vista St	Peak Hour	AM

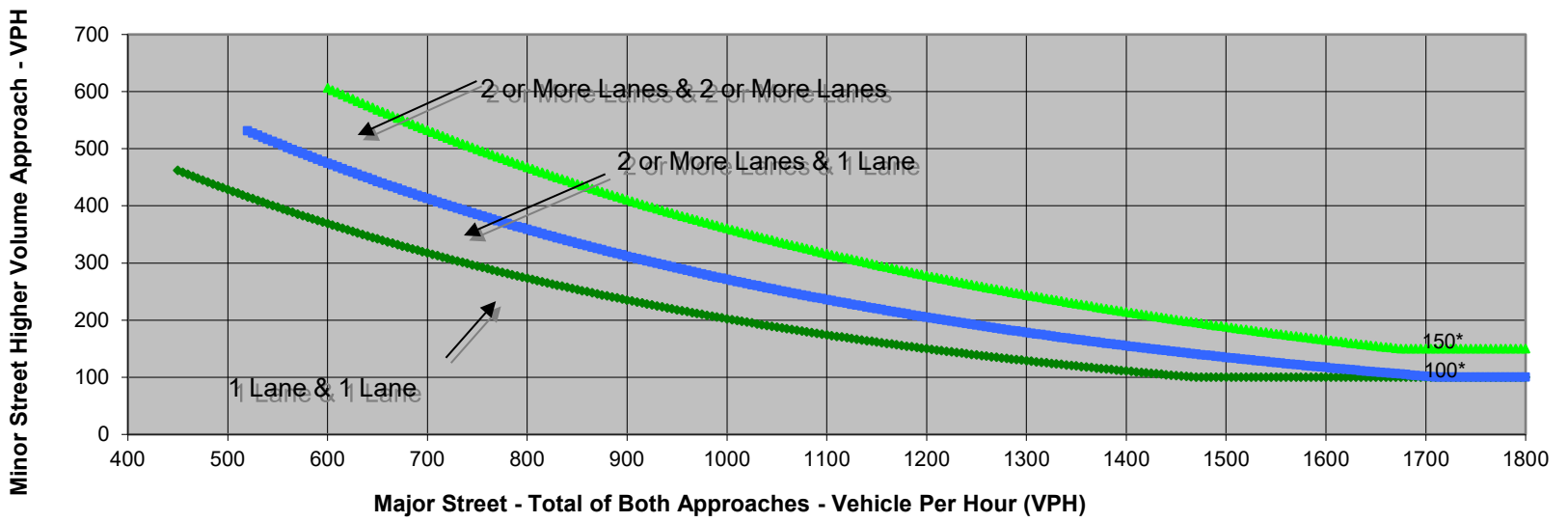
Turn Movement Volumes

	NB	SB	EB	WB
Left	9	27	68	5
Through	1	0	1173	1218
Right	21	103	39	99
Total	31	130	1,280	1,322

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	N Linda Vista St	
Number of Approach Lanes	**2**	**1**	**YES**
Traffic Volume (VPH) *	**2,602**	**130**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR & PEERS

Intersection	15	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Loma St	Scenario	Future Year (2045) Conditions
Minor Street	Cannon St	Peak Hour	AM

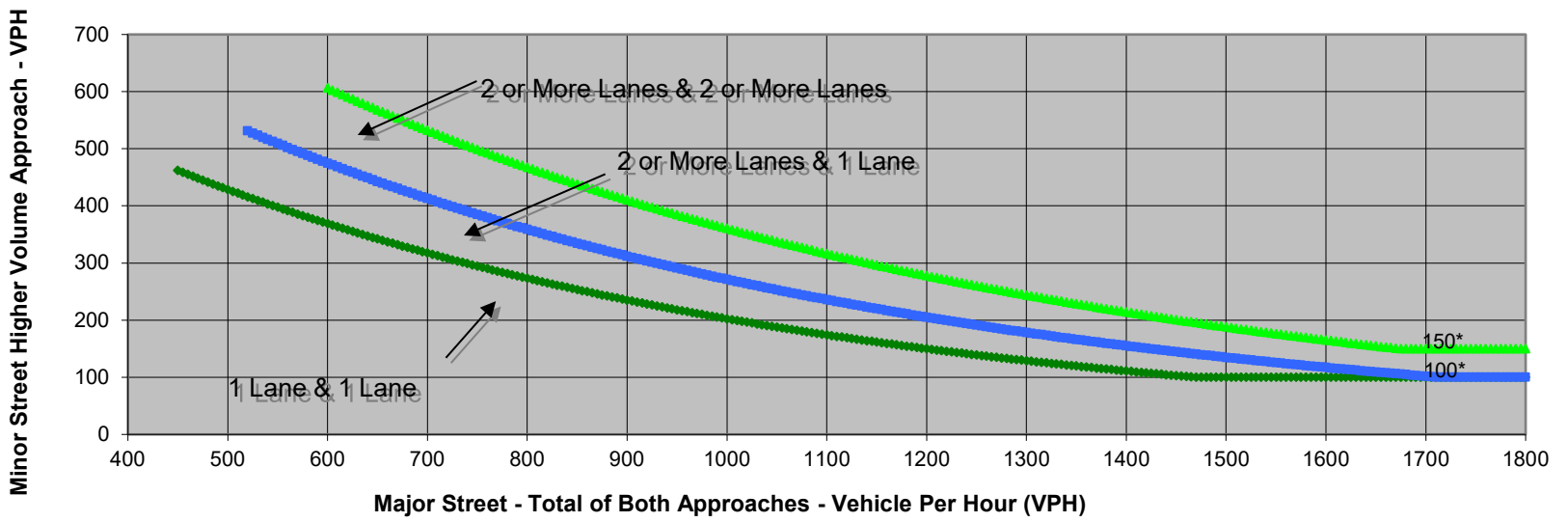
Turn Movement Volumes

	NB	SB	EB	WB
Left	28	0	34	0
Through	678	1419	0	0
Right	0	43	45	0
Total	706	1,462	79	0

Major Street Direction

x	North/South
	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Mesa Dr	Cannon St	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,168**	**79**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	2	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Kathleen Ln	Peak Hour	PM

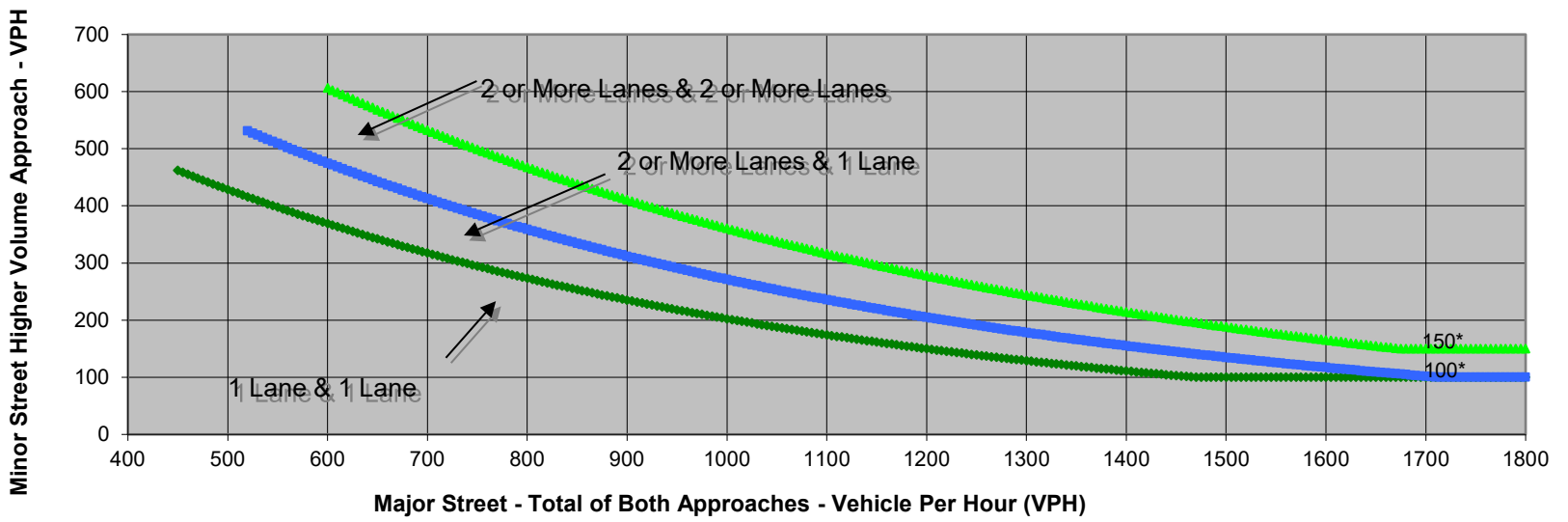
Turn Movement Volumes

	NB	SB	EB	WB
Left	0	0	4	0
Through	0	0	1065	1347
Right	0	2	0	0
Total	0	2	1,069	1,347

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	E Trenton Ave	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,416**	**2**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	3	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Morrow Cir	Peak Hour	PM

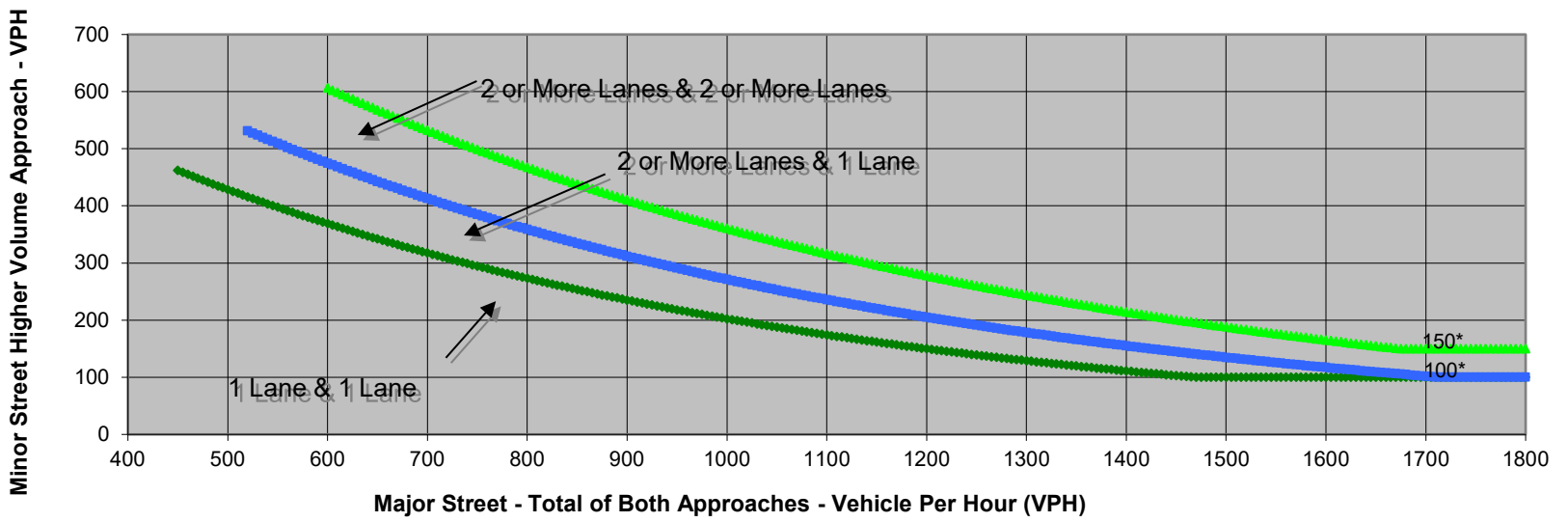
Turn Movement Volumes

	NB	SB	EB	WB
Left	4	0	0	2
Through	0	0	1064	1343
Right	1	0	1	0
Total	5	0	1,065	1,345

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	Morrow Cir	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,410**	**5**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	4	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Prado Woods Dr	Peak Hour	PM

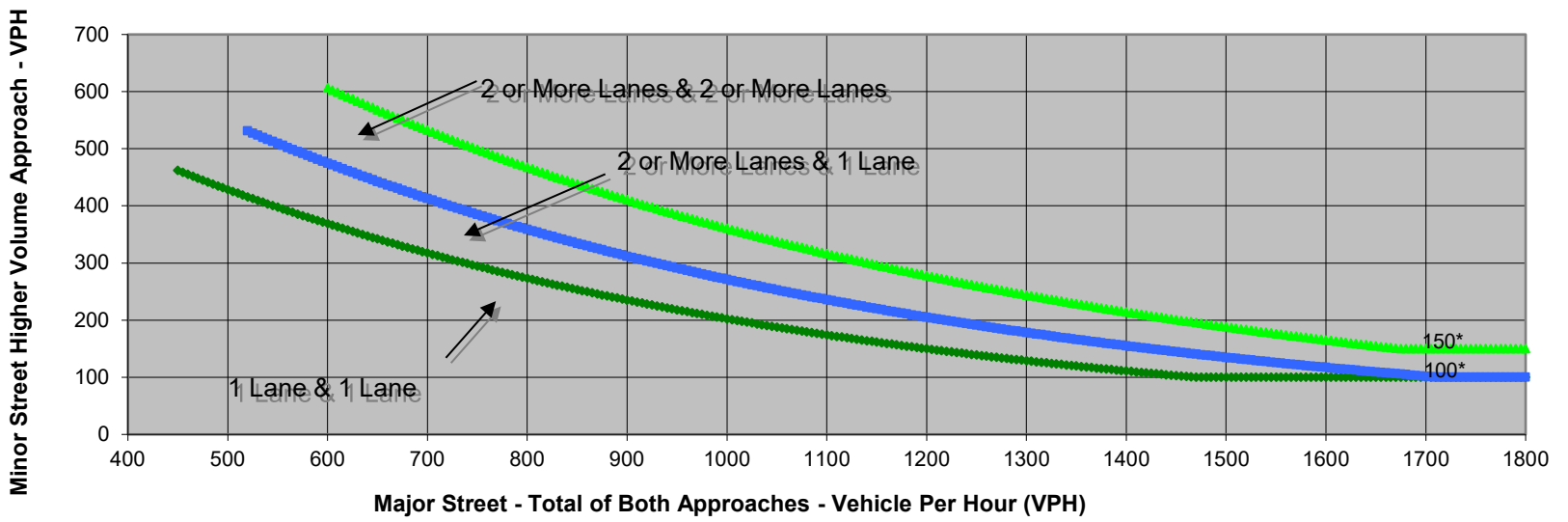
Turn Movement Volumes

	NB	SB	EB	WB
Left	0	2	18	0
Through	0	0	1047	1341
Right	0	4	0	4
Total	0	6	1,065	1,345

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	Prado Woods Dr	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,410**	**6**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR EX PEERS

Intersection	5	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Kenwick Dr	Peak Hour	PM

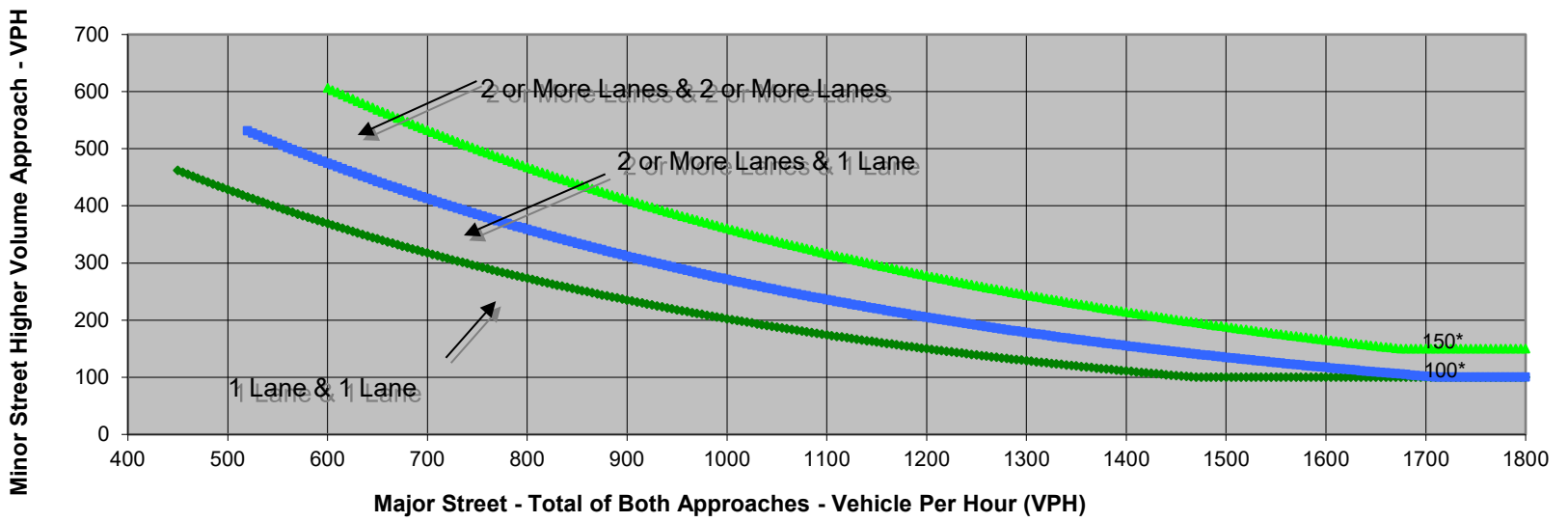
Turn Movement Volumes

	NB	SB	EB	WB
Left	0	0	6	0
Through	0	0	1043	1337
Right	0	8	0	3
Total	0	8	1,049	1,340

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	Kenwick Dr	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,389**	**8**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	6	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Radec Ct	Peak Hour	PM

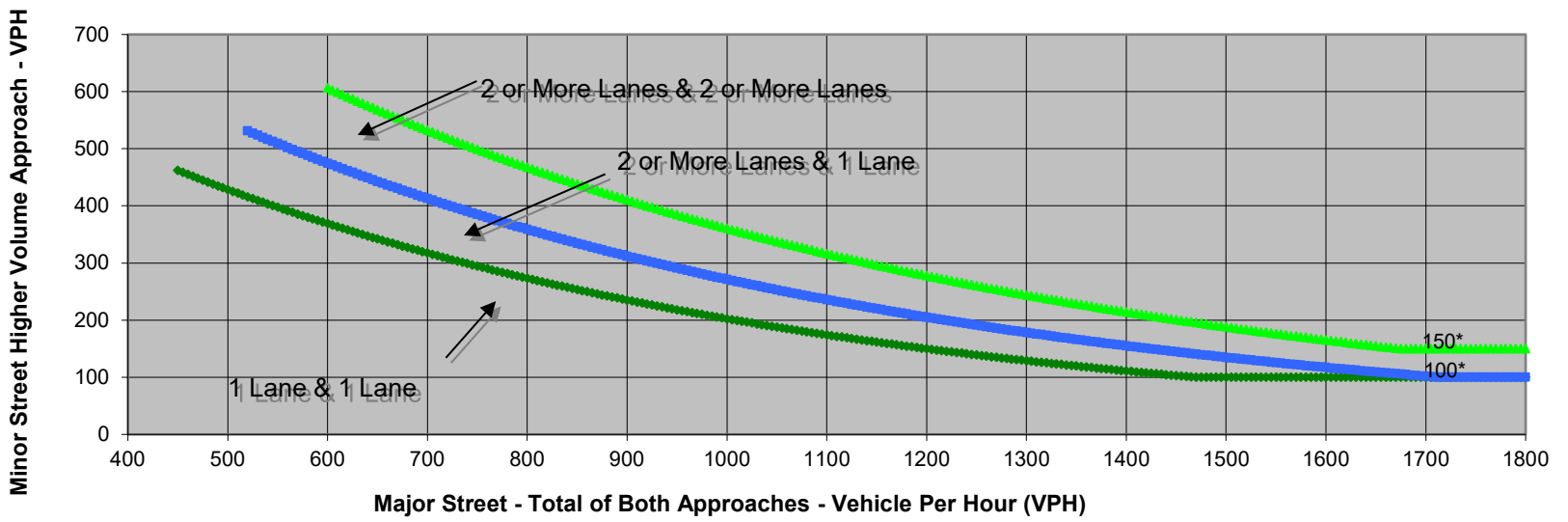
Turn Movement Volumes

	NB	SB	EB	WB
Left	1	0	0	1
Through	0	0	1041	1339
Right	0	0	2	0
Total	1	0	1,043	1,340

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	Radec Ct	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,383**	**1**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	8	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Park Villa Ln	Peak Hour	PM

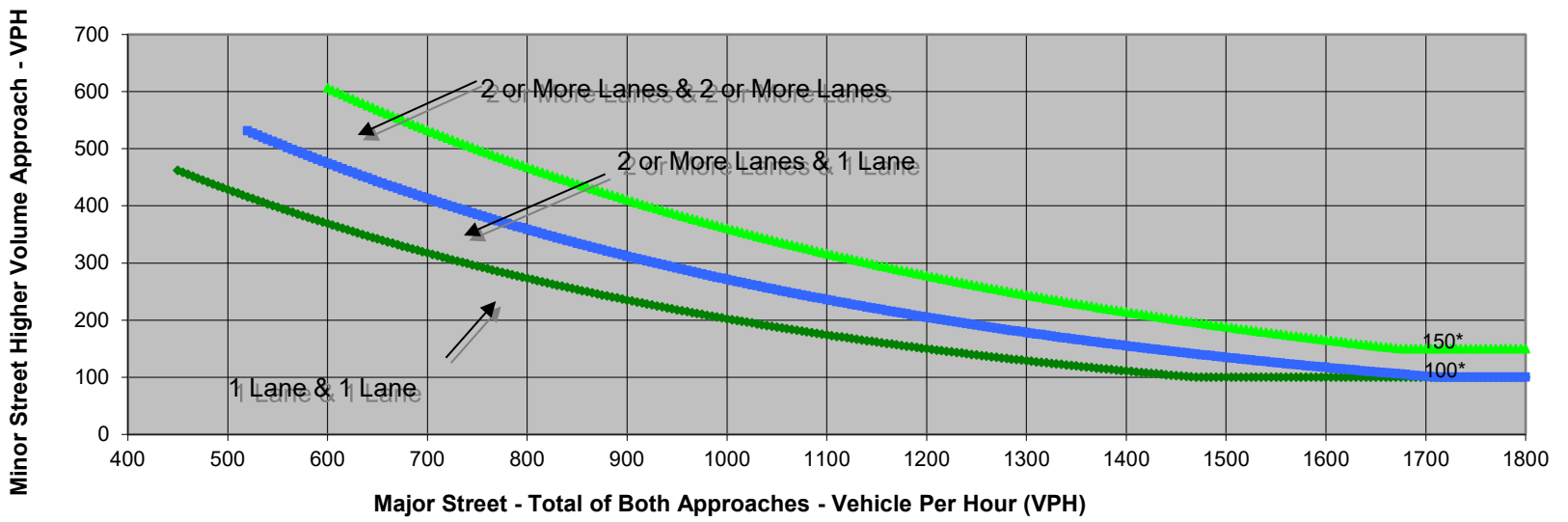
Turn Movement Volumes

	NB	SB	EB	WB
Left	6	0	0	3
Through	0	0	1005	1352
Right	2	0	5	0
Total	8	0	1,010	1,355

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	Park Villa Ln	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,365**	**8**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	11	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	N Linda Vista St	Peak Hour	PM

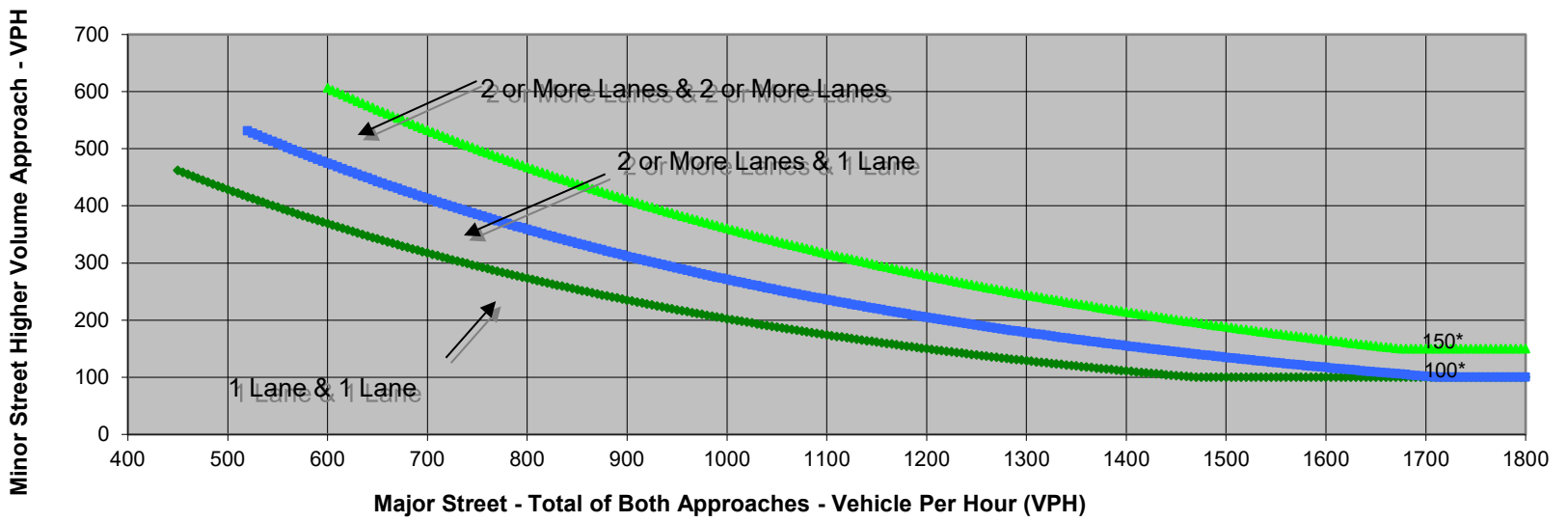
Turn Movement Volumes

	NB	SB	EB	WB
Left	6	9	8	2
Through	0	0	1120	1489
Right	6	19	22	13
Total	12	28	1,150	1,504

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	N Linda Vista St	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,654**	**28**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	15	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Loma St	Scenario	Future Year (2045) Conditions
Minor Street	Cannon St	Peak Hour	PM

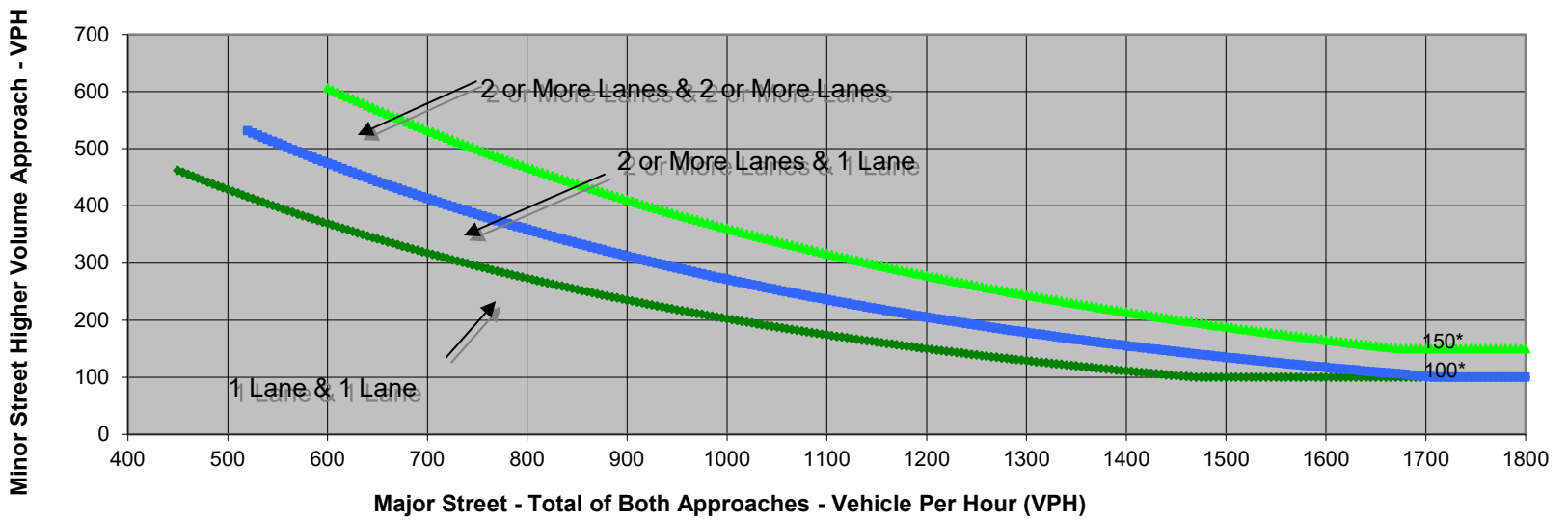
Turn Movement Volumes

	NB	SB	EB	WB
Left	38	0	25	0
Through	1269	751	0	0
Right	0	29	28	0
Total	1,307	780	53	0

Major Street Direction

x	North/South
	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Mesa Dr	Cannon St	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,087**	**53**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	2	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Kathleen Ln	Peak Hour	AM

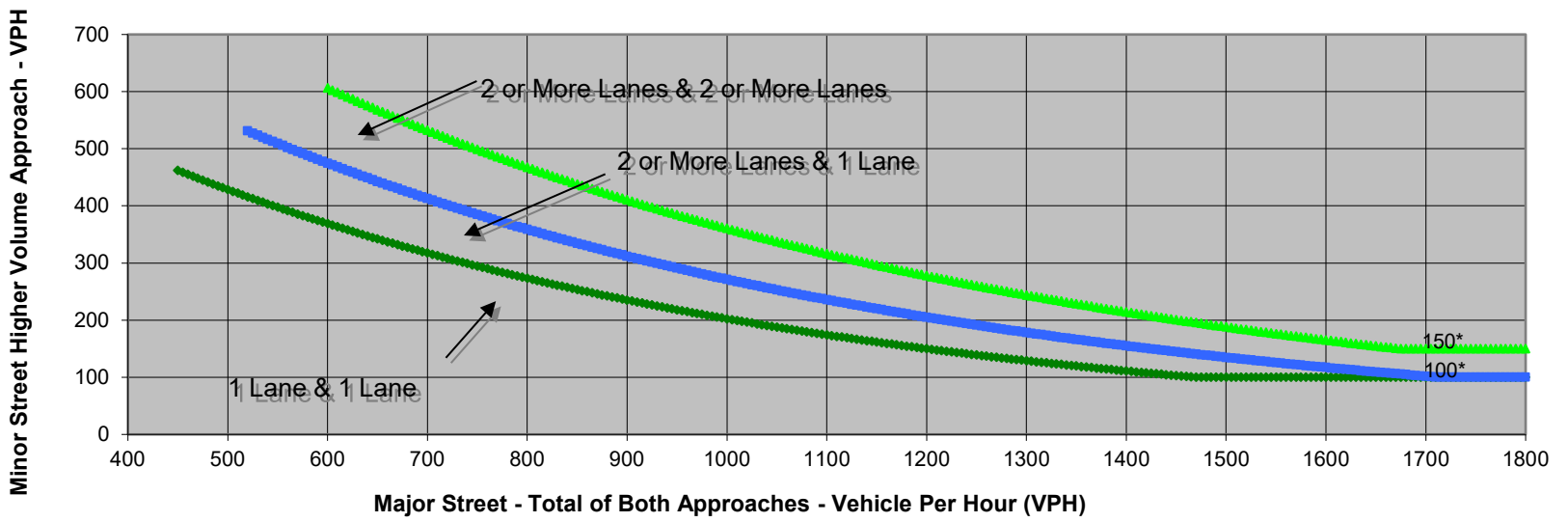
Turn Movement Volumes

	NB	SB	EB	WB
Left	0	10	10	0
Through	0	0	1300	1310
Right	0	10	0	0
Total	0	20	1,310	1,310

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	E Trenton Ave	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,620**	**20**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	3	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Morrow Cir	Peak Hour	AM

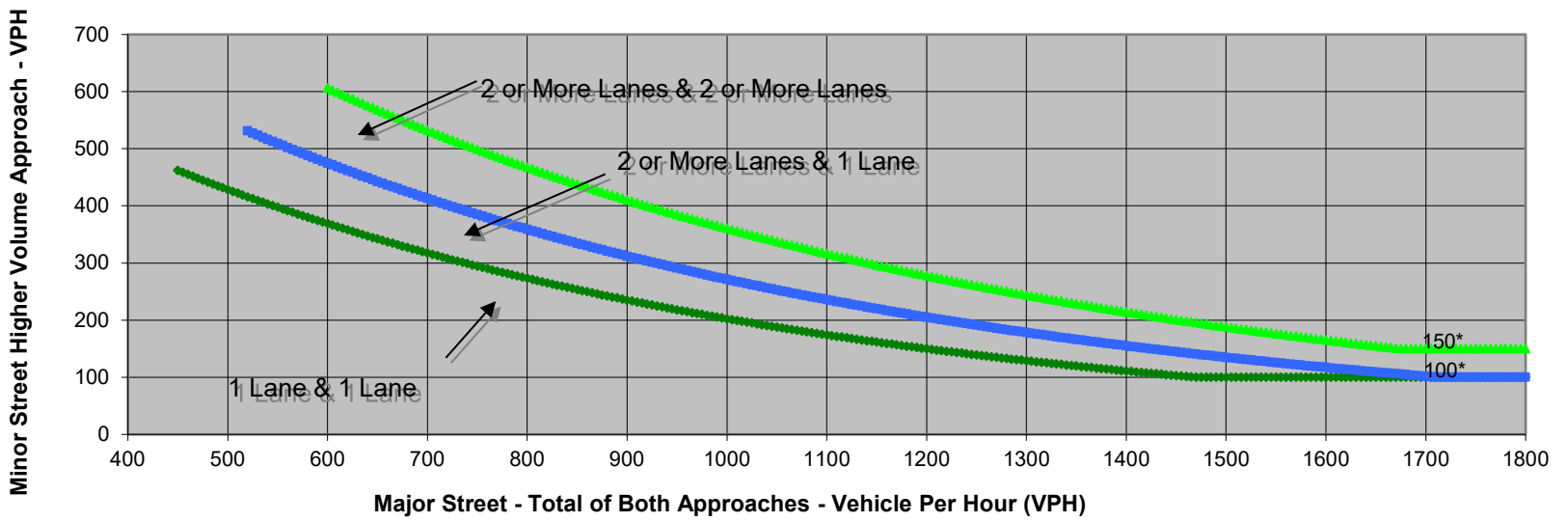
Turn Movement Volumes

	NB	SB	EB	WB
Left	10	0	0	10
Through	0	0	1300	1300
Right	10	0	10	0
Total	20	0	1,310	1,310

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	Morrow Cir	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,620**	**20**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	4	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Prado Woods Dr	Peak Hour	AM

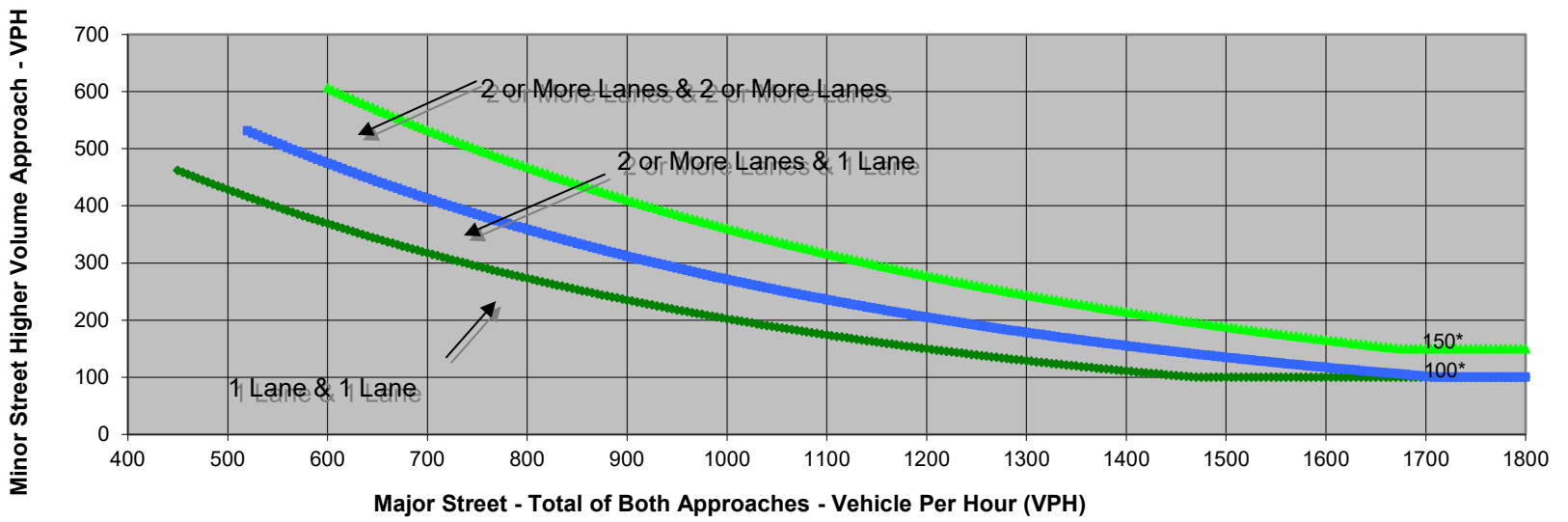
Turn Movement Volumes

	NB	SB	EB	WB
Left	0	10	20	0
Through	0	0	1290	1290
Right	0	20	0	10
Total	0	30	1,310	1,300

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	Prado Woods Dr	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,610**	**30**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	5	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Kenwick Dr	Peak Hour	AM

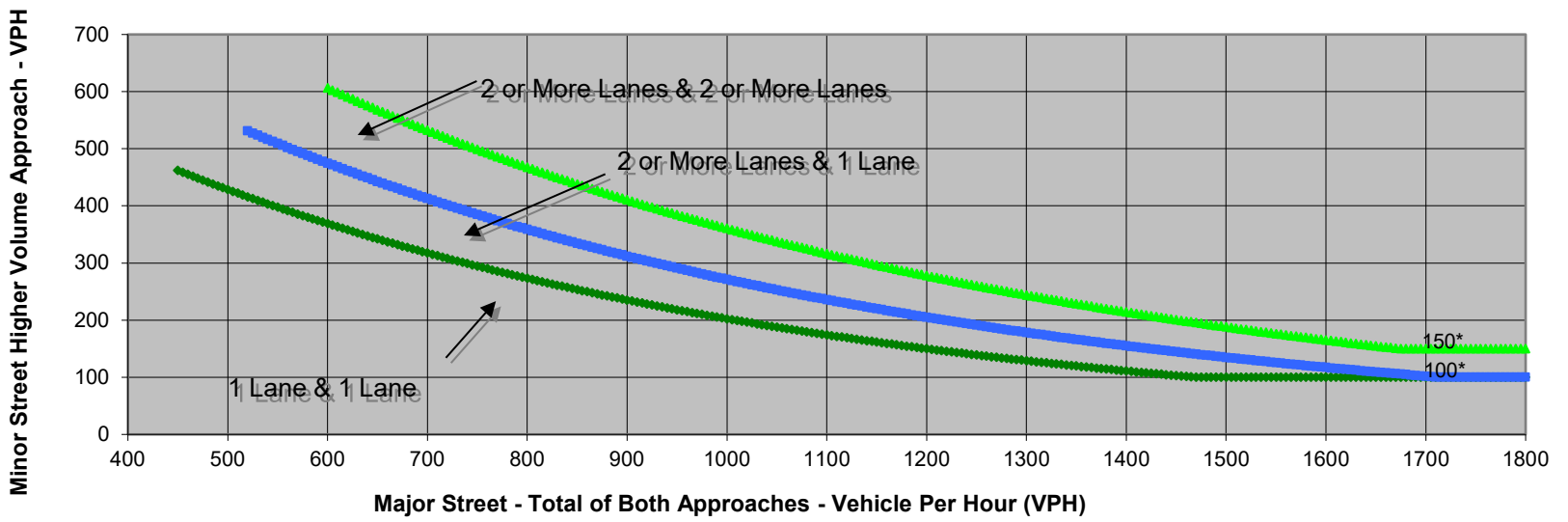
Turn Movement Volumes

	NB	SB	EB	WB
Left	0	10	10	0
Through	0	0	1290	1290
Right	0	10	0	0
Total	0	20	1,300	1,290

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	Kenwick Dr	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,590**	**20**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	6	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Radec Ct	Peak Hour	AM

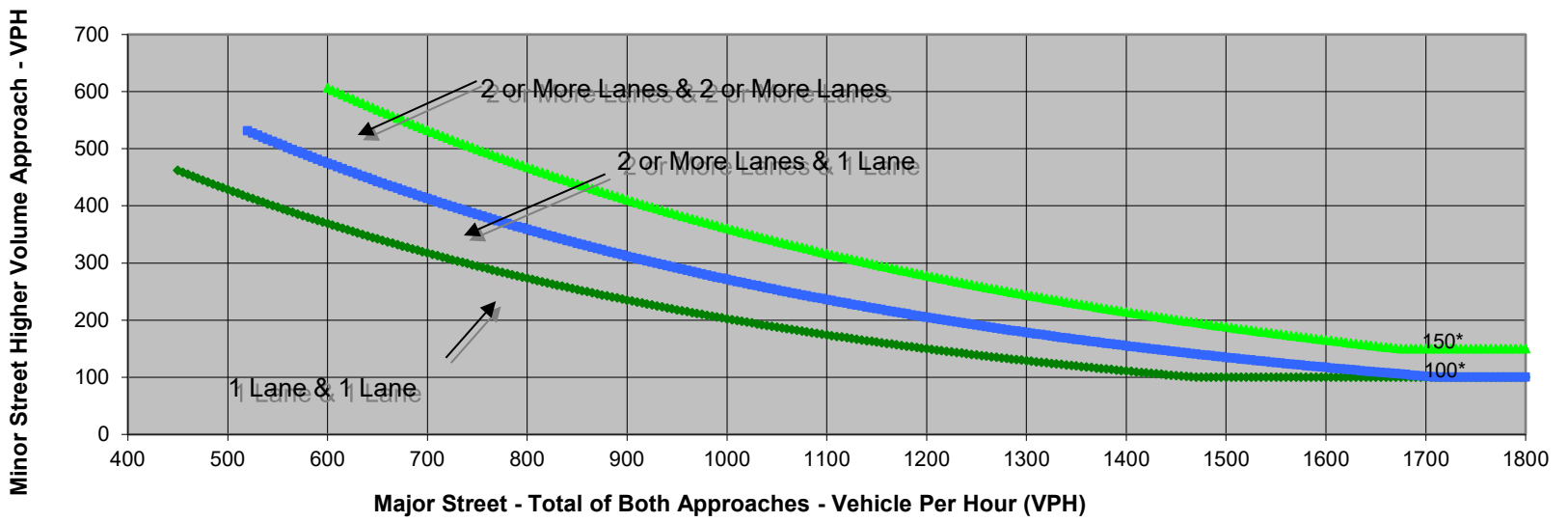
Turn Movement Volumes

	NB	SB	EB	WB
Left	10	0	0	10
Through	0	0	1290	1290
Right	10	0	10	0
Total	20	0	1,300	1,300

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	Radec Ct	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,600**	**20**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	8	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Park Villa Ln	Peak Hour	AM

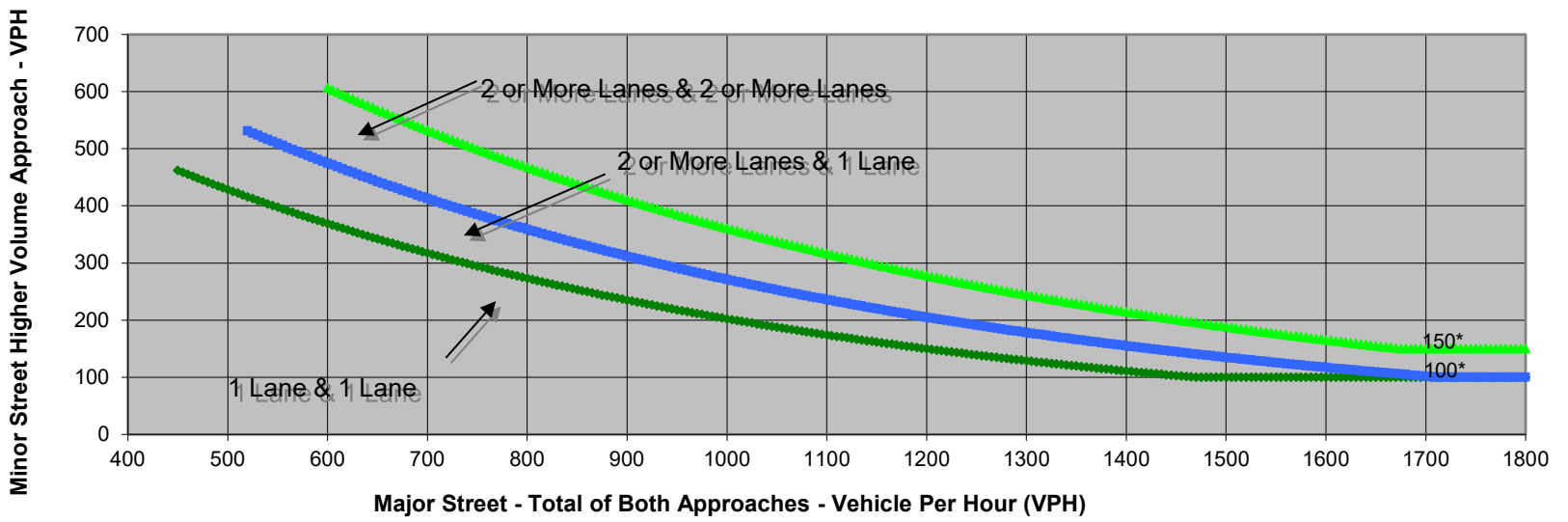
Turn Movement Volumes

	NB	SB	EB	WB
Left	10	0	0	10
Through	0	0	1210	1190
Right	10	0	10	0
Total	20	0	1,220	1,200

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	Park Villa Ln	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,420**	**20**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	11	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	N Linda Vista St	Peak Hour	AM

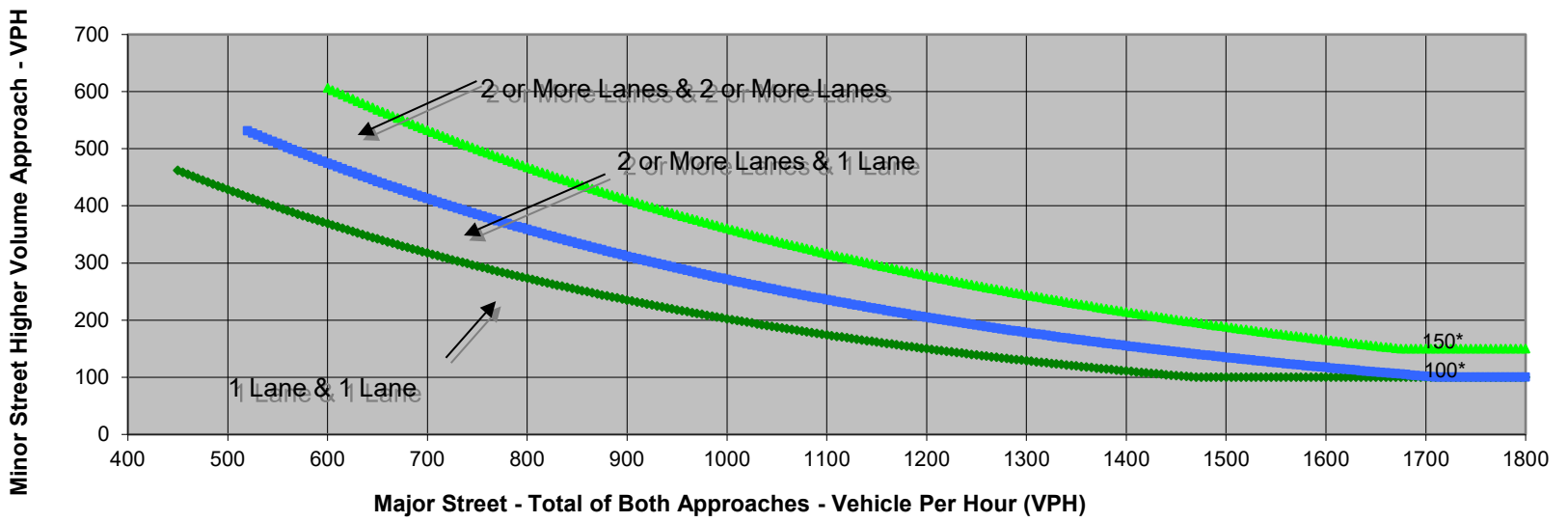
Turn Movement Volumes

	NB	SB	EB	WB
Left	10	30	70	10
Through	10	0	1250	1640
Right	30	110	40	100
Total	50	140	1,360	1,750

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	N Linda Vista St	
Number of Approach Lanes	**2**	**1**	**YES**
Traffic Volume (VPH) *	**3,110**	**140**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR & PEERS

Intersection	15	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Loma St	Scenario	Future Year (2045) Conditions
Minor Street	Cannon St	Peak Hour	AM

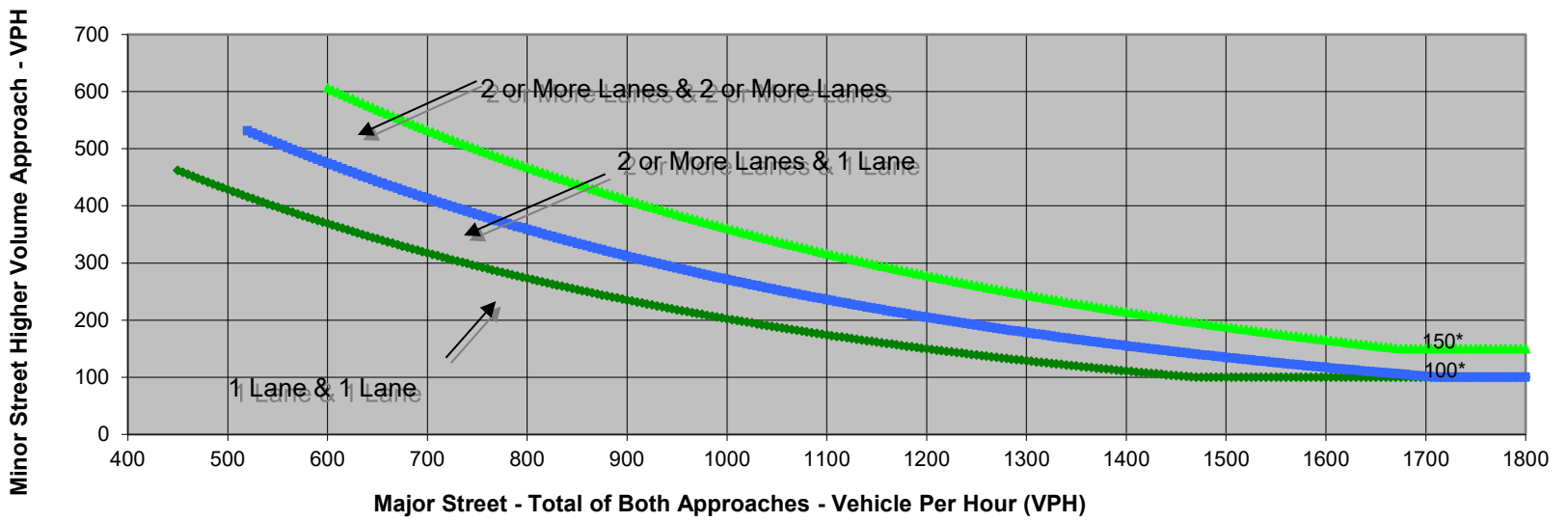
Turn Movement Volumes

	NB	SB	EB	WB
Left	30	0	40	0
Through	920	1770	0	0
Right	0	60	60	0
Total	950	1,830	100	0

Major Street Direction

x	North/South
	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Mesa Dr	Cannon St	
Number of Approach Lanes	**2**	**1**	**YES**
Traffic Volume (VPH) *	**2,780**	**100**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	2	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Kathleen Ln	Peak Hour	PM

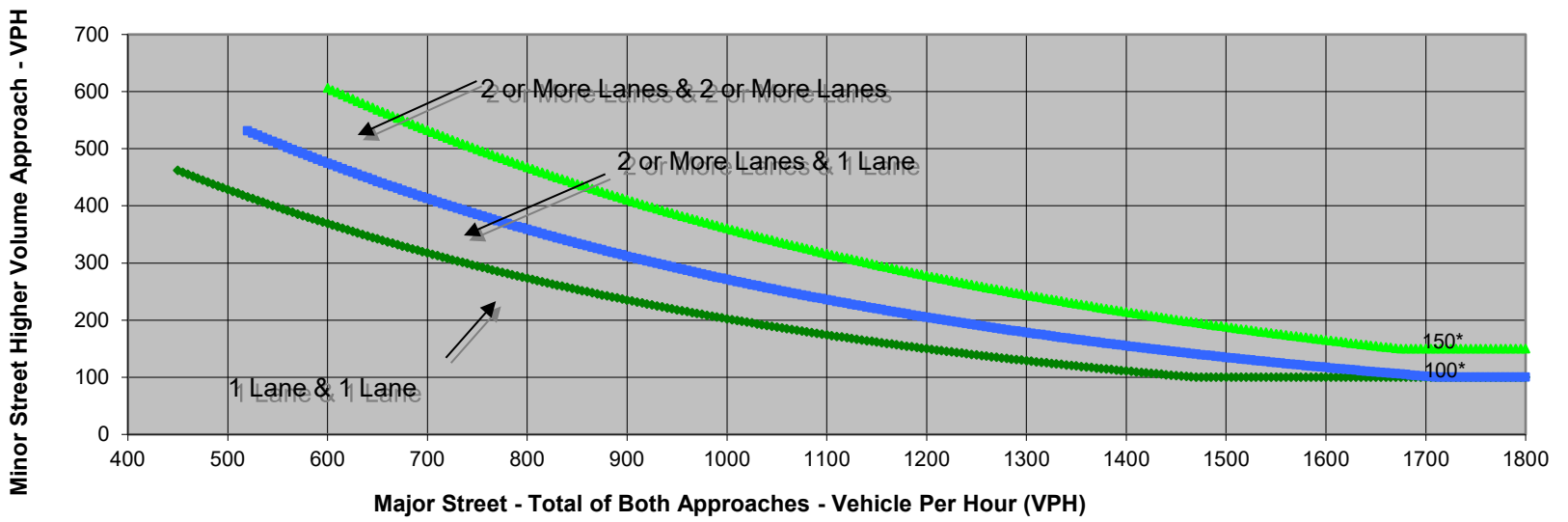
Turn Movement Volumes

	NB	SB	EB	WB
Left	0	0	10	0
Through	0	0	1280	1420
Right	0	10	0	0
Total	0	10	1,290	1,420

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	E Trenton Ave	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,710**	**10**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	3	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Morrow Cir	Peak Hour	PM

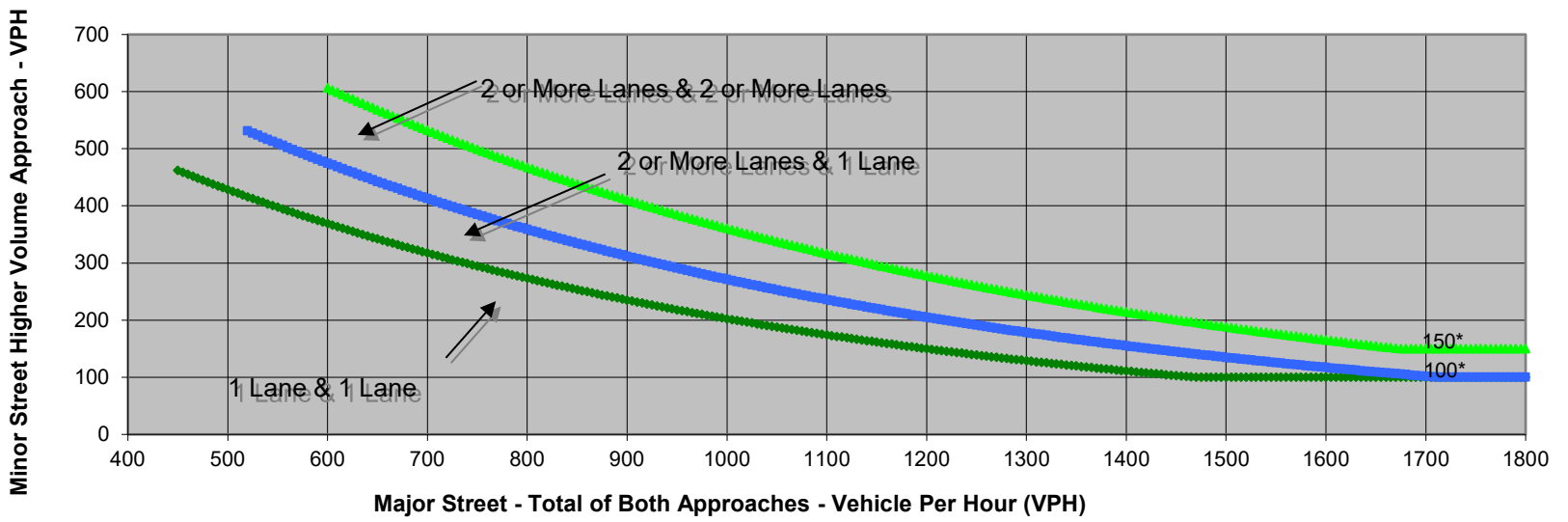
Turn Movement Volumes

	NB	SB	EB	WB
Left	10	0	0	10
Through	0	0	1270	1410
Right	10	0	10	0
Total	20	0	1,280	1,420

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	Morrow Cir	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,700**	**20**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	4	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Prado Woods Dr	Peak Hour	PM

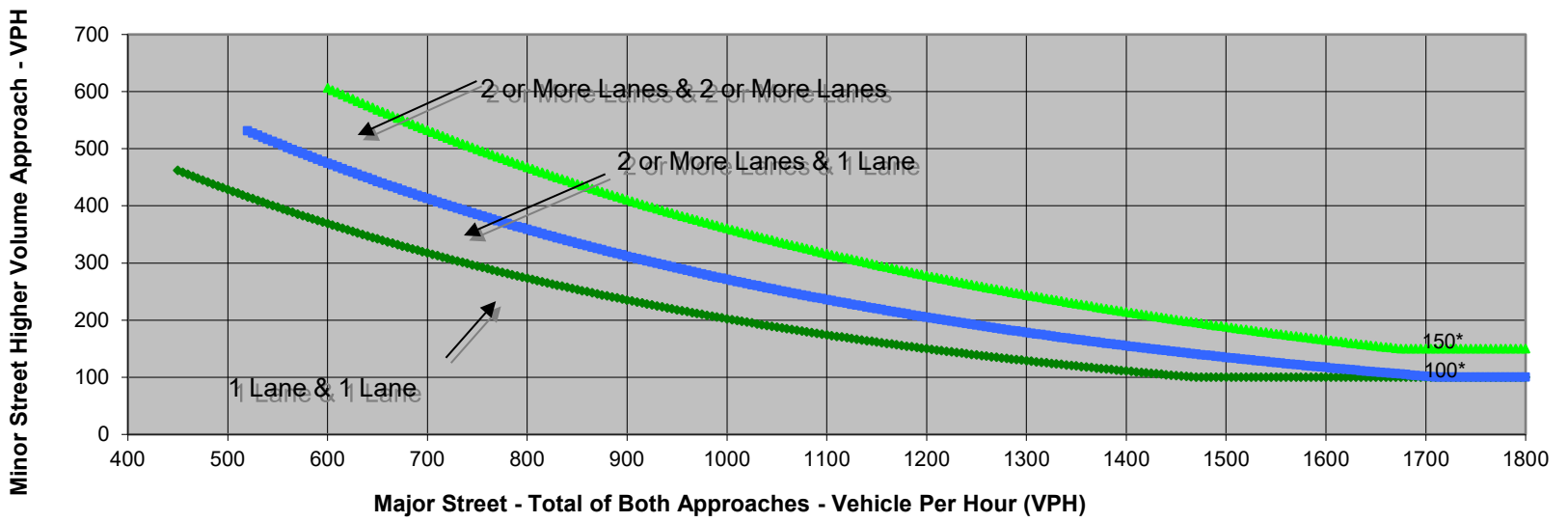
Turn Movement Volumes

	NB	SB	EB	WB
Left	0	10	20	0
Through	0	0	1260	1410
Right	0	10	0	10
Total	0	20	1,280	1,420

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	Prado Woods Dr	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,700**	**20**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	5	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Kenwick Dr	Peak Hour	PM

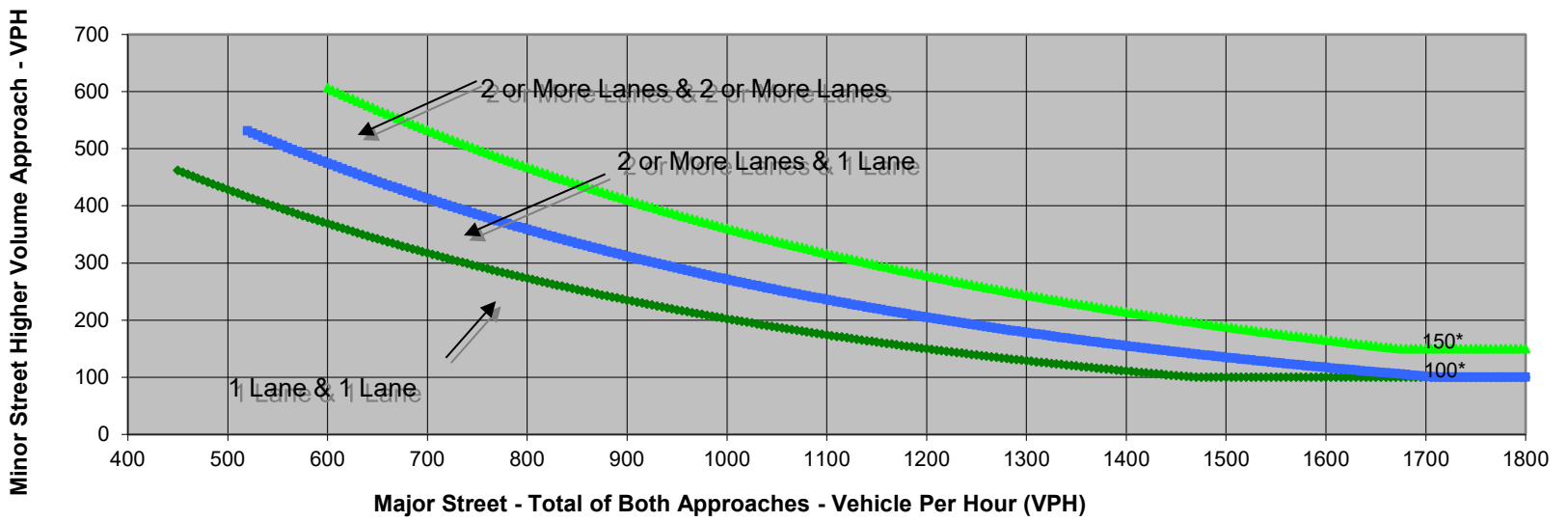
Turn Movement Volumes

	NB	SB	EB	WB
Left	0	0	10	0
Through	0	0	1260	1410
Right	0	10	0	10
Total	0	10	1,270	1,420

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	Kenwick Dr	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,690**	**10**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	6	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Radec Ct	Peak Hour	PM

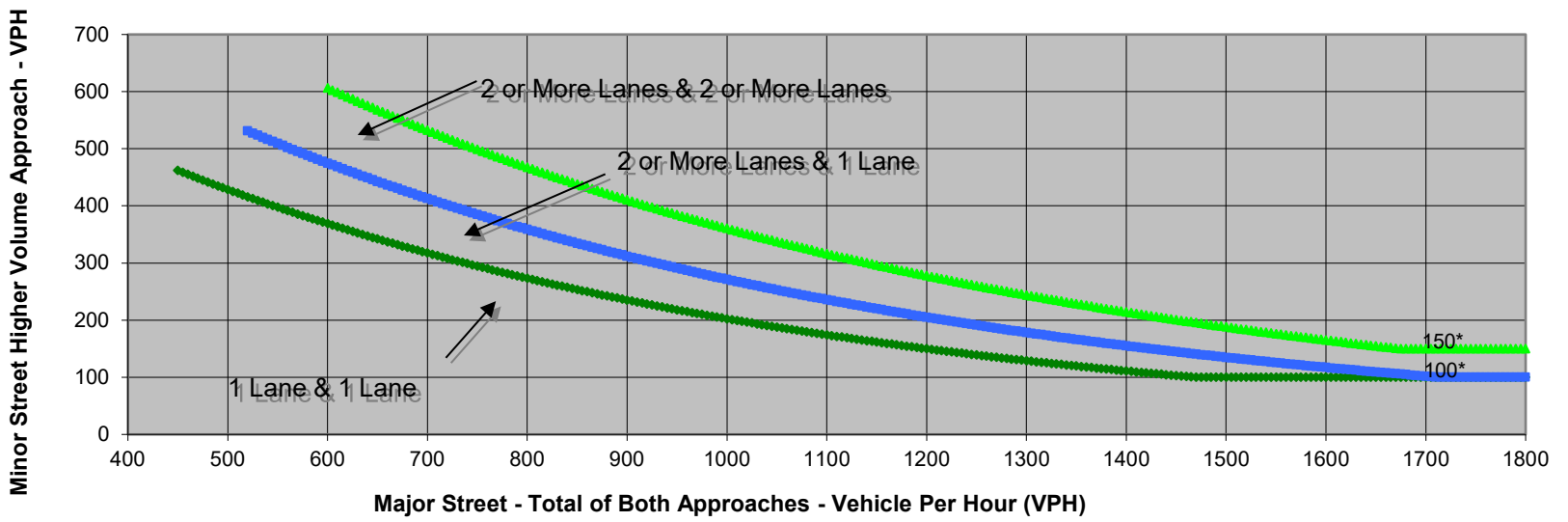
Turn Movement Volumes

	NB	SB	EB	WB
Left	10	0	0	10
Through	0	0	1250	1410
Right	0	0	10	0
Total	10	0	1,260	1,420

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	Radec Ct	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,680**	**10**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	8	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	Park Villa Ln	Peak Hour	PM

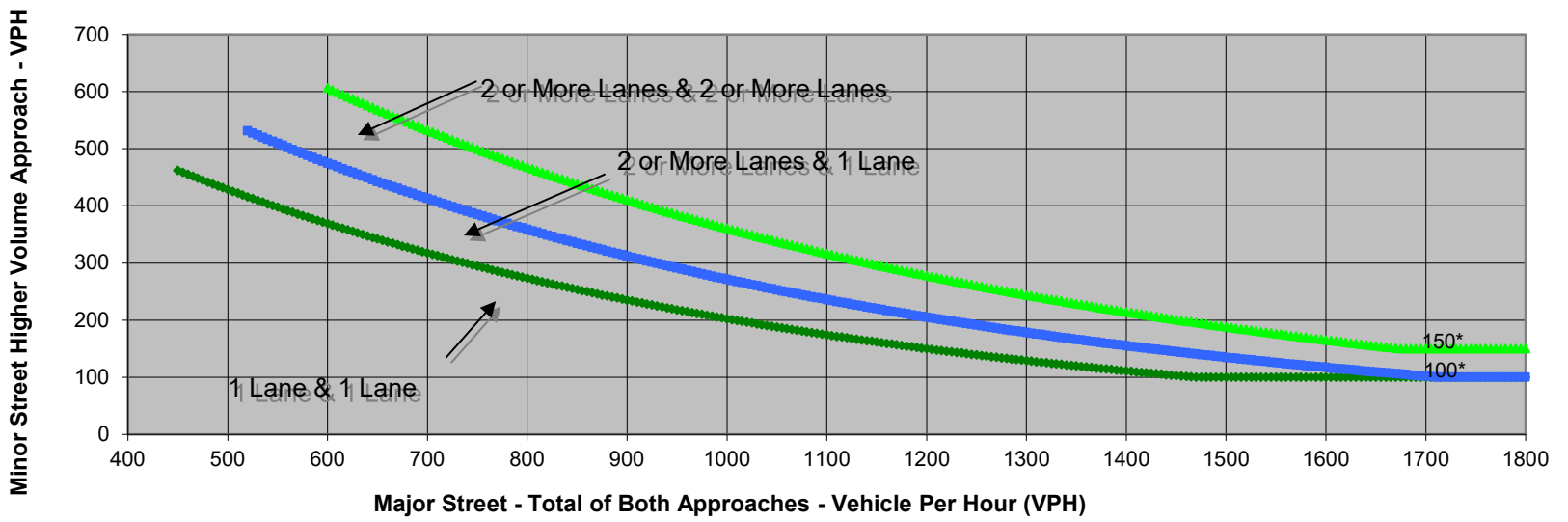
Turn Movement Volumes

	NB	SB	EB	WB
Left	10	0	0	10
Through	0	0	1200	1420
Right	10	0	10	0
Total	20	0	1,210	1,430

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	Park Villa Ln	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,640**	**20**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	11	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Villa Park Rd	Scenario	Future Year (2045) Conditions
Minor Street	N Linda Vista St	Peak Hour	PM

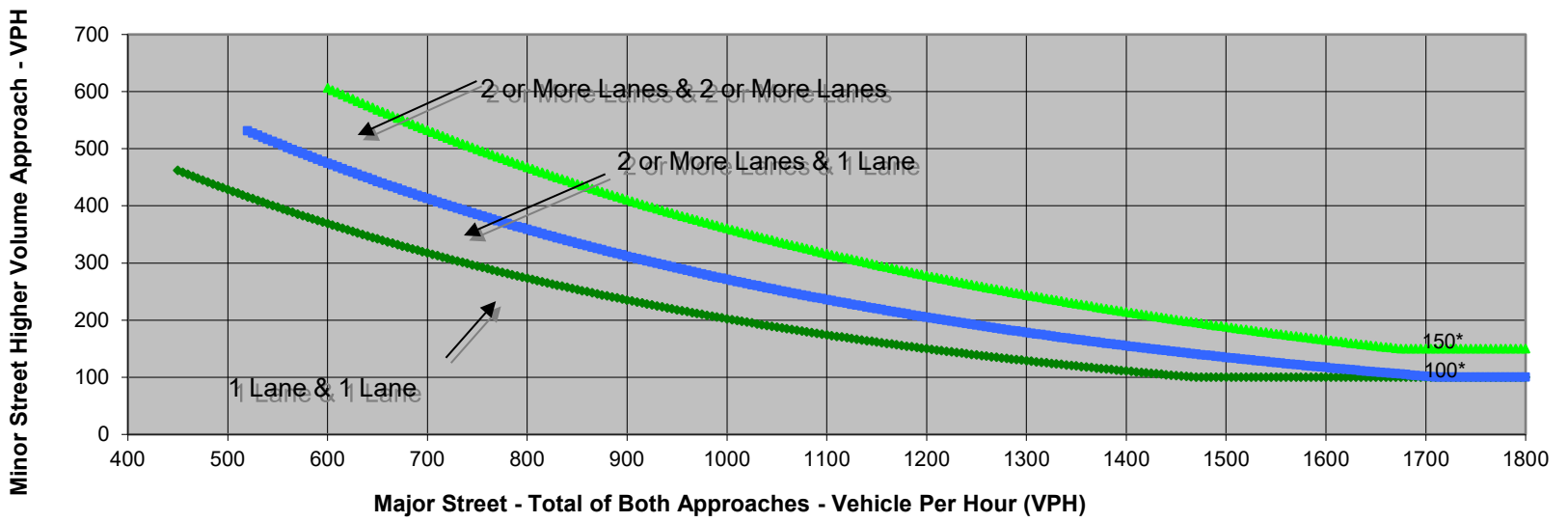
Turn Movement Volumes

	NB	SB	EB	WB
Left	10	10	10	10
Through	0	0	1600	1620
Right	10	20	30	20
Total	20	30	1,640	1,650

Major Street Direction

	North/South
x	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Villa Park Rd	N Linda Vista St	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**3,290**	**30**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

FEHR PEERS

Intersection	15	Project	MPAH Amendment Study - Villa Park Rd
Major Street	Loma St	Scenario	Future Year (2045) Conditions
Minor Street	Cannon St	Peak Hour	PM

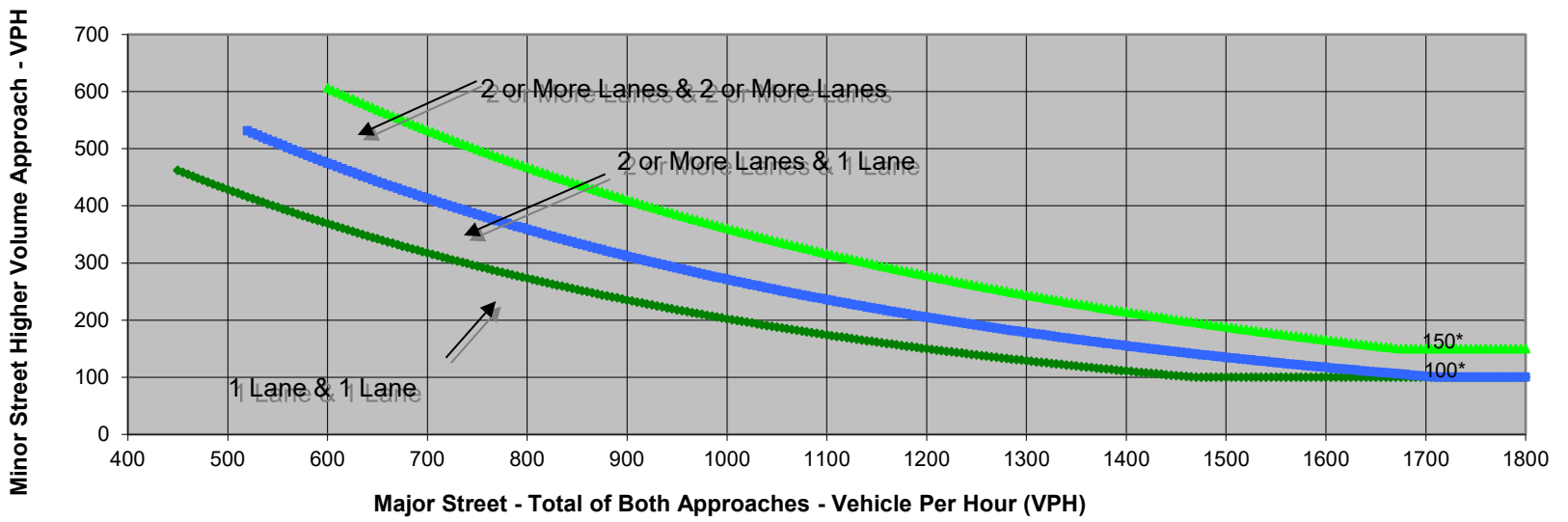
Turn Movement Volumes

	NB	SB	EB	WB
Left	40	0	30	0
Through	1580	940	0	0
Right	0	30	30	0
Total	1,620	970	60	0

Major Street Direction

x	North/South
	East/West

Figure 4C-3. Warrant 3, Peak Hour



* Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Source: *California Manual on Uniform Traffic Control Devices*, Caltrans, 2012

	Major Street	Minor Street	Warrant Met
	Mesa Dr	Cannon St	
Number of Approach Lanes	**2**	**1**	**NO**
Traffic Volume (VPH) *	**2,590**	**60**	

* Note: Traffic Volume for Major Street is Total Volume of Both Approches.
Traffic Volume for Minor Street is the Volume of High Volume Approach.

Attachment F: Collision Data

BerkeleySafeTREC

Crash Details for: Case ID 8717340

Crash Information

County	Orange
City	Orange
Date & Time (M/D/Y)	09/14/2018 15:19
Location (Intersection)	Villa Park Rd & N Wanda Rd
Dist. & Dir. from Intersection	At Intersection
State Highway	No
Geocoded Location	33.8104591, -117.8219681

Type of Crash	A - Head-On
Motor Vehicle Involved With	G - Bicycle
Crash Severity	3 - Injury (Other Visible)
PCF Violation Category	09 - Automobile Right of Way
Weather	A - Clear
Alcohol Involved	No

Pedestrian Accident	No	**Bicycle Accident**	Yes
Motorcycle Accident	No	**Truck Accident**	No

Map View



Street View



Parties: 2

Party Number	Party Type	Statewide Vehicle Type	At Fault	Party Direction	Movement Preceding Collision
1	4 - Bicyclist	L - Bicycle	Yes	South	B - Proceeding Straight
2	1 - Driver (including Hit and Run)	D - Pickup or Panel Truck	No	West	B - Proceeding Straight

Victims: 2

Party Number	Victim Role	Victim Gender	Victim Age	Victim Degree of Injury
1	4 - Bicyclist	M - Male	12	6 - Suspected Minor Injury
2	2 - Passenger	M - Male	58	0 - No Injury

BerkeleySafeTREC

Crash Details for: Case ID 8784782

Crash Information

County	Orange
City	Orange
Date & Time (M/D/Y)	01/09/2019 14:06
Location (Intersection)	East Katella Av & Wanda Rd
Dist. & Dir. from Intersection	At Intersection
State Highway	No
Geocoded Location	33.8104591, -117.8219681

Type of Crash	H - Other
Motor Vehicle Involved With	G - Bicycle
Crash Severity	4 - Injury (Complaint of Pain)
PCF Violation Category	09 - Automobile Right of Way
Weather	A - Clear
Alcohol Involved	No

Pedestrian Accident	No	**Bicycle Accident**	Yes
Motorcycle Accident	No	**Truck Accident**	No

Map View



Street View



Parties: 2

Party Number	Party Type	Statewide Vehicle Type	At Fault	Party Direction	Movement Preceding Collision
1	4 - Bicyclist	L - Bicycle	No	West	D - Making Right Turn
2	1 - Driver (including Hit and Run)	D - Pickup or Panel Truck	Yes	West	B - Proceeding Straight

Victims: 1

Party Number	Victim Role	Victim Gender	Victim Age	Victim Degree of Injury
1	4 - Bicyclist	M - Male	84	7 - Possible Injury

BerkeleySafeTREC

Crash Details for: Case ID 8874276

Crash Information

County	Orange
City	Orange
Date & Time (M/D/Y)	05/14/2019 06:36
Location (Intersection)	Katella Av & Wanda St
Dist. & Dir. from Intersection	At Intersection
State Highway	No
Geocoded Location	33.8104591, -117.8219681

Type of Crash	D - Broadside
Motor Vehicle Involved With	G - Bicycle
Crash Severity	3 - Injury (Other Visible)
PCF Violation Category	12 - Traffic Signals and Signs
Weather	A - Clear
Alcohol Involved	No

Pedestrian Accident	No	**Bicycle Accident**	Yes
Motorcycle Accident	No	**Truck Accident**	No

Map View



Street View



Parties: 2

Party Number	Party Type	Statewide Vehicle Type	At Fault	Party Direction	Movement Preceding Collision
1	1 - Driver (including Hit and Run)	A - Passenger Car/Station Wagon	Yes	West	B - Proceeding Straight
2	4 - Bicyclist	L - Bicycle	No	North	B - Proceeding Straight

Victims: 1

Party Number	Victim Role	Victim Gender	Victim Age	Victim Degree of Injury
2	4 - Bicyclist	M - Male	16	6 - Suspected Minor Injury

BerkeleySafeTREC

Crash Details for: Case ID 82028363

Crash Information

County	Orange
City	Orange
Date & Time (M/D/Y)	08/06/2022 05:30
Location (Intersection)	Santiago Canyon Rd & Cannon St
Dist. & Dir. from Intersection	225.00 ft West
State Highway	No
Geocoded Location	33.8111458, -117.7961502

Type of Crash	G - Vehicle/Pedestrian
Motor Vehicle Involved With	B - Pedestrian
Crash Severity	3 - Injury (Other Visible)
PCF Violation Category	21 - Unsafe Starting or Backing
Weather	A - Clear
Alcohol Involved	Yes

Pedestrian Accident	Yes	**Bicycle Accident**	No
Motorcycle Accident	No	**Truck Accident**	No

Map View



Street View



Parties: 2

Party Number	Party Type	Statewide Vehicle Type	At Fault	Party Direction	Movement Preceding Collision
1	1 - Driver (including Hit and Run)	A - Passenger Car/Station Wagon	Yes	West	G - Backing
2	2 - Pedestrian	N - Pedestrian	No	East	A - Stopped

Victims: 1

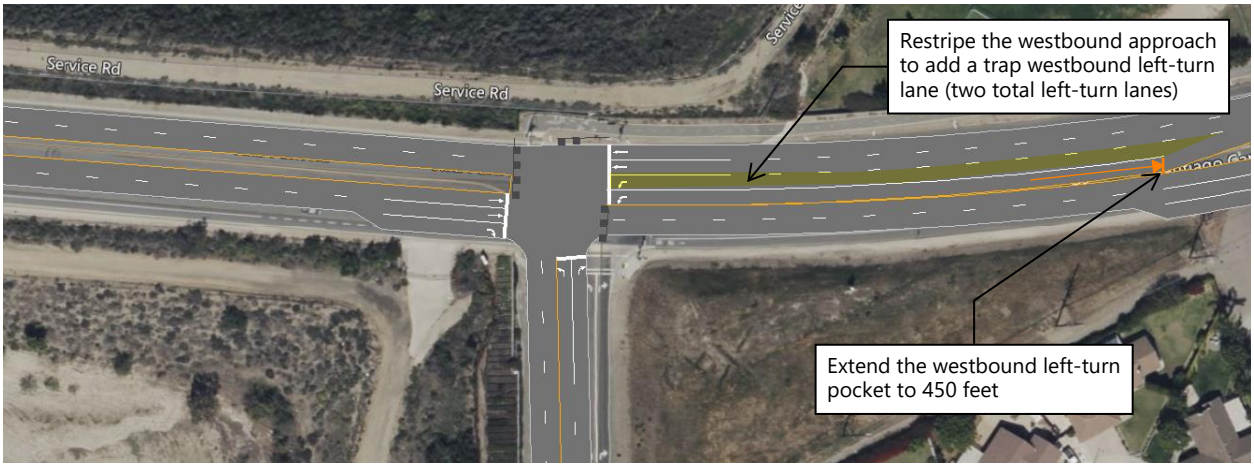
Party Number	Victim Role	Victim Gender	Victim Age	Victim Degree of Injury
2	3 - Pedestrian	M - Male	21	6 - Suspected Minor Injury

Attachment G: Intersection Improvements

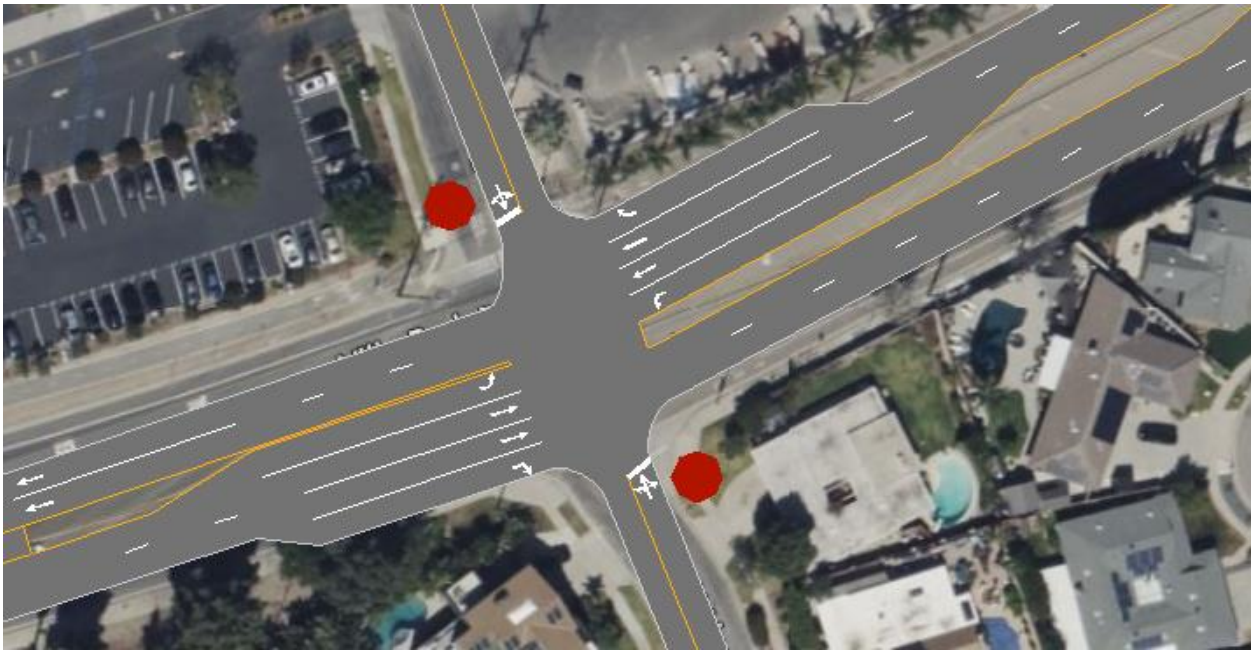
Intersection # 10: Hewes Street and Villa Park Road No Improvements



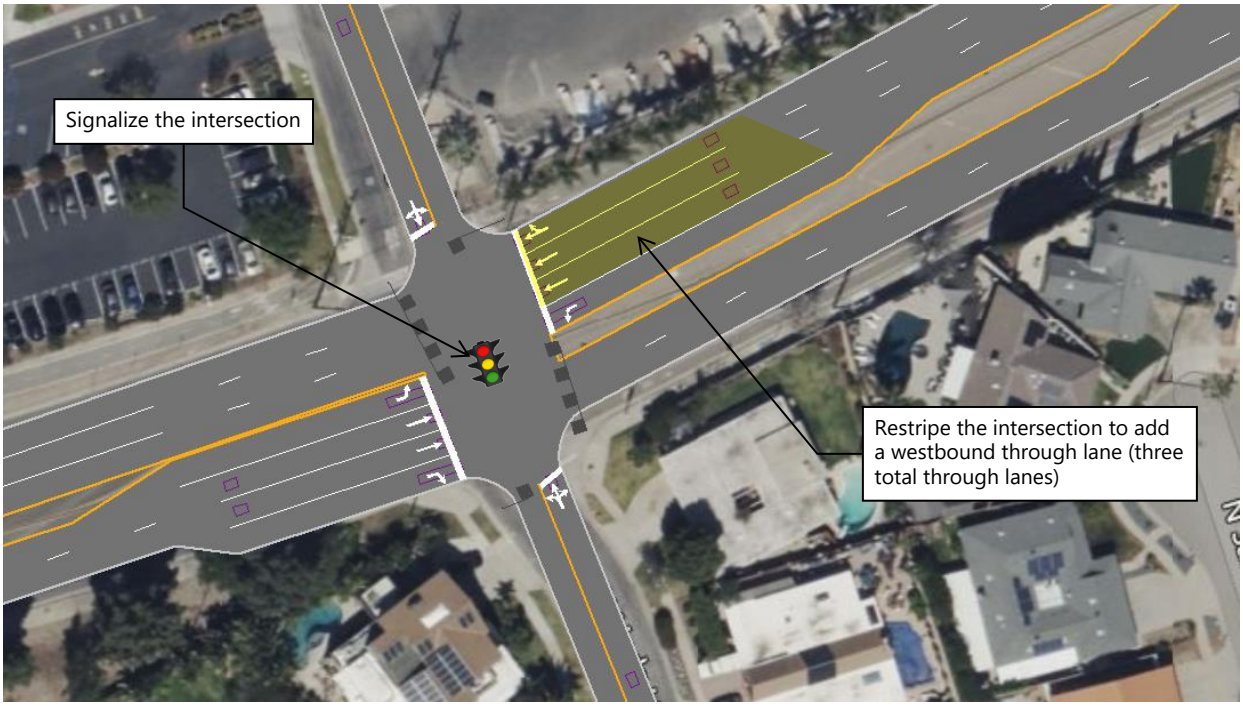
Intersection # 10: Hewes Street and Villa Park Road with Improvements



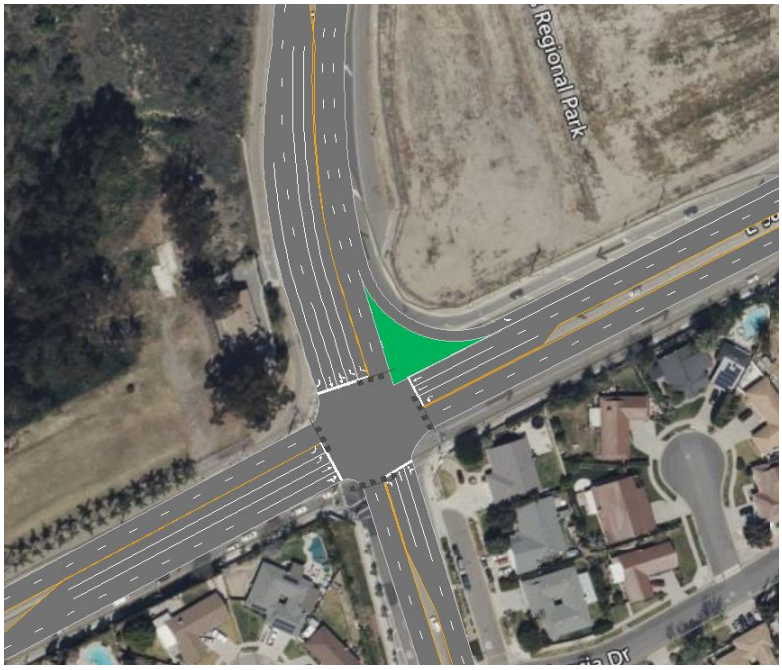
Intersection #11: Linda Vista Street and Villa Park Road No Improvements



Intersection #11: Linda Vista Street and Villa Park Road with Improvements



Intersection #12: Cannon Street and Santiago Canyon Road No Improvements



Intersection #12: Cannon Street and Santiago Canyon Road with Improvements



Agenda Item

AGENDA STAFF REPORT

ASR Control 25-000190

MEETING DATE:	05/06/25
LEGAL ENTITY TAKING ACTION:	Board of Supervisors
BOARD OF SUPERVISORS DISTRICT(S):	All Districts
SUBMITTING AGENCY/DEPARTMENT:	OC Public Works (Approved)
DEPARTMENT CONTACT PERSON(S):	Justin Kirk (714) 667-1627
	Cindy Salazar (714) 667-8870

SUBJECT: Adopt General Plan Amendment TGPA 24-03 - Transportation Element

CEO CONCUR	**COUNTY COUNSEL REVIEW**	**CLERK OF THE BOARD**
Concur	Approved Resolution to Form	Public Hearing 3 Votes Board Majority

Budgeted: N/A **Current Year Cost:** N/A **Annual Cost:** N/A

Staffing Impact: No **# of Positions:** **Sole Source:** N/A

Current Fiscal Year Revenue: N/A

Funding Source: N/A **County Audit in last 3 years:** N/A

Levine Act Review Completed: N/A

Prior Board Action: 10/8/2024 #15, 1/12/2021 #44-45, 11/17/2020 #30, 2/25/2020 #17-18

RECOMMENDED ACTION(S):

1. Find that the proposed project is exempt from the provisions of the CEQA, Common Sense Exemption pursuant to CEQA Guidelines Section 15061 (b)(3).

2. Conduct public hearing.

3. Consider the matter and adopt Resolution which will adopt General Plan Amendment TGPA 24-03 - Transportation Element.

SUMMARY:

Adoption of General Plan Amendment TGPA 24-03 - Transportation Element will reclassify roadway designation for Villa Park Road/Santiago Canyon Road to accurately reflect changing development and traffic patterns.

BACKGROUND INFORMATION:

The County of Orange (County) Transportation Element identifies goals, objectives, policies and implementation programs that affect the transportation system. The Transportation Element contains policies on the development of the transportation facilities necessary to accommodate orderly growth within

the County, provides guidance for future planning efforts within the unincorporated area and ensures a balanced transportation system.

OC Public Works initiated a Master Plan of Arterial Highways (MPAH) amendment request with the Orange County Transportation Authority (OCTA) after evaluation of a Transportation Assessment (TA) (Fehr and Peers, March 2024, Attachment D). The MPAH depicts a Countywide roadway network intended to ensure coordinated transportation system development among local jurisdictions in the County. The TA compared the existing MPAH roadway designations for Villa Park Road/Santiago Canyon Road against proposed modifications to those roadway designations to conform the designations to reduced growth projections through the year 2045. Villa Park Road/Santiago Canyon Road between Hewes Street and Cannon Street is a four-lane Primary Arterial. The MPAH designates the roadway as a six-lane Major Arterial.

The proposed reclassification of the roadway segment is meant to support the County's effort to improve traffic conditions and conform to changing traffic patterns and growth estimates. Over the last ten years, the population growth projections have indicated reduced future growth compared to previous years. As a result, some of the future planned roadway infrastructure to support the previously assumed higher population growth may no longer be necessary.

The TA analyzed future forecasts along Villa Park Road/Santiago Canyon Road and indicated that more capacity is needed between Hewes Street and Cannon Street to effectively serve future demand. However, the current Major Arterial designation does not fit within the existing right of way. The TA concluded that the roadway designation of a five lane Asymmetric Primary Arterial (three westbound lanes and two eastbound lanes) on Villa Park Road/Santiago Canyon Road between Hewes Street and Cannon Street will be sufficient to serve future traffic demand on the roadway, be the best use of the available right-of-way to increase capacity, meet operational goals of the County and limit impacts to adjacent properties. Further explanation of this review is detailed in Planning Commission Staff Report and Resolution No. 2025-03 (Attachment D). The five lane Asymmetric Primary Arterial roadway can fit within the existing 100-foot right-of-way.

Proposed Project:
The proposed General Plan Amendment TGPA 24-03 - Transportation Element (Amendment) includes the following revisions, including the Circulation Plan (Attachment C):

1. Reclassify the existing classification of Major Arterial (divided six-lane) to Asymmetric Primary Arterial (divided five-lane) for Villa Park Road/Santiago Canyon Road between Hewes Street and Cannon Street. The reclassification would provide three westbound lanes and two eastbound lanes and provide adequate right-of-way to accommodate a Class II bicycle lanes with three-foot painted buffers to improve traffic conditions on Villa Park Road/Santiago Canyon Road.

2. Include the definition of Asymmetric Lanes in the Transportation Element (Attachment B).

    a. Asymmetric Lanes: Capacity augmentation may be needed that creates more lanes in one direction than the other (asymmetric lanes) along some Smart Streets, Principal, Major and/or Primary arterials.

The MPAH amendment request was conditionally approved by OCTA's Regional Transportation Planning Committee on August 29, 2024, and by OCTA's Board on September 9, 2024. OCTA's conditional approval action remains conditional until OCTA receives documentation that the County has amended their General Plan and complies with the requirements of CEQA. OC Public Works recommends the Board adopt the Resolution to adopt the Amendment.

Planning Commission (Commission) Action:
OC Public Works received authorization from the Commission on December 11, 2024, to initiate the Amendment, including the Circulation Plan. On February 26, 2025, the Commission held a public hearing, considered the Amendment and voted to adopt Commission Resolution No. 2025-03, which recommends adoption by the Board of Supervisors (Board) to update the County's Transportation Element (Attachment D).

Prior Board actions related to the County's Transportation Element are summarized in the table below.

Board Date	Action
10/8/2024	Adopted General Plan Amendment T24-01 - County of Orange Transportation Element
1/12/2021	Adopted General Plan Amendment T20-02 - County of Orange Transportation Element Adopted General Plan Amendment T20-03 - County of Orange Transportation Element
11/17/2020	Adopted General Plan Amendment 20-01 Land Use, Transportation, Growth Management
2/25/2020	Adopted General Plan Amendment T19-01 - County of Orange Transportation Element Adopted General Plan Amendment T19-02 - County of Orange Transportation Element

Compliance with CEQA: The proposed Amendment is exempt (Common Sense Exemption) from the provisions of CEQA pursuant to CEQA Guidelines Sections 15061(b)(3) since there is no possibility that the revisions to the roadway designations may have a significant effect on the environment.

FINANCIAL IMPACT:

N/A

STAFFING IMPACT:

N/A

ATTACHMENT(S):

Attachment A - Resolution
Attachment B - Resolution Exhibit A (Transportation Element)
Attachment C - Resolution Exhibit B (Circulation Plan Map)
Attachment D - Planning Commission Staff Report and Resolution No. 2025-03

OCTA

COMMITTEE TRANSMITTAL

September 9, 2024

To: Members of the Board of Directors

From: Andrea West, Clerk of the Board

Subject: Amendments to the Master Plan of Arterial Highways

Regional Transportation Planning Committee Meeting of August 29, 2024

Present: Directors Dumitru, Federico, Foley, and Stephens
Absent: Directors Do, Harper, and Khan

Committee Vote

This item was passed by the Members present.

Committee Recommendation(s)

A. Approve the following Master Plan of Arterial Highways amendments:

County of Orange

1. Reclassify Villa Park Road/Santiago Canyon Road from a major (six-lane, divided) arterial to an asymmetric primary (five-lane, divided) arterial between Hewes Street and Cannon Street.
2. Reclassify Santiago Canyon Road from a primary (four-lane, divided) arterial to a divided collector (two-lane, divided) arterial between State Route 241 and Live Oak Canyon Road.
3. Reclassify El Toro Road from a major (six-lane, divided) arterial to a primary (four-lane, divided) arterial between Live Oak Canyon Road and Pheasant Creek.
4. Reclassify El Toro Road, from a major (six-lane, divided) arterial to an asymmetric primary (five-lane, divided) arterial between Pheasant Creek and Portola Parkway.
5. Delete Black Star Canyon Road between Silverado Canyon Road and the Orange County/Riverside County Line.
6. Reclassify Bucker Way from a secondary (four-lane, undivided) arterial to a divided collector (two-lane, divided) arterial between Ranch Canyon Road and Coyotes Road.
7. Reclassify Ranch Canyon Road from a primary (four-lane, divided) arterial to a divided collector (two-lane, divided) arterial between Bucker Way and Cow Camp Road.

City of Fullerton

8. Reclassify Harbor Boulevard, from a major (six-lane, divided) arterial to a primary (four-lane, divided) arterial between Bastanchury Road and Berkeley Avenue.

City of Laguna Hills

9. Reclassify Paseo De Valencia from a major (six-lane, divided) arterial to a divided collector (two-lane, divided) arterial between Alicia Parkway and La Paz Road.
10. Reclassify Paseo De Valencia from a secondary (four-lane, undivided) arterial to a divided collector (two-lane, divided) arterial between La Paz Road and Cabot Road.
11. Reclassify Cabot Road from a secondary (four-lane, undivided) arterial to a divided collector (two-lane, divided) arterial between Paseo de Valencia and El Paseo.

The Master Plan of Arterial Highways will be amended to reflect each approved request contingent upon receipt of documentation confirming that all affected general plans are consistent with the proposed amendment and are compliant with the California Environmental Quality Act. Amendment requests will expire if the Orange County Transportation Authority does not receive such documentation within three years of granting approval.

Should the proposed Master Plan of Arterial Highways amendment be modified for any reason after receiving approval, the modified Master Plan of Arterial Highways amendment must be returned to the Orange County Transportation Authority Board of Directors for reconsideration and action.

B. Direct the Executive Director of Planning, or his designee, to file a Notice of Exemption from the California Environmental Quality Act for the Master Plan of Arterial Highways amendments.

C. Receive and file a status report on the active Master Plan of Arterial Highways amendments.

OCTA

August 29, 2024

To: Regional Transportation Planning Committe

From: Darrell E. Johnson, Chief Executive Officer



Subject: Amendments to the Master Plan of Arterial Highways

Overview

The Orange County Transportation Authority administers the Master Plan of Arterial Highways, including the review and approval of amendments requested by local agencies. Approvals are recommended for amendment requests from the County of Orange, the City of Fullerton, and the City of Laguna Hills. Also, a status report on pending amendment requests is provided.

Recommendations

A. Approve the following Master Plan of Arterial Highways amendments:

<u>County of Orange</u>

1. Reclassify Villa Park Road/Santiago Canyon Road from a major (six-lane, divided) arterial to an asymmetric primary (five-lane, divided) arterial between Hewes Street and Cannon Street.
2. Reclassify Santiago Canyon Road from a primary (four-lane, divided) arterial to a divided collector (two-lane, divided) arterial between State Route 241 and Live Oak Canyon Road.
3. Reclassify El Toro Road from a major (six-lane, divided) arterial to a primary (four-lane, divided) arterial between Live Oak Canyon Road and Pheasant Creek.
4. Reclassify El Toro Road, from a major (six-lane, divided) arterial to an asymmetric primary (five-lane, divided) arterial between Pheasant Creek and Portola Parkway.
5. Delete Black Star Canyon Road between Silverado Canyon Road and the Orange County/Riverside County Line.
6. Reclassify Bucker Way from a secondary (four-lane, undivided) arterial to a divided collector (two-lane, divided) arterial between Ranch Canyon Road and Coyotes Road.
7. Reclassify Ranch Canyon Road from a primary (four-lane, divided) arterial to a divided collector (two-lane, divided) arterial between Bucker Way and Cow Camp Road.

City of Fullerton

8. Reclassify Harbor Boulevard, from a major (six-lane, divided) arterial to a primary (four-lane, divided) arterial between Bastanchury Road and Berkeley Avenue.

City of Laguna Hills

9. Reclassify Paseo De Valencia from a major (six-lane, divided) arterial to a divided collector (two-lane, divided) arterial between Alicia Parkway and La Paz Road.
10. Reclassify Paseo De Valencia from a secondary (four-lane, undivided) arterial to a divided collector (two-lane, divided) arterial between La Paz Road and Cabot Road.
11. Reclassify Cabot Road from a secondary (four-lane, undivided) arterial to a divided collector (two-lane, divided) arterial between Paseo de Valencia and El Paseo.

The Master Plan of Arterial Highways will be amended to reflect each approved request contingent upon receipt of documentation confirming that all affected general plans are consistent with the proposed amendment and are compliant with the California Environmental Quality Act. Amendment requests will expire if the Orange County Transportation Authority does not receive such documentation within three years of granting approval.

Should the proposed Master Plan of Arterial Highways amendment be modified for any reason after receiving approval, the modified Master Plan of Arterial Highways amendment must be returned to the Orange County Transportation Authority Board of Directors for reconsideration and action.

B. Direct the Executive Director of Planning, or his designee, to file a Notice of Exemption from the California Environmental Quality Act for the Master Plan of Arterial Highways amendments.

C. Receive and file a status report on the active Master Plan of Arterial Highways amendments.

Background

The Master Plan of Arterial Highways (MPAH) coordinates roadway system planning across Orange County jurisdictions. The MPAH was first adopted by the County of Orange in 1956, and the Orange County Transportation Authority (OCTA) assumed administration responsibilities in 1995.

OCTA is responsible for maintaining the integrity and continuity of the MPAH system as it evolves to fulfill transportation circulation needs by reviewing changes proposed by local jurisdictions. The reviews consider potential transportation-related concerns and ensure interagency collaboration to avoid unintended impacts in neighboring jurisdictions or regional transportation systems. Details on the requested amendments by the County of Orange (County), City of Fullerton (Fullerton), and City of Laguna Hills (Laguna Hills) are presented below, along with a status report of MPAH amendments that are in process.

Discussion

Consistent with the MPAH amendment process described in OCTA's Guidance for Administration of the MPAH, Attachment A documents OCTA's analysis of several requests for amendments to the MPAH. The requests were submitted to OCTA by the County (Attachments B, C, and D), Fullerton (Attachment E), and Laguna Hills (Attachment F). The table below summarizes the current and proposed roadway configurations.

The first five amendments in the table that are proposed by the County are illustrated in Attachment G and the cities of Lake Forest, Mission Viejo, Orange, and Villa Park have provided consent forms (Attachments H and I) to express their support. The remaining three amendments being proposed by the County are illustrated in Attachment J. Additionally, a map of the MPAH amendment request by the City of Fullerton is provided in Attachment K, and a map of the amendment request by the City of Laguna Hills is provided in Attachment L.

Most of the amendments are being requested to accommodate complete street initiatives. OCTA supports these initiatives through the MPAH amendment process, which also ensures that the integrity of the MPAH system is maintained and has no impacts on the ability of OCTA to deliver public transit. The amendment requests are supported by affected jurisdictions, where applicable, and they can be implemented within the existing right-of-way. The requested amendments are expected to sufficiently accommodate future traffic demand and, therefore, they do not adversely impact the MPAH and are recommended for approval.

Agency	Current Configuration	MPAH Classification	Requested Classification
1. Villa Park Road/Santiago Canyon Road - Hewes Street to Cannon Street			
County	4-lane, divided	6-lane, divided	5-lane, divided
2. Santiago Canyon Road - State Route 241 to Live Oak Canyon Road			
County	2-lane, undivided	4-lane, divided	2-lane, divided
3. El Toro Road - Live Oak Canyon Road to Pheasant Creek			
County	2-lane, undivided	6-lane, divided	4-lane, divided
4. El Toro Road - Pheasant Creek to Portola Parkway			
County	5-lane, divided	6-lane, divided	5-lane, divided
5. Black Star Canyon Road - Silverado Canyon Road to County Line			
County	Unbuilt	4-lane, undivided	Remove
6. Bucker Way - Ranch Canyon Road to Coyotes Road			
County	Unbuilt	4-lane, undivided	2-lane, divided
7. Ranch Canyon Road - Bucker Way to Cow Camp Road			
County	Unbuilt	4-lane, divided	2-lane, divided
8. Harbor Boulevard - Bastanchury Road to Berkeley Avenue			
Fullerton	4-lane, divided	6-lane, divided	4-lane, divided
9. Paseo De Valencia - Alicia Parkway to La Paz Road			
Laguna Hills	4-lane, divided	6-lane, divided	2-lane, divided
10. Paseo De Valencia - La Paz Road to Cabot Road			
Laguna Hills	4-lane, divided	4-lane, divided	2-lane, divided
11. Cabot Road - Paseo de Valencia to El Paseo			
Laguna Hills	4-lane, divided	4-lane, undivided	2-lane, divided

California Environmental Quality Act (CEQA)

Amendments to the MPAH plan are exempt from CEQA review. With direction from the Board, staff will file a Notice of Exemption from CEQA for the proposed MPAH amendment.

MPAH Amendment Status Update

There are currently 17 active amendments proposed for the MPAH (Attachment M). Several of the active amendments are awaiting local action to amend their respective general plans. Others are either under review, in the cooperative study process, pending resolution of issues with other agencies, or are awaiting refinement of development plans.

On July 12, 2021, the Board conditionally approved amendments to delete Tonner Canyon Road and Valencia Avenue in the City of Brea (Brea) from the MPAH. Since the general plan update was not completed within three years of the Board approval date, the amendment was not finalized and has expired. Staff is actively coordinating with Brea to potentially restart the amendment process when appropriate.

Staff is also coordinating with the cities of Costa Mesa and Newport Beach on the proposed 19th Street amendment. OCTA has met with both cities to discuss the status of this request. The setting for this change can have implications on roadways that involve the cities of Costa Mesa, Huntington Beach, Newport Beach, the County of Orange, and potentially the California Department of Transportation. OCTA is agreeable to lead a traffic study in the general area, including the Banning Ranch area, to gain consensus between the involved agencies on MPAH network needs before advancing the 19th Street amendment. However, OCTA was asked to pause the effort until after the cities of Costa Mesa and Newport Beach have updated their General Plan Housing Elements so the latest future housing assumptions can be reflected in the traffic analysis.

Summary

The County, Fullerton, and Laguna Hills have requested amendments to the MPAH. Based on an analysis of the requested changes, the MPAH Guidance has been satisfied, and staff recommends Board approval. A summary of active amendments proposed for the MPAH is also provided as information for the Board.

Attachments

A. Master Plan of Arterial Highways Amendments Detailed Discussion
B. Letter from Nardy Khan, PE, PMP, Deputy Director, OC Public Works, to Kurt Brotcke, Director, Strategic Planning, Orange County Transportation Authority, dated March 6, 2024, re: MPAH Amendment Request for Villa Park Road/Santiago Canyon Road
C. Letter from Nardy Khan, PE, PMP, Deputy Director, OC Public Works, to Kurt Brotcke, Director, Strategic Planning, Orange County Transportation Authority, dated March 27, 2024, re: MPAH Amendment Request for Santiago Canyon Road (SCR), El Toro Road, and Black Star Canyon Road
D. Letter from Nardy Khan, PE, PMP, Deputy Director, OC Public Works, to Kia Mortazavi, Executive Director, Strategic Planning, Orange County Transportation Authority, dated July 18, 2024, re: MPAH Amendment Request for Bucker Way and Ranch Canyon Road in RMV
E. Letter from Stephen Bise, PE, Public Works Director, City of Fullerton to Ivy Hang, Senior Transportation Analyst, Orange County Transportation

Authority, dated June 21, 2024, re: Master Plan of Arterial Highways Amendment – Harbor Boulevard

F. Letter from Jarad Hildenbrand, City Manager, City of Laguna Hills, to Kia Mortazavi, Executive Director, Planning Division, Orange County Transportation Authority, dated June 11, 2024, re: Master Plan of Arterial Highways Amendment – Paseo De Valencia and Cabot Road

G. County of Orange MPAH Amendments Map

H. Villa Park Road/Santiago Canyon Road – Jurisdiction Consent Forms

I. Santiago Canyon Road, El Toro Road, and Black Star Canyon Road – Jurisdiction Consent Forms

J. County of Orange MPAH Amendment Map – Bucker Way and Ranch Canyon Road

K. City of Fullerton MPAH Amendment Map

L. City of Laguna Hills MPAH Amendment Map

M. Status Report on Pending Master Plan of Arterial Highways Amendments

Prepared by:



Ivy Hang
Senior Transportation Analyst
(714) 560-5684

Approved by:



Kia Mortazavi
Executive Director, Planning
(714) 560-5741

Master Plan of Arterial Highways Amendments Detailed Discussion

County – Reclassify Villa Park Road/Santiago Canyon Road

Villa Park Road/Santiago Canyon Road, between Hewes Street and Cannon Street, is currently constructed as a primary (four-lane, divided) arterial, and is classified on the Master Plan of Arterial Highways (MPAH) as a future major (six-lane, divided) arterial. The County is requesting to reclassify this facility to an asymmetric primary (five-lane, divided) arterial. This reclassification would provide two eastbound lanes and three westbound lanes, while providing adequate right-of-way to accommodate class II bicycle lanes with three-foot painted buffers. The cities of Orange and Villa Park have also submitted consent forms to express their support for this MPAH amendment.

Based on analysis by Orange County Transportation Authority (OCTA) staff, using the Orange County Transportation Analysis Model (OCTAM) 5.1, 27,000 daily vehicle trips are forecasted for this segment in 2050. Since an asymmetric primary can carry up to 37,500 daily vehicle trips while maintaining an acceptable level of service, the proposed amendment provides ample capacity, and no trips are expected to be diverted to other facilities.

The requested amendment does not adversely impact the MPAH and is, therefore, recommended for approval.

County – Reclassify Santiago Canyon Road

Santiago Canyon Road, between State Route 241 and Live Oak Canyon Road, is currently constructed as a collector (two-lane, undivided) arterial, and is classified on the MPAH as a future primary (four-lane, divided) arterial. The County is requesting a reclassification of this facility to a divided collector (two-lane, divided) arterial. The proposed changes would support the County's effort to improve traffic conditions on Santiago Canyon Road. The cities of Lake Forest and Mission Viejo have also submitted consent forms to express their support for this MPAH amendment.

Based on analysis by OCTA staff, using OCTAM 5.1, between 9,000 and 14,000 daily vehicle trips are forecasted for this segment in 2050. Since a divided collector can carry up to 15,000 daily vehicle trips while maintaining an acceptable level of service, the proposed amendment provides sufficient capacity, and no trips are expected to be diverted to other facilities.

The requested amendment does not adversely impact the MPAH and is, therefore, recommended for approval.

County – Reclassify El Toro Road

El Toro Road, between Live Oak Canyon Road and Portola Parkway, currently has between two and five travel lanes. This facility is classified on the MPAH as a future major (six-lane, divided) arterial. The County is requesting to reclassify the segment of El Toro Road between Live Oak Canyon Road and Pheasant Creek to a primary

(four-lane, divided) arterial. For the segment of El Toro Road between Pheasant Creek and Portola Parkway, the County is requesting to reclassify to an asymmetric primary (five-lane, divided) arterial, consistent with the existing built configuration. The proposed changes would support the County's effort to accommodate class II bicycle lanes and improve traffic conditions on El Toro Road.

Based on analysis by OCTA staff, using OCTAM 5.1, between 10,000 and 15,000 daily vehicle trips are forecasted for the segment of El Toro Road between Live Oak Canyon Road and Portola Parkway in 2050. No trips are anticipated to be diverted to other facilities as a result of this amendment since the proposed classifications would have ample capacity for the forecasted volumes. Specifically, a primary can carry up to 30,000 daily vehicle trips while maintaining an acceptable level of service, and an asymmetric primary can carry up to 37,500 daily vehicle trips.

The requested amendment does not adversely impact the MPAH and is, therefore, recommended for approval.

County – Delete Black Star Canyon Road

Black Star Canyon Road is currently built as an approximately one-mile, single-lane road that ends at the Black Star Canyon Falls trailhead in the Black Star Canyon wilderness area. The MPAH classifies the facility as a secondary (four-lane, undivided) arterial that extends to the Orange/Riverside County Line. A corresponding road was deleted from the Riverside County highway plan several years ago and an open space preserve was established by the City of Corona. Due to this lack of connectivity to Riverside County and concerns of severe environmental and cultural impacts from construction in the Cleveland National Forest, the County does not intend to implement this planned facility. Therefore, the County is requesting to delete Black Star Canyon Road from the MPAH.

The requested amendment does not adversely impact the MPAH and is, therefore, recommended for approval.

County – Reclassify Bucker Way

Bucker Way is an unbuilt facility in the Rancho Mission Viejo community that is currently classified on the MPAH as a future Secondary (four-lane, undivided) arterial. The County is requesting to reclassify Bucker Way as a Divided Collector (two-lane, divided) arterial between Ranch Canyon Road and Coyotes Road to provide the appropriate capacity needed for the anticipated travel demand and accommodations for active transportation.

Based on analysis by OCTA staff, using OCTAM 5.1, approximately 3,000 daily vehicle trips are forecasted on this segment in 2050. Since a divided collector can carry up to 15,000 daily vehicle trips while maintaining an acceptable level of service, the proposed amendment provides sufficient capacity, and no trips are expected to be diverted to other facilities.

The requested amendment does not adversely impact the MPAH and is, therefore, recommended for approval.

County – Reclassify Ranch Canyon Road

Ranch Canyon Road is an unbuilt facility in the Rancho Mission Viejo community that is currently classified on the MPAH as a future Primary (four-lane, divided) arterial. The County is requesting to reclassify Rancho Canyon Road to a Divided Collector (two-lane, divided) arterial between Bucker Way and Cow Camp Road to provide the appropriate capacity needed for the anticipated travel demand and accommodations for active transportation.

Based on analysis by OCTA staff, using OCTAM 5.1, approximately 13,000 daily vehicle trips are forecasted on this segment in 2050, assuming the amended classification. Since a divided collector can carry up to 15,000 daily vehicle trips while maintaining an acceptable level of service, the proposed amendment provides sufficient capacity. This amendment does result in a diversion of approximately 2,000 trips to other facilities within the Rancho Mission Viejo area but does not cause any performance issues.

The requested amendment does not adversely impact the MPAH and is, therefore, recommended for approval.

Fullerton – Harbor Boulevard

Harbor Boulevard, between Bastanchury Road and Berkeley Avenue, currently has between four and six travel lanes. This segment is classified on the MPAH as a major (six-lane, divided) arterial. The City of Fullerton is requesting reclassification of this facility to a primary (four-lane, divided) arterial to accommodate active transportation improvements that are part of the Orange County Complete Streets Program (OCCSP) approved by the OCTA Board of Directors (Board) on February 12, 2024. This project requires a reduction of one lane in each direction north of Valley View Drive/Brea Boulevard and will provide six-foot bicycle lanes, a two-foot painted buffer, continuous sidewalks, and improved bus stops.

Based on analysis by OCTA staff, using OCTAM 5.1, approximately 35,000 daily vehicle trips are forecasted on Harbor Boulevard north of Valley View Drive/Brea Boulevard, and approximately 52,000 daily vehicle trips south of Valley View Drive/Brea Boulevard, assuming the amended classification. A primary arterial can carry up to 30,000 daily vehicle trips while maintaining an acceptable level of service. However, the City of Fullerton has established a level of service standard that allows a primary arterial to carry up to 33,800. While the projected volumes for 2050 exceed both the MPAH and locally-adopted level of service standards for roadway segments, additional analysis was performed that showed the corridor intersections would continue to meet the locally adopted level of service standards for intersections. Additionally, there are no adverse impacts that extend beyond the City of Fullerton’s jurisdictional boundary.

The requested amendment does not adversely impact the MPAH outside of the City of Fullerton, therefore, it is recommended for approval.

Laguna Hills – Paseo De Valencia

Paseo De Valencia, between Alicia Parkway and Cabot Road, is currently constructed as a primary (four-lane, divided) arterial, and is classified as a major (six-lane, divided) arterial and between Alicia Parkway and La Paz Road. The segment between La Paz Road and Cabot Road is classified as a secondary (four-lane, divided) arterial. The City of Laguna Hills is requesting the reclassification of both segments to a divided collector (two-lane, divided) arterial to provide traffic and safety improvements in the vicinity of Valencia Elementary School. This is part of a project included in the OCCSP that was approved by the Board on February 12, 2024.

Based on analysis by OCTA staff, using OCTAM 5.1, between 5,000 and 10,000 daily vehicle trips are forecasted on Paseo De Valencia in 2050. Since a divided collector can carry up to 15,000 daily vehicle trips while maintaining an acceptable level of service, the proposed amendment provides sufficient capacity with no substantial diversion of traffic.

The requested amendment does not adversely impact the MPAH and is, therefore, recommended for approval.

Laguna Hills – Cabot Road

Cabot Road, between Paseo De Valencia and El Paseo, is currently constructed as a primary (four-lane, divided) arterial, and is classified as a secondary (four-lane, undivided) arterial. The City of Laguna Hills is requesting the reclassification of this facility to a divided collector (two-lane, divided) arterial to accommodate multimodal improvements that are linked to the improvements on Paseo De Valencia, and are part of the same project noted above that is included in the OCCSP approved by the Board on February 12, 2024.

Based on analysis by OCTA staff, using OCTAM 5.1, approximately 10,000 daily vehicle trips are forecasted on Cabot Road in 2050. Since a divided collector can carry up to 15,000 daily vehicle trips while maintaining an acceptable level of service, the proposed amendment provides sufficient capacity with no substantial diversion of traffic.

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info@ocpw.ocgov.com

OCPublicWorks.com

Administrative Services

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OC Facilities Design & Construction Management

OC Facilities Maintenance & CUF

OC Fleet Services

OC Construction

OC Environmental Resources

OC Operations & Maintenance

OC Infrastructure Programs

OC Survey

March 6, 2024

Mr. Kurt Brotcke, Director
Strategic Planning
Orange County Transportation Authority
550s. Main Street
Orange, CA 92863-1584

Subject: MPAH Amendment Request for Villa Park Road/Santiago Canyon Road

Dear Mr. Brotcke,

Orange County Public Works (OCPW) is formally requesting the initiation of a Master Plan of Arterial Highways (MPAH) Amendment process for Villa Park Road and Santiago Canyon Road between Hewes Street and Cannon Street. This roadway segment is situated within unincorporated Orange County and City of Orange.

OCPW proposes the following modification to the MPAH:

- Reclassification of Villa Park Road/Santiago Canyon Road between Hewes Street and Cannon Street from a 6-lane Major Arterial Highway to a 5-lane Asymmetric Primary Arterial Highway.

Other stakeholders involved in the project include the City of Orange and the City of Villa Park, both of which endorse the amendment request (please refer to the attached consent forms). A Traffic Study conducted by OCPW has produced results that support the requested amendment (refer to the report attached).

Should you have any inquiries regarding this change, please feel free to contact Wei Zhu at (657) 303-3113.

Sincerely,

Nardy Khan, P.E., PMP
Deputy Director
OC Public Works/Infrastructure Programs

cc: Denis Bilodeau, OC Public Works/Traffic & Development Support
Wei Zhu, OC Public Works/Traffic

Attachments: Consent Form – City of Orange
Consent Form – City of Villa Park
Villa Park Road MPAH Amendment Traffic Study

ORANGE COUNTY
OC Public Works

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March 27, 2024

Mr. Kurt Brotcke, Director
Strategic Planning
Orange County Transportation Authority
550 South Main Street
Orange, CA. 92863-1584

Subject: MPAH Amendment Request for Santiago Canyon Road (SCR), El Toro Road, and Black Star Canyon Road

Dear Mr. Brotcke,

On May 8, 2017, the Orange County Transportation Authority (OCTA) Board of Directors (Board) approved an amendment to the Master Plan of Arterial Highways (MPAH), which included three changes: reclassification of Santiago Canyon Road (SCR), deletion of the Jeffrey Road Extension, and deletion of Black Star Canyon Road between Silverado Canyon Road and the Orange County/Riverside County line. The approval was contingent upon fulfilling several requirements, including amending each agency's general plan. This amendment was based on a traffic study that recommended reclassifying SCR, from east of the State Route 241 and Live Oak Canyon Road, from a primary (four-lane, divided) to a collector (two-lane undivided). However, this recommendation did not secure approval from the Orange County Planning Commission. Consequently, the County's General Plan was not amended, and the County's original MPAH Amendment request was indefinitely put on hold.

In 2023, Orange County Public Works (OCPW) embarked on another comprehensive Traffic Study for three roadways: SCR, El Toro Road, and Black Star Canyon Road. These roadway segments are situated within unincorporated Orange County, the City of Lake Forest, and the City of Mission Viejo. Please note that the deletion of the Jeffrey Road Extension is not included in our current request as it is not needed at this time. The outcome from our 2023 study for SCR diverges from the 2017 one and now takes precedence.

OCPW formally requests the withdrawal of the previous MPAH Amendment from 2017, and the initiation of a MPAH Amendment process for the segments of SCR, El Toro Road, and Black Star Canyon Road for the following modifications:

- Reclassify SCR between SR-241 and Live Oak Canyon Road from its currently planned designation as a Primary Arterial (four-lane divided roadway) to a Divided Collector (two-lane divided roadway with a center turn-lane)

- Reclassify El Toro Road (ETR) between Live Oak Canyon Road and Portola Parkway from its currently planned designation as a Major Arterial (six-lane divided roadway) to a Primary Arterial (four-lane divided roadway) with the exception that the southern end segment between Pheasant Creek and Portola Parkway remain at its current condition as an asymmetric Primary Arterial (five-lane divided roadway)

- Delete Black Star Canyon Road (BSCR) between Silverado Canyon Road and the Orange County/Riverside County line.

Both the City of Lake Forest and the City of Mission Viejo, as stakeholders of the project, endorse the amendment request (please refer to the attached consent forms). The report of the 2023 Traffic Study supporting the requested amendment is enclosed for your reference.

Should you have any inquiries regarding the amendment, please feel free to contact Wei Zhu at (657) 303-3113.

Sincerely,

Nardy Khan, P.E./P.M.P.
Deputy Director
Orange County Public Works/ Infrastructure Programs

cc: Denis Bilodeau, Manager, Traffic & Development Support
Wei Zhu, Sr. Civil Engineer, Traffic & Development Support

Attachment: Consent Form – City of Lake Forest
Consent Form – City of Mission Viejo
MPAH Amendment Study for SCR, ETR, BSCR

ORANGE COUNTY OC Public Works

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July 18, 2024

Mr. Kia Mortazavi, Executive Director
Strategic Planning
Orange County Transportation Authority
550 South Main Street
Orange, CA. 92863-1584

Subject: MPAH Amendment Request for Bucker Way and Ranch Canyon Road in RMV

Dear Mr. Mortazavi,

Orange County Public Works (OCPW) is formally requesting the initiation of a Master Plan of Arterial Highways (MPAH) Amendment for Bucker Way and Ranch Canyon Road within the Rancho Mission Viejo Planning Area 3 (PA3). The roadway segments are situated within unincorporated Orange County.

OCPW proposes the following modification to the MPAH:

- Reclassify Bucker Way between Ranch Canyon Road to Coyotes Road (S) from Secondary Arterial (4-lane) to Divided Collector (2-lane)

- Reclassify Ranch Canyon Road between Bucker Way to Cow Camp Road from Primary Arterial (4-lane) to Divided Collector (2-lane)

The report of the Traffic Study dated June 2024 prepared by Fehr & Peers supports the requested amendment and is enclosed for your reference.

Should you have any inquiries regarding the amendment, please feel free to contact Wei Zhu at (657) 303-3113.

Sincerely,

Nardy Khan, P.E./P.M.P.
Deputy Director
Orange County Public Works/ Infrastructure Programs

cc: Denis Bilodeau, Manager, Traffic & Development Support
Wei Zhu, Sr. Civil Engineer, Traffic & Development Support

Attachment: MPAH and GP Amendment for Bucker Way & Ranch Canyon Road

CITY OF FULLERTON
Public Works Department – *Engineering Division*

June 21, 2024

Orange County Transportation Authority
ATTN: Ivy Hang, Senior Transportation Analyst
550 South Main Street, P.O. Box 14184
Orange, CA 92863

Via Email: IHang@octa.net

Subject: **Master Plan of Arterial Highways Amendment – Harbor Boulevard**

The City of Fullerton is requesting an amendment to the Master Plan of Arterial Highways (MPAH). Harbor Boulevard is currently classified as a 6-lane Major Divided Arterial spanning from Berkely Avenue to Lambert Avenue within the City of Fullerton. Below is a summary of the existing lane geometry throughout that segment.

Segment:	Existing Lane Geometry:
Berkeley Avenue to West Valley View Drive/North Brea Boulevard	4-lanes
Valley View Drive/North Brea Boulevard to Bastanchury Road	6-lanes
Bastanchury Road to Imperial Highway	4-lanes
Harbor Boulevard to Lambert Road	6-lanes

NOTE: Harbor Boulevard is classified as a 4-lane Primary Divided Arterial south of Berkeley Avenue to Orangethorpe Avenue and north of Lambert Avenue to La Habra City Limits.

The City is requesting the segment between Berkely Avenue to Bastanchury Road be reclassified to a 4-lane Primary Divided Arterial. The reclassification will support the City's objective to provide various 'complete streets' improvements and fulfil our grant obligations for the recently awarded 2023 Orange County Complete Streets Program (OCCSP) grant application for the Harbor Boulevard Complete Streets Improvement Project. Attachment 'A' illustrates the limits and existing/proposed cross-section.

The reclassification requires lane reduction between Valley View Drive/North Brea Boulevard to Bastanchury Road, which was determined feasible based on the Response to OCTA Comments, dated April 16, 2024; see Attachment 'B' and Revised Traffic Analysis dated April 16, 2024; see Attachment 'C'.

Upon receiving conditional approval from OCTA's Board of Directors, Fullerton will take necessary steps to update the Circulation Element in our General Plan to reflect the changes identified above.

Please contact me if you have any questions or require additional information.

Regards,

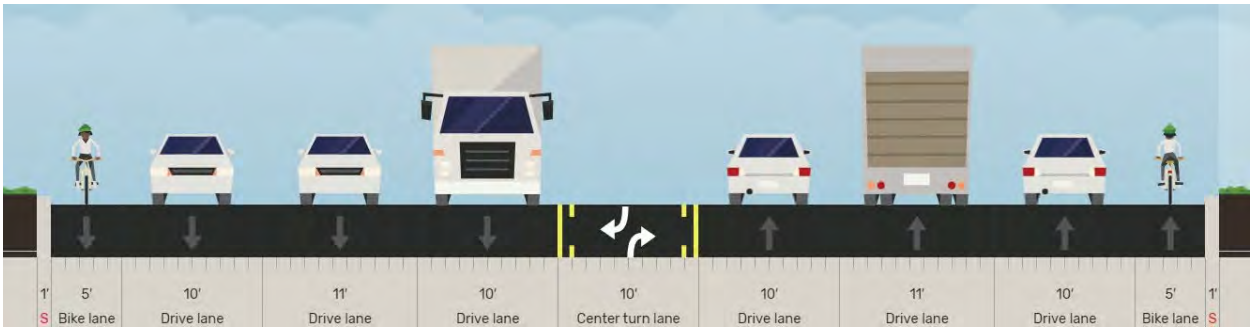
for Stephen Bise, PE, TE
Public Works Director
714-738-6852
Stephen.Bise@cityoffullerton.com

cc: David Grantham, City Engineer
Jerome Joaquin, Public Works Administration Manger
Dave Roseman, City Traffic Engineer
Chris Schaefer, Planning Manager

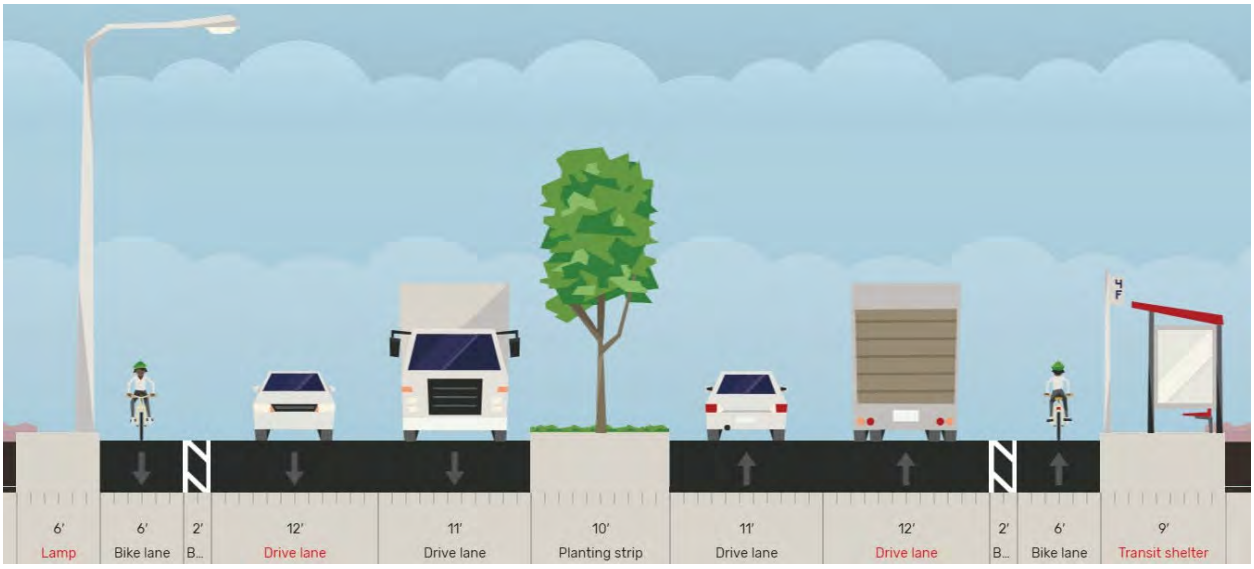
Attachment A

Typical Section

Roadway: **Harbor Boulevard**
Limits: **Valley View Drive/North Brea Boulevard to Bastanchury Road**
Length: **~5000 feet**
Right-of-Way Width: **104-115 feet**



Existing
10–11-ft travel lanes
10-ft two-way left turn lane
5-ft Class II Bike lanes
MPAH Classification = Major Divided Arterial



Proposed
11-12-ft travel lanes
10-ft median or left turn pocket
6-ft buffered Class II Bike Lanes
New, continuous sidewalk with upgrade transit stops
MPAH Classification = Primary Divided Arterial

Attachment B

MEMORANDUM

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Pasadena
Irvine
San Diego

To: Dave Roseman
City Traffic Engineer – City of Fullerton

Date: April 16, 2024

From: Richard E. Barretto, P.E., Principal
Linscott, Law & Greenspan, Engineers

LLG Ref: 1.20.4391.4

Subject: ***Response to Comments Memorandum for the Harbor Boulevard "Complete Streets" Improvements Project Traffic Analysis***

Linscott, Law & Greenspan, Engineers (LLG) is pleased to provide the following responses to address comments from OCTA, summarized in an email dated February 5, 2024, as it pertains to the *Traffic Analysis for the Harbor Boulevard "Complete Streets" Improvements Project*, dated September 20, 2023. The comments from OCTA are attached and referenced in our responses provided below.

Response to OCTA Comments:

- **Response to Comment 1:** *Upon reviewing the MPAH network map, we noted that Harbor Boulevard north of Berkeley Avenue is classified as a Major 6-lane divided Arterial, while south of Berkeley Avene is a Primary 4-lane Arterial. Suppose the City of Fullerton decided to pursue an MPAH amendment to align with the proposed reconfiguration. In that case, OCTA suggests including the segment between Brea Boulevard and Berkeley Avenue for consistent MPAH classification from Berkeley Avenue to Valencia Mesa Drive.*

  The traffic analysis has been revised to include the segments between Brea Boulevard/Valley View Drive and Berkeley Avenue.

- **Response to Comment 2:** *Specify the version of OCTAM used or provide the date of the model to ensure clarity in the analysis.*

  As noted on Page 5 of the report, under sub header *Year 2045 Buildout Traffic Volumes*, OCTAM 5.0 traffic model was utilized for Year 2045 traffic volume forecasts, which was the most current version of the model at the time of analysis.

- **Response to Comment 3:** *Clarify that the volumes were not provided directly by OCTA or directly from a version of OCTAM approved by OCTA.*

  Comment noted, the report has been updated accordingly.

- **Response to Comment 4:** *Clarify and provide a more comprehensive discussion on traffic volume development: a) Provide additional details on the methodology for forecasting future traffic volumes, particularly the rationale behind determining the 1.55% traffic growth; b) Clarify the justifications for not utilizing the model growth on the specific roadways.*

  As noted on Page 5 of the report, under sub header *Year 2045 Buildout Traffic Volumes*, the development of Year 2045 buildout traffic volumes for all study

David S. Shender, PE
John A. Boarman, PE
Richard E. Barretto, PE
Keil D. Maberry, PE
KC Yellapu, PE
Dave Roseman, PE
Shankar Ramakrishnan, PE

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intersections and roadway segments were based on the OCTAM 5.0 model.

The reference to 1.55% was included for informational purposes only. We were merely documenting the annual growth rate when comparing the existing traffic counts to the Year 2045 model generated traffic volumes. However, the report has been updated to remove this reference to avoid potential confusion.

- **Response to Comment 5:** *Clarify why traffic volumes for the with and without project scenarios are the same. The analysis should encompass potential traffic diversions or impacts to parallel/adjacent roadways due to the road diet.*

  It is our understanding that due to the existing topography and street network, there are not any viable parallel routes within the vicinity of the Project that would result in potential traffic diversions due to the road diet. Therefore, traffic volumes are expected to remain the same for both with and without the proposed Project. It should be noted that the weekday peak hour and daily service levels are generally considered adequate with the implementation of the proposed Project and therefore motorists will not feel the need to adjust their driving patterns.

- **Response to Comment 6:** *Identify the specific thresholds for determining significant impacts at intersections and on roadway segments (e.g., LOS threshold and changes in V/C).*

  As noted on Page 7 of the report, under sub header *Level of Service Standards and Thresholds,* the City of Fullerton has identified LOS D as the minimum acceptable condition that should be maintained at both study intersections and roadway segments. Improvements should be considered if the Project causes an intersection or roadway segment operating at or above acceptable LOS D to an unacceptable condition.

* * * * * * * * * * *

Please let us know if you have any comments or questions regarding this response memorandum.

Attachments

cc: Shane S. Green, P.E. LLG
File

OCTA COMMENTS DATED FEBRUARY 5, 2024

Fw: [EXTERNAL MAIL]MPAH - Harbor Boulevard Complete Street Traffic Study Comments

From: Ivy Hang <IHang@octa.net>
Sent: Monday, February 5, 2024 12:33 PM
To: Stephen Bise <Stephen.Bise@cityoffullerton.com>
Cc: Gregory Nord <gnord@octa.net>; Anup Kulkarni <akulkarni@octa.net>; Archie Tan <atan@octa.net>
Subject: [EXTERNAL MAIL]MPAH - Harbor Boulevard Complete Street Traffic Study Comments

Hi Stephen,

Thank you for providing the Traffic Analysis for the Harbor Boulevard "Complete Streets" Improvements Project. The traffic study recommends reconfiguring the segment from a 6-lane divided Major Arterial to a 4-lane divided Primary Arterial on Harbor Boulevard between Valencia Mesa Drive and Valley View Drive/Brea Boulevard.

Upon reviewing the MPAH network map, we noted that Harbor Boulevard north of Berkeley Avenue is classified as a Major 6-lane divided Arterial, while south of Berkeley Avenue is a Primary 4-lane divided Arterial. Suppose the City of Fullerton decides to pursue an MPAH amendment to align with the proposed reconfiguration. In that case, OCTA suggests including the segment between Brea Boulevard and Berkeley Avenue for consistent MPAH classification from Berkeley Avenue to Valencia Mesa Drive. In addition, please consider revising the traffic study following the comments provided below.

- Specify the version of OCTAM used or provide the date of the model to ensure clarity in the analysis.
- Clarify that the volumes were not provided directly by OCTA or directly from a version of OCTAM approved by OCTA.
- Clarify and provide a more comprehensive discussion on traffic volume development.
    - Provide additional details on the methodology for forecasting future traffic volumes, particularly the rationale behind determining the 1.55% traffic growth.
    - Clarify the justifications for not utilizing the model growth on the specific roadways.
- Clarify why traffic volumes for the with and without project scenarios are the same.
    - The analysis should encompass potential traffic diversions or impacts to parallel/adjacent roadways due to the road diet.
- Identify the specific thresholds for determining significant impacts at intersections and on roadway segments (e.g., LOS threshold and changes in V/C).

Feel free to reach out if you have any questions.

Thank You,

Ivy Hang
Senior Transportation Analyst
Long-Range Planning & Corridor Studies
Orange County Transportation Authority
550 South Main Street, P.O. Box 14184, Orange, CA 92863
P: (714) 560-5684 | **E:** ihang@octa.net

Attachment C

MEMORANDUM

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Pasadena
Irvine
San Diego

To: Dave Roseman
City Traffic Engineer – City of Fullerton

Date: April 16, 2024

From: Richard E. Barretto, P.E., Principal
Linscott, Law & Greenspan, Engineers

LLG Ref: 2.23.4708.1

Subject: ***Revised Traffic Analysis for the Harbor Boulevard "Complete Streets" Improvements Project Fullerton, California***

Linscott, Law & Greenspan, Engineers (LLG) is pleased to submit this revised traffic analysis memorandum for the proposed Harbor Boulevard "Complete Streets" Improvements Project (hereinafter referred to as Project) located in Fullerton, California. This traffic analysis has been prepared to identify the traffic implications associated with the proposed "complete streets" modifications to Harbor Boulevard, between Bastanchury Road and Brea Boulevard/Valley View Drive. Further, the traffic analysis has been prepared in support of the City of Fullerton's proposed reclassification of Harbor Boulevard, between Valencia Mesa Drive and Berkeley Avenue, from a Major Arterial Highway to a Primary Arterial Highway. This letter was revised to address comments provided by OCTA via an email on February 5, 2024.

Harbor Boulevard, along the northern end and southern end, consists of a medical/medical office and commercial/retail properties at the street frontage, whereas the midblock section is fronted by open space. The Project involves multimodal improvements along the middle section of Harbor Boulevard, from Valencia Mesa Drive to Brea Boulevard/Valley View Drive, to provide bicycle and pedestrian connectivity in this corridor. The Project is proposing to modify Harbor Boulevard, between Valencia Mesa Drive and Brea Boulevard/Valley View Drive, to a four-lane divided roadway with on-street "buffered" bike lanes and curb adjacent sidewalks.

Included in this memorandum are:

- AM and PM peak hour and Daily capacity analyses for existing conditions,
- AM and PM peak hour and Daily capacity analyses for existing plus project conditions,
- AM and PM peak hour and Daily capacity analyses for future buildout (Year 2045) conditions,
- AM and PM peak hour and Daily capacity analyses for future buildout (Year 2045) plus project conditions, and
- Intersection vehicle queueing analyses along Harbor Boulevard.

This study satisfies the traffic analysis requirements of the City of Fullerton.

David S. Shender, PE
John A. Boarman, PE
Richard E. Barretto, PE
Keil D. Maberry, PE
KC Yellapu, PE
Dave Roseman, PE
Shankar Ramakrishnan, PE

An LG2WB Company Founded 19

STUDY AREA/PROJECT LIMITS

The following six (6) study intersections and four (4) roadway segments located within the limits of the Project have been identified for evaluation:

Study Intersections:

1. Harbor Boulevard at Bastanchury Road
2. Harbor Boulevard at Valencia Mesa Drive
3. Harbor Boulevard at Brea Boulevard/Valley View Drive
4. Harbor Boulevard at Brea Dam Park (Future Signalized Intersection)
5. Harbor Boulevard at E. Valley View Drive
6. Harbor Boulevard at Berkeley Avenue

Study Roadway Segments:

A. Harbor Boulevard, between Bastanchury Road and Valencia Mesa Drive
B. Harbor Boulevard, between Valencia Mesa Drive and Brea Boulevard/Valley View Drive
C. Harbor Boulevard, between Brea Boulevard/Valley View Drive and E. Valley View Drive
D. Harbor Boulevard, between E. Valley View Drive and Berkeley Avenue

Figure 1, which is located at the end of this memorandum, presents the vicinity map which illustrates the limits of the Harbor Boulevard improvements and depicts the study locations and surrounding street system.

The Level of Service (LOS) investigations at these key locations were used to evaluate the potential traffic implications associated with implementation of the proposed Project. Our methods of analysis, findings and conclusions are described in detail in the following sections of this memorandum.

PROJECT DESCRIPTION

Harbor Boulevard is currently classified as a Major Arterial Highway in the City's Circulation Elements as well as the Orange County Master Plan of Arterial Highways (MPAH). The existing configuration of Harbor Boulevard, between Bastanchury Road and Brea Boulevard/Valley View Drive, is currently three through lanes in the southbound direction and three through lanes in the northbound direction, with Class II bike lanes provided in the middle section from just north of Valencia Mesa Drive to north of Brea Boulevard/Valley View Drive. The existing configuration of Harbor Boulevard, between Brea Boulevard/Valley View Drive and E. Valley View Drive, is

currently two through lanes in the southbound direction and three through lanes in the northbound direction. The existing configuration of Harbor Boulevard, between E. Valley View Drive and Berkeley Avenue is two through lanes in the southbound direction and two through lanes in the northbound direction. Daily traffic volumes on Harbor Boulevard, between Bastanchury Road and Berkeley Avenue, range between 28,271 ADT and 44,041 ADT.

With approval of the proposed reclassification to a Primary Arterial Highway, the proposed Project would allow for construction of "complete streets" improvements on Harbor Boulevard, between Bastanchury Road and Brea Boulevard/Valley View Drive, which include the removal of a through lane in both the northbound and southbound directions, the installation of a Class II bike lane with buffer in both the northbound and southbound directions, and the construction of sidewalks on both sides of the roadway, where applicable, which will connect to the existing sidewalk network. It should be noted that the proposed Project will maintain the existing lane geometrics south of Brea Boulevard/Valley View Drive. As such, upon completion of the Project, this section of Harbor Boulevard will generally match the number of travels lanes on the section of Harbor Boulevard south of Brea Boulevard/Valley View Drive.

The Project improvements are proposed to begin at Valencia Mesa Drive and terminate south of Brea Boulevard. Given the focus of the Project is on the midblock segments of Harbor Boulevard, the existing lane geometrics at the intersections of Harbor Boulevard at Bastanchury Road (Intersection No. 1) will be maintained as part of the proposed Project. The "Complete Streets" improvements consist of the following:

- **Harbor Boulevard at Bastanchury Road:** The implementation of a "Complete Streets" on Harbor Boulevard will not affect the intersection; existing lane geometrics will be maintained.

- **Harbor Boulevard at Valencia Mesa Drive:** The implementation of a "Complete Streets" on Harbor Boulevard will replace the outermost through-lane for the northbound approach with a Class II bike land and buffer, the outermost through-lane for the southbound approach with a trap right-turn lane, and the outermost through-lane for the southbound departure with a Class II bike lane and buffer. The northbound left-turn pocket will be restriped to provide 185-feet of storage. Sidewalks will be constructed for the southwest corner of the intersection and proceed southerly to connect to the existing

sidewalk on the west side of Harbor Boulevard at Brea Boulevard/Valley View Drive.

- **Harbor Boulevard at Brea Boulevard/Valley View Drive:** The implementation of a "Complete Streets" on Harbor Boulevard will replace the outermost through-lane for the southbound approach with a right-turn pocket providing 140-feet of storage and restripe the left-turn lane to provide 140-feet of storage. The outermost through-lane for the northbound approach will be removed and the outermost through-lane for the northbound departure will be replaced with a Class II bike lane and buffer.

It should also be noted that a new signalized intersection for the existing Brea Dam Park along Harbor Boulevard, located between Valencia Mesa Drive and Brea Boulevard/Valley View Drive, has been assumed as part of the proposed Project and consists of the following:

- **Harbor Boulevard at Brea Dam Park:** Construct a three-leg intersection to provide two through lanes and a 200-foot right-turn pocket in the northbound direction, two through lanes and a 250-foot left-turn pocket in the southbound direction, and shared left/right-turn lane in the westbound direction. Install a two-phase traffic signal. The implementation of a "Complete Streets" on Harbor Boulevard will construct sidewalks along Harbor Boulevard and provide a Class II bike lane and buffer in both the northbound and southbound directions.

Figure 2 presents the aerial map for Harbor Boulevard, between Bastanchury Road and Berkeley Avenue, under existing conditions. ***Figure 3*** presents the proposed lane geometry on Harbor Boulevard with the Project and compares it to the existing lane geometry.

EXISTING AND FUTURE TRAFFIC VOLUMES

Existing Traffic Volumes

Six (6) study intersections and four (4) roadway segments have been identified as the locations at which to evaluate existing and future traffic operating conditions. Existing weekday daily and peak hour traffic volumes for the five (5) existing study intersections and four (4) roadway segments evaluated in this report were obtained from manual turning movement counts conducted by Transportation Studies, Inc. in May 2023 and March 2024. ***Figure 4*** presents the existing weekday AM peak hour,

PM peak hour, and daily traffic volumes at the five (5) key study intersections and four (4) roadway segments evaluated in this report. ***Appendix A*** contains the detailed count sheets for the locations evaluated in this report.

Since the proposed Project is the implementation of "Complete Streets" improvements along Harbor Boulevard, the project does not generate any additional vehicular trips. As such, Existing Plus Project traffic volumes are identical to the Existing without Project traffic volumes.

Year 2045 Buildout Traffic Volumes

The Year 2045 traffic volume forecasts were developed via the utilization of the OCTAM 5.0 Year 2045 traffic model. Specifically, daily, AM peak period and PM peak period intersection traffic svolumes were provided by OCTA for the existing base year (i.e. Year 2016) and for the Year 2045 year. The AM peak period corresponds to a three-hour morning commute period while the PM peak period corresponds to a four-hour afternoon commute period. Using the peak period model runs and the OCTA approved peak hour factors (i.e. AM = 0.3566 and PM = 0.2662), the one-hour peak hour intersection traffic volumes were determined. ***Figure 5*** presents the Year 2045 Buildout weekday AM peak hour, PM peak hour, and daily traffic volumes at the six (6) key study intersections and four (4) roadway segments evaluated in this report.

Since the proposed Project is the implementation of "Complete Streets" improvements along Harbor Boulevard, the project does not generate any additional vehicular trips. As such, Year 2045 Buildout Plus Project traffic volumes are identical to the Year 2045 Buildout without Project traffic volumes.

LEVEL OF SERVICE CRITERIA

Level of Service Analysis Methodologies

In conformance with the City of Fullerton criteria, weekday AM and PM peak hour operating conditions for the six (6) key study intersections were evaluated using the methodology outlined in the *Highway Capacity Manual 6* (HCM 6) for signalized intersections. The analysis of the four (4) roadway segments were evaluated per the Volume to Capacity (V/C) Ratio methodology.

Highway Capacity Manual (HCM) Method of Analysis (Signalized)

Based on the HCM 6th Edition operations method of analysis, level of service for signalized intersections is defined in terms of control delay, which is a measure of driver discomfort, frustration, fuel consumption and lost travel time. The delay experienced by a motorist is made up of a number of factors that relate to control, geometries, traffic, and incidents. Total delay is the difference between the travel time actually experienced and the reference travel time that would result during ideal conditions: in the absence of traffic control, in the absence of geometric delay, in the absence of any incidents and when there are no other vehicles on the road.

In the HCM, only the portion of total delay attributed to the control facility is quantified. This delay is called *control delay*. Control delay includes initial deceleration delay, queue move-up time, stopped delay and final acceleration delay. Specifically, LOS criteria for traffic signals are stated in terms of the average control delay per vehicle. The six qualitative categories of Level of Service that have been defined along with the corresponding HCM control delay value range for signalized intersections are shown in ***Table 1***.

Daily Volume to Capacity (V/C) Ratio Method of Analysis (Roadway Segments)

Daily operating conditions for the key study roadway segments have been investigated according to the Volume to Capacity (V/C) Ratio of each roadway segment. The V/C relationship is used to estimate the LOS of the roadway segment with the volume based on the 24-hour traffic volumes and the capacity based on the City's classification of each roadway. The six qualitative categories of Level of Service have been defined along with the corresponding Volume to Capacity (V/C) value range and are shown in ***Table 2***.

The roadway link capacity of each street classification according to the Orange County Master Plan of Arterial Highways (MPAH) is presented in ***Table 3***, along with the six corresponding service levels and associated V/C ratios.

Level of Service Standards and Thresholds

According to the City of Fullerton, LOS D is the minimum acceptable condition that should be maintained during the peak commute hours. LOS D has also been assumed as the threshold for daily.

Based on the City of Fullerton, the need for potential improvements will be assessed based on the following criteria:

- The Project causes a signalized or unsignalized intersection or roadway segment operating at or above an acceptable operating condition to degrade to an unacceptable condition.

EXISTING CONDITIONS TRAFFIC ANALYSIS

Existing Traffic Conditions Intersection Capacity Analysis

Table 4 summarizes the peak hour level of service results at the six (6) study intersections for Existing Plus Project traffic conditions. The first column (1) of HCM/LOS values in presents a summary of existing AM and PM peak hour traffic conditions utilizing the existing lane configurations. The second column (2) lists existing plus project traffic conditions utilizing the proposed project lane configurations. The third column (3) indicates whether the Project will exceed the LOS thresholds defined in this report and thus require improvements.

As shown in column (1) of *Table 4*, all five (5) existing key study intersections currently operate at an acceptable service level (LOS D or better) during the AM and PM peak hours.

As shown in column (2) of *Table 4*, all six (6) key study intersections are forecast to operate at an acceptable service level (LOS D or better) during the AM and PM peak hours with the addition of the proposed Project. As such, it can be concluded that the proposed Project will not have a negative effect on existing traffic conditions. ***Appendix B*** presents the HCM/LOS calculations for the study intersections.

Existing Traffic Conditions Roadway Segment Analysis

Table 5 summarizes the roadway segment level of service results at the four (4) roadway segments for Existing Plus Project traffic conditions. The first column (1) presents a summary of existing daily traffic conditions utilizing the existing lane configurations. The second column (2) lists existing plus project traffic conditions utilizing the proposed project lane configurations and indicates whether the intersection will operate adversely based on the criteria defined in this report and thus require improvements.

As shown in column (1) of *Table 5*, the two (2) out of the four (4) roadway segments currently operate adversely on a daily basis. Harbor Boulevard, between Brea

Boulevard/Valley View Drive and E. Valley View Drive (Segment C) and Harbor Boulevard, between E. Valley View Drive and Berkeley Avenue (Segment D) currently operate at LOS E and LOS F, respectively. The remaining roadway segments currently operate at an acceptable service level (LOS D or better) on a daily basis.

As shown in column (2) of *Table 5*, Harbor Boulevard, between Brea Boulevard/Valley View Drive and E. Valley View Drive (Segment C) and Harbor Boulevard, between E. Valley View Drive and Berkeley Avenue (Segment D) are forecast to operate adversely with the addition of the proposed Project. However, the proposed Project will maintain the existing lane geometrics along these roadway segments and thus the adverse service levels are not caused or worsened by the Project. The remaining two (2) roadway segments will continue to operate at an acceptable service level (LOS D or better) on a daily basis with the addition of the proposed Project. As such, it can be concluded that the proposed Project will not have a negative effect on existing traffic conditions.

YEAR 2045 BUILDOUT CONDITIONS TRAFFIC ANALYSIS

Year 2045 Buildout Traffic Conditions Intersection Capacity Analysis

Table 6 summarizes the peak hour level of service results at the six (6) study intersections for Year 2045 buildout Plus Project traffic conditions. The first column (1) of HCM/LOS values presents a summary of Year 2045 buildout AM and PM peak hour traffic conditions utilizing the existing lane configurations. The second column (2) lists Year 2045 buildout plus project traffic conditions utilizing the proposed project lane configurations. The third column (3) indicates whether the Project will exceed the LOS thresholds defined in this report and thus require additional improvements.

As shown in column (1) of *Table 6*, the intersection of Harbor Boulevard at Bastanchury Road (Intersection No. 1) is forecast to operate adversely during the AM and PM peak hours under Year 2045 Buildout traffic conditions. The remaining four (4) existing study intersections are forecast to operate at an acceptable service level (LOS D or better) during the AM and PM peak hours.

As shown in column (2) of *Table 6*, the intersection of Harbor Boulevard at Bastanchury Road (Intersection No. 1) will continue to operate adversely during the AM and PM peak hours with the addition of the proposed Project. Although the study intersection is forecast to operate adversely, the implementation of the Project does

not affect the forecast LOS E conditions. As such, it can be concluded that the proposed Project will not have a negative effect on future traffic conditions. *Appendix B* presents the HCM/LOS calculations for the study intersections.

Year 2045 Buildout Traffic Conditions Roadway Segment Analysis

Table 7 summarizes the roadway segment level of service results at the four (4) roadway segments for Year 2045 buildout Plus Project traffic conditions. The first column (1) presents a summary of Year 2045 Buildout daily traffic conditions utilizing the existing lane configurations. The second column (2) lists Year 2045 buildout plus project traffic conditions utilizing the proposed project lane configurations and indicates whether the intersection will operate adversely based on the criteria defined in this report and thus require additional improvements.

As shown in column (1) of *Table 7*, two (2) out of the four (4) roadway segments are forecast to operate adversely on a daily basis. Harbor Boulevard, between Brea Boulevard/Valley View Drive and E. Valley View Drive (Segment C) and Harbor Boulevard, between E. Valley View Drive and Berkeley Avenue (Segment D) are forecast to operate at LOS F. The remaining roadway segments are forecast to operate at an acceptable service level (LOS D or better) on a daily basis under Year 2045 Buildout traffic conditions and assuming a six-lane arterial is maintained.

As shown in column (2) of *Table 7*, Harbor Boulevard between Bastanchury Road and Valencia Mesa Drive (Segment A), Harbor Boulevard, between Brea Boulevard/Valley View Drive and E. Valley View Drive (Segment C) and Harbor Boulevard, between E. Valley View Drive and Berkeley Avenue (Segment D) are forecast to operate adversely with the addition of the proposed Project, whereas the segment between Valencia Mesa Drive and Brea Boulevard/Valley Drive (Segment D) is forecast to operate at LOS D with the conversion to a four-lane arterial.

The proposed Project will maintain the existing lane geometrics along Harbor Boulevard, between Brea Boulevard/Valley View Drive and E. Valley View Drive (Segment C) and Harbor Boulevard, between E. Valley View Drive and Berkeley Avenue (Segment D) and thus the adverse service levels are not caused or worsened by the Project.

To determine if the proposed Project will result in a deficiency for Harbor Boulevard between Bastanchury Road and Valencia Mesa Drive (Segment A), the roadway segment was further analyzed under peak hour conditions, as presented in ***Table 8***. Review of *Table 8* indicates that Segment A is forecast to operate at an acceptable

level of service during the AM and PM peak hours. As such, it can be concluded that the proposed Project will not have a negative effect on future traffic conditions.

INTERSECTION QUEUING EVALUATION

To validate the adequacy of the existing stacking/storage lengths along Harbor Boulevard with the addition of the proposed Project, a Synchro queuing evaluation was prepared for the northbound and southbound left-turn and right-turn pockets at the six (6) study intersections. The queuing evaluation was conducted for Existing Plus Project and Year 2045 Buildout Plus Project traffic conditions utilizing the Synchro 11.0 SimTraffic 95th percentile delay methodology. It should be noted that vehicular queues from Synchro 11.0 SimTraffic are computed based on the average queues of the microsimulation model, and therefore, each simulation is unique.

Existing Traffic Conditions

Table 9 presents the lane queuing analyses results for the six (6) signalized study intersections for existing traffic conditions. The first column (1) presents the lane queuing results for Existing traffic conditions. The second column (2) presents the results for Existing Plus Project traffic conditions.

Review of column (1) of *Table 9* indicates that the five (5) existing study intersections have queues which exceed the existing storage capacity for one or more intersection approaches under existing traffic conditions.

Review of column (2) of *Table 9* indicates that six (6) study intersections have queues which exceed the existing storage capacity for one or more intersection approaches with the addition of the proposed Project. However, with the exception of the southbound right-turn at Harbor Boulevard and Brea Boulevard/Valley View Drive (Intersection No. 3) and the northbound left-turn at Harbor Boulevard and Berkeley Avenue (Intersection No 6), the approaches which provide insufficient storage are the same as those identified under existing without project traffic conditions. As such, it can be concluded that the effect of the proposed Project on the existing roadway network is considered minimal when taking into consideration the already deficient storage under existing traffic conditions. It should be noted that the spillover queue for the southbound right-turn at Harbor Boulevard and Brea Boulevard/Valley View Drive (Intersection No. 3) can be accommodated within the transition area of the turn pocket. Similarly, the spillover queue for the northbound left-turn at Harbor Boulevard and Berkeley Avenue (Intersection No. 6) can be accommodated within

the upstream two-way left-turn lane. ***Appendix C*** presents the Synchro 11.0 SimTraffic worksheets for the study intersections.

Year 2045 Buildout Traffic Conditions

Table 10 presents the lane queuing analyses results for the six (6) signalized study intersections for Year 2045 Buildout traffic conditions. The first column (1) presents the lane queuing results for Year 2045 Buildout traffic conditions. The second column (2) presents the results for Year 2045 Buildout Plus Project traffic conditions.

Review of column (1) *Table 10* indicates that the five (5) existing study intersections have queues which exceed the existing storage capacity for one or more intersection approaches under Year 2045 Buildout traffic conditions.

Review of column (2) of *Table 10* indicates that six (6) study intersections have queues which exceed the existing storage capacity for one or more intersection approaches with the addition of the proposed Project. However, with the exception of the southbound right-turn at Harbor Boulevard and Brea Boulevard/Valley View Drive (Intersection No. 3), the approaches which provide insufficient storage are the same as those identified under Year 2045 Buildout without project traffic conditions. As such, it can be concluded that the effect of the proposed Project on the future roadway network is considered minimal when taking into consideration the already deficient storage under Year 2045 buildout traffic conditions. It should be noted that the spillover queue for the southbound right-turn at Harbor Boulevard and Brea Boulevard/Valley View Drive (Intersection No. 3) can be accommodated within the transition area of the turn pocket. *Appendix C* presents the Synchro 11.0 SimTraffic worksheets for the study intersections.

CONCLUSIONS

Based on the above, the proposed "Complete Streets" Project is not expected to negatively affect existing or future traffic conditions along Harbor Boulevard. Based on the City of Fullerton's level of service standards and threshold criteria, the weekday peak hour and daily service levels are considered adequate with the implementation of the proposed Project.

Although some queues at the six (6) study intersections are considered deficient, the implementation of the proposed Project will ensure bicycle and pedestrian connectivity along the corridor and will allow for a more balanced, multimodal

transportation network that meets the needs of all users of streets, roads, and highways, including motorists, pedestrians, and bicyclists. In addition, the proposed Project would generally provide continuity to Harbor Boulevard, which already is generally a four lane roadway south of Valley View Drive/Brea Boulevard. An added benefit of the proposed lane drop is that it helps to improve OCTA bus services since the bus will no longer be required to stop in a through lane and block traffic. It should be noted that some cities are adopting more flexible policies to solve traffic problems. For example, some communities have started accepting a lower (worse) automobile level of service standard in their downtowns and in urban neighborhoods or along transit corridors. By doing so, these cities are increasing capacity for other modes like transit and/or bikes. Therefore, opportunities and constraints both exist and should be thoroughly considered prior to approval of the "Complete Streets".

As such, LLG has concluded that the peak hour and daily traffic volumes (current and projected) on Harbor Boulevard do not warrant its current classification of Major Arterial Highway. The City's proposed reclassification of Harbor Boulevard, between Valencia Mesa Drive and Brea Boulevard/Valley View Drive, from a Major Arterial Highway to a Primary Arterial Highway, to install a new protected bike lane, with the provision of a new continuous sidewalk system, would appear feasible with the removal of one northbound through lane and one southbound through lane. It is also recommended to reclassify Harbor Boulevard, between Brea Boulevard/Valley View Drive and Berkeley Avenue, from a Major Arterial Highway to a Primary Arterial Highway for consistent MPAH classification. However, it should be noted that the proposed Project will maintain the existing lane geometrics along Harbor Boulevard, between Brea Boulevard/Valley View Drive and E. Valley View Drive, which currently has two through lanes in the southbound direction and three through lanes in the northbound direction with the third lane functioning primarily as a dedicated right turn for direct access to Brea Boulevard.

Therefore, it is recommended that the City of Fullerton formally request approval from Orange County Transportation Authority (OCTA) of the following classification for Harbor Boulevard:

Street	Limits	Existing Classification	Proposed Classification
Harbor Boulevard	Valencia Mesa Drive to Berkeley Avenue	Major Arterial	Primary Arterial

* * * * * * * * * * *

We appreciate the opportunity to work on this project. If you have any questions regarding this letter, please do not hesitate to call me at (949) 825-6175.

Sincerely,
Linscott, Law & Greenspan, Engineers

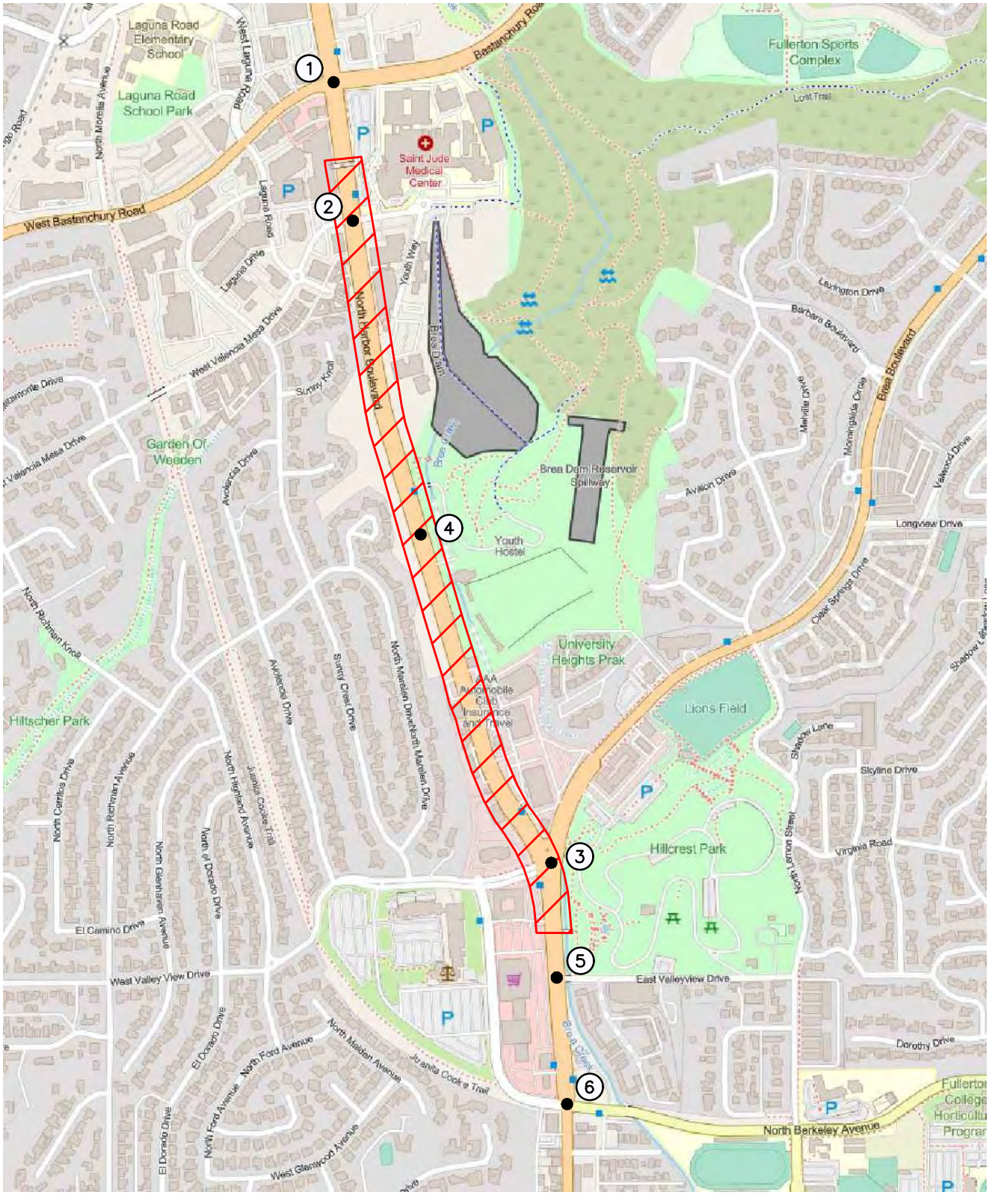




Richard E. Barretto, P.E.
Principal

cc: Shane S. Green, P.E., Senior Transportation Engineer

Attachments



n:\4700\2234708 - harbor boulevard road diet (billing ref. 1.20.4391.4), fullerton\dwg\4708 f-1.dwg LDP 07:37:47 04-16-2024 aguilar

LINSCOTT LAW & GREENSPAN engineers

N NO SCALE

KEY

(#) = STUDY INTERSECTION

= PROJECT LIMITS

FIGURE 1

VICINITY MAP

HARBOR BOULEVARD COMPLETE STREETS IMPROVEMENT PROJECT, FULLERTON



n:\4700\2234708 - harbor boulevard road diet (billing ref. 1.20.4391.4), fullerton\dwg\4708 f-2.dwg LDP 07:44:28 04-16-2024 aguilar

LINSCOTT LAW & GREENSPAN engineers

KEY

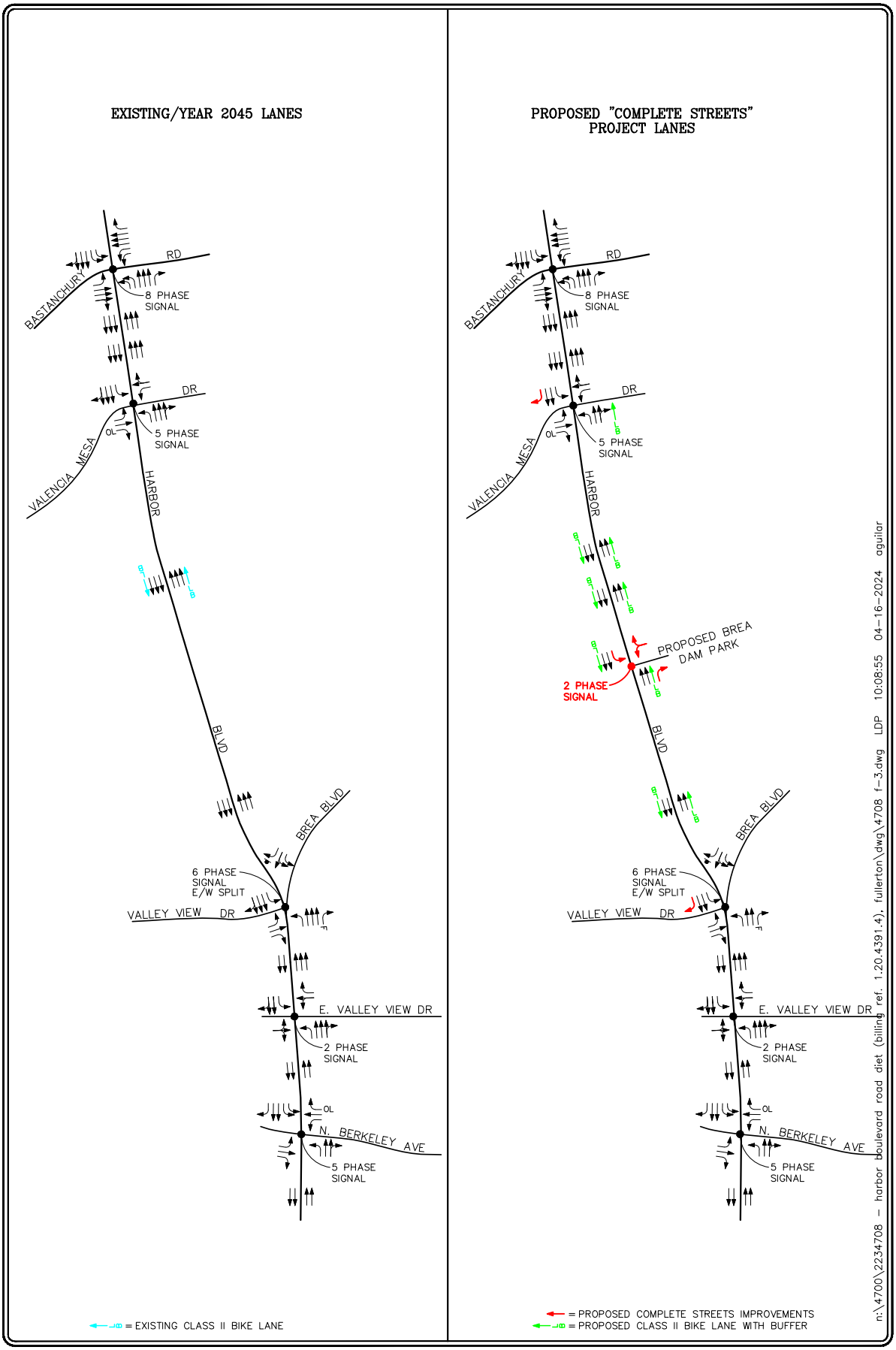
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FIGURE 2

EXISTING SITE AERIAL

HARBOR BOULEVARD COMPLETE STREETS IMPROVEMENT PROJECT, FULLERTON

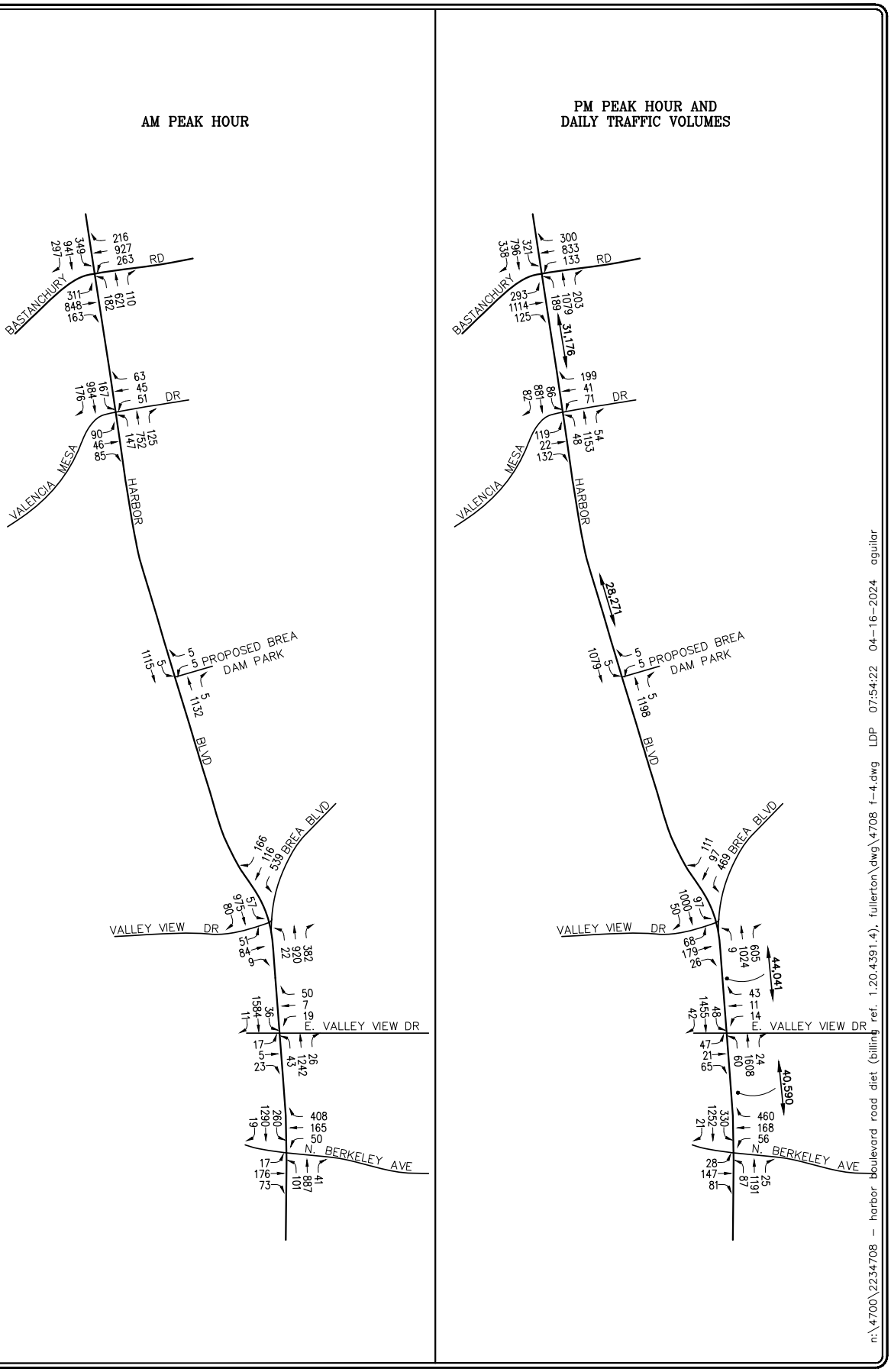


NO SCALE

FIGURE 3

HARBOR BOULEVARD EXISTING VS COMPLETE STREETS LANE GEOMETRICS

HARBOR BOULEVARD COMPLETE STREETS IMPROVEMENT PROJECT, FULLERTON



KEY

XX,XXX = DAILY TRAFFIC VOLUMES

NO SCALE

FIGURE 4

EXISTING TRAFFIC VOLUMES

HARBOR BOULEVARD COMPLETE STREETS IMPROVEMENT PROJECT, FULLERTON

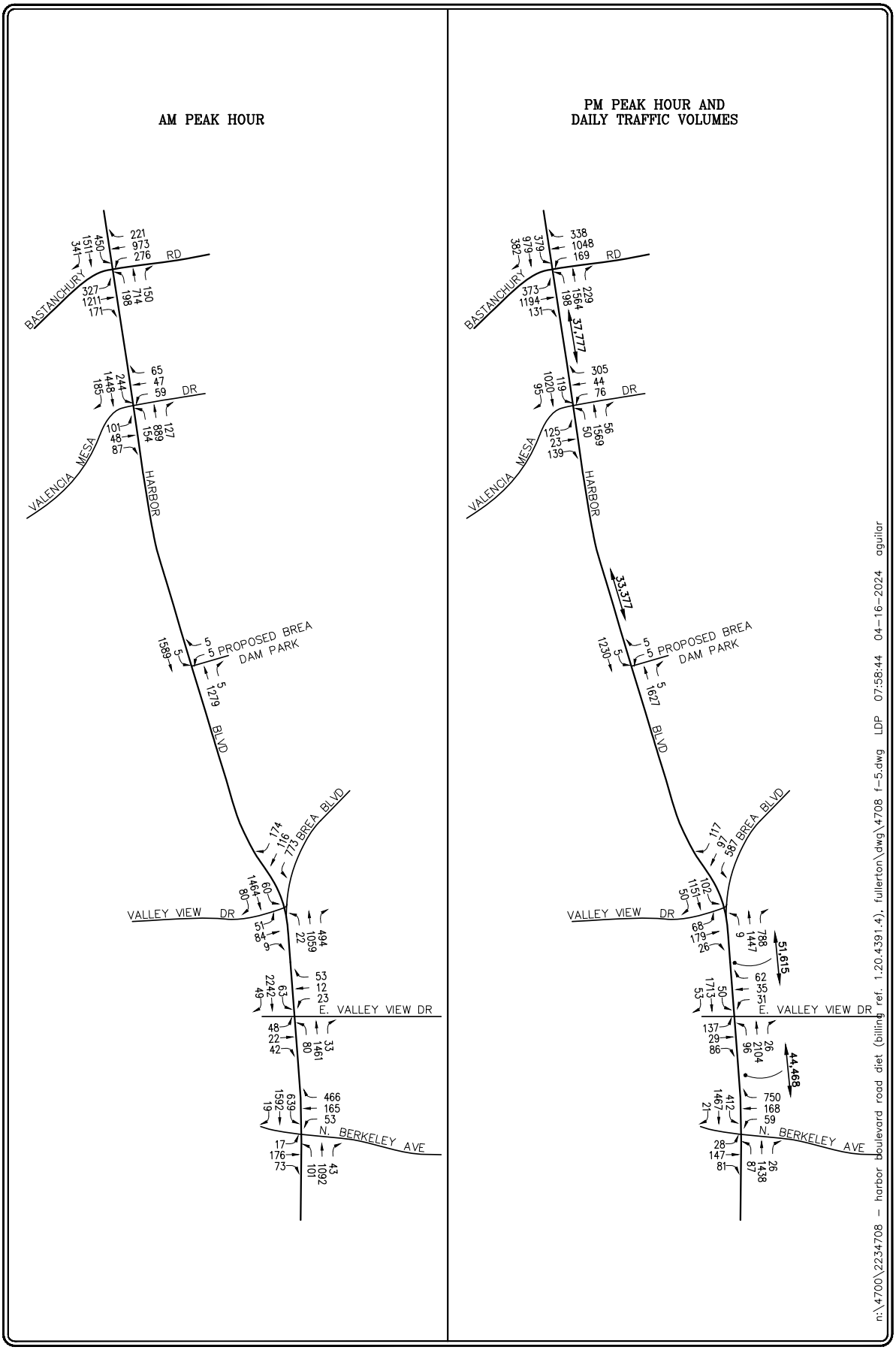


FIGURE 5

NO SCALE

YEAR 2045 BUILDOUT TRAFFIC VOLUMES

HARBOR BOULEVARD COMPLETE STREETS IMPROVEMENT PROJECT, FULLERTON

TABLE 1
LEVEL OF SERVICE CRITERIA FOR SIGNALIZED INTERSECTIONS (HCM)[1]
HARBOR BOULEVARD COMPLETE STREETS IMPROVEMENT PROJECT, FULLERTON

Level of Service (LOS)	Control Delay Per Vehicle (seconds/vehicle)	Level of Service Description
A	≤ 10.0	This level of service occurs when progression is extremely favorable and most vehicles arrive during the green phase. Most vehicles do not stop at all. Short cycle lengths may also contribute to low delay.
B	> 10.0 and ≤ 20.0	This level generally occurs with good progression, short cycle lengths, or both. More vehicles stop than with LOS A, causing higher levels of average delay.
C	> 20.0 and ≤ 35.0	Average traffic delays. These higher delays may result from fair progression, longer cycle lengths, or both. Individual cycle failures may begin to appear at this level. The number of vehicles stopping is significant at this level, though many still pass through the intersection without stopping.
D	> 35.0 and ≤ 55.0	Long traffic delays. At level D, the influence of congestion becomes more noticeable. Longer delays may result from some combination of unfavorable progression, long cycle lengths, or high *v/c* ratios. Many vehicles stop, and the proportion of vehicles not stopping declines. Individual cycle failures are noticeable.
E	> 55.0 and ≤ 80.0	Very long traffic delays. This level is considered by many agencies (i.e. SANBAG) to be the limit of acceptable delay. These high delay values generally indicate poor progression, long cycle lengths, and high *v/c* ratios. Individual cycle failures are frequent occurrences.
F	≥ 80.0	Severe congestion. This level, considered to be unacceptable to most drivers, often occurs with over saturation, that is, when arrival flow rates exceed the capacity of the intersection. It may also occur at high *v/c* ratios below 1.0 with many individual cycle failures. Poor progression and long cycle lengths may also be major contributing factors to such delay levels.

[1] Source: *Highway Capacity Manual* (Signalized Intersections).

TABLE 2
LEVEL OF SERVICE CRITERIA FOR ROADWAY SEGMENTS (V/C METHODOLOGY)[2]
HARBOR BOULEVARD COMPLETE STREETS IMPROVEMENT PROJECT, FULLERTON

Level of Service (LOS)	Volume to Capacity Ratio (V/C)	Level of Service Description
A	≤ 0.600	**EXCELLENT**. Describes primarily free flow operations at average travel speeds, usually about 90% of the free flow speed for the arterial class. Vehicles are completely unimpeded in their ability to maneuver within the traffic stream. Stopped delay at signalized intersections is minimal.
B	0.601 – 0.700	**VERY GOOD**. Represents reasonably unimpeded operations at average travel speeds, usually about 70% of the free flow speed for the arterial class. The ability to maneuver within the traffic stream is only slightly restricted and stopped delays are not bothersome. Drivers are not generally subjected to appreciable tension.
C	0.701 – 0.800	**GOOD**. Represents stable conditions; however, ability to maneuver and change lanes in mid-block location may be more restricted than in LOS B, and longer queues and/or adverse signal coordination may contribute to lower average travel speeds of about 50% of the average free flow speed for the arterial class. Motorists will experience appreciable tension while driving.
D	0.801 – 0.900	**FAIR**. Borders on a range in which small increases in flow may cause substantial increases in approach delay and, hence, decreases in arterial speed. This may be due to adverse signal progression, inappropriate signal timing, high volumes, or some combination of these. Average travel speeds are about 40% of free flow speed.
E	0.901 – 1.000	**POOR**. Characterized by significant approach delays and average travel speeds of one-third the free flow speed or lower. Such operations are caused by some combination of adverse progression, high signal density, extensive queuing at critical intersections, and inappropriate signal timing.
F	> 1.000	**FAILURE**. Characterizes arterial flow at extremely low speeds below one-third to one-quarter of the free flow speed. Intersection congestion is likely at critical signalized locations, with resultant high approach delays. Adverse progression is frequently a contributor to this condition.

[2] Source: *Transportation Research Board 2000.*

TABLE 3
LEVEL OF SERVICE CRITERIA FOR ROADWAY SEGMENTS (V/C METHODOLOGY)[3]
HARBOR BOULEVARD COMPLETE STREETS IMPROVEMENT PROJECT, FULLERTON

Facility Type	Number of Lanes	Level of Service Criteria With Associated Roadway Capacity					
		Daily Values (VPD)					
		Level of Service (LOS)					
		A	B	C	D	E	F
Principal	8-lanes divided	45,000	52,500	60,000	67,500	75,000	--
Smart Street[4]	6-lanes divided	36,300	42,200	48,200	54,200	60,200	--
Major	6-lanes divided	33,900	39,400	45,000	50,600	56,300	--
Primary	4-lanes divided	22,500	26,300	30,000	33,800	37,500	--
Secondary	4-lanes undivided	15,000	17,500	20,000	22,500	25,000	--
Commuter	2-lanes undivided	7,500	8,800	10,000	11,300	12,500	--
V/C Ratio		≤ 0.600	0.601-0.700	0.701-0.800	0.801-0.900	0.901-1.000	≥ 1.01

Notes:

- VPD = vehicles per day

[3] Source: *Transportation Research Board 2000.*

[4] The capacity of a Smart Street is seven percent higher than the capacity of a major arterial, due to traffic flow operation enhancements that are associated with the Smart Street facility designation.

TABLE 4

EXISTING PEAK HOUR INTERSECTION CAPACITY ANALYSIS SUMMARY
HARBOR BOULEVARD COMPLETE STREETS IMPROVEMENT PROJECT, FULLERTON

Study Intersection		Time Period	(1) Existing Traffic Conditions		(2) Existing Plus Project Traffic Conditions		(3) Exceeds LOS Thresholds (2) – (1)	
			Delay (s/v)	LOS	Delay (s/v)	LOS	Increase	Yes/No
1.	Harbor Boulevard at	AM	50.9	D	51.8	D	0.9	No
	Bastanchury Road	PM	44.1	D	48.9	D	4.8	No
2.	Harbor Boulevard at	AM	20.9	C	27.6	C	6.7	No
	Valencia Mesa Drive	PM	23.3	C	29.6	C	6.3	No
3.	Harbor Boulevard at	AM	35.3	D	35.7	D	0.4	No
	Brea Boulevard/Valley View Drive	PM	38.1	D	40.1	D	2.0	No
4.	Harbor Boulevard at	AM	--	--	1.5	A	--	No
	Proposed Brea Dam Park	PM	--	--	1.5	A	--	No
5.	Harbor Boulevard at	AM	5.1	A	5.1	A	0.0	No
	E. Valley View Drive	PM	7.5	A	7.5	A	0.0	No
6.	Harbor Boulevard at	AM	28.1	C	28.2	C	0.1	No
	Berkeley Avenue	PM	31.5	C	31.5	C	0.0	No

Notes:

- s/v = seconds per vehicle (delay)
- **Bold Delay/LOS values** indicate adverse service levels based on the LOS standards mentioned in this report

TABLE 5

EXISTING DAILY ROADWAY SEGMENT ANALYSIS SUMMARY
HARBOR BOULEVARD COMPLETE STREETS IMPROVEMENT PROJECT, FULLERTON

Roadway Segment	(1) Existing Traffic Conditions					(2) Existing Plus Project Traffic Conditions					
	No. of Lanes	LOS "E" Capacity (VPD)[5]	Daily Volume	V/C Ratio	LOS	No. of Lanes	LOS "E" Capacity (VPD)	Daily Volume	V/C Ratio	LOS	Adverse (Yes/No)
A. Harbor Boulevard, between Bastanchury Road and Valencia Mesa Drive	6D	56,300	31,176	0.554	A	4D	37,500	31,176	0.831	D	No
B. Harbor Boulevard, between Valencia Mesa Drive and Brea Boulevard/Valley View Drive	6D	56,300	28,271	0.502	A	4D	37,500	28,271	0.754	C	No
C. Harbor Boulevard, between Brea Boulevard/Valley View Drive and E. Valley View Drive	5D	46,900	44,041	**0.939**	**E**	5D	46,900	44,041	**0.939**	**E**	No[6]
D. Harbor Boulevard, between E. Valley View Drive and Berkeley Avenue	4D	37,500	40,590	**1.082**	**F**	4D	37,500	40,590	**1.082**	**F**	No[6]

Notes:

- D = Divided; U = Undivided; VPD = Vehicles per day; V/C = Volume over capacity
- **Bold LOS values** indicate adverse service levels

[5] Source: *Guidance for Administration of the Orange County Master Plan of Arterial Highways, dated November 1995 and Amended April 1998.*

[6] The roadway segment is forecast to operate at deficient levels of service both with and without the proposed Project. The proposed Project will maintain the existing lane geometrics along the roadway segment and thus the adverse service levels are not caused by the Project.

TABLE 6

YEAR 2045 BUILDOUT PEAK HOUR INTERSECTION CAPACITY ANALYSIS SUMMARY
HARBOR BOULEVARD COMPLETE STREETS IMPROVEMENT PROJECT, FULLERTON

Study Intersection		Time Period	(1) Year 2045 Buildout Traffic Conditions		(2) Year 2045 Buildout Plus Project Traffic Conditions		(3) Exceeds LOS Thresholds (2) – (1)	
			Delay (s/v)	LOS	Delay (s/v)	LOS	Increase	Yes/No
1.	Harbor Boulevard at	AM	**67.4**	**E**	**69.8**	**E**	2.4	No
	Bastanchury Road	PM	**56.9**	**E**	**60.8**	**E**	3.9	No
2.	Harbor Boulevard at	AM	21.2	C	34.1	C	12.9	No
	Valencia Mesa Drive	PM	26.1	C	37.1	D	11.0	No
3.	Harbor Boulevard at	AM	37.0	D	42.7	D	5.7	No
	Brea Boulevard/Valley View Drive	PM	38.3	D	43.7	D	5.4	No
4.	Harbor Boulevard at	AM	--	--	1.7	A	--	No
	Proposed Brea Dam Park	PM	--	--	1.7	A	--	No
5.	Harbor Boulevard at	AM	9.4	A	9.4	A	0.0	No
	E. Valley View Drive	PM	14.2	B	14.2	B	0.0	No
6.	Harbor Boulevard at	AM	31.8	C	31.8	C	0.0	No
	Berkeley Avenue	PM	47.0	D	47.0	D	0.0	No

Notes:

- s/v = seconds per vehicle (delay)
- **Bold Delay/LOS values** indicate adverse service levels based on the LOS standards mentioned in this report

TABLE 7

YEAR 2045 BUILDOUT DAILY ROADWAY SEGMENT ANALYSIS SUMMARY
HARBOR BOULEVARD COMPLETE STREETS IMPROVEMENT PROJECT, FULLERTON

Roadway Segment	(1) Year 2045 Buildout Traffic Conditions					(2) Year 2045 Buildout Plus Project Traffic Conditions					
	No. of Lanes	LOS "E" Capacity (VPD)[7]	Daily Volume	V/C Ratio	LOS	No. of Lanes	LOS "E" Capacity (VPD)	Daily Volume	V/C Ratio	LOS	Adverse (Yes/No)
A. Harbor Boulevard, between Bastanchury Road and Valencia Mesa Drive	6D	56,300	37,777	0.671	B	4D	37,500	37,777	**1.007**	**F**	**Yes**
B. Harbor Boulevard, between Valencia Mesa Drive and Brea Boulevard/Valley View Drive	6D	56,300	33,377	0.593	A	4D	37,500	33,377	0.890	D	No
C. Harbor Boulevard, between Brea Boulevard/Valley View Drive and E. Valley View Drive	5D	46,900	51,615	**1.101**	**F**	5D	46,900	51,615	**1.101**	**F**	No[8]
D. Harbor Boulevard, between and E. Valley View Drive and Berkeley Avenue	4D	37,500	55,468	**1.186**	**F**	4D	37,500	55,468	**1.186**	**F**	No[8]

Notes:

- D = Divided; U = Undivided; VPD = Vehicles per day; V/C = Volume over capacity
- **Bold LOS values** indicate adverse service levels

[7] Source: *Guidance for Administration of the Orange County Master Plan of Arterial Highways, dated November 1995 and Amended April 1998.*

[8] The roadway segment is forecast to operate at deficient levels of service both with and without the proposed Project. The proposed Project will maintain the existing lane geometrics along the roadway segment and thus the adverse service levels are not caused by the Project.

TABLE 8

YEAR 2045 BUILDOUT PLUS PROJECT PEAK HOUR ROADWAY SEGMENTS ANALYSIS SUMMARY
HARBOR BOULEVARD COMPLETE STREETS IMPROVEMENT PROJECT, FULLERTON

Roadway Segment	Approach	Time Period	Link Capacity (VPHPL)	(1) Year 2045 Buildout Plus Project Traffic Conditions					
				Lanes	Total Link Capacity (VPH)	Peak Hour Volume	V/C Ratio	LOS	Adverse (Yes/No)
A. Harbor Boulevard, between Bastanchury Road and Valencia Mesa Drive	Northbound	AM	1,600	2	3,200	1,284	0.401	A	No
		PM	1,600	2	3,200	1,604	0.501	A	No
	Southbound	AM	1,600	2	3,200	1,632	0.510	A	No
		PM	1,600	2	3,200	1,303	0.407	A	No

Notes:

- VPHPL = Vehicles Per Hour Per Lane
- VPH = Vehicles Per Hour
- V/C = Volume to Capacity Ratio

TABLE 9
EXISTING PEAK HOUR INTERSECTION QUEUING ANALYSIS[9]
HARBOR BOULEVARD COMPLETE STREETS IMPROVEMENT PROJECT, FULLERTON

Key Intersection	Storage Provided (feet)	(1) Existing Traffic Conditions				(2) Existing Plus Project Traffic Conditions			
		AM Peak Hour		PM Peak Hour		AM Peak Hour		PM Peak Hour	
		Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)	Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)	Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)	Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)
1. Harbor Boulevard at Bastanchury Road									
Northbound Left-Turn	460[10]	244	Yes	267	Yes	277	Yes	330	Yes
Northbound Right-Turn	150	68	Yes	**213**	**No**	80	Yes	**264**	**No**
Southbound Left-Turn	550[11]	**617**	**No**	**702**	**No**	**690**	**No**	**633**	**No**
2. Harbor Boulevard at Valencia Mesa Drive									
Northbound Left-Turn	175 / 185[12]	**192**	**No**	84	Yes	**198**	**No**	139	Yes
Southbound Left-Turn	230	212	Yes	125	Yes	214	Yes	143	Yes
Southbound Right-Turn[13]	705	--	--	--	--	55	Yes	40	Yes

9 Queues are based on Synchro 11.0 SimTraffic 95th Percentile methodology.

10 Movement consists of dual left-turn lanes. Both lanes provide approximately 230' of storage. The storage reported is the total of both lanes.

11 Movement consists of dual left-turn lanes. The first lane provides approximately 140' of storage and the second lane provides approximately 410' of storage. The storage reported is the total of both lanes.

12 Restriping the northbound left-turn from 175-feet to 185-feet is proposed as part of the Project.

13 Restriping the southbound through/right-turn lane into a southbound right-turn pocket is proposed as part of the Project.

TABLE 9 (CONTINUED)
EXISTING PEAK HOUR INTERSECTION QUEUING ANALYSIS[14]
HARBOR BOULEVARD COMPLETE STREETS IMPROVEMENT PROJECT, FULLERTON

Key Intersection	Storage Provided (feet)	(1) Existing Traffic Conditions				(2) Existing Plus Project Traffic Conditions			
		AM Peak Hour		PM Peak Hour		AM Peak Hour		PM Peak Hour	
		Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)	Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)	Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)	Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)
3. Harbor Boulevard at Brea Boulevard/Valley View Drive									
Northbound Left-Turn	95	35	Yes	32	Yes	34	Yes	33	Yes
Southbound Left-Turn	150 / 140[15]	108	Yes	**156**	**No**	119	Yes	**196**	**No**
Southbound Right-Turn[16]	140	--	--	--	--	**164**	**No**	**190**	**No**
4. Harbor Boulevard at Proposed Brea Dam Park									
Northbound Right-Turn	200	--	--	--	--	25	Yes	25	Yes
Southbound Left-Turn	250	--	--	--	--	25	Yes	25	Yes

14 Queues are based on Synchro 11.0 SimTraffic 95th Percentile methodology.

15 Restriping the southbound left-turn from 150-feet to 140-feet is proposed as part of the Project.

16 Restriping the southbound through/right-turn lane into a southbound right-turn pocket is proposed as part of the Project.

TABLE 9 (CONTINUED)
EXISTING PEAK HOUR INTERSECTION QUEUING ANALYSIS[17]
HARBOR BOULEVARD COMPLETE STREETS IMPROVEMENT PROJECT, FULLERTON

Key Intersection	Storage Provided (feet)	(1) Existing Traffic Conditions				(2) Existing Plus Project Traffic Conditions			
		AM Peak Hour		PM Peak Hour		AM Peak Hour		PM Peak Hour	
		Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)	Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)	Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)	Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)
5. Harbor Boulevard at E. Valley View Drive									
Northbound Left-Turn	95	59	Yes	70	Yes	52	Yes	72	Yes
Southbound Left-Turn	100	76	Yes	75	Yes	69	Yes	62	Yes
6. Harbor Boulevard at Berkeley Avenue									
Northbound Left-Turn	205	173	Yes	204	Yes	172	Yes	**219**	**No**
Southbound Left-Turn	600[18]	279	Yes	315	Yes	290	Yes	297	Yes
Southbound Right-Turn	165	54	Yes	25	Yes	69	Yes	25	Yes

[17] Queues are based on Synchro 11.0 SimTraffic 95th Percentile methodology.

[18] Movement consists of dual left-turn lanes. Both lanes provide approximately 300' of storage. The storage reported is the total of both lanes.

TABLE 10

YEAR 2045 BUILDOUT PEAK HOUR INTERSECTION QUEUING ANALYSIS[19]

HARBOR BOULEVARD COMPLETE STREETS IMPROVEMENT PROJECT, FULLERTON

Key Intersection	Storage Provided (feet)	(1) Year 2045 Buildout Traffic Conditions				(2) Year 2045 Buildout Plus Project Traffic Conditions			
		AM Peak Hour		PM Peak Hour		AM Peak Hour		PM Peak Hour	
		Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)	Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)	Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)	Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)
1. Harbor Boulevard at Bastanchury Road									
Northbound Left-Turn	460[20]	302	Yes	**476**	**No**	424	Yes	**498**	**No**
Northbound Right-Turn	150	109	Yes	**278**	**No**	130	Yes	**274**	**No**
Southbound Left-Turn	550[21]	**729**	**No**	**730**	**No**	**703**	**No**	**723**	**No**
2. Harbor Boulevard at Valencia Mesa Drive									
Northbound Left-Turn	175 / 185[22]	**177**	**No**	**191**	**No**	**278**	**No**	**278**	**No**
Southbound Left-Turn	230	**260**	**No**	146	Yes	**247**	**No**	155	Yes
Southbound Right-Turn[23]	705	--	--	--	--	55	Yes	28	Yes

19 Queues are based on Synchro 11.0 SimTraffic 95th Percentile methodology.

20 Movement consists of dual left-turn lanes. Both lanes provide approximately 230' of storage. The storage reported is the total of both lanes.

21 Movement consists of dual left-turn lanes. The first lane provides approximately 140' of storage and the second lane provides approximately 410' of storage. The storage reported is the total of both lanes.

22 Restriping the northbound left-turn from 175-feet to 185-feet is proposed as part of the Project.

23 Restriping the southbound through/right-turn lane into a southbound right-turn pocket is proposed as part of the Project.

TABLE 10 (CONTINUED)
YEAR 2045 BUILDOUT PEAK HOUR INTERSECTION QUEUING ANALYSIS[24]
HARBOR BOULEVARD COMPLETE STREETS IMPROVEMENT PROJECT, FULLERTON

Key Intersection	Storage Provided (feet)	(1) Year 2045 Buildout Traffic Conditions				(2) Year 2045 Buildout Plus Project Traffic Conditions			
		AM Peak Hour		PM Peak Hour		AM Peak Hour		PM Peak Hour	
		Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)	Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)	Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)	Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)
3. Harbor Boulevard at Brea Boulevard/Valley View Drive									
Northbound Left-Turn	95	25	Yes	50	Yes	48	Yes	57	Yes
Southbound Left-Turn	150 / 140[25]	**157**	**No**	**163**	**No**	**165**	**No**	**196**	**No**
Southbound Right-Turn[26]	140	--	--	--	--	**180**	**No**	128	Yes
4. Harbor Boulevard at Proposed Brea Dam Park									
Northbound Right-Turn	200	--	--	--	--	25	Yes	25	Yes
Southbound Left-Turn	250	--	--	--	--	33	Yes	25	Yes

24 Queues are based on Synchro 11.0 SimTraffic 95th Percentile methodology.

25 Restriping the southbound left-turn from 150-feet to 140-feet is proposed as part of the Project.

26 Restriping the southbound through/right-turn lane into a southbound right-turn pocket is proposed as part of the Project.

TABLE 10 (CONTINUED)
YEAR 2045 BUILDOUT PEAK HOUR INTERSECTION QUEUING ANALYSIS[27]
HARBOR BOULEVARD COMPLETE STREETS IMPROVEMENT PROJECT, FULLERTON

Key Intersection	Storage Provided (feet)	(1) Year 2045 Buildout Traffic Conditions				(2) Year 2045 Buildout Plus Project Traffic Conditions			
		AM Peak Hour		PM Peak Hour		AM Peak Hour		PM Peak Hour	
		Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)	Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)	Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)	Max. Queue/ Min. Storage Required (feet)	Adequate Storage (Yes/No)
5. Harbor Boulevard at E. Valley View Drive									
Northbound Left-Turn	95	**144**	**No**	**130**	**No**	**131**	**No**	**125**	**No**
Southbound Left-Turn	100	75	Yes	96	Yes	65	Yes	84	Yes
6. Harbor Boulevard at Berkeley Avenue									
Northbound Left-Turn	205	**242**	**No**	**286**	**No**	**246**	**No**	**300**	**No**
Southbound Left-Turn	600[28]	449	Yes	336	Yes	472	Yes	357	Yes
Southbound Right-Turn	165	76	Yes	37	Yes	63	Yes	49	Yes

[27] Queues are based on Synchro 11.0 SimTraffic 95th Percentile methodology.

[28] Movement consists of dual left-turn lanes. Both lanes provide approximately 300' of storage. The storage reported is the total of both lanes.

APPENDIX A

EXISTING TRAFFIC COUNT DATA

Transportation Studies, Inc
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: FULLERTON
N-S Direction: HARBOR BOULEVAR
E-W Direction: BASTANCHURY ROAD

File Name : H2305016
Site Code : 00000000
Start Date : 5/24/2023
Page No : 1

Groups Printed- Turning Movement Count

	HARBOR BOULEVARD Southbound				BASTANCHURY ROAD Westbound				HARBOR BOULEVARD Northbound				BASTANCHURY ROAD Eastbound				
Start Time	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Int. Total
00:00	15	15	2	1	6	13	1	1	4	28	2	1	0	15	11	0	115
00:15	2	16	4	1	5	16	2	0	1	30	3	0	1	13	3	0	97
00:30	4	13	0	1	9	15	3	1	3	36	4	0	1	4	2	0	96
00:45	7	11	5	1	5	6	2	0	0	22	2	0	3	15	1	0	80
Total	28	55	11	4	25	50	8	2	8	116	11	1	5	47	17	0	388
01:00	10	15	1	1	2	2	2	2	3	21	3	0	0	8	3	0	73
01:15	1	15	1	1	2	7	2	1	0	14	0	0	2	4	3	0	53
01:30	6	6	1	1	2	2	3	0	2	12	0	0	1	2	4	0	42
01:45	0	12	1	0	3	1	1	1	1	14	0	0	1	2	1	0	38
Total	17	48	4	3	9	12	8	4	6	61	3	0	4	16	11	0	206
02:00	0	11	3	0	2	4	1	0	1	9	2	0	0	2	0	0	35
02:15	2	11	0	0	1	4	3	2	1	7	1	0	0	3	1	0	36
02:30	4	6	4	1	2	4	1	0	1	15	0	0	1	8	2	0	49
02:45	1	10	1	1	1	3	2	0	0	14	0	0	0	7	4	0	44
Total	7	38	8	2	6	15	7	2	3	45	3	0	1	20	7	0	164
03:00	1	12	0	0	3	3	1	0	1	14	0	0	1	2	2	0	40
03:15	2	11	2	0	0	6	1	0	0	19	0	0	1	4	9	0	55
03:30	1	21	4	0	4	3	1	0	2	29	0	0	0	6	5	0	76
03:45	4	23	6	0	2	7	1	1	1	23	0	0	1	11	7	0	87
Total	8	67	12	0	9	19	4	1	4	85	0	0	3	23	23	0	258
04:00	3	14	4	1	3	9	2	0	2	11	0	1	0	9	5	0	64
04:15	8	23	3	0	6	7	1	0	4	31	0	1	0	8	5	0	97
04:30	6	41	12	0	4	30	3	0	0	30	0	0	0	8	12	0	146
04:45	8	40	15	0	7	28	0	2	2	52	1	0	1	15	14	0	185
Total	25	118	34	1	20	74	6	2	8	124	1	2	1	40	36	0	492
05:00	12	35	11	1	6	20	5	0	8	31	4	0	0	8	16	0	157
05:15	19	92	18	1	11	22	6	3	7	53	2	0	3	25	15	0	277
05:30	26	88	15	4	12	51	10	4	5	74	4	1	2	34	24	0	354
05:45	35	87	34	1	23	54	12	3	29	82	6	0	1	45	31	0	443
Total	92	302	78	7	52	147	33	10	49	240	16	1	6	112	86	0	1231
06:00	26	83	39	4	25	57	13	4	14	57	6	0	6	44	25	0	403
06:15	36	122	49	3	16	70	14	2	21	66	4	0	6	61	26	0	496
06:30	39	167	66	3	31	79	31	2	19	77	10	1	8	93	33	0	659
06:45	43	222	74	7	43	148	35	2	36	96	12	1	20	119	63	0	921
Total	144	594	228	17	115	354	93	10	90	296	32	2	40	317	147	0	2479
07:00	40	255	94	4	36	155	33	7	19	104	15	1	31	171	41	0	1006
07:15	53	281	100	3	47	192	36	6	21	124	23	1	26	188	45	0	1146
07:30	74	165	87	6	57	243	41	4	25	164	31	1	36	219	42	0	1195
07:45	75	271	99	5	68	244	69	3	22	187	38	4	39	185	76	0	1385
Total	242	972	380	18	208	834	179	20	87	579	107	7	132	763	204	0	4732
08:00	92	259	71	6	47	240	73	8	33	133	53	1	41	195	72	0	1324
08:15	68	207	70	3	41	228	55	4	26	136	52	3	44	221	84	0	1242
08:30	62	204	89	6	60	215	47	4	29	165	29	2	39	247	79	0	1277
08:45	60	218	77	5	49	176	52	4	34	143	18	2	43	162	52	0	1095
Total	282	888	307	20	197	859	227	20	122	577	152	8	167	825	287	0	4938

Transportation Studies, Inc
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City: FULLERTON
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File Name : H2305016
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Groups Printed- Turning Movement Count

	HARBOR BOULEVARD Southbound				BASTANCHURY ROAD Westbound				HARBOR BOULEVARD Northbound				BASTANCHURY ROAD Eastbound				
Start Time	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Int. Total
09:00	49	178	60	6	67	208	47	3	35	112	22	2	32	148	45	0	1014
09:15	39	145	65	5	52	156	64	4	33	116	26	0	38	145	52	0	940
09:30	49	132	64	5	56	161	44	9	44	122	37	1	37	130	60	0	951
09:45	44	158	50	5	48	160	46	5	27	128	30	0	46	150	54	0	951
Total	181	613	239	21	223	685	201	21	139	478	115	3	153	573	211	0	3856
10:00	58	136	56	4	62	155	54	4	40	110	30	1	46	134	63	0	953
10:15	53	152	61	4	75	134	42	5	32	125	31	3	31	135	54	0	937
10:30	53	157	62	3	48	164	49	4	45	135	31	1	35	133	65	0	985
10:45	38	151	44	3	74	148	69	5	39	128	27	0	33	166	64	0	989
Total	202	596	223	14	259	601	214	18	156	498	119	5	145	568	246	0	3864
11:00	57	157	49	4	70	152	44	8	47	152	28	2	20	141	57	0	988
11:15	58	152	69	8	50	153	31	6	36	135	27	1	48	160	80	0	1014
11:30	54	137	62	8	62	171	33	3	48	165	33	2	27	158	56	0	1019
11:45	47	132	55	5	39	138	25	5	58	166	35	1	34	145	56	0	941
Total	216	578	235	25	221	614	133	22	189	618	123	6	129	604	249	0	3962
12:00	54	138	56	6	58	143	41	7	48	165	47	0	48	168	59	0	1038
12:15	51	144	65	6	50	148	53	7	43	164	41	1	31	177	60	0	1041
12:30	41	135	63	7	74	135	45	3	45	165	44	0	30	164	59	0	1010
12:45	45	152	50	6	73	229	55	6	37	161	51	2	35	160	56	0	1118
Total	191	569	234	25	255	655	194	23	173	655	183	3	144	669	234	0	4207
13:00	53	140	72	8	64	163	40	12	28	149	39	0	52	200	71	0	1091
13:15	40	149	55	8	63	146	53	7	39	169	36	1	34	185	54	0	1039
13:30	63	168	54	5	65	190	40	8	42	168	45	5	26	193	53	0	1125
13:45	60	177	55	9	59	188	56	8	52	146	27	0	43	158	64	0	1102
Total	216	634	236	30	251	687	189	35	161	632	147	6	155	736	242	0	4357
14:00	81	183	67	9	71	183	48	8	38	155	41	0	41	192	60	0	1177
14:15	62	156	70	4	65	161	34	6	45	168	35	2	38	216	66	0	1128
14:30	75	210	68	7	72	179	43	1	50	175	38	2	31	212	64	0	1227
14:45	69	176	78	7	71	178	39	11	49	219	47	4	40	230	67	0	1285
Total	287	725	283	27	279	701	164	26	182	717	161	8	150	850	257	0	4817
15:00	62	191	97	9	66	201	43	10	46	188	40	1	39	196	69	0	1258
15:15	78	203	102	8	63	189	41	13	60	232	35	0	45	230	68	0	1367
15:30	75	205	83	7	86	183	45	13	56	242	50	3	45	252	63	0	1408
15:45	83	197	86	9	74	204	47	4	56	259	47	2	38	311	60	0	1477
Total	298	796	368	33	289	777	176	40	218	921	172	6	167	989	260	0	5510
16:00	84	214	82	3	79	222	25	11	53	240	47	2	36	288	61	0	1447
16:15	76	190	107	4	79	183	35	6	33	264	47	0	39	255	63	0	1381
16:30	82	220	75	12	81	181	24	3	63	269	38	4	33	264	68	0	1417
16:45	81	173	69	8	69	223	30	6	42	286	36	1	39	248	72	0	1383
Total	323	797	333	27	308	809	114	26	191	1059	168	7	147	1055	264	0	5628
17:00	94	194	75	3	71	189	27	8	61	267	60	1	24	292	75	0	1441
17:15	81	209	74	5	79	240	32	3	37	257	48	1	29	310	78	0	1483
17:30	92	200	63	3	68	201	12	7	51	220	48	0	25	231	58	0	1279
17:45	66	183	73	3	55	186	27	6	28	185	28	1	27	226	68	0	1162
Total	333	786	285	14	273	816	98	24	177	929	184	3	105	1059	279	0	5365
18:00	89	153	51	5	57	193	21	7	33	218	25	1	30	190	77	0	1150
18:15	74	141	49	3	59	161	32	8	30	196	30	0	20	166	56	0	1025

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Groups Printed- Turning Movement Count

	HARBOR BOULEVARD Southbound				BASTANCHURY ROAD Westbound				HARBOR BOULEVARD Northbound				BASTANCHURY ROAD Eastbound				
Start Time	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Int. Total
18:30	41	145	61	8	66	163	28	15	33	224	37	2	14	170	49	0	1056
18:45	52	125	42	2	68	152	17	5	22	151	21	1	29	143	43	0	873
Total	256	564	203	18	250	669	98	35	118	789	113	4	93	669	225	0	4104
19:00	38	115	32	7	44	120	12	5	23	159	33	0	13	133	40	0	774
19:15	36	105	40	7	50	115	19	10	22	141	27	0	28	124	38	0	762
19:30	36	102	36	8	59	116	18	14	13	116	22	0	15	76	32	0	663
19:45	29	107	36	4	52	115	19	6	20	86	19	1	20	67	37	0	618
Total	139	429	144	26	205	466	68	35	78	502	101	1	76	400	147	0	2817
20:00	41	84	35	8	55	138	20	7	18	137	30	0	19	98	29	0	719
20:15	36	73	35	3	41	92	21	7	28	113	18	0	20	72	30	0	589
20:30	20	84	30	10	40	104	25	4	16	95	23	0	25	83	26	0	585
20:45	23	70	32	5	31	91	11	3	16	107	18	1	10	83	28	0	529
Total	120	311	132	26	167	425	77	21	78	452	89	1	74	336	113	0	2422
21:00	21	75	30	3	30	74	9	4	22	95	11	2	5	68	29	0	478
21:15	21	65	22	0	25	69	17	4	10	90	16	0	10	66	28	0	443
21:30	18	74	14	4	25	57	13	3	8	79	14	0	7	63	21	0	400
21:45	19	44	14	0	20	46	10	2	10	85	21	0	10	52	19	0	352
Total	79	258	80	7	100	246	49	13	50	349	62	2	32	249	97	0	1673
22:00	18	51	17	4	16	44	6	7	10	68	8	0	1	53	17	0	320
22:15	21	37	13	6	17	33	4	2	4	75	6	1	2	32	23	0	276
22:30	13	48	9	2	10	35	5	0	7	66	10	0	3	31	21	0	260
22:45	12	28	7	2	14	32	7	2	4	43	10	0	2	31	15	0	209
Total	64	164	46	14	57	144	22	11	25	252	34	1	8	147	76	0	1065
23:00	19	36	7	2	13	32	5	3	9	53	6	0	3	33	18	0	239
23:15	15	23	5	2	14	27	6	2	3	45	2	0	1	32	12	0	189
23:30	7	29	7	2	9	13	5	2	2	52	5	0	4	11	4	0	152
23:45	9	12	7	2	11	12	3	0	6	38	4	0	0	21	18	0	143
Total	50	100	26	8	47	84	19	7	20	188	17	0	8	97	52	0	723
Grand Total	3800	11002	4129	387	3825	10743	2381	428	2332	11162	2113	77	1945	11164	3770	0	69258
Apprch %	19.7	57	21.4	2	22	61.8	13.7	2.5	14.9	71.2	13.5	0.5	11.5	66.1	22.3	0	
Total %	5.5	15.9	6	0.6	5.5	15.5	3.4	0.6	3.4	16.1	3.1	0.1	2.8	16.1	5.4	0	

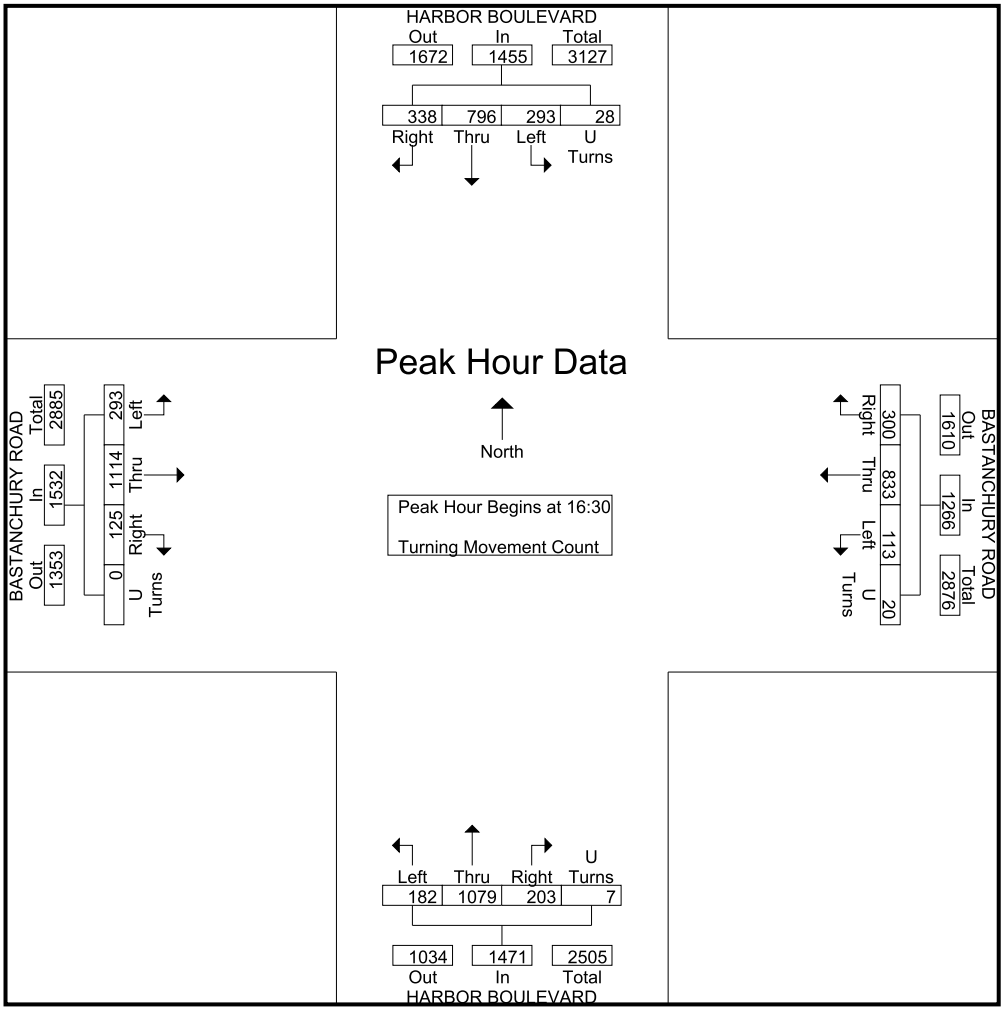
	HARBOR BOULEVARD Southbound				Westbound				HARBOR BOULEVARD Northbound				Eastbound			
	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns

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City: FULLERTON
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	HARBOR BOULEVARD Southbound					BASTANCHURY ROAD Westbound					HARBOR BOULEVARD Northbound					BASTANCHURY ROAD Eastbound					
Start Time	Right	Thru	Left	U Turns	App. Total	Right	Thru	Left	U Turns	App. Total	Right	Thru	Left	U Turns	App. Total	Right	Thru	Left	U Turns	App. Total	Int. Total
Peak Hour Analysis From 00:00 to 23:45 - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 16:30																					
16:30	82	220	75	12	389	81	181	24	3	289	63	269	38	4	374	33	264	68	0	365	1417
16:45	81	173	69	8	331	69	223	30	6	328	42	286	36	1	365	39	248	72	0	359	1383
17:00	94	194	75	3	366	71	189	27	8	295	61	267	60	1	389	24	292	75	0	391	1441
17:15	81	209	74	5	369	79	240	32	3	354	37	257	48	1	343	29	310	78	0	417	1483
Total Volume	338	796	293	28	1455	300	833	113	20	1266	203	1079	182	7	1471	125	1114	293	0	1532	5724
% App. Total	23.2	54.7	20.1	1.9		23.7	65.8	8.9	1.6		13.8	73.4	12.4	0.5		8.2	72.7	19.1	0		
PHF	.899	.905	.977	.583	.935	.926	.868	.883	.625	.894	.806	.943	.758	.438	.945	.801	.898	.939	.000	.918	.965



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City: FULLERTON
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File Name : H2305018
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Groups Printed- Turning Movement Count

	HARBOR BOULEVARD Southbound				BREA BOULEVARD Westbound				HARBOR BOULEVARD Northbound				VALLEY VIEW DRIVE Eastbound				
Start Time	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Int. Total
00:00	2	16	1	0	1	3	7	0	14	29	0	0	0	2	0	0	75
00:15	1	14	1	0	1	3	7	0	10	25	0	1	0	0	1	0	64
00:30	2	13	3	0	2	2	5	0	13	37	1	0	1	0	0	0	79
00:45	0	17	0	0	1	1	7	0	8	21	0	0	0	1	0	0	56
Total	5	60	5	0	5	9	26	0	45	112	1	1	1	3	1	0	274
01:00	0	11	2	0	1	1	7	0	6	23	0	0	1	0	0	0	52
01:15	0	19	1	0	0	0	1	0	5	10	0	0	2	1	0	0	39
01:30	0	10	1	0	0	0	4	0	3	14	2	0	0	0	0	0	34
01:45	1	14	0	0	0	0	1	0	8	15	0	0	0	1	1	0	41
Total	1	54	4	0	1	1	13	0	22	62	2	0	3	2	1	0	166
02:00	0	12	0	0	1	1	3	0	4	9	0	0	0	0	0	0	30
02:15	0	10	0	0	1	0	4	0	2	8	0	0	0	0	1	0	26
02:30	0	9	0	0	1	0	1	0	2	16	0	0	0	0	0	0	29
02:45	0	12	0	0	0	1	4	0	2	12	0	0	0	1	1	0	33
Total	0	43	0	0	3	2	12	0	10	45	0	0	0	1	2	0	118
03:00	0	14	0	0	0	0	2	0	4	14	0	0	0	0	0	0	34
03:15	0	13	0	0	0	0	2	0	5	20	0	0	0	0	1	0	41
03:30	0	22	0	0	0	0	4	0	16	30	2	0	0	1	0	0	75
03:45	0	24	0	0	0	0	6	0	16	22	0	0	0	0	0	0	68
Total	0	73	0	0	0	0	14	0	41	86	2	0	0	1	1	0	218
04:00	0	13	0	0	2	1	6	0	6	16	1	0	1	0	1	0	47
04:15	0	29	1	0	0	0	5	0	11	29	1	0	0	0	0	0	76
04:30	1	37	1	0	1	0	11	0	10	25	0	0	0	3	1	0	90
04:45	2	48	0	0	2	1	22	0	21	56	1	0	0	2	0	0	155
Total	3	127	2	0	5	2	44	0	48	126	3	0	1	5	2	0	368
05:00	0	34	0	0	0	0	24	0	11	46	1	0	0	3	1	0	120
05:15	0	59	16	0	3	3	25	0	20	58	0	0	2	4	2	0	192
05:30	1	75	7	0	3	0	38	1	20	102	1	0	0	5	2	0	255
05:45	0	98	1	0	9	2	42	0	41	107	1	0	0	1	3	0	305
Total	1	266	24	0	15	5	129	1	92	313	3	0	2	13	8	0	872
06:00	0	85	1	0	5	3	45	0	26	67	2	0	0	4	6	0	244
06:15	1	109	3	0	7	0	63	0	31	110	2	0	0	4	3	0	333
06:30	5	150	8	0	13	5	69	0	46	131	1	0	0	6	7	0	441
06:45	4	213	1	0	16	7	108	0	54	149	3	0	1	10	6	0	572
Total	10	557	13	0	41	15	285	0	157	457	8	0	1	24	22	0	1590
07:00	9	151	9	0	24	15	149	0	69	125	4	0	3	16	10	0	584
07:15	8	271	15	0	30	19	152	0	86	186	2	0	2	17	8	0	796
07:30	17	273	11	0	45	18	120	0	103	247	1	0	1	13	12	0	861
07:45	22	241	17	0	34	30	141	0	101	249	5	0	3	27	11	0	881
Total	56	936	52	0	133	82	562	0	359	807	12	0	9	73	41	0	3122
08:00	20	245	12	0	40	34	130	0	75	210	3	0	1	15	16	0	801
08:15	21	216	17	0	47	34	148	0	103	214	12	1	4	29	12	0	858
08:30	16	222	22	0	32	25	129	0	105	235	4	0	7	28	20	0	845
08:45	16	209	15	0	27	28	148	0	95	194	11	1	7	39	9	0	799
Total	73	892	66	0	146	121	555	0	378	853	30	2	19	111	57	0	3303

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Groups Printed- Turning Movement Count

	HARBOR BOULEVARD Southbound				BREA BOULEVARD Westbound				HARBOR BOULEVARD Northbound				VALLEY VIEW DRIVE Eastbound				
Start Time	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Int. Total
09:00	14	200	15	0	30	25	142	0	86	176	3	0	9	6	14	0	720
09:15	14	172	11	0	18	19	117	0	89	163	5	0	10	26	19	0	663
09:30	10	188	12	0	25	13	93	0	82	170	9	0	9	29	13	0	653
09:45	8	186	23	0	19	23	120	0	95	156	5	0	8	25	17	0	685
Total	46	746	61	0	92	80	472	0	352	665	22	0	36	86	63	0	2721
10:00	15	200	14	0	18	21	119	0	70	142	3	0	8	20	12	1	643
10:15	15	177	18	0	17	18	108	0	81	142	7	0	13	25	12	0	633
10:30	11	191	15	0	30	14	77	1	82	178	4	0	7	17	26	0	653
10:45	10	204	14	0	19	13	102	0	76	158	6	0	6	14	16	0	638
Total	51	772	61	0	84	66	406	1	309	620	20	0	34	76	66	1	2567
11:00	6	210	11	0	24	17	102	0	87	179	4	1	10	15	15	0	681
11:15	16	202	16	0	24	15	109	0	90	151	8	0	8	15	19	0	673
11:30	7	187	10	0	25	20	100	0	94	183	8	0	5	34	16	0	689
11:45	10	172	17	0	29	22	130	0	100	159	7	0	10	28	26	0	710
Total	39	771	54	0	102	74	441	0	371	672	27	1	33	92	76	0	2753
12:00	14	191	24	0	21	17	119	0	102	205	3	0	20	40	35	0	791
12:15	8	177	14	0	26	8	88	0	105	166	4	0	8	28	15	0	647
12:30	13	196	16	0	16	21	95	0	99	215	8	0	6	16	17	0	718
12:45	18	182	13	0	32	28	123	0	104	181	10	0	9	22	16	0	738
Total	53	746	67	0	95	74	425	0	410	767	25	0	43	106	83	0	2894
13:00	21	191	12	0	23	40	106	0	99	196	6	0	5	21	18	0	738
13:15	17	183	19	0	25	28	110	0	108	181	8	0	12	32	21	0	744
13:30	14	195	17	0	22	19	108	0	111	212	7	0	13	15	8	0	741
13:45	6	193	14	0	33	21	129	0	126	182	9	0	11	18	14	0	756
Total	58	762	62	0	103	108	453	0	444	771	30	0	41	86	61	0	2979
14:00	12	217	19	0	9	27	111	0	95	200	7	0	9	20	12	0	738
14:15	11	206	16	0	23	22	114	0	102	176	7	0	7	22	11	0	717
14:30	13	238	15	0	24	32	95	0	123	225	12	1	5	20	14	0	817
14:45	12	212	17	0	24	23	116	0	145	243	12	0	10	31	18	0	863
Total	48	873	67	0	80	104	436	0	465	844	38	1	31	93	55	0	3135
15:00	13	245	22	0	26	23	109	0	113	230	3	0	6	14	14	0	818
15:15	16	230	9	0	27	24	120	0	111	223	8	0	10	26	20	0	824
15:30	12	218	20	0	26	32	104	0	140	253	8	1	10	26	13	0	863
15:45	17	270	16	0	26	32	104	0	143	284	5	0	16	41	23	0	977
Total	58	963	67	0	105	111	437	0	507	990	24	1	42	107	70	0	3482
16:00	12	249	16	0	37	38	109	0	143	252	2	0	6	36	20	0	920
16:15	19	241	24	0	26	16	119	0	119	265	3	0	11	32	19	0	894
16:30	16	261	21	1	33	26	107	0	137	263	0	0	13	50	25	0	953
16:45	7	221	30	0	23	25	130	0	164	243	3	0	4	46	12	0	908
Total	54	972	91	1	119	105	465	0	563	1023	8	0	34	164	76	0	3675
17:00	12	274	24	0	26	21	115	0	156	268	3	0	4	46	18	0	967
17:15	15	244	21	0	29	25	117	0	148	250	3	0	5	37	13	0	907
17:30	14	237	32	0	21	22	120	0	152	250	6	0	4	24	12	0	894
17:45	11	172	25	0	19	21	114	0	124	200	3	0	5	31	10	0	735
Total	52	927	102	0	95	89	466	0	580	968	15	0	18	138	53	0	3503
18:00	9	184	18	0	24	19	124	0	137	203	1	0	3	23	12	0	757
18:15	9	153	14	0	21	14	101	0	126	215	2	0	5	20	11	0	691

Transportation Studies, Inc
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: FULLERTON
N-S Direction: HARBOR BOULEVARD
E-W Direction: BREA BLVD/VALLEY VIEW

File Name : H2305018
Site Code : 00000000
Start Date : 5/24/2023
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Groups Printed- Turning Movement Count

	HARBOR BOULEVARD Southbound				BREA BOULEVARD Westbound				HARBOR BOULEVARD Northbound				VALLEY VIEW DRIVE Eastbound				
Start Time	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Int. Total
18:30	6	159	22	0	18	7	98	0	127	214	2	0	2	14	11	0	680
18:45	10	146	25	0	18	16	87	0	128	180	3	0	2	12	13	0	640
Total	34	642	79	0	81	56	410	0	518	812	8	0	12	69	47	0	2768
19:00	3	153	15	0	24	33	97	0	107	157	4	0	8	9	9	0	619
19:15	5	125	20	0	28	13	88	0	105	158	5	0	3	18	9	0	577
19:30	3	129	10	0	14	15	88	0	78	125	2	1	2	14	8	0	489
19:45	5	128	10	0	7	8	76	0	83	98	5	0	3	20	4	0	447
Total	16	535	55	0	73	69	349	0	373	538	16	1	16	61	30	0	2132
20:00	6	101	17	0	14	11	73	0	82	142	3	0	6	14	5	0	474
20:15	5	112	9	0	13	16	78	0	92	131	3	0	2	8	10	0	479
20:30	4	130	11	0	12	26	53	0	76	127	5	0	3	12	4	0	463
20:45	3	93	9	0	7	12	40	0	84	115	2	0	6	5	6	0	382
Total	18	436	46	0	46	65	244	0	334	515	13	0	17	39	25	0	1798
21:00	3	98	11	0	21	20	93	0	70	116	1	0	0	7	2	0	442
21:15	2	96	9	0	5	3	54	0	36	87	2	0	5	7	3	0	309
21:30	1	93	4	0	5	10	30	0	45	91	2	0	1	5	3	0	290
21:45	1	63	3	0	7	5	39	0	40	67	0	0	3	6	5	0	239
Total	7	350	27	0	38	38	216	0	191	361	5	0	9	25	13	0	1280
22:00	0	63	7	0	4	1	16	0	50	75	5	0	4	1	4	0	230
22:15	0	45	2	0	2	2	27	0	42	70	1	0	2	0	5	0	198
22:30	0	63	5	0	4	2	21	0	33	77	1	0	3	1	1	0	211
22:45	2	37	4	0	0	3	22	0	29	46	2	0	1	1	2	0	149
Total	2	208	18	0	10	8	86	0	154	268	9	0	10	3	12	0	788
23:00	2	43	6	0	1	3	27	0	22	63	0	0	1	3	0	0	171
23:15	0	34	1	0	2	3	17	0	24	47	2	0	1	2	2	0	135
23:30	0	41	1	0	3	1	13	0	16	46	2	0	1	2	2	0	128
23:45	0	17	0	0	1	2	5	0	19	36	1	0	1	1	0	0	83
Total	2	135	8	0	7	9	62	0	81	192	5	0	4	8	4	0	517
Grand Total	687	12846	1031	1	1479	1293	7008	2	6804	12867	326	7	416	1386	869	1	47023
Apprch %	4.7	88.2	7.1	0	15.1	13.2	71.6	0	34	64.3	1.6	0	15.6	51.9	32.5	0	
Total %	1.5	27.3	2.2	0	3.1	2.7	14.9	0	14.5	27.4	0.7	0	0.9	2.9	1.8	0	

Groups Printed- Turning Movement Count

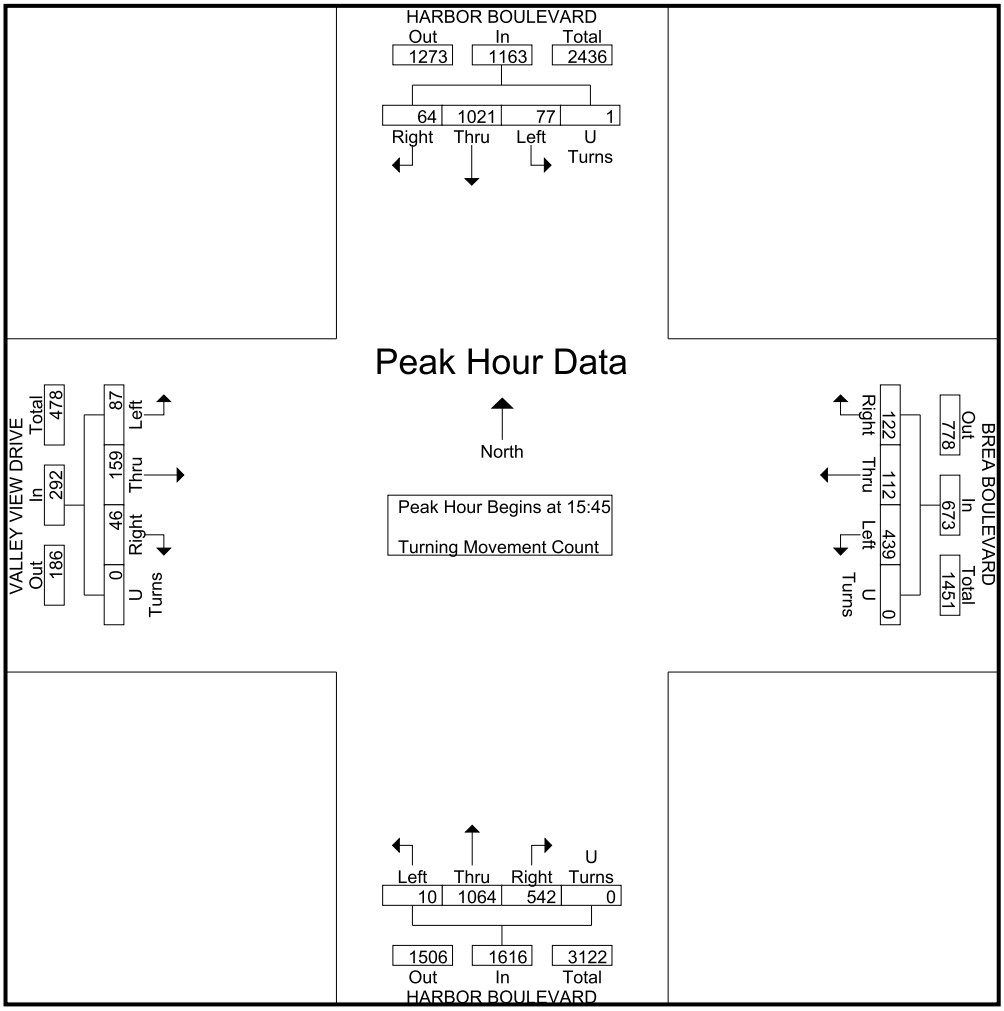
HARBOR BOULEVARD Southbound				BREA BOULEVARD Westbound				HARBOR BOULEVARD Northbound				VALLEY VIEW DRIVE Eastbound			
Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns

Transportation Studies, Inc
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City: FULLERTON
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File Name : H2305018
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	HARBOR BOULEVARD Southbound					BREA BOULEVARD Westbound					HARBOR BOULEVARD Northbound					VALLEY VIEW DRIVE Eastbound					
Start Time	Right	Thru	Left	U Turns	App. Total	Right	Thru	Left	U Turns	App. Total	Right	Thru	Left	U Turns	App. Total	Right	Thru	Left	U Turns	App. Total	Int. Total
Peak Hour Analysis From 00:00 to 23:45 - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 15:45																					
15:45	17	270	16	0	303	26	32	104	0	162	143	284	5	0	432	16	41	23	0	80	977
16:00	12	249	16	0	277	37	38	109	0	184	143	252	2	0	397	6	36	20	0	62	920
16:15	19	241	24	0	284	26	16	119	0	161	119	265	3	0	387	11	32	19	0	62	894
16:30	16	261	21	1	299	33	26	107	0	166	137	263	0	0	400	13	50	25	0	88	953
Total Volume	64	1021	77	1	1163	122	112	439	0	673	542	1064	10	0	1616	46	159	87	0	292	3744
% App. Total	5.5	87.8	6.6	0.1		18.1	16.6	65.2	0		33.5	65.8	0.6	0		15.8	54.5	29.8	0		
PHF	.842	.945	.802	.250	.960	.824	.737	.922	.000	.914	.948	.937	.500	.000	.935	.719	.795	.870	.000	.830	.958



Transportation Studies, Inc
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City: FULLERTON
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File Name : H2305017
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Groups Printed- Turning Movement Count

	HARBOR BOULEVARD Southbound				VALENCIA MESA DRIVE Westbound				HARBOR BOULEVARD Northbound				VALENCIA MESA DRIVE Eastbound				
Start Time	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Int. Total
00:00	2	14	0	0	7	0	2	0	0	28	0	0	2	0	0	0	55
00:15	0	13	6	0	5	0	0	0	1	28	0	0	0	0	1	0	54
00:30	2	14	2	0	6	0	4	0	3	34	1	0	0	0	4	0	70
00:45	0	11	3	0	1	0	3	0	1	22	0	0	0	0	1	0	42
Total	4	52	11	0	19	0	9	0	5	112	1	0	2	0	6	0	221
01:00	2	12	3	0	4	0	1	0	1	22	0	0	1	0	2	0	48
01:15	0	17	1	0	2	0	0	0	0	10	1	0	0	0	2	0	33
01:30	0	9	2	0	1	0	3	0	1	12	0	0	1	0	0	0	29
01:45	0	13	1	0	2	0	2	0	1	15	1	0	0	0	0	0	35
Total	2	51	7	0	9	0	6	0	3	59	2	0	2	0	4	0	145
02:00	1	11	0	0	3	0	0	0	1	9	0	0	0	0	0	0	25
02:15	2	10	1	0	0	0	0	0	2	6	0	0	0	0	2	0	23
02:30	0	8	0	0	1	0	1	0	0	15	2	0	0	0	0	0	27
02:45	0	10	2	0	0	0	0	0	0	13	0	0	2	0	0	0	27
Total	3	39	3	0	4	0	1	0	3	43	2	0	2	0	2	0	102
03:00	0	12	0	0	1	1	1	0	0	15	0	0	0	0	0	0	30
03:15	0	13	0	0	1	0	1	0	1	17	0	0	0	0	0	0	33
03:30	0	21	0	0	1	0	2	0	0	31	0	0	0	0	1	0	56
03:45	0	21	2	0	1	0	0	0	0	21	1	0	0	0	2	0	48
Total	0	67	2	0	4	1	4	0	1	84	1	0	0	0	3	0	167
04:00	0	17	0	0	4	0	0	0	1	12	3	0	0	1	0	0	38
04:15	0	20	0	0	5	0	1	0	1	31	0	0	3	0	1	0	62
04:30	0	39	3	0	0	0	1	0	0	28	0	0	3	1	0	0	75
04:45	0	38	2	0	0	0	2	0	2	55	0	0	0	0	0	0	99
Total	0	114	5	0	9	0	4	0	4	126	3	0	6	2	1	0	274
05:00	2	34	6	0	0	0	1	0	2	36	3	0	0	2	5	0	91
05:15	2	86	8	0	5	0	1	0	2	57	4	0	0	6	0	0	171
05:30	5	83	12	0	0	1	1	0	5	98	5	0	4	3	0	0	217
05:45	9	94	4	0	3	4	1	0	5	105	7	0	3	0	4	0	239
Total	18	297	30	0	8	5	4	0	14	296	19	0	7	11	9	0	718
06:00	4	82	11	0	10	1	4	0	6	60	9	0	4	4	5	0	200
06:15	15	112	11	0	6	4	1	0	15	82	11	0	6	5	8	0	276
06:30	14	161	21	0	7	3	7	0	17	83	27	0	3	5	4	0	352
06:45	16	216	28	0	12	6	6	0	16	127	23	0	7	9	10	0	476
Total	49	571	71	0	35	14	18	0	54	352	70	0	20	23	27	0	1304
07:00	21	281	22	0	8	4	7	0	17	127	14	0	12	12	10	0	535
07:15	25	287	27	0	6	4	3	0	24	157	28	0	12	10	12	0	595
07:30	31	274	31	0	7	6	6	0	15	212	30	0	13	6	17	0	648
07:45	56	260	45	0	5	14	14	0	30	207	41	0	17	12	20	0	721
Total	133	1102	125	0	26	28	30	0	86	703	113	0	54	40	59	0	2499
08:00	41	271	47	0	20	4	12	0	23	178	36	0	14	10	23	0	679
08:15	42	234	35	0	21	18	17	0	38	177	37	0	22	13	18	0	672
08:30	37	219	40	0	17	9	8	0	34	190	33	0	32	11	29	0	659
08:45	53	223	26	0	20	5	12	0	21	146	28	0	27	18	30	0	609
Total	173	947	148	0	78	36	49	0	116	691	134	0	95	52	100	0	2619

Transportation Studies, Inc
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City: FULLERTON
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File Name : H2305017
Site Code : 00000000
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Groups Printed- Turning Movement Count

	HARBOR BOULEVARD Southbound				VALENCIA MESA DRIVE Westbound				HARBOR BOULEVARD Northbound				VALENCIA MESA DRIVE Eastbound				
Start Time	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Int. Total
09:00	18	186	43	0	23	6	19	0	19	129	33	0	34	7	23	0	540
09:15	34	170	40	0	27	14	12	0	26	136	26	0	32	24	21	0	562
09:30	28	152	28	0	31	11	17	0	19	125	24	0	28	8	32	0	503
09:45	30	172	40	1	29	12	24	0	19	128	20	0	23	12	24	0	534
Total	110	680	151	1	110	43	72	0	83	518	103	0	117	51	100	0	2139
10:00	27	173	29	0	31	11	25	0	17	121	14	0	27	8	33	0	516
10:15	22	164	38	0	39	19	30	0	18	123	19	0	31	13	34	0	550
10:30	22	183	33	0	24	10	13	0	24	152	19	0	24	20	25	0	549
10:45	24	173	45	0	42	11	22	0	29	146	24	0	24	10	18	0	568
Total	95	693	145	0	136	51	90	0	88	542	76	0	106	51	110	0	2183
11:00	30	175	23	0	38	5	21	0	25	156	15	0	38	5	29	0	560
11:15	22	184	34	0	32	10	24	0	16	149	18	0	29	4	21	0	543
11:30	16	153	22	1	35	18	25	0	18	179	13	0	41	9	40	0	570
11:45	12	157	19	0	25	11	40	0	23	188	17	0	25	8	33	0	558
Total	80	669	98	1	130	44	110	0	82	672	63	0	133	26	123	0	2231
12:00	24	172	20	0	37	6	19	0	19	198	19	0	29	5	21	0	569
12:15	31	154	40	2	27	10	23	0	22	190	17	0	21	9	26	0	572
12:30	25	163	24	3	26	11	18	0	19	188	16	0	30	5	30	0	558
12:45	28	176	35	1	28	6	15	0	10	200	23	0	18	6	27	0	573
Total	108	665	119	6	118	33	75	0	70	776	75	0	98	25	104	0	2272
13:00	34	165	30	0	30	14	16	0	15	170	26	0	21	6	22	0	549
13:15	30	170	31	1	27	7	22	0	20	189	18	0	21	10	31	0	577
13:30	31	172	35	0	43	10	16	0	16	194	19	0	31	10	21	0	598
13:45	39	189	39	0	31	3	12	0	17	169	24	0	14	12	25	0	574
Total	134	696	135	1	131	34	66	0	68	722	87	0	87	38	99	0	2298
14:00	33	199	24	0	26	6	12	0	9	189	10	0	22	8	27	0	565
14:15	19	180	42	0	35	7	16	0	18	194	10	0	30	11	24	0	586
14:30	26	222	31	0	29	8	15	0	11	200	22	0	23	4	31	0	622
14:45	30	207	38	0	52	14	14	0	24	259	21	0	31	16	26	0	732
Total	108	808	135	0	142	35	57	0	62	842	63	0	106	39	108	0	2505
15:00	21	227	26	0	37	10	20	0	23	220	16	0	31	6	33	0	670
15:15	28	212	35	0	41	6	13	0	16	237	20	0	20	4	35	0	667
15:30	26	245	19	1	44	11	17	0	12	273	20	0	27	6	30	0	731
15:45	22	234	37	0	47	7	14	0	16	281	12	0	29	9	31	0	739
Total	97	918	117	1	169	34	64	0	67	1011	68	0	107	25	129	0	2807
16:00	23	237	18	0	54	13	28	0	11	290	8	0	26	4	31	0	743
16:15	24	221	29	0	43	9	9	0	18	287	11	0	27	8	25	0	711
16:30	18	244	15	0	48	8	14	0	9	294	13	0	38	3	32	0	736
16:45	22	206	21	0	40	7	10	0	14	273	17	0	25	9	33	0	677
Total	87	908	83	0	185	37	61	0	52	1144	49	0	116	24	121	0	2867
17:00	18	210	21	0	68	17	38	0	13	299	7	0	42	2	29	0	764
17:15	12	245	24	0	41	13	21	0	16	267	6	0	18	8	32	0	703
17:30	12	223	10	0	30	6	12	0	9	277	6	0	15	8	26	0	634
17:45	25	188	21	0	18	4	9	0	11	210	6	0	10	7	7	0	516
Total	67	866	76	0	157	40	80	0	49	1053	25	0	85	25	94	0	2617
18:00	11	179	14	0	34	10	19	0	13	232	7	0	7	8	13	0	547
18:15	19	155	18	0	28	2	10	0	9	201	5	0	5	11	17	0	480

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Groups Printed- Turning Movement Count

	HARBOR BOULEVARD Southbound				VALENCIA MESA DRIVE Westbound				HARBOR BOULEVARD Northbound				VALENCIA MESA DRIVE Eastbound				
Start Time	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Int. Total
18:30	14	156	17	0	29	9	14	0	13	253	3	0	8	0	17	0	533
18:45	12	137	24	1	21	6	14	0	18	177	11	0	11	4	8	0	444
Total	56	627	73	1	112	27	57	0	53	863	26	0	31	23	55	0	2004
19:00	5	132	9	0	31	9	20	0	12	178	3	0	10	9	7	0	425
19:15	11	117	18	0	25	2	10	0	13	164	5	0	6	2	5	0	378
19:30	6	127	7	0	15	1	14	0	15	127	4	0	9	3	5	1	334
19:45	9	123	17	1	22	5	8	0	10	95	2	0	7	4	5	0	308
Total	31	499	51	1	93	17	52	0	50	564	14	0	32	18	22	1	1445
20:00	6	113	11	0	31	5	15	0	10	147	5	0	6	4	7	0	360
20:15	4	87	17	1	22	3	13	0	4	135	3	0	3	2	6	0	300
20:30	9	114	10	1	14	3	15	0	9	118	5	0	4	3	4	0	309
20:45	4	86	4	0	15	0	11	0	11	120	5	0	4	1	4	0	265
Total	23	400	42	2	82	11	54	0	34	520	18	0	17	10	21	0	1234
21:00	10	83	3	0	19	2	12	0	4	111	4	0	8	1	3	0	260
21:15	7	82	6	0	7	1	14	0	5	105	3	0	3	0	2	0	235
21:30	4	76	7	0	4	1	7	0	3	96	1	0	3	1	1	0	204
21:45	6	56	2	0	14	2	13	0	5	89	3	0	1	0	1	0	192
Total	27	297	18	0	44	6	46	0	17	401	11	0	15	2	7	0	891
22:00	1	53	4	0	6	6	5	0	2	81	0	0	1	0	0	0	159
22:15	4	38	4	0	6	2	8	0	6	69	2	0	2	3	4	0	148
22:30	2	53	5	0	8	0	6	0	4	78	1	0	0	0	4	0	161
22:45	6	28	3	0	0	0	4	0	6	51	2	0	2	0	1	0	103
Total	13	172	16	0	20	8	23	0	18	279	5	0	5	3	9	0	571
23:00	0	42	2	0	7	0	2	0	2	61	1	0	0	0	0	0	117
23:15	4	25	1	0	4	1	5	0	0	42	1	0	0	0	1	0	84
23:30	3	35	1	0	4	0	6	0	1	52	0	0	0	0	0	0	102
23:45	0	12	3	0	5	0	1	0	0	38	2	0	3	0	6	0	70
Total	7	114	7	0	20	1	14	0	3	193	4	0	3	0	7	0	373
Grand Total	1425	12252	1668	14	1841	505	1046	0	1082	12566	1032	0	1246	488	1320	1	36486
Apprch %	9.3	79.8	10.9	0.1	54.3	14.9	30.8	0	7.4	85.6	7	0	40.8	16	43.2	0	
Total %	3.9	33.6	4.6	0	5	1.4	2.9	0	3	34.4	2.8	0	3.4	1.3	3.6	0	

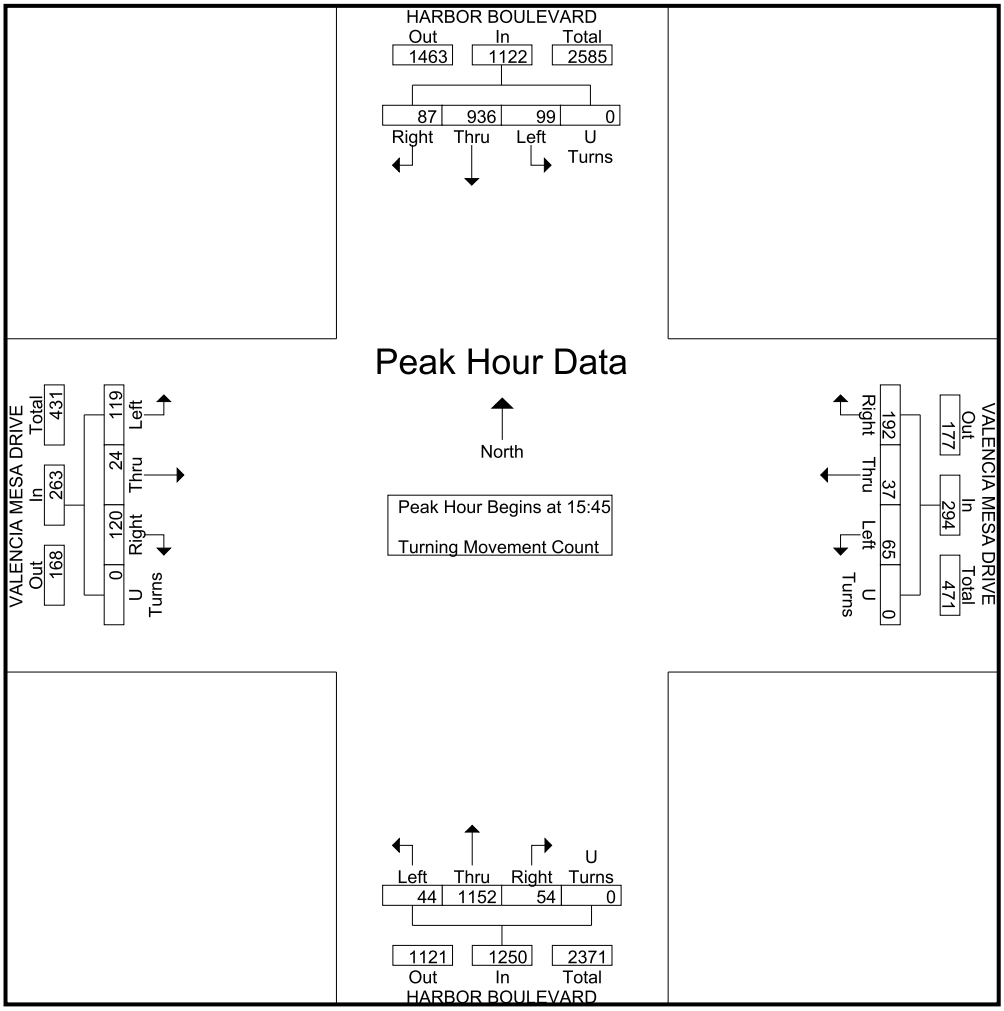
	HARBOR BOULEVARD Southbound				Westbound				HARBOR BOULEVARD Northbound				Eastbound			
	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns

Transportation Studies, Inc
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: FULLERTON
N-S Direction: HARBOR BOULEVARD
E-W Direction: VALENCIA MESA DR

File Name : H2305017
Site Code : 00000000
Start Date : 5/24/2023
Page No : 4

	HARBOR BOULEVARD Southbound					VALENCIA MESA DRIVE Westbound					HARBOR BOULEVARD Northbound					VALENCIA MESA DRIVE Eastbound					
Start Time	Right	Thru	Left	U Turns	App. Total	Right	Thru	Left	U Turns	App. Total	Right	Thru	Left	U Turns	App. Total	Right	Thru	Left	U Turns	App. Total	Int. Total
Peak Hour Analysis From 00:00 to 23:45 - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 15:45																					
15:45	22	234	37	0	293	47	7	14	0	68	16	281	12	0	309	29	9	31	0	69	739
16:00	23	237	18	0	278	54	13	28	0	95	11	290	8	0	309	26	4	31	0	61	743
16:15	24	221	29	0	274	43	9	9	0	61	18	287	11	0	316	27	8	25	0	60	711
16:30	18	244	15	0	277	48	8	14	0	70	9	294	13	0	316	38	3	32	0	73	736
Total Volume	87	936	99	0	1122	192	37	65	0	294	54	1152	44	0	1250	120	24	119	0	263	2929
% App. Total	7.8	83.4	8.8	0		65.3	12.6	22.1	0		4.3	92.2	3.5	0		45.6	9.1	45.2	0		
PHF	.906	.959	.669	.000	.957	.889	.712	.580	.000	.774	.750	.980	.846	.000	.989	.789	.667	.930	.000	.901	.986



Transportation Studies, Inc
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: FULLERTON
N-S Direction: HARBOR BOULEVARD
E-W Direction: E. VALLEY VIEW DRIVE

File Name : H2403021
Site Code : 00000000
Start Date : 3/27/2024
Page No : 1

Groups Printed- Turning Movement Count

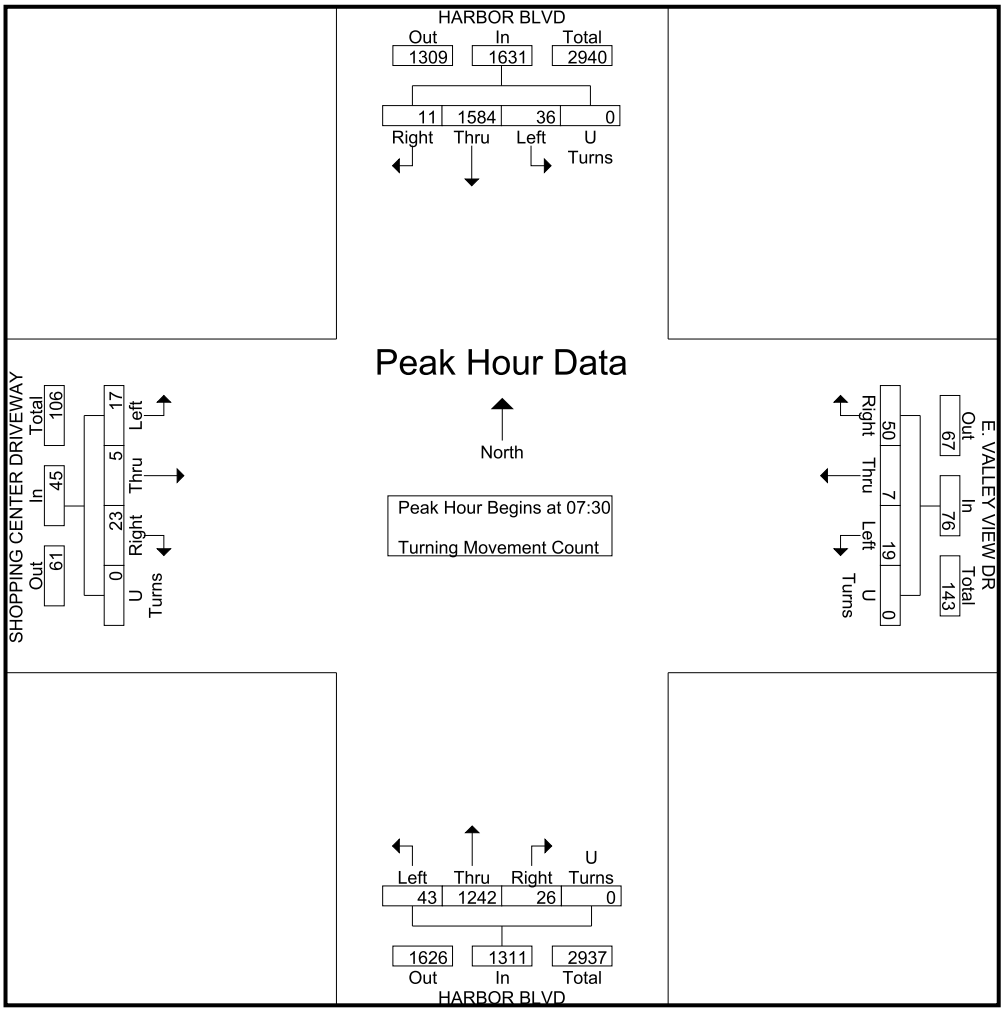
	HARBOR BLVD Southbound				E. VALLEY VIEW DR Westbound				HARBOR BLVD Northbound				SHOPPING CENTER DRIVEWAY Eastbound				
Start Time	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Int. Total
07:00	4	328	15	0	7	2	2	0	6	184	4	0	7	0	6	0	565
07:15	1	381	11	0	7	2	2	0	8	253	10	0	7	1	3	0	686
07:30	0	433	8	0	12	2	6	0	8	317	4	0	8	1	4	0	803
07:45	3	386	10	0	17	1	5	0	4	339	10	0	2	3	6	0	786
Total	8	1528	44	0	43	7	15	0	26	1093	28	0	24	5	19	0	2840
08:00	3	404	9	0	7	2	6	0	7	259	9	0	6	0	3	0	715
08:15	5	361	9	0	14	2	2	0	7	327	20	0	7	1	4	0	759
08:30	5	384	6	2	8	1	4	0	6	300	16	0	6	1	6	0	745
08:45	4	340	8	3	13	2	6	0	8	291	13	0	6	0	1	0	695
Total	17	1489	32	5	42	7	18	0	28	1177	58	0	25	2	14	0	2914
16:00	11	338	14	0	7	3	5	0	13	363	22	0	13	6	7	0	802
16:15	6	377	9	0	9	4	2	0	6	393	22	0	13	7	11	0	859
16:30	6	372	9	0	6	4	3	0	4	404	14	0	13	6	10	0	851
16:45	16	352	10	1	16	0	2	0	10	394	10	0	17	4	9	0	841
Total	39	1439	42	1	38	11	12	0	33	1554	68	0	56	23	37	0	3353
17:00	14	367	13	1	9	3	5	0	5	401	18	0	24	4	12	0	876
17:15	6	364	12	2	12	4	4	0	5	409	18	0	11	7	16	0	870
17:30	9	365	8	0	8	6	7	0	11	356	21	0	11	4	9	0	815
17:45	6	381	8	0	10	2	4	0	10	378	12	0	21	11	11	0	854
Total	35	1477	41	3	39	15	20	0	31	1544	69	0	67	26	48	0	3415
Grand Total	99	5933	159	9	162	40	65	0	118	5368	223	0	172	56	118	0	12522
Apprch %	1.6	95.7	2.6	0.1	60.7	15	24.3	0	2.1	94	3.9	0	49.7	16.2	34.1	0	
Total %	0.8	47.4	1.3	0.1	1.3	0.3	0.5	0	0.9	42.9	1.8	0	1.4	0.4	0.9	0	

Transportation Studies, Inc
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: FULLERTON
N-S Direction: HARBOR BOULEVARD
E-W Direction: E. VALLEY VIEW DRIVE

File Name : H2403021
Site Code : 00000000
Start Date : 3/27/2024
Page No : 2

	HARBOR BLVD Southbound					E. VALLEY VIEW DR Westbound					HARBOR BLVD Northbound					SHOPPING CENTER DRIVEWAY Eastbound					
Start Time	Right	Thru	Left	U Turns	App. Total	Right	Thru	Left	U Turns	App. Total	Right	Thru	Left	U Turns	App. Total	Right	Thru	Left	U Turns	App. Total	Int. Total
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 07:30																					
07:30	0	433	8	0	441	12	2	6	0	20	8	317	4	0	329	8	1	4	0	13	803
07:45	3	386	10	0	399	17	1	5	0	23	4	339	10	0	353	2	3	6	0	11	786
08:00	3	404	9	0	416	7	2	6	0	15	7	259	9	0	275	6	0	3	0	9	715
08:15	5	361	9	0	375	14	2	2	0	18	7	327	20	0	354	7	1	4	0	12	759
Total Volume	11	1584	36	0	1631	50	7	19	0	76	26	1242	43	0	1311	23	5	17	0	45	3063
% App. Total	0.7	97.1	2.2	0		65.8	9.2	25	0		2	94.7	3.3	0		51.1	11.1	37.8	0		
PHF	.550	.915	.900	.000	.925	.735	.875	.792	.000	.826	.813	.916	.538	.000	.926	.719	.417	.708	.000	.865	.954

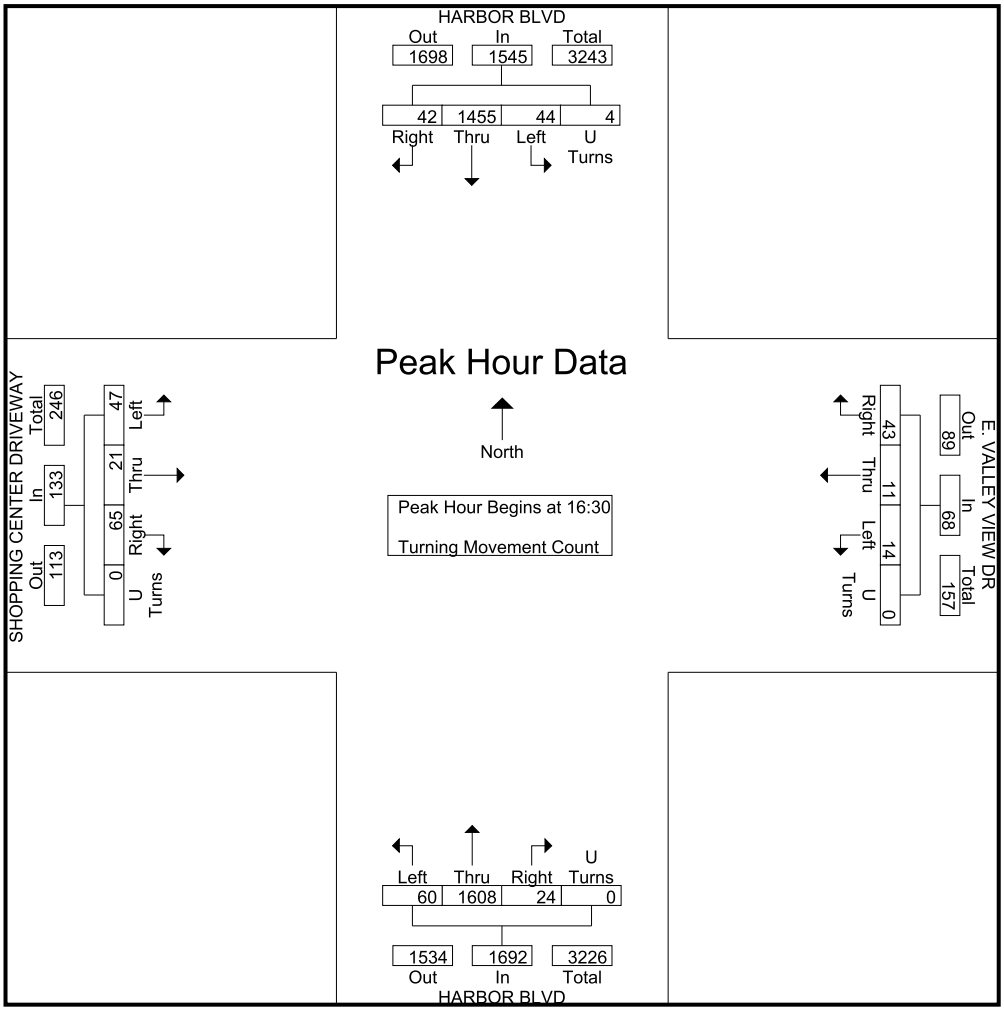


Transportation Studies, Inc
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: FULLERTON
N-S Direction: HARBOR BOULEVARD
E-W Direction: E. VALLEY VIEW DRIVE

File Name : H2403021
Site Code : 00000000
Start Date : 3/27/2024
Page No : 3

	HARBOR BLVD Southbound					E. VALLEY VIEW DR Westbound					HARBOR BLVD Northbound					SHOPPING CENTER DRIVEWAY Eastbound					
Start Time	Right	Thru	Left	U Turns	App. Total	Right	Thru	Left	U Turns	App. Total	Right	Thru	Left	U Turns	App. Total	Right	Thru	Left	U Turns	App. Total	Int. Total
Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 16:30																					
16:30	6	372	9	0	387	6	4	3	0	13	4	404	14	0	422	13	6	10	0	29	851
16:45	16	352	10	1	379	16	0	2	0	18	10	394	10	0	414	17	4	9	0	30	841
17:00	14	367	13	1	395	9	3	5	0	17	5	401	18	0	424	24	4	12	0	40	876
17:15	6	364	12	2	384	12	4	4	0	20	5	409	18	0	432	11	7	16	0	34	870
Total Volume	42	1455	44	4	1545	43	11	14	0	68	24	1608	60	0	1692	65	21	47	0	133	3438
% App. Total	2.7	94.2	2.8	0.3		63.2	16.2	20.6	0		1.4	95	3.5	0		48.9	15.8	35.3	0		
PHF	.656	.978	.846	.500	.978	.672	.688	.700	.000	.850	.600	.983	.833	.000	.979	.677	.750	.734	.000	.831	.981



Transportation Studies, Inc
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: FULLERTON
N-S Direction: HARBOR BOULEVARD
E-W Direction: BERKEKEY AVE

File Name : H2403022
Site Code : 00000000
Start Date : 3/27/2024
Page No : 1

Groups Printed- Turning Movement Count

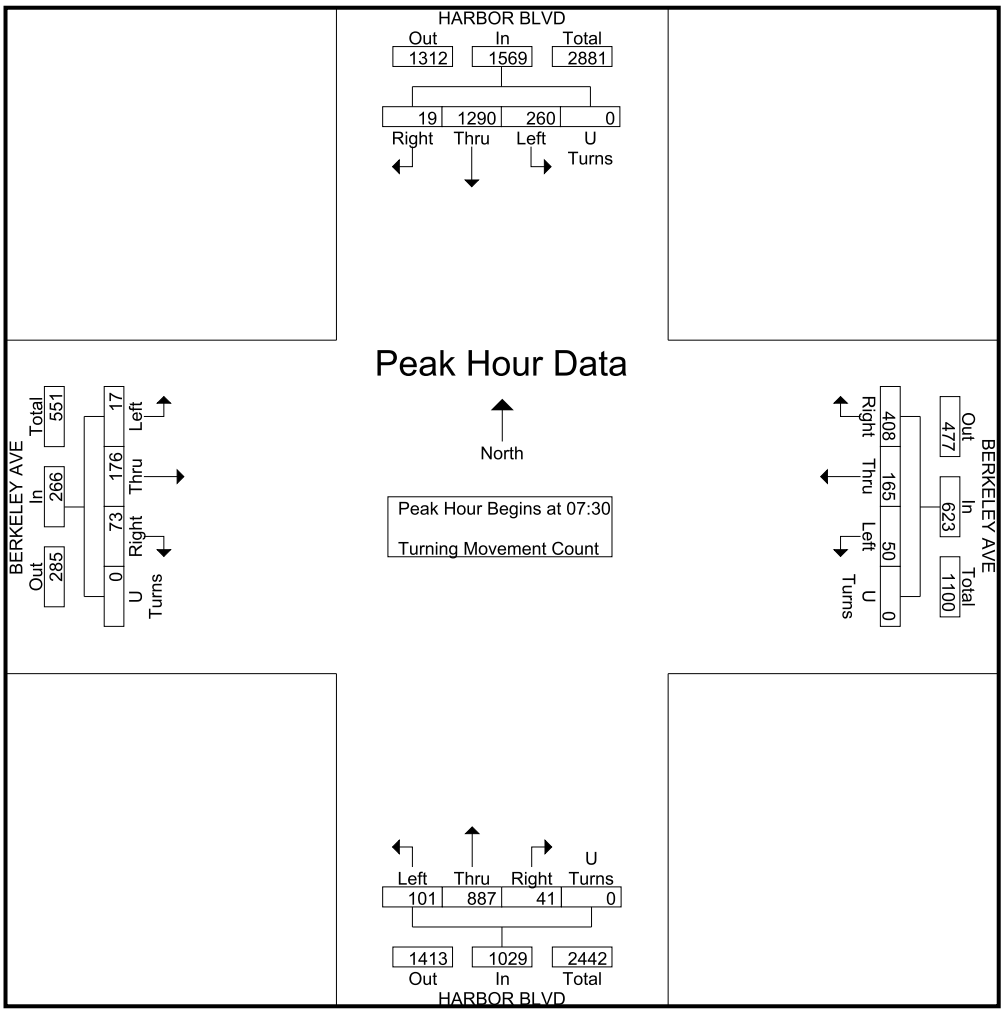
	HARBOR BLVD Southbound				BERKELEY AVE Westbound				HARBOR BLVD Northbound				BERKELEY AVE Eastbound				
Start Time	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Right	Thru	Left	U Turns	Int. Total
07:00	1	275	63	0	55	18	7	0	3	141	9	0	7	26	2	0	607
07:15	9	312	64	0	80	32	6	0	10	187	15	0	15	33	3	0	766
07:30	3	345	66	0	112	46	7	0	10	226	18	0	14	41	2	0	890
07:45	7	317	68	0	119	38	13	0	7	211	23	0	13	34	1	0	851
Total	20	1249	261	0	366	134	33	0	30	765	65	0	49	134	8	0	3114
08:00	4	327	74	0	95	30	18	0	12	187	26	0	21	58	7	0	859
08:15	5	301	52	0	82	51	12	0	12	263	34	0	25	43	7	0	887
08:30	3	310	58	0	100	27	9	0	8	205	26	0	18	25	0	0	789
08:45	2	316	44	0	74	26	13	0	3	238	24	0	10	27	4	0	781
Total	14	1254	228	0	351	134	52	0	35	893	110	0	74	153	18	0	3316
16:00	4	330	83	0	113	32	12	0	7	274	24	0	27	33	5	0	944
16:15	4	240	69	0	112	49	13	0	8	298	25	0	16	31	8	0	873
16:30	6	301	79	0	114	48	14	0	6	294	15	0	29	50	6	0	962
16:45	4	281	71	0	107	34	14	0	4	303	24	0	13	30	11	0	896
Total	18	1152	302	0	446	163	53	0	25	1169	88	0	85	144	30	0	3675
17:00	5	346	90	0	110	40	15	0	7	303	21	0	25	33	4	0	999
17:15	6	324	90	0	129	46	13	0	8	291	27	0	14	34	7	0	989
17:30	5	286	74	0	101	52	11	0	3	280	18	0	10	31	8	0	879
17:45	9	252	80	0	96	32	5	0	7	293	21	0	7	33	14	0	849
Total	25	1208	334	0	436	170	44	0	25	1167	87	0	56	131	33	0	3716
Grand Total	77	4863	1125	0	1599	601	182	0	115	3994	350	0	264	562	89	0	13821
Apprch %	1.3	80.2	18.5	0	67.1	25.2	7.6	0	2.6	89.6	7.8	0	28.9	61.4	9.7	0	
Total %	0.6	35.2	8.1	0	11.6	4.3	1.3	0	0.8	28.9	2.5	0	1.9	4.1	0.6	0	

Transportation Studies, Inc
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: FULLERTON
N-S Direction: HARBOR BOULEVARD
E-W Direction: BERKEKEY AVE

File Name : H2403022
Site Code : 00000000
Start Date : 3/27/2024
Page No : 2

	HARBOR BLVD Southbound					BERKELEY AVE Westbound					HARBOR BLVD Northbound					BERKELEY AVE Eastbound					
Start Time	Right	Thru	Left	U Turns	App. Total	Right	Thru	Left	U Turns	App. Total	Right	Thru	Left	U Turns	App. Total	Right	Thru	Left	U Turns	App. Total	Int. Total
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 07:30																					
07:30	3	345	66	0	414	112	46	7	0	165	10	226	18	0	254	14	41	2	0	57	890
07:45	7	317	68	0	392	119	38	13	0	170	7	211	23	0	241	13	34	1	0	48	851
08:00	4	327	74	0	405	95	30	18	0	143	12	187	26	0	225	21	58	7	0	86	859
08:15	5	301	52	0	358	82	51	12	0	145	12	263	34	0	309	25	43	7	0	75	887
Total Volume	19	1290	260	0	1569	408	165	50	0	623	41	887	101	0	1029	73	176	17	0	266	3487
% App. Total	1.2	82.2	16.6	0		65.5	26.5	8	0		4	86.2	9.8	0		27.4	66.2	6.4	0		
PHF	.679	.935	.878	.000	.947	.857	.809	.694	.000	.916	.854	.843	.743	.000	.833	.730	.759	.607	.000	.773	.979

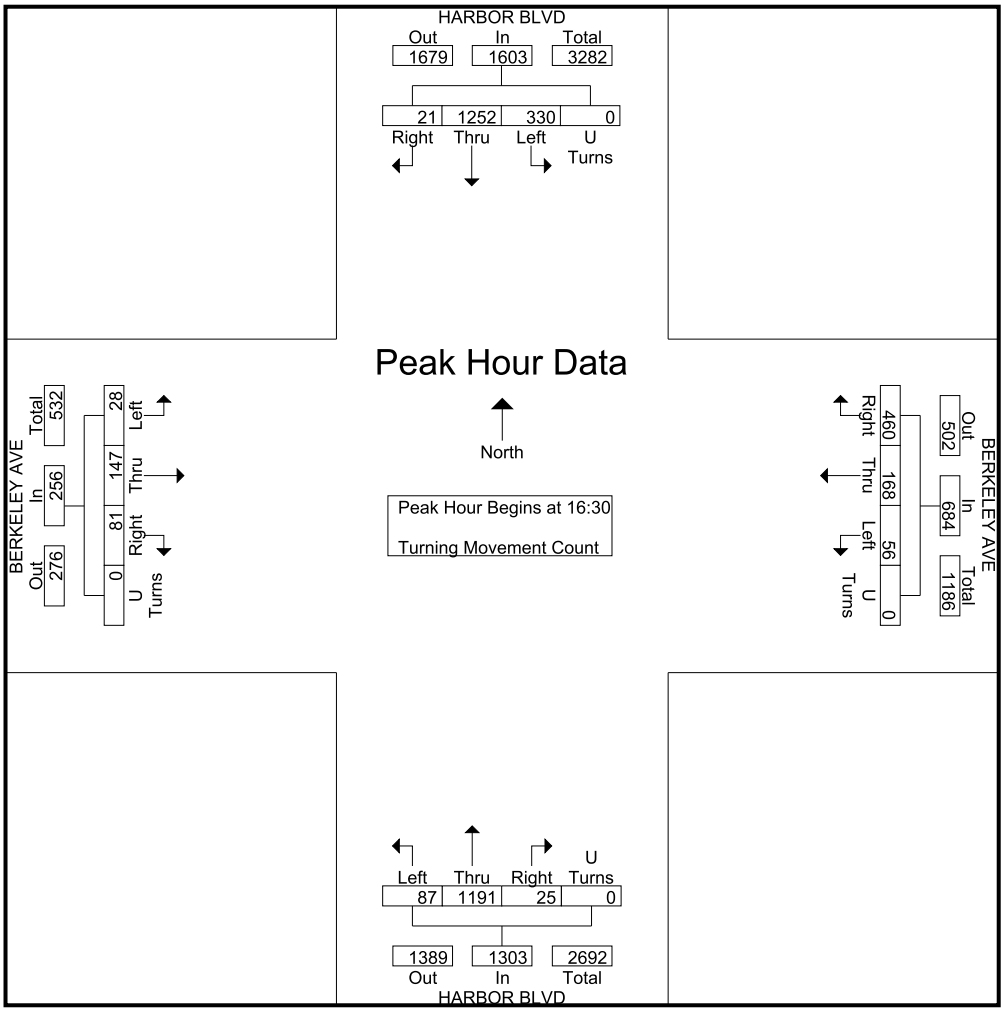


Transportation Studies, Inc
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: FULLERTON
N-S Direction: HARBOR BOULEVARD
E-W Direction: BERKEKEY AVE

File Name : H2403022
Site Code : 00000000
Start Date : 3/27/2024
Page No : 3

	HARBOR BLVD Southbound					BERKELEY AVE Westbound					HARBOR BLVD Northbound					BERKELEY AVE Eastbound					
Start Time	Right	Thru	Left	U Turns	App. Total	Right	Thru	Left	U Turns	App. Total	Right	Thru	Left	U Turns	App. Total	Right	Thru	Left	U Turns	App. Total	Int. Total
Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 16:30																					
16:30	6	301	79	0	386	114	48	14	0	176	6	294	15	0	315	29	50	6	0	85	962
16:45	4	281	71	0	356	107	34	14	0	155	4	303	24	0	331	13	30	11	0	54	896
17:00	5	346	90	0	441	110	40	15	0	165	7	303	21	0	331	25	33	4	0	62	999
17:15	6	324	90	0	420	129	46	13	0	188	8	291	27	0	326	14	34	7	0	55	989
Total Volume	21	1252	330	0	1603	460	168	56	0	684	25	1191	87	0	1303	81	147	28	0	256	3846
% App. Total	1.3	78.1	20.6	0		67.3	24.6	8.2	0		1.9	91.4	6.7	0		31.6	57.4	10.9	0		
PHF	.875	.905	.917	.000	.909	.891	.875	.933	.000	.910	.781	.983	.806	.000	.984	.698	.735	.636	.000	.753	.962



Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : HARBOR BOULEVARD — Site: FULLERTON

Segment : VALLEY MESA TO BASTANCHURY — Date: 05/25/23

Client : CITY FULLERTON

Interval	NB				SB				Combined				Day: Thursday
Begin	AM		PM		AM		PM		AM		PM		
12:00	53	152	244	955	23	72	228	887	76	224	472	1,842	
12:15	28		274		17		212		45		486		
12:30	38		214		18		214		56		428		
12:45	33		223		14		233		47		456		
01:00	23	80	222	894	15	55	204	931	38	135	426	1,825	
01:15	19		217		19		210		38		427		
01:30	21		220		13		244		34		464		
01:45	17		235		8		273		25		508		
02:00	16	65	254	1,092	8	46	270	1,094	24	111	524	2,186	
02:15	16		253		15		252		31		505		
02:30	17		256		13		284		30		540		
02:45	16		329		10		288		26		617		
03:00	24	100	299	1,321	20	79	260	1,095	44	179	559	2,416	
03:15	18		276		11		275		29		551		
03:30	32		334		24		258		56		592		
03:45	26		412		24		302		50		714		
04:00	18	126	398	1,359	17	134	310	1,098	35	260	708	2,457	
04:15	26		310		31		268		57		578		
04:30	24		344		38		254		62		598		
04:45	58		307		48		266		106		573		
05:00	36	288	407	1,386	59	336	268	1,073	95	624	675	2,459	
05:15	67		388		60		281		127		669		
05:30	82		302		91		264		173		566		
05:45	103		289		126		260		229		549		
06:00	56	374	244	955	99	648	220	811	155	1,022	464	1,766	
06:15	82		254		128		211		210		465		
06:30	116		246		189		204		305		450		
06:45	120		211		232		176		352		387		
07:00	131	769	228	764	294	1,379	140	571	425	2,148	368	1,335	
07:15	160		182		353		166		513		348		
07:30	200		198		336		133		536		331		
07:45	278		156		396		132		674		288		
08:00	200	782	180	644	341	1,263	129	472	541	2,045	309	1,116	
08:15	202		180		326		109		528		289		
08:30	208		144		326		112		534		256		
08:45	172		140		270		122		442		262		
09:00	214	798	164	501	276	969	99	317	490	1,767	263	818	
09:15	182		122		250		86		432		208		
09:30	192		107		193		72		385		179		
09:45	210		108		250		60		460		168		
10:00	166	764	96	362	248	948	54	232	414	1,712	150	594	
10:15	192		88		224		60		416		148		
10:30	208		102		252		62		460		164		
10:45	198		76		224		56		422		132		
11:00	216	876	68	235	218	882	42	142	434	1,758	110	377	
11:15	222		64		200		36		422		100		
11:30	206		54		234		33		440		87		
11:45	232		49		230		31		462		80		
Totals	5,174		10,468		6,811		8,723		11,985		19,191		
Split%	43.2		54.5		56.8		45.5						
Day Totals		15,642				15,534				31,176			
Day Splits		50.2				49.8							
Peak Hour	07:45		03:45		07:15		03:15		07:30		03:45		
Volume	888		1,464		1,426		1,145		2,279		2,598		
Factor	0.80		0.89		0.90		0.92		0.85		0.91		

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : HARBOR BOULEVARD — Site: FULLERTON

Segment : VALENCIA MESA TO VALLEY VIEW — Date: 05/25/23

Client : CITY FULLERTON

Interval	NB				SB				Combined				Day: Thursday
Begin	AM		PM		AM		PM		AM		PM		
12:00	47	135	212	889	28	74	235	808	75	209	447	1,697	
12:15	30		242		16		200		46		442		
12:30	28		201		18		186		46		387		
12:45	30		234		12		187		42		421		
01:00	19	73	200	836	14	61	156	781	33	134	356	1,617	
01:15	16		192		20		186		36		378		
01:30	23		222		18		194		41		416		
01:45	15		222		9		245		24		467		
02:00	15	62	224	1,005	6	47	224	983	21	109	448	1,988	
02:15	18		215		14		223		32		438		
02:30	16		270		14		264		30		534		
02:45	13		296		13		272		26		568		
03:00	23	96	249	1,091	15	72	244	1,002	38	168	493	2,093	
03:15	16		236		10		248		26		484		
03:30	31		274		24		238		55		512		
03:45	26		332		23		272		49		604		
04:00	18	124	314	1,129	18	138	294	1,064	36	262	608	2,193	
04:15	24		271		30		260		54		531		
04:30	22		274		42		270		64		544		
04:45	60		270		48		240		108		510		
05:00	39	299	324	1,149	51	296	284	1,058	90	595	608	2,207	
05:15	61		300		50		260		111		560		
05:30	87		265		84		262		171		527		
05:45	112		260		111		252		223		512		
06:00	68	439	234	878	91	551	197	726	159	990	431	1,604	
06:15	101		244		101		188		202		432		
06:30	132		209		160		184		292		393		
06:45	138		191		199		157		337		348		
07:00	134	842	192	690	243	1,091	152	556	377	1,933	344	1,246	
07:15	177		170		296		154		473		324		
07:30	224		188		256		128		480		316		
07:45	307		140		296		122		603		262		
08:00	223	803	157	581	284	1,026	111	436	507	1,829	268	1,017	
08:15	204		158		264		100		468		258		
08:30	202		139		258		120		460		259		
08:45	174		127		220		105		394		232		
09:00	205	800	143	447	209	816	90	312	414	1,616	233	759	
09:15	195		112		223		88		418		200		
09:30	175		90		156		76		331		166		
09:45	225		102		228		58		453		160		
10:00	157	709	76	332	211	808	56	240	368	1,517	132	572	
10:15	178		78		183		69		361		147		
10:30	184		99		216		60		400		159		
10:45	190		79		198		55		388		134		
11:00	178	732	58	212	196	820	43	152	374	1,552	101	364	
11:15	176		56		190		34		366		90		
11:30	178		50		226		39		404		89		
11:45	200		48		208		36		408		84		
Totals	5,114		9,239		5,800		8,118		10,914		17,357		
Split%	46.9		53.2		53.1		46.8						
Day Totals		14,353				13,918				28,271			
Day Splits		50.8				49.2							
Peak Hour	07:30		03:30		07:15		03:45		07:15		03:45		
Volume	958		1,191		1,132		1,096		2,063		2,287		
Factor	0.78		0.90		0.96		0.93		0.86		0.94		

* Data File : D2305033

Transportation Studies, Inc.

Location : HARBOR BOULEVARD
Segment : BREA BLVD TO E. VALLEY VIEW DR
Client : LL&G

Site: FULLERTON
Date: 03/27/24
Day: Wednesday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	52	142	289	1,186	24	69	394	1,549	76	211	683	2,735
12:15	42		297		17		356		59		653	
12:30	25		313		12		358		37		671	
12:45	23		287		16		441		39		728	
01:00	26	82	305	1,300	21	66	430	1,599	47	148	735	2,899
01:15	26		317		20		341		46		658	
01:30	15		354		13		375		28		729	
01:45	15		324		12		453		27		777	
02:00	13	60	318	1,357	12	52	336	1,520	25	112	654	2,877
02:15	18		324		13		370		31		694	
02:30	13		348		13		402		26		750	
02:45	16		367		14		412		30		779	
03:00	20	116	357	1,459	12	98	434	1,864	32	214	791	3,323
03:15	25		343		24		518		49		861	
03:30	45		377		32		462		77		839	
03:45	26		382		30		450		56		832	
04:00	26	170	398	1,549	28	174	401	1,689	54	344	799	3,238
04:15	26		388		28		402		54		790	
04:30	48		368		54		402		102		770	
04:45	70		395		64		484		134		879	
05:00	51	376	402	1,582	66	403	462	1,641	117	779	864	3,223
05:15	81		382		83		428		164		810	
05:30	94		404		112		441		206		845	
05:45	150		394		142		310		292		704	
06:00	94	605	413	1,465	113	817	370	1,308	207	1,422	783	2,773
06:15	139		348		176		352		315		700	
06:30	174		382		230		296		404		678	
06:45	198		322		298		290		496		612	
07:00	188	1,133	289	1,061	398	2,014	313	1,155	586	3,147	602	2,216
07:15	250		288		494		276		744		564	
07:30	331		256		572		304		903		560	
07:45	364		228		550		262		914		490	
08:00	268	1,220	210	812	571	2,057	242	819	839	3,277	452	1,631
08:15	337		208		466		223		803		431	
08:30	318		216		550		183		868		399	
08:45	297		178		470		171		767		349	
09:00	262	967	159	590	382	1,420	180	548	644	2,387	339	1,138
09:15	252		179		334		150		586		329	
09:30	221		142		300		112		521		254	
09:45	232		110		404		106		636		216	
10:00	216	970	92	359	340	1,311	92	290	556	2,281	184	649
10:15	240		104		300		61		540		165	
10:30	261		81		336		81		597		162	
10:45	253		82		335		56		588		138	
11:00	249	1,092	62	248	378	1,511	54	166	627	2,603	116	414
11:15	270		66		340		43		610		109	
11:30	281		62		389		33		670		95	
11:45	292		58		404		36		696		94	
Totals	6,933		12,968		9,992		14,148		16,925		27,116	
Split%	41.0		47.8		59.0		52.2					
Day Totals		19,901				24,140				44,041		
Day Splits		45.2				54.8						
Peak Hour	07:30		05:15		07:15		03:00		07:30		04:45	
Volume	1,300		1,593		2,187		1,864		3,459		3,398	
Factor	0.89		0.96		0.96		0.90		0.95		0.97	

Transportation Studies, Inc.

Location : HARBOR BOULEVARD
Segment : E. VALLEY VIEW DR TO BERKELEY
Client : LL&G

Site: FULLERTON
Date: 03/27/24
Day: Wednesday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	51	145	277	1,212	27	73	296	1,206	78	218	573	2,418
12:15	47		302		17		276		64		578	
12:30	27		332		14		302		41		634	
12:45	20		301		15		332		35		633	
01:00	33	86	299	1,291	19	67	313	1,235	52	153	612	2,526
01:15	24		324		20		270		44		594	
01:30	16		341		12		302		28		643	
01:45	13		327		16		350		29		677	
02:00	12	64	321	1,389	21	64	280	1,258	33	128	601	2,647
02:15	22		332		15		310		37		642	
02:30	15		362		14		334		29		696	
02:45	15		374		14		334		29		708	
03:00	21	122	369	1,486	14	94	358	1,530	35	216	727	3,016
03:15	30		344		22		378		52		722	
03:30	46		383		30		377		76		760	
03:45	25		390		28		417		53		807	
04:00	26	177	404	1,618	29	176	353	1,467	55	353	757	3,085
04:15	29		414		30		372		59		786	
04:30	50		383		49		358		99		741	
04:45	72		417		68		384		140		801	
05:00	51	385	384	1,618	66	384	402	1,453	117	769	786	3,071
05:15	84		400		76		390		160		790	
05:30	96		426		110		360		206		786	
05:45	154		408		132		301		286		709	
06:00	93	614	418	1,505	122	771	318	1,179	215	1,385	736	2,684
06:15	142		376		156		327		298		703	
06:30	174		380		211		260		385		640	
06:45	205		331		282		274		487		605	
07:00	191	1,143	294	1,103	332	1,519	247	968	523	2,662	541	2,071
07:15	268		292		380		268		648		560	
07:30	328		259		418		241		746		500	
07:45	356		258		389		212		745		470	
08:00	287	1,264	214	833	391	1,494	202	752	678	2,758	416	1,585
08:15	349		228		368		196		717		424	
08:30	320		209		390		178		710		387	
08:45	308		182		345		176		653		358	
09:00	258	989	163	618	318	1,211	162	529	576	2,200	325	1,147
09:15	256		182		296		153		552		335	
09:30	232		146		265		114		497		260	
09:45	243		127		332		100		575		227	
10:00	214	977	94	379	275	1,107	92	292	489	2,084	186	671
10:15	234		110		270		68		504		178	
10:30	265		84		286		82		551		166	
10:45	264		91		276		50		540		141	
11:00	257	1,099	74	275	302	1,196	56	173	559	2,295	130	448
11:15	260		73		278		41		538		114	
11:30	280		65		308		38		588		103	
11:45	302		63		308		38		610		101	
Totals	7,065		13,327		8,156		12,042		15,221		25,369	
Split%	46.4		52.5		53.6		47.5					
Day Totals		20,392				20,198				40,590		
Day Splits		50.2				49.8						
Peak Hour	07:30		05:15		07:15		04:45		07:30		04:45	
Volume	1,320		1,652		1,578		1,536		2,886		3,163	
Factor	0.93		0.97		0.94		0.96		0.97		0.99	

APPENDIX B

INTERSECTION LEVEL OF SERVICE CALCULATION WORKSHEETS

APPENDIX B-I

EXISTING TRAFFIC CONDITIONS

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	311	848	163	263	927	216	182	621	110	349	941	297
Future Volume (vph)	311	848	163	263	927	216	182	621	110	349	941	297
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	325		0	193		145	230		150	275		0
Storage Lanes	1		0	2		1	2		1	2		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	0.91	0.91	0.97	0.91	1.00	0.97	0.91	1.00	0.97	0.91	0.91
Frt		0.976				0.850			0.850		0.964	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	4963	0	3433	5085	1583	3433	5085	1583	3433	4902	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	4963	0	3433	5085	1583	3433	5085	1583	3433	4902	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		33				170			126		64	
Link Speed (mph)		40			40			45			50	
Link Distance (ft)		1043			981			833			926	
Travel Time (s)		17.8			16.7			12.6			12.6	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	331	902	173	280	986	230	194	661	117	371	1001	316
Shared Lane Traffic (%)												
Lane Group Flow (vph)	331	1075	0	280	986	230	194	661	117	371	1317	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		24			24			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2	1	1	2	1	1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru	Right	Left	Thru	
Leading Detector (ft)	20	100		20	100	20	20	100	20	20	100	
Trailing Detector (ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Position(ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Size(ft)	20	6		20	6	20	20	6	20	20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases						8			2			

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	7	4		3	8	8	5	2	2	1	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Minimum Split (s)	10.0	43.0		10.0	43.0	43.0	10.0	43.0	43.0	10.0	36.0	
Total Split (s)	27.0	49.0		21.0	43.0	43.0	14.0	43.0	43.0	17.0	46.0	
Total Split (%)	20.8%	37.7%		16.2%	33.1%	33.1%	10.8%	33.1%	33.1%	13.1%	35.4%	
Maximum Green (s)	23.0	44.0		17.0	38.0	38.0	10.0	38.0	38.0	13.0	41.0	
Yellow Time (s)	3.0	4.0		3.0	4.0	4.0	3.0	4.0	4.0	3.0	4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	4.0	5.0		4.0	5.0	5.0	4.0	5.0	5.0	4.0	5.0	
Lead/Lag	Lead	Lead		Lag	Lag	Lag	Lag	Lead	Lead	Lag	Lead	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None		None	None	None	None	C-Max	C-Max	None	C-Max	
Walk Time (s)		7.0			7.0	7.0		7.0	7.0		7.0	
Flash Dont Walk (s)		31.0			31.0	31.0		31.0	31.0		24.0	
Pedestrian Calls (#/hr)		5			5	5		5	5		5	
Act Effct Green (s)	23.0	35.9		20.9	33.9	33.9	10.0	42.1	42.1	13.0	45.1	
Actuated g/C Ratio	0.18	0.28		0.16	0.26	0.26	0.08	0.32	0.32	0.10	0.35	
v/c Ratio	1.06	0.77		0.51	0.74	0.43	0.73	0.40	0.20	1.08	0.76	
Control Delay	117.6	45.6		53.8	47.6	13.3	66.2	14.1	2.2	126.1	39.6	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	117.6	45.6		53.8	47.6	13.3	66.2	14.1	2.2	126.1	39.6	
LOS	F	D		D	D	B	E	B	A	F	D	
Approach Delay		62.5			43.5			23.1			58.6	
Approach LOS		E			D			C			E	
Queue Length 50th (ft)	~305	297		112	279	39	89	157	9	~179	344	
Queue Length 95th (ft)	#494	326		163	318	108	#141	59	8	#281	421	
Internal Link Dist (ft)		963			901			753			846	
Turn Bay Length (ft)	325			193		145	230		150	275		
Base Capacity (vph)	313	1701		552	1486	583	264	1648	598	343	1743	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	1.06	0.63		0.51	0.66	0.39	0.73	0.40	0.20	1.08	0.76	

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 59 (45%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 120			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 1.08			
Intersection Signal Delay: 49.3		Intersection LOS: D	
Intersection Capacity Utilization 80.1%		ICU Level of Service D	
Analysis Period (min) 15			

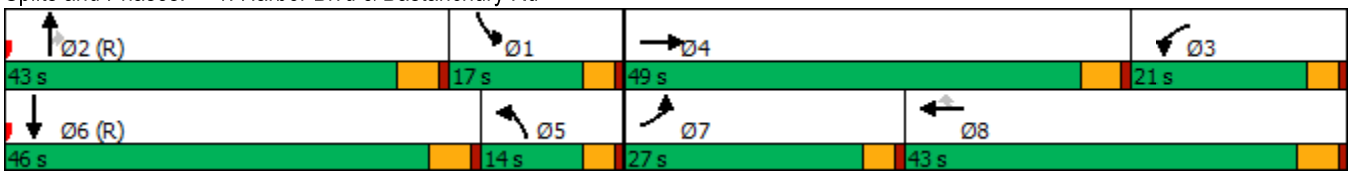
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Harbor Blvd & Bastanchury Rd



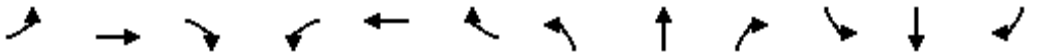
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	311	848	163	263	927	216	182	621	110	349	941	297
Future Volume (veh/h)	311	848	163	263	927	216	182	621	110	349	941	297
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	331	902	173	280	986	230	194	661	117	371	1001	316
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	315	1123	214	518	1232	383	442	1493	463	522	1212	382
Arrive On Green	0.18	0.26	0.26	0.15	0.24	0.24	0.17	0.39	0.39	0.15	0.32	0.32
Sat Flow, veh/h	1781	4304	822	3456	5106	1585	3456	5106	1585	3456	3844	1212
Grp Volume(v), veh/h	331	712	363	280	986	230	194	661	117	371	886	431
Grp Sat Flow(s),veh/h/ln	1781	1702	1722	1728	1702	1585	1728	1702	1585	1728	1702	1652
Q Serve(g_s), s	23.0	25.4	25.6	9.7	23.6	11.9	6.5	12.4	6.5	13.3	31.3	31.4
Cycle Q Clear(g_c), s	23.0	25.4	25.6	9.7	23.6	11.9	6.5	12.4	6.5	13.3	31.3	31.4
Prop In Lane	1.00		0.48	1.00		1.00	1.00		1.00	1.00		0.73
Lane Grp Cap(c), veh/h	315	888	449	518	1232	383	442	1493	463	522	1074	521
V/C Ratio(X)	1.05	0.80	0.81	0.54	0.80	0.60	0.44	0.44	0.25	0.71	0.83	0.83
Avail Cap(c_a), veh/h	315	1152	583	518	1493	463	442	1493	463	522	1074	521
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.33	1.33	1.33	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	0.94	0.94	0.94	1.00	1.00	1.00
Uniform Delay (d), s/veh	53.5	44.9	45.0	51.1	46.4	22.0	49.8	31.9	30.1	52.5	41.2	41.2
Incr Delay (d2), s/veh	64.5	3.2	6.4	1.1	2.7	1.5	0.6	0.9	1.2	4.5	7.3	14.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	15.7	10.9	11.5	4.3	10.1	4.5	2.8	4.8	2.6	5.9	13.6	14.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	118.0	48.1	51.3	52.3	49.0	23.6	50.4	32.8	31.3	57.0	48.5	55.2
LnGrp LOS	F	D	D	D	D	C	D	C	C	E	D	E
Approach Vol, veh/h		1406			1496			972			1688	
Approach Delay, s/veh		65.4			45.7			36.1			52.0	
Approach LOS		E			D			D			D	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	23.6	43.0	24.5	38.9	20.6	46.0	27.0	36.4
Change Period (Y+Rc), s	4.0	5.0	5.0	* 5	4.0	5.0	4.0	5.0
Max Green Setting (Gmax), s	13.0	38.0	17.0	* 44	10.0	41.0	23.0	38.0
Max Q Clear Time (g_c+I1), s	15.3	14.4	11.7	27.6	8.5	33.4	25.0	25.6
Green Ext Time (p_c), s	0.0	4.6	0.4	6.3	0.1	4.5	0.0	5.8

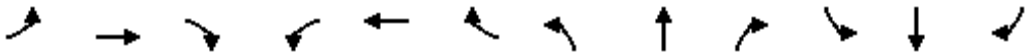
Intersection Summary	
HCM 6th Ctrl Delay	50.9
HCM 6th LOS	D

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	90	46	85	51	45	63	147	752	125	167	984	176
Future Volume (vph)	90	46	85	51	45	63	147	752	125	167	984	176
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	55		85	0		0	175		0	230		0
Storage Lanes	1		1	1		0	1		0	1		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.91	0.91
Frt			0.850		0.912			0.979			0.977	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1770	1699	0	1770	4979	0	1770	4968	0
Flt Permitted	0.583			0.726			0.950			0.950		
Satd. Flow (perm)	1086	1863	1583	1352	1699	0	1770	4979	0	1770	4968	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			59		53			30			34	
Link Speed (mph)		30			30			50			45	
Link Distance (ft)		813			705			3958			833	
Travel Time (s)		18.5			16.0			54.0			12.6	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	95	48	89	54	47	66	155	792	132	176	1036	185
Shared Lane Traffic (%)												
Lane Group Flow (vph)	95	48	89	54	113	0	155	924	0	176	1221	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2	1	1	2		1	2		1	2	
Detector Template	Left	Thru	Right	Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100	20	20	100		20	100		20	100	
Trailing Detector (ft)	0	0	0	0	0		0	0		0	0	
Detector 1 Position(ft)	0	0	0	0	0		0	0		0	0	
Detector 1 Size(ft)	20	6	20	20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	pm+ov	Perm	NA		Prot	NA		Prot	NA	
Protected Phases		4	5		8		5	2		1	6	
Permitted Phases	4		4	8								

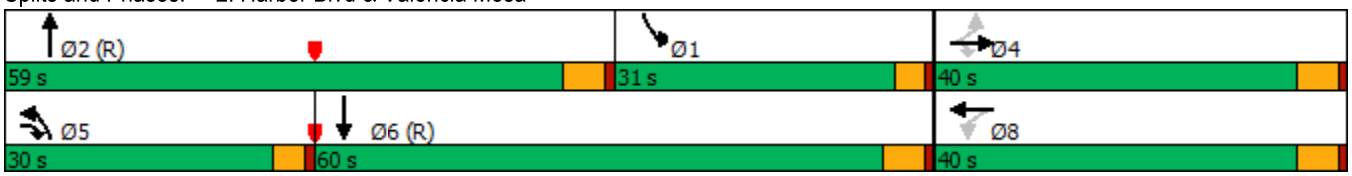


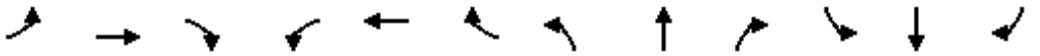
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	5	8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0		6.0	6.0		6.0	6.0	
Minimum Split (s)	11.0	11.0	10.0	36.0	36.0		10.0	26.0		10.0	30.0	
Total Split (s)	40.0	40.0	30.0	40.0	40.0		30.0	59.0		31.0	60.0	
Total Split (%)	30.8%	30.8%	23.1%	30.8%	30.8%		23.1%	45.4%		23.8%	46.2%	
Maximum Green (s)	35.0	35.0	26.0	35.0	35.0		26.0	54.0		27.0	55.0	
Yellow Time (s)	4.0	4.0	3.0	4.0	4.0		3.0	4.0		3.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.0	5.0	4.0	5.0	5.0		4.0	5.0		4.0	5.0	
Lead/Lag			Lead				Lead	Lead		Lag	Lag	
Lead-Lag Optimize?			Yes				Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None	None	None	None		None	C-Max		None	C-Max	
Walk Time (s)				7.0	7.0			7.0			7.0	
Flash Dont Walk (s)				24.0	24.0			14.0			18.0	
Pedestrian Calls (#/hr)				5	5			5			5	
Act Effct Green (s)	17.2	17.2	38.8	17.2	17.2		16.7	71.8		27.0	82.2	
Actuated g/C Ratio	0.13	0.13	0.30	0.13	0.13		0.13	0.55		0.21	0.63	
v/c Ratio	0.66	0.20	0.17	0.30	0.42		0.68	0.33		0.48	0.39	
Control Delay	73.5	48.5	11.6	52.2	30.8		52.0	6.6		48.0	8.1	
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	73.5	48.5	11.6	52.2	30.8		52.0	6.6		48.0	8.1	
LOS	E	D	B	D	C		D	A		D	A	
Approach Delay		44.6			37.7			13.1			13.1	
Approach LOS		D			D			B			B	
Queue Length 50th (ft)	78	37	18	42	47		81	20		153	109	
Queue Length 95th (ft)	122	66	46	74	93		152	226		m211	304	
Internal Link Dist (ft)		733			625			3878			753	
Turn Bay Length (ft)	55		85				175			230		
Base Capacity (vph)	292	501	623	364	496		354	2764		367	3153	
Starvation Cap Reductn	0	0	0	0	0		0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0		0	0		0	0	
Storage Cap Reductn	0	0	0	0	0		0	0		0	0	
Reduced v/c Ratio	0.33	0.10	0.14	0.15	0.23		0.44	0.33		0.48	0.39	

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 84 (65%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 80			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.68			
Intersection Signal Delay: 17.1		Intersection LOS: B	
Intersection Capacity Utilization 54.4%		ICU Level of Service A	
Analysis Period (min) 15			

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Harbor Blvd & Valencia Mesa





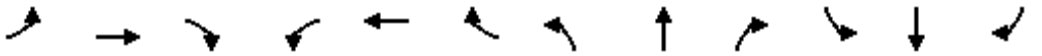
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	90	46	85	51	45	63	147	752	125	167	984	176
Future Volume (veh/h)	90	46	85	51	45	63	147	752	125	167	984	176
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	95	48	89	54	47	66	155	792	132	176	1036	185
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	175	288	407	220	108	152	183	1833	303	561	2769	494
Arrive On Green	0.15	0.15	0.15	0.15	0.15	0.15	0.10	0.42	0.42	0.63	1.00	1.00
Sat Flow, veh/h	1280	1870	1585	1252	704	988	1781	4413	730	1781	4358	777
Grp Volume(v), veh/h	95	48	89	54	0	113	155	610	314	176	809	412
Grp Sat Flow(s),veh/h/ln	1280	1870	1585	1252	0	1692	1781	1702	1739	1781	1702	1731
Q Serve(g_s), s	9.4	2.9	5.7	5.1	0.0	7.9	11.1	16.6	16.8	5.9	0.0	0.0
Cycle Q Clear(g_c), s	17.3	2.9	5.7	8.0	0.0	7.9	11.1	16.6	16.8	5.9	0.0	0.0
Prop In Lane	1.00		1.00	1.00		0.58	1.00		0.42	1.00		0.45
Lane Grp Cap(c), veh/h	175	288	407	220	0	261	183	1414	722	561	2163	1100
V/C Ratio(X)	0.54	0.17	0.22	0.25	0.00	0.43	0.85	0.43	0.44	0.31	0.37	0.37
Avail Cap(c_a), veh/h	322	504	590	365	0	456	356	1414	722	561	2163	1100
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	2.00	2.00	2.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	0.92	0.92	0.92	0.63	0.63	0.63
Uniform Delay (d), s/veh	57.7	47.7	38.0	51.2	0.0	49.8	57.3	27.1	27.1	17.5	0.0	0.0
Incr Delay (d2), s/veh	2.6	0.3	0.3	0.6	0.0	1.1	9.5	0.9	1.8	0.2	0.3	0.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.2	1.4	2.3	1.6	0.0	3.4	5.3	6.6	7.0	2.2	0.1	0.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	60.3	48.0	38.3	51.8	0.0	51.0	66.8	27.9	28.9	17.7	0.3	0.6
LnGrp LOS	E	D	D	D	A	D	E	C	C	B	A	A
Approach Vol, veh/h		232			167			1079			1397	
Approach Delay, s/veh		49.3			51.2			33.8			2.6	
Approach LOS		D			D			C			A	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	46.0	59.0		25.0	17.4	87.6		25.0				
Change Period (Y+Rc), s	5.0	* 5		5.0	4.0	5.0		5.0				
Max Green Setting (Gmax), s	27.0	* 54		35.0	26.0	55.0		35.0				
Max Q Clear Time (g_c+I1), s	7.9	18.8		19.3	13.1	2.0		10.0				
Green Ext Time (p_c), s	0.4	6.1		0.7	0.3	9.9		0.8				

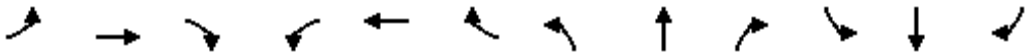
Intersection Summary			
HCM 6th Ctrl Delay		20.9	
HCM 6th LOS		C	

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.



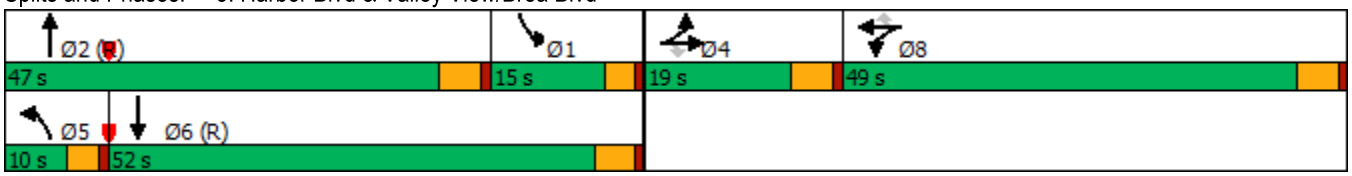
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	51	84	9	539	116	166	22	920	382	57	975	80
Future Volume (vph)	51	84	9	539	116	166	22	920	382	57	975	80
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	85		0	0		0	95		0	150		0
Storage Lanes	1		1	1		1	1		1	1		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00	1.00	0.91	1.00	1.00	0.91	0.91
Frt			0.850			0.850			0.850		0.989	
Flt Protected	0.950			0.950	0.969		0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1681	1715	1583	1770	5085	1583	1770	5029	0
Flt Permitted	0.950			0.950	0.969		0.950			0.950		
Satd. Flow (perm)	1770	1863	1583	1681	1715	1583	1770	5085	1583	1770	5029	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			134			171			346		11	
Link Speed (mph)		25			45			35			50	
Link Distance (ft)		871			1039			280			3958	
Travel Time (s)		23.8			15.7			5.5			54.0	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	53	87	9	556	120	171	23	948	394	59	1005	82
Shared Lane Traffic (%)				40%								
Lane Group Flow (vph)	53	87	9	334	342	171	23	948	394	59	1087	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	
Leading Detector (ft)	20	100	20	20	100	20	20	100	20	20	100	
Trailing Detector (ft)	0	0	0	0	0	0	0	0	0	0	0	
Detector 1 Position(ft)	0	0	0	0	0	0	0	0	0	0	0	
Detector 1 Size(ft)	20	6	20	20	6	20	20	6	20	20	6	
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Split	NA	Perm	Split	NA	Perm	Prot	NA	Free	Prot	NA	
Protected Phases	4	4		8	8		5	2		1	6	
Permitted Phases			4			8			Free			

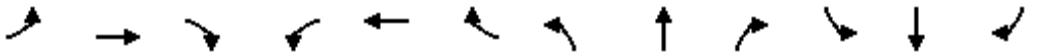


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	4	8	8	8	5	2		1	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0	
Minimum Split (s)	11.0	11.0	11.0	36.0	36.0	36.0	10.0	23.0		10.0	30.0	
Total Split (s)	19.0	19.0	19.0	49.0	49.0	49.0	10.0	47.0		15.0	52.0	
Total Split (%)	14.6%	14.6%	14.6%	37.7%	37.7%	37.7%	7.7%	36.2%		11.5%	40.0%	
Maximum Green (s)	14.0	14.0	14.0	44.0	44.0	44.0	6.0	42.0		11.0	47.0	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	3.0	4.0		3.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	4.0	5.0		4.0	5.0	
Lead/Lag							Lead	Lead		Lag	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None	None	None	None	None	None	C-Max		None	C-Max	
Walk Time (s)				7.0	7.0	7.0		7.0			7.0	
Flash Dont Walk (s)				24.0	24.0	24.0		11.0			18.0	
Pedestrian Calls (#/hr)				5	5	5		5			5	
Act Effct Green (s)	11.1	11.1	11.1	32.8	32.8	32.8	6.8	59.1	130.0	10.0	64.3	
Actuated g/C Ratio	0.09	0.09	0.09	0.25	0.25	0.25	0.05	0.45	1.00	0.08	0.49	
v/c Ratio	0.35	0.55	0.04	0.79	0.79	0.32	0.25	0.41	0.25	0.44	0.44	
Control Delay	61.7	69.7	0.2	58.0	58.1	6.2	79.6	17.8	0.4	49.8	8.6	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	61.7	69.7	0.2	58.0	58.1	6.2	79.6	17.8	0.4	49.8	8.6	
LOS	E	E	A	E	E	A	E	B	A	D	A	
Approach Delay		62.7			47.6			13.8			10.7	
Approach LOS		E			D			B			B	
Queue Length 50th (ft)	43	71	0	276	284	0	20	141	0	53	144	
Queue Length 95th (ft)	84	126	0	355	363	50	48	172	0	102	57	
Internal Link Dist (ft)		791			959			200			3878	
Turn Bay Length (ft)	85						95			150		
Base Capacity (vph)	190	200	290	568	580	648	92	2311	1583	149	2492	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.28	0.43	0.03	0.59	0.59	0.26	0.25	0.41	0.25	0.40	0.44	

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 90			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.79			
Intersection Signal Delay: 23.0		Intersection LOS: C	
Intersection Capacity Utilization 61.9%		ICU Level of Service B	
Analysis Period (min) 15			

Splits and Phases: 3: Harbor Blvd & Valley View/Brea Blvd





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	51	84	9	539	116	166	22	920	382	57	975	80
Future Volume (veh/h)	51	84	9	539	116	166	22	920	382	57	975	80
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	53	87	9	642	0	0	23	948	0	59	1005	82
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	112	118	100	737	0		46	1650		451	2685	219
Arrive On Green	0.06	0.06	0.06	0.21	0.00	0.00	0.03	0.32	0.00	0.25	0.56	0.56
Sat Flow, veh/h	1781	1870	1585	3563	0	1585	1781	5106	1585	1781	4812	392
Grp Volume(v), veh/h	53	87	9	642	0	0	23	948	0	59	710	377
Grp Sat Flow(s),veh/h/ln	1781	1870	1585	1781	0	1585	1781	1702	1585	1781	1702	1800
Q Serve(g_s), s	3.7	5.9	0.7	22.7	0.0	0.0	1.7	20.1	0.0	3.3	15.2	15.2
Cycle Q Clear(g_c), s	3.7	5.9	0.7	22.7	0.0	0.0	1.7	20.1	0.0	3.3	15.2	15.2
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		0.22
Lane Grp Cap(c), veh/h	112	118	100	737	0		46	1650		451	1900	1004
V/C Ratio(X)	0.47	0.74	0.09	0.87	0.00		0.50	0.57		0.13	0.37	0.37
Avail Cap(c_a), veh/h	192	201	171	1206	0		82	1650		451	1900	1004
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00	0.00	0.94	0.94	0.94
Uniform Delay (d), s/veh	58.8	59.9	57.4	49.9	0.0	0.0	62.5	36.6	0.0	37.5	16.0	16.1
Incr Delay (d2), s/veh	3.1	8.7	0.4	4.1	0.0	0.0	8.0	1.5	0.0	0.1	0.5	1.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.8	3.1	0.3	10.2	0.0	0.0	0.9	8.5	0.0	1.4	5.6	6.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	61.9	68.6	57.8	54.0	0.0	0.0	70.4	38.0	0.0	37.6	16.6	17.1
LnGrp LOS	E	E	E	D	A		E	D		D	B	B
Approach Vol, veh/h		149			642			971			1146	
Approach Delay, s/veh		65.6			54.0			38.8			17.8	
Approach LOS		E			D			D			B	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	37.9	47.0		13.2	7.4	77.6		31.9				
Change Period (Y+Rc), s	5.0	* 5		5.0	4.0	5.0		5.0				
Max Green Setting (Gmax), s	11.0	* 42		14.0	6.0	47.0		44.0				
Max Q Clear Time (g_c+I1), s	5.3	22.1		7.9	3.7	17.2		24.7				
Green Ext Time (p_c), s	0.0	6.5		0.3	0.0	7.3		2.2				

Intersection Summary			
HCM 6th Ctrl Delay		35.3	
HCM 6th LOS		D	

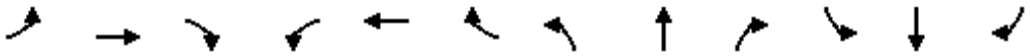
Notes

User approved volume balancing among the lanes for turning movement.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	17	5	23	19	7	50	43	1242	26	36	1584	11
Future Volume (vph)	17	5	23	19	7	50	43	1242	26	36	1584	11
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		65	95		0	100		0
Storage Lanes	0		0	0		1	1		0	1		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.95	0.95
Frt		0.931				0.850		0.997			0.999	
Flt Protected		0.981			0.964		0.950			0.950		
Satd. Flow (prot)	0	1701	0	0	1796	1583	1770	5070	0	1770	3536	0
Flt Permitted		0.867			0.810		0.125			0.189		
Satd. Flow (perm)	0	1504	0	0	1509	1583	233	5070	0	352	3536	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		24				53		6			1	
Link Speed (mph)		25			25			35			30	
Link Distance (ft)		390			645			190			410	
Travel Time (s)		10.6			17.6			3.7			9.3	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	18	5	24	20	7	53	45	1307	27	38	1667	12
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	47	0	0	27	53	45	1334	0	38	1679	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100	20	20	100		20	100	
Trailing Detector (ft)	0	0		0	0	0	0	0		0	0	
Detector 1 Position(ft)	0	0		0	0	0	0	0		0	0	
Detector 1 Size(ft)	20	6		20	6	20	20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		

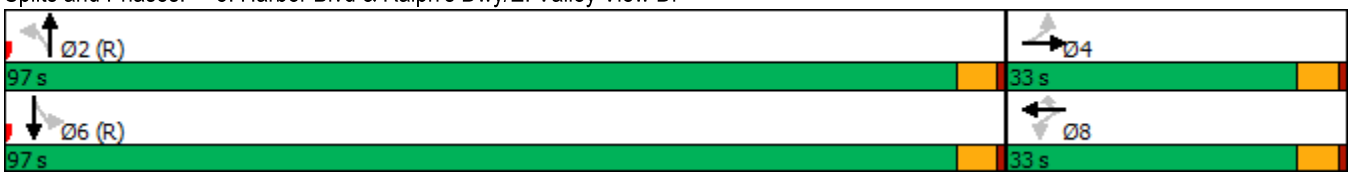


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4		8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0		6.0	6.0	
Minimum Split (s)	33.0	33.0		33.0	33.0	33.0	26.0	26.0		26.0	26.0	
Total Split (s)	33.0	33.0		33.0	33.0	33.0	97.0	97.0		97.0	97.0	
Total Split (%)	25.4%	25.4%		25.4%	25.4%	25.4%	74.6%	74.6%		74.6%	74.6%	
Maximum Green (s)	28.0	28.0		28.0	28.0	28.0	92.0	92.0		92.0	92.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	4.0	4.0		4.0	4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.0			5.0	5.0	5.0	5.0		5.0	5.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None	None	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	21.0	21.0		21.0	21.0	21.0	14.0	14.0		14.0	14.0	
Pedestrian Calls (#/hr)	5	5		5	5	5	5	5		5	5	
Act Effct Green (s)		11.4			11.4	11.4	111.8	111.8		111.8	111.8	
Actuated g/C Ratio		0.09			0.09	0.09	0.86	0.86		0.86	0.86	
v/c Ratio		0.31			0.20	0.28	0.23	0.31		0.13	0.55	
Control Delay		34.5			54.6	15.7	5.6	2.5		1.4	3.0	
Queue Delay		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		34.5			54.6	15.7	5.6	2.5		1.4	3.0	
LOS		C			D	B	A	A		A	A	
Approach Delay		34.5			28.8			2.6			3.0	
Approach LOS		C			C			A			A	
Queue Length 50th (ft)		19			22	0	5	56		0	71	
Queue Length 95th (ft)		50			45	35	m17	101		m7	190	
Internal Link Dist (ft)		310			565			110			330	
Turn Bay Length (ft)						65	95			100		
Base Capacity (vph)		342			325	382	200	4360		302	3040	
Starvation Cap Reductn		0			0	0	0	0		0	116	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.14			0.08	0.14	0.23	0.31		0.13	0.57	

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 98 (75%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green			
Natural Cycle: 75			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.55			
Intersection Signal Delay: 3.9		Intersection LOS: A	
Intersection Capacity Utilization 61.8%		ICU Level of Service B	
Analysis Period (min) 15			

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Harbor Blvd & Ralph's Dwy/E. Valley View Dr

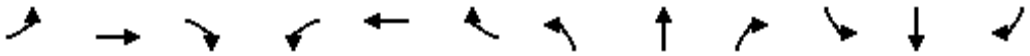


Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	17	5	23	19	7	50	43	1242	26	36	1584	11
Future Volume (veh/h)	17	5	23	19	7	50	43	1242	26	36	1584	11
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	18	5	24	20	7	53	45	1307	27	38	1667	12
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	59	14	36	103	29	79	281	4497	93	396	3159	23
Arrive On Green	0.05	0.05	0.05	0.05	0.05	0.05	0.87	0.87	0.87	0.87	0.87	0.87
Sat Flow, veh/h	411	289	731	1114	590	1585	294	5149	106	410	3617	26
Grp Volume(v), veh/h	47	0	0	27	0	53	45	864	470	38	818	861
Grp Sat Flow(s),veh/h/ln	1431	0	0	1704	0	1585	294	1702	1851	410	1777	1866
Q Serve(g_s), s	2.5	0.0	0.0	0.0	0.0	4.3	5.5	5.6	5.6	2.3	14.1	14.1
Cycle Q Clear(g_c), s	4.3	0.0	0.0	1.9	0.0	4.3	19.6	5.6	5.6	7.8	14.1	14.1
Prop In Lane	0.38		0.51	0.74		1.00	1.00		0.06	1.00		0.01
Lane Grp Cap(c), veh/h	109	0	0	133	0	79	281	2973	1617	396	1552	1630
V/C Ratio(X)	0.43	0.00	0.00	0.20	0.00	0.67	0.16	0.29	0.29	0.10	0.53	0.53
Avail Cap(c_a), veh/h	359	0	0	378	0	341	281	2973	1617	396	1552	1630
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	60.8	0.0	0.0	59.6	0.0	60.7	4.2	1.4	1.4	2.1	1.9	1.9
Incr Delay (d2), s/veh	2.7	0.0	0.0	0.7	0.0	9.6	1.2	0.2	0.5	0.5	1.3	1.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.6	0.0	0.0	0.9	0.0	1.9	0.4	1.0	1.1	0.2	3.1	3.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	63.4	0.0	0.0	60.3	0.0	70.3	5.5	1.6	1.9	2.5	3.2	3.2
LnGrp LOS	E	A	A	E	A	E	A	A	A	A	A	A
Approach Vol, veh/h		47			80			1379			1717	
Approach Delay, s/veh		63.4			66.9			1.8			3.2	
Approach LOS		E			E			A			A	

Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		118.5		11.5		118.5		11.5				
Change Period (Y+Rc), s		5.0		5.0		5.0		5.0				
Max Green Setting (Gmax), s		92.0		28.0		92.0		28.0				
Max Q Clear Time (g_c+I1), s		21.6		6.3		16.1		6.3				
Green Ext Time (p_c), s		15.0		0.2		24.9		0.2				

Intersection Summary												
HCM 6th Ctrl Delay			5.1									
HCM 6th LOS			A									

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	17	176	73	50	165	408	101	887	41	260	1290	19
Future Volume (vph)	17	176	73	50	165	408	101	887	41	260	1290	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	130		135	200		0	205		0	300		165
Storage Lanes	1		1	1		1	1		0	2		1
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.97	0.95	1.00
Frt			0.850			0.850		0.993				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1770	1863	1583	1770	3514	0	3433	3539	1583
Flt Permitted	0.401			0.367			0.950			0.950		
Satd. Flow (perm)	747	1863	1583	684	1863	1583	1770	3514	0	3433	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			101			77		5				50
Link Speed (mph)		35			35			35			30	
Link Distance (ft)		538			720			654			541	
Travel Time (s)		10.5			14.0			12.7			12.3	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	17	180	74	51	168	416	103	905	42	265	1316	19
Shared Lane Traffic (%)												
Lane Group Flow (vph)	17	180	74	51	168	416	103	947	0	265	1316	19
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes										
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2	1	1	2	1	1	2		1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (ft)	20	100	20	20	100	20	20	100		20	100	20
Trailing Detector (ft)	0	0	0	0	0	0	0	0		0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0	0	0		0	0	0
Detector 1 Size(ft)	20	6	20	20	6	20	20	6		20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	Perm	Perm	NA	pm+ov	Prot	NA		Prot	NA	Perm
Protected Phases		4			8	1	5	2		1	6	
Permitted Phases	4		4	8		8						6

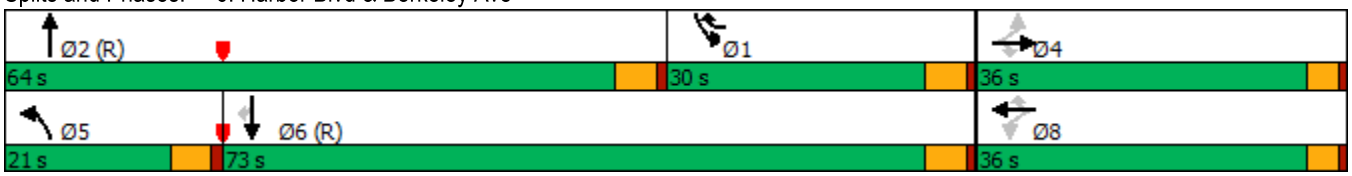


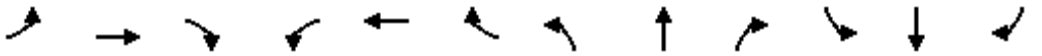
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	4	8	8	1	5	2		1	6	6
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0	6.0
Minimum Split (s)	29.0	29.0	29.0	35.0	35.0	11.0	11.0	30.0		11.0	30.0	30.0
Total Split (s)	36.0	36.0	36.0	36.0	36.0	30.0	21.0	64.0		30.0	73.0	73.0
Total Split (%)	27.7%	27.7%	27.7%	27.7%	27.7%	23.1%	16.2%	49.2%		23.1%	56.2%	56.2%
Maximum Green (s)	32.0	32.0	32.0	32.0	32.0	25.0	16.0	59.0		25.0	68.0	68.0
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	4.0	4.0	4.0		4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	5.0	5.0	5.0		5.0	5.0	5.0
Lead/Lag						Lag	Lead	Lead		Lag	Lag	Lag
Lead-Lag Optimize?						Yes	Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max		None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0			7.0			7.0	7.0
Flash Dont Walk (s)	18.0	18.0	18.0	24.0	24.0			18.0			18.0	18.0
Pedestrian Calls (#/hr)	5	5	5	5	5			5			5	5
Act Effct Green (s)	19.3	19.3	19.3	19.3	19.3	48.3	12.7	71.7		25.0	84.0	84.0
Actuated g/C Ratio	0.15	0.15	0.15	0.15	0.15	0.37	0.10	0.55		0.19	0.65	0.65
v/c Ratio	0.15	0.65	0.23	0.50	0.61	0.65	0.60	0.49		0.40	0.58	0.02
Control Delay	47.6	62.3	4.9	65.8	60.1	31.7	69.8	19.9		37.3	9.3	0.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.1	0.0
Total Delay	47.6	62.3	4.9	65.8	60.1	31.7	69.8	19.9		37.3	9.4	0.7
LOS	D	E	A	E	E	C	E	B		D	A	A
Approach Delay		45.7			41.9			24.8			13.9	
Approach LOS		D			D			C			B	
Queue Length 50th (ft)	13	147	0	41	136	242	84	239		85	93	0
Queue Length 95th (ft)	33	200	21	78	188	302	142	368		121	427	m1
Internal Link Dist (ft)		458			640			574			461	
Turn Bay Length (ft)	130		135	200			205			300		165
Base Capacity (vph)	183	458	465	168	458	636	220	1941		660	2287	1040
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	168	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	0
Reduced v/c Ratio	0.09	0.39	0.16	0.30	0.37	0.65	0.47	0.49		0.40	0.62	0.02

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 122 (94%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 80			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.65			
Intersection Signal Delay: 24.6		Intersection LOS: C	
Intersection Capacity Utilization 70.5%		ICU Level of Service C	
Analysis Period (min) 15			

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Harbor Blvd & Berkeley Ave

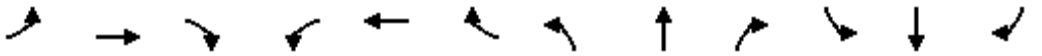




Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	17	176	73	50	165	408	101	887	41	260	1290	19
Future Volume (veh/h)	17	176	73	50	165	408	101	887	41	260	1290	19
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	17	180	74	51	168	416	103	905	42	265	1316	19
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	124	309	262	141	309	695	127	1569	73	944	2330	1039
Arrive On Green	0.17	0.17	0.17	0.17	0.17	0.17	0.07	0.45	0.45	0.27	0.66	0.66
Sat Flow, veh/h	831	1870	1585	1126	1870	1585	1781	3458	160	3456	3554	1585
Grp Volume(v), veh/h	17	180	74	51	168	416	103	465	482	265	1316	19
Grp Sat Flow(s),veh/h/ln	831	1870	1585	1126	1870	1585	1781	1777	1841	1728	1777	1585
Q Serve(g_s), s	2.5	11.6	5.3	5.7	10.7	0.0	7.4	25.2	25.2	7.8	26.3	0.5
Cycle Q Clear(g_c), s	13.2	11.6	5.3	17.3	10.7	0.0	7.4	25.2	25.2	7.8	26.3	0.5
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.09	1.00		1.00
Lane Grp Cap(c), veh/h	124	309	262	141	309	695	127	806	836	944	2330	1039
V/C Ratio(X)	0.14	0.58	0.28	0.36	0.54	0.60	0.81	0.58	0.58	0.28	0.56	0.02
Avail Cap(c_a), veh/h	191	460	390	232	460	823	219	806	836	944	2330	1039
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	55.8	50.1	47.5	58.1	49.8	27.8	59.5	26.3	26.3	37.2	12.2	7.8
Incr Delay (d2), s/veh	0.5	1.7	0.6	1.5	1.5	0.9	11.5	3.0	2.9	0.2	1.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.5	5.5	2.1	1.7	5.1	9.8	3.7	11.1	11.5	3.4	10.3	0.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	56.3	51.9	48.1	59.6	51.3	28.7	70.9	29.3	29.2	37.3	13.2	7.8
LnGrp LOS	E	D	D	E	D	C	E	C	C	D	B	A
Approach Vol, veh/h		271			635			1050			1600	
Approach Delay, s/veh		51.1			37.1			33.3			17.2	
Approach LOS		D			D			C			B	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	40.5	64.0		25.5	14.3	90.2		25.5				
Change Period (Y+Rc), s	5.0	5.0		4.0	5.0	5.0		4.0				
Max Green Setting (Gmax), s	25.0	59.0		32.0	16.0	68.0		32.0				
Max Q Clear Time (g_c+I1), s	9.8	27.2		15.2	9.4	28.3		19.3				
Green Ext Time (p_c), s	0.8	6.8		1.1	0.1	13.7		2.2				

Intersection Summary			
HCM 6th Ctrl Delay		28.1	
HCM 6th LOS		C	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	293	1114	125	133	833	300	189	1079	203	321	796	338
Future Volume (vph)	293	1114	125	133	833	300	189	1079	203	321	796	338
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	325		0	193		145	230		150	275		0
Storage Lanes	1		0	2		1	2		1	2		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	0.91	0.91	0.97	0.91	1.00	0.97	0.91	1.00	0.97	0.91	0.91
Frt		0.985				0.850			0.850		0.955	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	5009	0	3433	5085	1583	3433	5085	1583	3433	4856	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	5009	0	3433	5085	1583	3433	5085	1583	3433	4856	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		18				248			141		84	
Link Speed (mph)		40			40			45			50	
Link Distance (ft)		1043			981			833			926	
Travel Time (s)		17.8			16.7			12.6			12.6	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	302	1148	129	137	859	309	195	1112	209	331	821	348
Shared Lane Traffic (%)												
Lane Group Flow (vph)	302	1277	0	137	859	309	195	1112	209	331	1169	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		24			24			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2	1	1	2	1	1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru	Right	Left	Thru	
Leading Detector (ft)	20	100		20	100	20	20	100	20	20	100	
Trailing Detector (ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Position(ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Size(ft)	20	6		20	6	20	20	6	20	20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases						8			2			

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	7	4		3	8	8	5	2	2	1	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Minimum Split (s)	10.0	43.0		10.0	43.0	43.0	10.0	43.0	43.0	10.0	36.0	
Total Split (s)	27.0	57.0		13.0	43.0	43.0	16.0	43.0	43.0	17.0	44.0	
Total Split (%)	20.8%	43.8%		10.0%	33.1%	33.1%	12.3%	33.1%	33.1%	13.1%	33.8%	
Maximum Green (s)	23.0	52.0		9.0	38.0	38.0	12.0	38.0	38.0	13.0	39.0	
Yellow Time (s)	3.0	4.0		3.0	4.0	4.0	3.0	4.0	4.0	3.0	4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	4.0	5.0		4.0	5.0	5.0	4.0	5.0	5.0	4.0	5.0	
Lead/Lag	Lag	Lag		Lead	Lead	Lead	Lag	Lag	Lag	Lead	Lead	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None		None	None	None	None	C-Max	C-Max	None	C-Max	
Walk Time (s)		7.0			7.0	7.0		7.0	7.0		7.0	
Flash Dont Walk (s)		31.0			31.0	31.0		31.0	31.0		24.0	
Pedestrian Calls (#/hr)		5			5	5		5	5		5	
Act Effct Green (s)	26.4	48.9		8.7	31.2	31.2	12.0	40.0	40.0	14.4	42.4	
Actuated g/C Ratio	0.20	0.38		0.07	0.24	0.24	0.09	0.31	0.31	0.11	0.33	
v/c Ratio	0.84	0.67		0.60	0.70	0.54	0.62	0.71	0.36	0.87	0.71	
Control Delay	70.5	35.1		70.1	48.2	12.7	45.7	24.5	3.9	80.0	39.2	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	70.5	35.1		70.1	48.2	12.7	45.7	24.5	3.9	80.0	39.2	
LOS	E	D		E	D	B	D	C	A	F	D	
Approach Delay		41.9			42.1			24.4			48.2	
Approach LOS		D			D			C			D	
Queue Length 50th (ft)	239	309		58	243	41	85	332	18	145	307	
Queue Length 95th (ft)	#438	360		93	272	121	m118	182	6	#241	364	
Internal Link Dist (ft)		963			901			753			846	
Turn Bay Length (ft)	325			193		145	230		150	275		
Base Capacity (vph)	360	2014		237	1486	638	316	1563	584	380	1639	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.84	0.63		0.58	0.58	0.48	0.62	0.71	0.36	0.87	0.71	

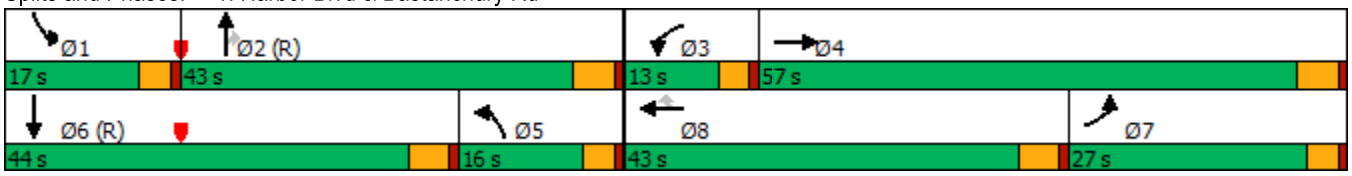
Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 5 (4%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 120			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.87			
Intersection Signal Delay: 39.0		Intersection LOS: D	
Intersection Capacity Utilization 77.3%		ICU Level of Service D	
Analysis Period (min) 15			

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Harbor Blvd & Bastanchury Rd



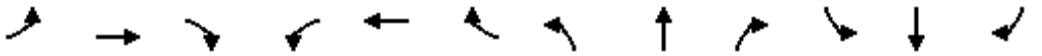
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	293	1114	125	133	833	300	189	1079	203	321	796	338
Future Volume (veh/h)	293	1114	125	133	833	300	189	1079	203	321	796	338
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	302	1148	129	137	859	309	195	1112	209	331	821	348
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	315	1629	183	188	1122	348	517	1824	566	346	1057	445
Arrive On Green	0.18	0.35	0.35	0.05	0.22	0.22	0.30	0.71	0.71	0.10	0.30	0.30
Sat Flow, veh/h	1781	4657	523	3456	5106	1585	3456	5106	1585	3456	3522	1485
Grp Volume(v), veh/h	302	839	438	137	859	309	195	1112	209	331	793	376
Grp Sat Flow(s),veh/h/ln	1781	1702	1776	1728	1702	1585	1728	1702	1585	1728	1702	1603
Q Serve(g_s), s	21.8	27.6	27.7	5.1	20.5	19.2	5.8	14.3	6.6	12.4	27.7	27.9
Cycle Q Clear(g_c), s	21.8	27.6	27.7	5.1	20.5	19.2	5.8	14.3	6.6	12.4	27.7	27.9
Prop In Lane	1.00		0.29	1.00		1.00	1.00		1.00	1.00		0.93
Lane Grp Cap(c), veh/h	315	1191	621	188	1122	348	517	1824	566	346	1021	481
V/C Ratio(X)	0.96	0.70	0.71	0.73	0.77	0.89	0.38	0.61	0.37	0.96	0.78	0.78
Avail Cap(c_a), veh/h	315	1362	710	239	1493	463	517	1824	566	346	1021	481
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	2.00	2.00	2.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	0.86	0.86	0.86	1.00	1.00	1.00
Uniform Delay (d), s/veh	53.0	36.5	36.5	60.5	47.6	30.2	40.8	14.0	12.9	58.2	41.5	41.6
Incr Delay (d2), s/veh	39.5	1.4	2.7	7.9	1.7	15.0	0.4	1.3	1.6	37.3	5.8	11.9
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	13.0	11.5	12.2	2.4	8.7	8.6	2.3	3.8	2.2	7.0	11.9	12.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	92.5	37.9	39.2	68.4	49.3	45.2	41.2	15.3	14.5	95.5	47.3	53.5
LnGrp LOS	F	D	D	E	D	D	D	B	B	F	D	D
Approach Vol, veh/h		1579			1305			1516			1500	
Approach Delay, s/veh		48.7			50.3			18.5			59.5	
Approach LOS		D			D			B			E	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	17.0	51.4	11.1	50.5	24.4	44.0	28.0	33.6
Change Period (Y+Rc), s	4.0	5.0	4.0	5.0	5.0	* 5	5.0	* 5
Max Green Setting (Gmax), s	13.0	38.0	9.0	52.0	12.0	* 39	23.0	* 38
Max Q Clear Time (g_c+I1), s	14.4	16.3	7.1	29.7	7.8	29.9	23.8	22.5
Green Ext Time (p_c), s	0.0	8.3	0.1	8.8	0.2	4.6	0.0	6.0

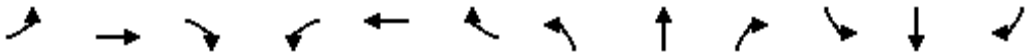
Intersection Summary	
HCM 6th Ctrl Delay	44.1
HCM 6th LOS	D

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	119	22	132	71	41	199	48	1153	54	86	881	82
Future Volume (vph)	119	22	132	71	41	199	48	1153	54	86	881	82
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	55		85	0		0	175		0	230		0
Storage Lanes	1		1	1		0	1		0	1		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.91	0.91
Frt			0.850		0.876			0.993			0.987	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1770	1632	0	1770	5050	0	1770	5019	0
Flt Permitted	0.293			0.742			0.950			0.950		
Satd. Flow (perm)	546	1863	1583	1382	1632	0	1770	5050	0	1770	5019	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			82		201			7			16	
Link Speed (mph)		30			30			50			45	
Link Distance (ft)		813			705			3958			833	
Travel Time (s)		18.5			16.0			54.0			12.6	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	125	23	139	75	43	209	51	1214	57	91	927	86
Shared Lane Traffic (%)												
Lane Group Flow (vph)	125	23	139	75	252	0	51	1271	0	91	1013	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2	1	1	2		1	2		1	2	
Detector Template	Left	Thru	Right	Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100	20	20	100		20	100		20	100	
Trailing Detector (ft)	0	0	0	0	0		0	0		0	0	
Detector 1 Position(ft)	0	0	0	0	0		0	0		0	0	
Detector 1 Size(ft)	20	6	20	20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	pm+ov	Perm	NA		Prot	NA		Prot	NA	
Protected Phases		4	5		8		5	2		1	6	
Permitted Phases	4		4	8								



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	5	8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0		6.0	6.0		6.0	6.0	
Minimum Split (s)	11.0	11.0	10.0	36.0	36.0		10.0	26.0		10.0	30.0	
Total Split (s)	48.0	48.0	15.0	48.0	48.0		15.0	61.0		21.0	67.0	
Total Split (%)	36.9%	36.9%	11.5%	36.9%	36.9%		11.5%	46.9%		16.2%	51.5%	
Maximum Green (s)	43.0	43.0	11.0	43.0	43.0		11.0	56.0		17.0	62.0	
Yellow Time (s)	4.0	4.0	3.0	4.0	4.0		3.0	4.0		3.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.0	5.0	4.0	5.0	5.0		4.0	5.0		4.0	5.0	
Lead/Lag			Lag				Lag	Lead		Lag	Lead	
Lead-Lag Optimize?			Yes				Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None	None	None	None		None	C-Max		None	C-Max	
Walk Time (s)				7.0	7.0			7.0			7.0	
Flash Dont Walk (s)				24.0	24.0			14.0			18.0	
Pedestrian Calls (#/hr)				5	5			5			5	
Act Effct Green (s)	23.9	23.9	37.9	23.9	23.9		9.0	77.1		15.0	83.1	
Actuated g/C Ratio	0.18	0.18	0.29	0.18	0.18		0.07	0.59		0.12	0.64	
v/c Ratio	1.25	0.07	0.27	0.30	0.54		0.42	0.42		0.45	0.32	
Control Delay	213.3	39.9	14.4	46.0	14.7		72.1	3.0		55.0	5.8	
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	213.3	39.9	14.4	46.0	14.7		72.1	3.0		55.0	5.8	
LOS	F	D	B	D	B		E	A		D	A	
Approach Delay		103.1			21.9			5.7			9.8	
Approach LOS		F			C			A			A	
Queue Length 50th (ft)	~131	16	35	55	37		45	23		75	54	
Queue Length 95th (ft)	#222	38	76	93	107		m89	39		m112	80	
Internal Link Dist (ft)		733			625			3878			753	
Turn Bay Length (ft)	55		85				175			230		
Base Capacity (vph)	180	616	512	457	674		149	2996		231	3213	
Starvation Cap Reductn	0	0	0	0	0		0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0		0	0		0	0	
Storage Cap Reductn	0	0	0	0	0		0	0		0	0	
Reduced v/c Ratio	0.69	0.04	0.27	0.16	0.37		0.34	0.42		0.39	0.32	

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 2 (2%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 80			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 1.25			
Intersection Signal Delay: 18.1		Intersection LOS: B	
Intersection Capacity Utilization 65.3%		ICU Level of Service C	
Analysis Period (min) 15			

~ Volume exceeds capacity, queue is theoretically infinite.

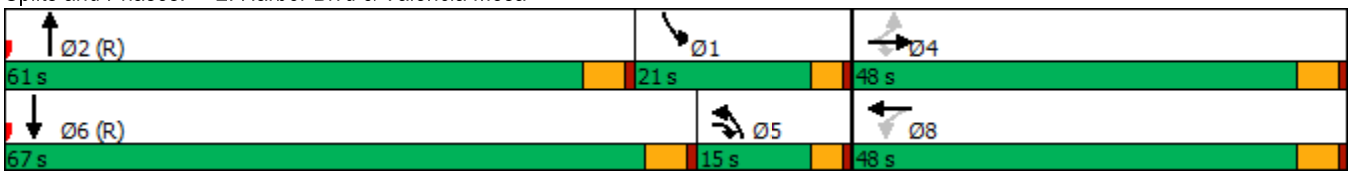
Queue shown is maximum after two cycles.

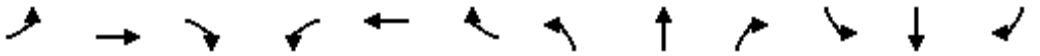
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Harbor Blvd & Valencia Mesa

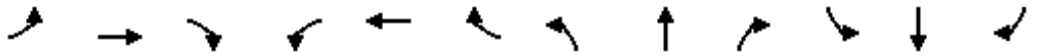




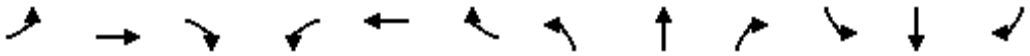
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	119	22	132	71	41	199	48	1153	54	86	881	82
Future Volume (veh/h)	119	22	132	71	41	199	48	1153	54	86	881	82
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	125	23	139	75	43	209	51	1214	57	91	927	86
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	201	494	658	368	73	357	269	2153	101	351	2268	210
Arrive On Green	0.26	0.26	0.26	0.26	0.26	0.26	0.15	0.43	0.43	0.39	0.95	0.95
Sat Flow, veh/h	1128	1870	1585	1224	278	1350	1781	4998	235	1781	4755	440
Grp Volume(v), veh/h	125	23	139	75	0	252	51	827	444	91	663	350
Grp Sat Flow(s),veh/h/ln	1128	1870	1585	1224	0	1627	1781	1702	1828	1781	1702	1791
Q Serve(g_s), s	14.1	1.2	0.0	6.3	0.0	17.5	3.3	23.7	23.8	4.5	1.9	1.9
Cycle Q Clear(g_c), s	31.6	1.2	0.0	7.5	0.0	17.5	3.3	23.7	23.8	4.5	1.9	1.9
Prop In Lane	1.00		1.00	1.00		0.83	1.00		0.13	1.00		0.25
Lane Grp Cap(c), veh/h	201	494	658	368	0	430	269	1466	788	351	1623	854
V/C Ratio(X)	0.62	0.05	0.21	0.20	0.00	0.59	0.19	0.56	0.56	0.26	0.41	0.41
Avail Cap(c_a), veh/h	276	619	764	449	0	538	269	1466	788	351	1623	854
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	2.00	2.00	2.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	0.88	0.88	0.88	0.65	0.65	0.65
Uniform Delay (d), s/veh	55.4	35.6	24.4	38.4	0.0	41.6	48.2	27.8	27.8	32.9	1.6	1.6
Incr Delay (d2), s/veh	3.1	0.0	0.2	0.3	0.0	1.3	0.3	1.4	2.6	0.3	0.5	0.9
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	4.2	0.6	2.8	1.9	0.0	7.2	1.4	9.4	10.4	1.8	0.6	0.7
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	58.5	35.7	24.5	38.7	0.0	42.9	48.5	29.2	30.4	33.2	2.1	2.6
LnGrp LOS	E	D	C	D	A	D	D	C	C	C	A	A
Approach Vol, veh/h		287			327			1322			1104	
Approach Delay, s/veh		40.2			41.9			30.3			4.8	
Approach LOS		D			D			C			A	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	29.6	61.0		39.4	23.6	67.0		39.4				
Change Period (Y+Rc), s	4.0	5.0		5.0	4.0	5.0		5.0				
Max Green Setting (Gmax), s	17.0	56.0		43.0	11.0	62.0		43.0				
Max Q Clear Time (g_c+I1), s	6.5	25.8		33.6	5.3	3.9		19.5				
Green Ext Time (p_c), s	0.1	8.9		0.7	0.0	7.5		1.9				

Intersection Summary												
HCM 6th Ctrl Delay			23.3									
HCM 6th LOS			C									



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	68	179	26	469	97	111	9	1024	605	97	1000	50
Future Volume (vph)	68	179	26	469	97	111	9	1024	605	97	1000	50
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	85		0	0		0	95		0	150		0
Storage Lanes	1		1	1		1	1		1	1		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00	1.00	0.91	1.00	1.00	0.91	0.91
Frt			0.850			0.850			0.850		0.993	
Flt Protected	0.950			0.950	0.968		0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1681	1713	1583	1770	5085	1583	1770	5050	0
Flt Permitted	0.950			0.950	0.968		0.950			0.950		
Satd. Flow (perm)	1770	1863	1583	1681	1713	1583	1770	5085	1583	1770	5050	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			134			134			491		7	
Link Speed (mph)		25			45			35			50	
Link Distance (ft)		871			1039			280			3958	
Travel Time (s)		23.8			15.7			5.5			54.0	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	70	185	27	484	100	114	9	1056	624	100	1031	52
Shared Lane Traffic (%)				40%								
Lane Group Flow (vph)	70	185	27	290	294	114	9	1056	624	100	1083	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	
Leading Detector (ft)	20	100	20	20	100	20	20	100	20	20	100	
Trailing Detector (ft)	0	0	0	0	0	0	0	0	0	0	0	
Detector 1 Position(ft)	0	0	0	0	0	0	0	0	0	0	0	
Detector 1 Size(ft)	20	6	20	20	6	20	20	6	20	20	6	
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Split	NA	Perm	Split	NA	Perm	Prot	NA	Free	Prot	NA	
Protected Phases	4	4		8	8		5	2		1	6	
Permitted Phases			4			8			Free			

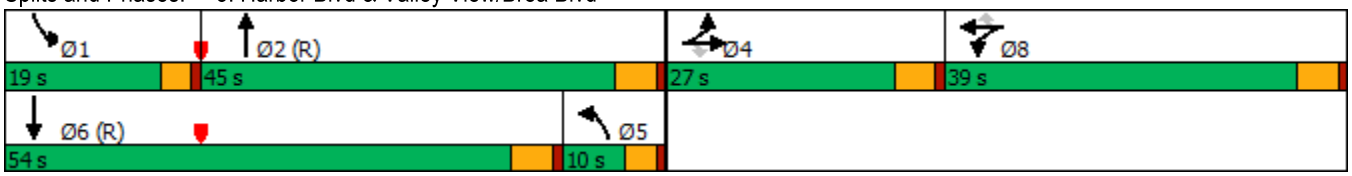


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	4	8	8	8	5	2		1	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0	
Minimum Split (s)	11.0	11.0	11.0	36.0	36.0	36.0	10.0	23.0		10.0	30.0	
Total Split (s)	27.0	27.0	27.0	39.0	39.0	39.0	10.0	45.0		19.0	54.0	
Total Split (%)	20.8%	20.8%	20.8%	30.0%	30.0%	30.0%	7.7%	34.6%		14.6%	41.5%	
Maximum Green (s)	22.0	22.0	22.0	34.0	34.0	34.0	6.0	40.0		15.0	49.0	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	3.0	4.0		3.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	4.0	5.0		4.0	5.0	
Lead/Lag							Lag	Lag		Lead	Lead	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None	None	None	None	None	None	C-Max		None	C-Max	
Walk Time (s)				7.0	7.0	7.0		7.0			7.0	
Flash Dont Walk (s)				24.0	24.0	24.0		11.0			18.0	
Pedestrian Calls (#/hr)				5	5	5		5			5	
Act Effct Green (s)	17.6	17.6	17.6	27.8	27.8	27.8	6.0	53.5	130.0	12.1	67.6	
Actuated g/C Ratio	0.14	0.14	0.14	0.21	0.21	0.21	0.05	0.41	1.00	0.09	0.52	
v/c Ratio	0.29	0.73	0.08	0.81	0.80	0.26	0.11	0.50	0.39	0.61	0.41	
Control Delay	52.5	70.6	0.5	65.6	64.9	5.3	50.8	21.3	0.8	78.7	13.5	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	52.5	70.6	0.5	65.6	64.9	5.3	50.8	21.3	0.8	78.7	13.5	
LOS	D	E	A	E	E	A	D	C	A	E	B	
Approach Delay		59.4			55.4			13.9			19.1	
Approach LOS		E			E			B			B	
Queue Length 50th (ft)	54	151	0	245	248	0	7	230	4	87	134	
Queue Length 95th (ft)	98	227	0	337	341	33	m18	276	13	126	236	
Internal Link Dist (ft)		791			959			200			3878	
Turn Bay Length (ft)	85						95			150		
Base Capacity (vph)	299	315	379	439	448	512	81	2092	1583	204	2630	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.23	0.59	0.07	0.66	0.66	0.22	0.11	0.50	0.39	0.49	0.41	

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 72 (55%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 90			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.81			
Intersection Signal Delay: 26.4		Intersection LOS: C	
Intersection Capacity Utilization 66.2%		ICU Level of Service C	
Analysis Period (min) 15			

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Harbor Blvd & Valley View/Brea Blvd



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	68	179	26	469	97	111	9	1024	605	97	1000	50
Future Volume (veh/h)	68	179	26	469	97	111	9	1024	605	97	1000	50
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	70	185	27	555	0	0	9	1056	0	100	1031	52
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	210	221	187	639	0		306	2486		124	1876	95
Arrive On Green	0.12	0.12	0.12	0.18	0.00	0.00	0.17	0.49	0.00	0.07	0.38	0.38
Sat Flow, veh/h	1781	1870	1585	3563	0	1585	1781	5106	1585	1781	4978	251
Grp Volume(v), veh/h	70	185	27	555	0	0	9	1056	0	100	705	378
Grp Sat Flow(s),veh/h/ln	1781	1870	1585	1781	0	1585	1781	1702	1585	1781	1702	1825
Q Serve(g_s), s	4.7	12.6	2.0	19.7	0.0	0.0	0.5	17.4	0.0	7.2	21.1	21.2
Cycle Q Clear(g_c), s	4.7	12.6	2.0	19.7	0.0	0.0	0.5	17.4	0.0	7.2	21.1	21.2
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		0.14
Lane Grp Cap(c), veh/h	210	221	187	639	0		306	2486		124	1283	688
V/C Ratio(X)	0.33	0.84	0.14	0.87	0.00		0.03	0.42		0.81	0.55	0.55
Avail Cap(c_a), veh/h	301	317	268	932	0		306	2486		206	1283	688
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00	0.00	0.96	0.96	0.96
Uniform Delay (d), s/veh	52.6	56.1	51.4	51.8	0.0	0.0	44.8	21.6	0.0	59.6	31.8	31.8
Incr Delay (d2), s/veh	0.9	12.5	0.3	6.2	0.0	0.0	0.0	0.5	0.0	11.2	1.6	3.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.2	6.8	0.8	9.1	0.0	0.0	0.2	7.0	0.0	3.5	8.6	9.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	53.5	68.6	51.8	58.0	0.0	0.0	44.9	22.1	0.0	70.9	33.4	34.9
LnGrp LOS	D	E	D	E	A		D	C		E	C	C
Approach Vol, veh/h		282			555			1065			1183	
Approach Delay, s/veh		63.2			58.0			22.3			37.1	
Approach LOS		E			E			C			D	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	13.0	68.3		20.4	27.3	54.0		28.3				
Change Period (Y+Rc), s	4.0	5.0		5.0	5.0	* 5		5.0				
Max Green Setting (Gmax), s	15.0	40.0		22.0	6.0	* 49		34.0				
Max Q Clear Time (g_c+I1), s	9.2	19.4		14.6	2.5	23.2		21.7				
Green Ext Time (p_c), s	0.1	7.4		0.8	0.0	6.9		1.6				

Intersection Summary			
HCM 6th Ctrl Delay		38.1	
HCM 6th LOS		D	

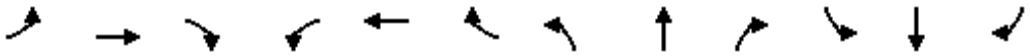
Notes

User approved volume balancing among the lanes for turning movement.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	47	21	65	14	11	43	60	1608	24	48	1455	42
Future Volume (vph)	47	21	65	14	11	43	60	1608	24	48	1455	42
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		65	95		0	100		0
Storage Lanes	0		0	0		1	1		0	1		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.95	0.95
Frt		0.934				0.850		0.998			0.996	
Flt Protected		0.983			0.973		0.950			0.950		
Satd. Flow (prot)	0	1710	0	0	1812	1583	1770	5075	0	1770	3525	0
Flt Permitted		0.872			0.775		0.143			0.125		
Satd. Flow (perm)	0	1517	0	0	1444	1583	266	5075	0	233	3525	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		34				44		4			5	
Link Speed (mph)		25			25			35			35	
Link Distance (ft)		390			645			190			410	
Travel Time (s)		10.6			17.6			3.7			8.0	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	48	21	66	14	11	44	61	1641	24	49	1485	43
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	135	0	0	25	44	61	1665	0	49	1528	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60		60	60		60	60		60	60		60
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100	20	20	100		20	100	
Trailing Detector (ft)	0	0		0	0	0	0	0		0	0	
Detector 1 Position(ft)	0	0		0	0	0	0	0		0	0	
Detector 1 Size(ft)	20	6		20	6	20	20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4		8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0		6.0	6.0	
Minimum Split (s)	33.0	33.0		33.0	33.0	33.0	26.0	26.0		26.0	26.0	
Total Split (s)	35.0	35.0		35.0	35.0	35.0	95.0	95.0		95.0	95.0	
Total Split (%)	26.9%	26.9%		26.9%	26.9%	26.9%	73.1%	73.1%		73.1%	73.1%	
Maximum Green (s)	30.0	30.0		30.0	30.0	30.0	90.0	90.0		90.0	90.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	4.0	4.0		4.0	4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.0			5.0	5.0	5.0	5.0		5.0	5.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None	None	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	21.0	21.0		21.0	21.0	21.0	14.0	14.0		14.0	14.0	
Pedestrian Calls (#/hr)	5	5		5	5	5	5	5		5	5	
Act Effct Green (s)		15.5			15.5	15.5	104.5	104.5		104.5	104.5	
Actuated g/C Ratio		0.12			0.12	0.12	0.80	0.80		0.80	0.80	
v/c Ratio		0.64			0.15	0.19	0.29	0.41		0.26	0.54	
Control Delay		53.0			49.2	14.4	6.0	3.2		3.5	2.4	
Queue Delay		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		53.0			49.2	14.4	6.0	3.2		3.5	2.4	
LOS		D			D	B	A	A		A	A	
Approach Delay		53.0			27.0			3.3			2.4	
Approach LOS		D			C			A			A	
Queue Length 50th (ft)		83			19	0	8	91		0	31	
Queue Length 95th (ft)		136			42	32	m16	120		m1	88	
Internal Link Dist (ft)		310			565			110			330	
Turn Bay Length (ft)						65	95			100		
Base Capacity (vph)		376			333	399	213	4081		187	2835	
Starvation Cap Reductn		0			0	0	0	0		0	50	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.36			0.08	0.11	0.29	0.41		0.26	0.55	

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 32 (25%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green			
Natural Cycle: 70			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.64			
Intersection Signal Delay: 5.3		Intersection LOS: A	
Intersection Capacity Utilization 72.6%		ICU Level of Service C	
Analysis Period (min) 15			

m Volume for 95th percentile queue is metered by upstream signal.

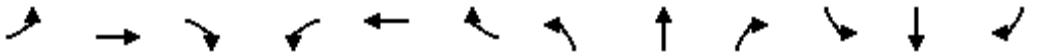
Splits and Phases: 5: Harbor Blvd & Ralph's Dwy/E. Valley View Dr



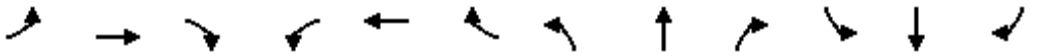
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	47	21	65	14	11	43	60	1608	24	48	1455	42
Future Volume (veh/h)	47	21	65	14	11	43	60	1608	24	48	1455	42
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	48	21	66	14	11	44	61	1641	24	49	1485	43
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	86	35	80	110	75	165	289	4247	62	275	2889	84
Arrive On Green	0.10	0.10	0.10	0.10	0.10	0.10	0.82	0.82	0.82	0.82	0.82	0.82
Sat Flow, veh/h	469	336	770	643	722	1585	341	5185	76	298	3527	102
Grp Volume(v), veh/h	135	0	0	25	0	44	61	1077	588	49	747	781
Grp Sat Flow(s),veh/h/ln	1576	0	0	1365	0	1585	341	1702	1857	298	1777	1852
Q Serve(g_s), s	8.9	0.0	0.0	0.0	0.0	3.3	8.9	10.9	10.9	6.8	17.1	17.1
Cycle Q Clear(g_c), s	10.9	0.0	0.0	1.6	0.0	3.3	26.0	10.9	10.9	17.6	17.1	17.1
Prop In Lane	0.36		0.49	0.56		1.00	1.00		0.04	1.00		0.06
Lane Grp Cap(c), veh/h	201	0	0	185	0	165	289	2788	1521	275	1455	1517
V/C Ratio(X)	0.67	0.00	0.00	0.14	0.00	0.27	0.21	0.39	0.39	0.18	0.51	0.51
Avail Cap(c_a), veh/h	398	0	0	376	0	366	289	2788	1521	275	1455	1517
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	57.0	0.0	0.0	52.9	0.0	53.7	7.7	3.1	3.1	5.4	3.7	3.7
Incr Delay (d2), s/veh	3.8	0.0	0.0	0.3	0.0	0.9	1.7	0.4	0.7	1.4	1.3	1.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	4.6	0.0	0.0	0.8	0.0	1.4	0.7	2.9	3.3	0.5	4.9	5.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	60.8	0.0	0.0	53.2	0.0	54.5	9.4	3.5	3.9	6.9	5.0	4.9
LnGrp LOS	E	A	A	D	A	D	A	A	A	A	A	A
Approach Vol, veh/h		135			69			1726			1577	
Approach Delay, s/veh		60.8			54.1			3.8			5.0	
Approach LOS		E			D			A			A	

Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		111.5		18.5		111.5		18.5				
Change Period (Y+Rc), s		5.0		5.0		5.0		5.0				
Max Green Setting (Gmax), s		90.0		30.0		90.0		30.0				
Max Q Clear Time (g_c+I1), s		28.0		12.9		19.6		5.3				
Green Ext Time (p_c), s		21.6		0.7		20.1		0.2				

Intersection Summary												
HCM 6th Ctrl Delay			7.5									
HCM 6th LOS			A									



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	28	147	81	56	168	460	87	1191	25	330	1252	21
Future Volume (vph)	28	147	81	56	168	460	87	1191	25	330	1252	21
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	130		135	200		0	205		0	300		165
Storage Lanes	1		1	1		1	1		0	2		1
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.97	0.95	1.00
Frt			0.850			0.850		0.997				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1770	1863	1583	1770	3529	0	3433	3539	1583
Flt Permitted	0.410			0.475			0.950			0.950		
Satd. Flow (perm)	764	1863	1583	885	1863	1583	1770	3529	0	3433	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			92			50		2				50
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		538			720			654			541	
Travel Time (s)		10.5			14.0			12.7			10.5	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	29	153	84	58	175	479	91	1241	26	344	1304	22
Shared Lane Traffic (%)												
Lane Group Flow (vph)	29	153	84	58	175	479	91	1267	0	344	1304	22
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes										
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60		60	60		60	60		60	60		60
Number of Detectors	1	2	1	1	2	1	1	2		1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (ft)	20	100	20	20	100	20	20	100		20	100	20
Trailing Detector (ft)	0	0	0	0	0	0	0	0		0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0	0	0		0	0	0
Detector 1 Size(ft)	20	6	20	20	6	20	20	6		20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	Perm	Perm	NA	pm+ov	Prot	NA		Prot	NA	Perm
Protected Phases		4			8	1	5	2		1	6	
Permitted Phases	4		4	8		8						6

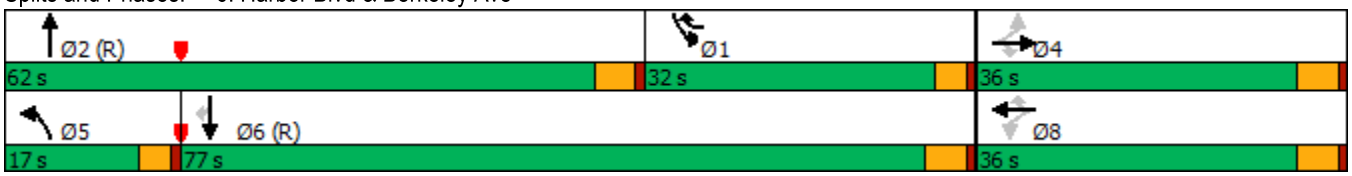


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	4	8	8	1	5	2		1	6	6
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0	6.0
Minimum Split (s)	30.0	30.0	30.0	36.0	36.0	10.0	10.0	30.0		10.0	30.0	30.0
Total Split (s)	36.0	36.0	36.0	36.0	36.0	32.0	17.0	62.0		32.0	77.0	77.0
Total Split (%)	27.7%	27.7%	27.7%	27.7%	27.7%	24.6%	13.1%	47.7%		24.6%	59.2%	59.2%
Maximum Green (s)	31.0	31.0	31.0	31.0	31.0	28.0	13.0	57.0		28.0	72.0	72.0
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	3.0	3.0	4.0		3.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	4.0	4.0	5.0		4.0	5.0	5.0
Lead/Lag						Lag	Lead	Lead		Lag	Lag	Lag
Lead-Lag Optimize?						Yes	Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max		None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0			7.0			7.0	7.0
Flash Dont Walk (s)	18.0	18.0	18.0	24.0	24.0			18.0			18.0	18.0
Pedestrian Calls (#/hr)	5	5	5	5	5			5			5	5
Act Effct Green (s)	19.0	19.0	19.0	19.0	19.0	52.0	11.3	69.0		28.0	85.7	85.7
Actuated g/C Ratio	0.15	0.15	0.15	0.15	0.15	0.40	0.09	0.53		0.22	0.66	0.66
v/c Ratio	0.26	0.56	0.27	0.45	0.64	0.72	0.59	0.68		0.47	0.56	0.02
Control Delay	51.9	58.3	9.1	59.5	62.1	35.3	72.9	25.8		31.1	3.8	0.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.1	0.0
Total Delay	51.9	58.3	9.1	59.5	62.1	35.3	72.9	25.8		31.1	3.9	0.1
LOS	D	E	A	E	E	D	E	C		C	A	A
Approach Delay		42.1			43.9			29.0			9.5	
Approach LOS		D			D			C			A	
Queue Length 50th (ft)	22	123	0	46	143	311	75	385		135	48	0
Queue Length 95th (ft)	48	173	37	83	196	367	133	579		145	188	m1
Internal Link Dist (ft)		458			640			574			461	
Turn Bay Length (ft)	130		135	200			205			300		165
Base Capacity (vph)	182	444	447	211	444	663	179	1874		739	2334	1061
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	165	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	0
Reduced v/c Ratio	0.16	0.34	0.19	0.27	0.39	0.72	0.51	0.68		0.47	0.60	0.02

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 52 (40%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 90			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.72			
Intersection Signal Delay: 24.4		Intersection LOS: C	
Intersection Capacity Utilization 78.9%		ICU Level of Service D	
Analysis Period (min) 15			

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Harbor Blvd & Berkeley Ave



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	28	147	81	56	168	460	87	1191	25	330	1252	21
Future Volume (veh/h)	28	147	81	56	168	460	87	1191	25	330	1252	21
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	29	153	84	58	175	479	91	1241	26	344	1304	22
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	138	291	247	155	291	707	114	1561	33	1004	2391	1067
Arrive On Green	0.16	0.16	0.16	0.16	0.16	0.16	0.06	0.44	0.44	0.29	0.67	0.67
Sat Flow, veh/h	1210	1870	1585	1234	1870	1585	1781	3559	75	3456	3554	1585
Grp Volume(v), veh/h	29	153	84	58	175	479	91	619	648	344	1304	22
Grp Sat Flow(s),veh/h/ln	1210	1870	1585	1234	1870	1585	1781	1777	1857	1728	1777	1585
Q Serve(g_s), s	3.0	9.8	6.1	5.9	11.3	0.0	6.6	39.1	39.1	10.2	24.6	0.6
Cycle Q Clear(g_c), s	14.3	9.8	6.1	15.7	11.3	0.0	6.6	39.1	39.1	10.2	24.6	0.6
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.04	1.00		1.00
Lane Grp Cap(c), veh/h	138	291	247	155	291	707	114	779	814	1004	2391	1067
V/C Ratio(X)	0.21	0.53	0.34	0.38	0.60	0.68	0.80	0.79	0.80	0.34	0.55	0.02
Avail Cap(c_a), veh/h	238	446	378	257	446	839	178	779	814	1004	2391	1067
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	57.8	50.5	48.9	57.7	51.1	28.6	60.0	31.5	31.5	36.3	11.0	7.1
Incr Delay (d2), s/veh	0.7	1.5	0.8	1.5	2.0	1.7	13.0	8.2	7.9	0.2	0.9	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.9	4.7	2.5	1.9	5.4	11.9	3.4	18.1	18.9	4.3	9.3	0.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	58.5	51.9	49.8	59.2	53.1	30.3	73.0	39.7	39.4	36.5	11.9	7.1
LnGrp LOS	E	D	D	E	D	C	E	D	D	D	B	A
Approach Vol, veh/h		266			712			1358			1670	
Approach Delay, s/veh		52.0			38.3			41.8			16.9	
Approach LOS		D			D			D			B	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	42.8	62.0		25.2	12.3	92.5		25.2				
Change Period (Y+Rc), s	5.0	* 5		5.0	4.0	5.0		5.0				
Max Green Setting (Gmax), s	28.0	* 57		31.0	13.0	72.0		31.0				
Max Q Clear Time (g_c+I1), s	12.2	41.1		16.3	8.6	26.6		17.7				
Green Ext Time (p_c), s	1.1	7.6		1.0	0.1	13.3		2.6				

Intersection Summary			
HCM 6th Ctrl Delay		31.5	
HCM 6th LOS		C	

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

APPENDIX B-II

EXISTING PLUS PROJECT TRAFFIC CONDITIONS

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	311	848	163	263	927	216	182	621	110	349	941	297
Future Volume (vph)	311	848	163	263	927	216	182	621	110	349	941	297
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	325		0	193		145	230		150	275		0
Storage Lanes	1		0	2		1	2		1	2		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	0.91	0.91	0.97	0.91	1.00	0.97	0.91	1.00	0.97	0.91	0.91
Frt		0.976				0.850			0.850		0.964	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	4963	0	3433	5085	1583	3433	5085	1583	3433	4902	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	4963	0	3433	5085	1583	3433	5085	1583	3433	4902	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		33				170			126		64	
Link Speed (mph)		40			40			45			50	
Link Distance (ft)		1043			981			428			926	
Travel Time (s)		17.8			16.7			6.5			12.6	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Adj. Flow (vph)	331	902	173	280	986	230	194	661	117	371	1001	316
Shared Lane Traffic (%)												
Lane Group Flow (vph)	331	1075	0	280	986	230	194	661	117	371	1317	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		24			24			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2	1	1	2	1	1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru	Right	Left	Thru	
Leading Detector (ft)	20	100		20	100	20	20	100	20	20	100	
Trailing Detector (ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Position(ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Size(ft)	20	6		20	6	20	20	6	20	20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases						8			2			

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	7	4		3	8	8	5	2	2	1	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Minimum Split (s)	10.0	43.0		10.0	43.0	43.0	10.0	43.0	43.0	10.0	36.0	
Total Split (s)	27.0	49.0		21.0	43.0	43.0	14.0	43.0	43.0	17.0	46.0	
Total Split (%)	20.8%	37.7%		16.2%	33.1%	33.1%	10.8%	33.1%	33.1%	13.1%	35.4%	
Maximum Green (s)	23.0	44.0		17.0	38.0	38.0	10.0	38.0	38.0	13.0	41.0	
Yellow Time (s)	3.0	4.0		3.0	4.0	4.0	3.0	4.0	4.0	3.0	4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	4.0	5.0		4.0	5.0	5.0	4.0	5.0	5.0	4.0	5.0	
Lead/Lag	Lead	Lead		Lag	Lag	Lag	Lag	Lead	Lead	Lag	Lead	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None		None	None	None	None	C-Max	C-Max	None	C-Max	
Walk Time (s)		7.0			7.0	7.0		7.0	7.0		7.0	
Flash Dont Walk (s)		31.0			31.0	31.0		31.0	31.0		24.0	
Pedestrian Calls (#/hr)		5			5	5		5	5		5	
Act Effct Green (s)	23.0	35.9		20.9	33.9	33.9	10.0	42.1	42.1	13.0	45.1	
Actuated g/C Ratio	0.18	0.28		0.16	0.26	0.26	0.08	0.32	0.32	0.10	0.35	
v/c Ratio	1.06	0.77		0.51	0.74	0.43	0.73	0.40	0.20	1.08	0.76	
Control Delay	117.6	45.6		53.8	47.6	13.3	55.9	25.1	5.0	126.1	39.6	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	117.6	45.6		53.8	47.6	13.3	55.9	25.1	5.0	126.1	39.6	
LOS	F	D		D	D	B	E	C	A	F	D	
Approach Delay		62.5			43.5			28.8			58.6	
Approach LOS		E			D			C			E	
Queue Length 50th (ft)	~305	297		112	279	39	85	94	4	~179	344	
Queue Length 95th (ft)	#494	326		163	318	108	#134	137	35	#281	421	
Internal Link Dist (ft)		963			901			348			846	
Turn Bay Length (ft)	325			193		145	230		150	275		
Base Capacity (vph)	313	1701		552	1486	583	264	1648	598	343	1743	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	1.06	0.63		0.51	0.66	0.39	0.73	0.40	0.20	1.08	0.76	

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 59 (45%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 120			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 1.08			
Intersection Signal Delay: 50.3		Intersection LOS: D	
Intersection Capacity Utilization 80.1%		ICU Level of Service D	
Analysis Period (min) 15			

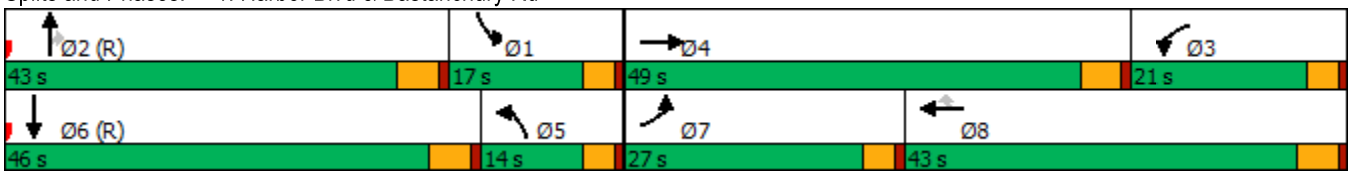
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Harbor Blvd & Bastanchury Rd



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	311	848	163	263	927	216	182	621	110	349	941	297
Future Volume (veh/h)	311	848	163	263	927	216	182	621	110	349	941	297
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	331	902	173	280	986	230	194	661	117	371	1001	316
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	315	1123	214	518	1232	383	442	1493	463	522	1212	382
Arrive On Green	0.18	0.26	0.26	0.15	0.24	0.24	0.13	0.29	0.29	0.15	0.32	0.32
Sat Flow, veh/h	1781	4304	822	3456	5106	1585	3456	5106	1585	3456	3844	1212
Grp Volume(v), veh/h	331	712	363	280	986	230	194	661	117	371	886	431
Grp Sat Flow(s),veh/h/ln	1781	1702	1722	1728	1702	1585	1728	1702	1585	1728	1702	1652
Q Serve(g_s), s	23.0	25.4	25.6	9.7	23.6	11.9	6.7	13.7	7.3	13.3	31.3	31.4
Cycle Q Clear(g_c), s	23.0	25.4	25.6	9.7	23.6	11.9	6.7	13.7	7.3	13.3	31.3	31.4
Prop In Lane	1.00		0.48	1.00		1.00	1.00		1.00	1.00		0.73
Lane Grp Cap(c), veh/h	315	888	449	518	1232	383	442	1493	463	522	1074	521
V/C Ratio(X)	1.05	0.80	0.81	0.54	0.80	0.60	0.44	0.44	0.25	0.71	0.83	0.83
Avail Cap(c_a), veh/h	315	1152	583	518	1493	463	442	1493	463	522	1074	521
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	53.5	44.9	45.0	51.1	46.4	22.0	52.4	37.4	35.1	52.5	41.2	41.2
Incr Delay (d2), s/veh	64.5	3.2	6.4	1.1	2.7	1.5	0.7	1.0	1.3	4.5	7.3	14.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	15.7	10.9	11.5	4.3	10.1	4.5	2.9	5.7	3.0	5.9	13.6	14.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	118.0	48.1	51.3	52.3	49.0	23.6	53.1	38.3	36.5	57.0	48.5	55.2
LnGrp LOS	F	D	D	D	D	C	D	D	D	E	D	E
Approach Vol, veh/h		1406			1496			972			1688	
Approach Delay, s/veh		65.4			45.7			41.1			52.0	
Approach LOS		E			D			D			D	

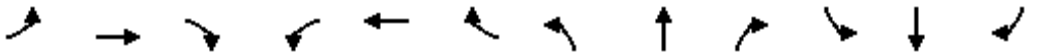
Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	23.6	43.0	24.5	38.9	20.6	46.0	27.0	36.4
Change Period (Y+Rc), s	4.0	5.0	5.0	* 5	4.0	5.0	4.0	5.0
Max Green Setting (Gmax), s	13.0	38.0	17.0	* 44	10.0	41.0	23.0	38.0
Max Q Clear Time (g_c+I1), s	15.3	15.7	11.7	27.6	8.7	33.4	25.0	25.6
Green Ext Time (p_c), s	0.0	4.6	0.4	6.3	0.1	4.5	0.0	5.8

Intersection Summary	
HCM 6th Ctrl Delay	51.8
HCM 6th LOS	D

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	90	46	85	51	45	63	147	752	125	167	984	176
Future Volume (vph)	90	46	85	51	45	63	147	752	125	167	984	176
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	55		85	0		0	185		0	230		0
Storage Lanes	1		1	1		0	1		0	1		1
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	1.00
Frt			0.850		0.912			0.979				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1770	1699	0	1770	3465	0	1770	3539	1583
Flt Permitted	0.583			0.726			0.950			0.950		
Satd. Flow (perm)	1086	1863	1583	1352	1699	0	1770	3465	0	1770	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			59		52			19				185
Link Speed (mph)		30			30			50			45	
Link Distance (ft)		813			705			616			406	
Travel Time (s)		18.5			16.0			8.4			6.2	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	95	48	89	54	47	66	155	792	132	176	1036	185
Shared Lane Traffic (%)												
Lane Group Flow (vph)	95	48	89	54	113	0	155	924	0	176	1036	185
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2	1	1	2		1	2		1	2	1
Detector Template	Left	Thru	Right	Left	Thru		Left	Thru		Left	Thru	Right
Leading Detector (ft)	20	100	20	20	100		20	100		20	100	20
Trailing Detector (ft)	0	0	0	0	0		0	0		0	0	0
Detector 1 Position(ft)	0	0	0	0	0		0	0		0	0	0
Detector 1 Size(ft)	20	6	20	20	6		20	6		20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	pm+ov	Perm	NA		Prot	NA		Prot	NA	Perm
Protected Phases		4	5		8		5	2		1	6	
Permitted Phases	4		4	8								6

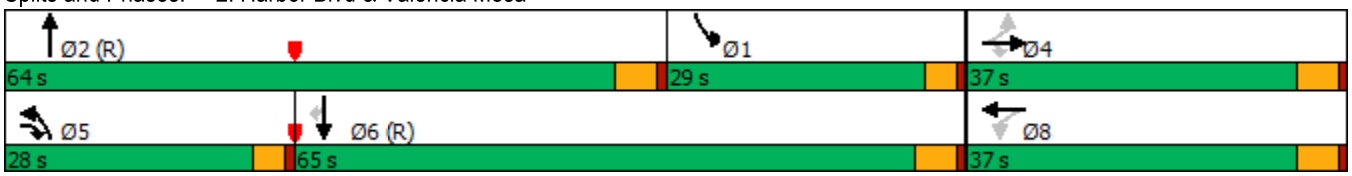


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	5	8	8		5	2		1	6	6
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0		6.0	6.0		6.0	6.0	6.0
Minimum Split (s)	11.0	11.0	10.0	36.0	36.0		10.0	26.0		10.0	30.0	30.0
Total Split (s)	37.0	37.0	28.0	37.0	37.0		28.0	64.0		29.0	65.0	65.0
Total Split (%)	28.5%	28.5%	21.5%	28.5%	28.5%		21.5%	49.2%		22.3%	50.0%	50.0%
Maximum Green (s)	32.0	32.0	24.0	32.0	32.0		24.0	59.0		25.0	60.0	60.0
Yellow Time (s)	4.0	4.0	3.0	4.0	4.0		3.0	4.0		3.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0		1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	4.0	5.0	5.0		4.0	5.0		4.0	5.0	5.0
Lead/Lag			Lead				Lead	Lead		Lag	Lag	Lag
Lead-Lag Optimize?			Yes				Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0
Recall Mode	None	None	None	None	None		None	C-Max		None	C-Max	C-Max
Walk Time (s)				7.0	7.0			7.0			7.0	7.0
Flash Dont Walk (s)				24.0	24.0			14.0			18.0	18.0
Pedestrian Calls (#/hr)				5	5			5			5	5
Act Effct Green (s)	17.2	17.2	38.8	17.2	17.2		16.6	73.8		25.0	82.2	82.2
Actuated g/C Ratio	0.13	0.13	0.30	0.13	0.13		0.13	0.57		0.19	0.63	0.63
v/c Ratio	0.66	0.20	0.17	0.30	0.42		0.69	0.47		0.52	0.46	0.17
Control Delay	73.2	48.4	11.6	52.1	31.2		91.7	4.9		52.9	10.9	2.1
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Delay	73.2	48.4	11.6	52.1	31.2		91.7	4.9		52.9	10.9	2.1
LOS	E	D	B	D	C		F	A		D	B	A
Approach Delay		44.5			37.9			17.4			15.0	
Approach LOS		D			D			B			B	
Queue Length 50th (ft)	78	37	18	42	48		134	42		150	133	8
Queue Length 95th (ft)	122	66	46	74	94		150	69		m207	534	m27
Internal Link Dist (ft)		733			625			536			326	
Turn Bay Length (ft)	55		85				185			230		
Base Capacity (vph)	267	458	600	332	457		326	1975		340	2237	1069
Starvation Cap Reductn	0	0	0	0	0		0	0		0	0	0
Spillback Cap Reductn	0	0	0	0	0		0	0		0	0	0
Storage Cap Reductn	0	0	0	0	0		0	0		0	0	0
Reduced v/c Ratio	0.36	0.10	0.15	0.16	0.25		0.48	0.47		0.52	0.46	0.17

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 84 (65%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 80			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.69			
Intersection Signal Delay: 19.6		Intersection LOS: B	
Intersection Capacity Utilization 58.7%		ICU Level of Service B	
Analysis Period (min) 15			

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Harbor Blvd & Valencia Mesa



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	90	46	85	51	45	63	147	752	125	167	984	176
Future Volume (veh/h)	90	46	85	51	45	63	147	752	125	167	984	176
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	95	48	89	54	47	66	155	792	132	176	1036	185
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	174	287	406	220	108	152	183	1383	231	494	2261	1008
Arrive On Green	0.15	0.15	0.15	0.15	0.15	0.15	0.10	0.45	0.45	0.28	0.64	0.64
Sat Flow, veh/h	1280	1870	1585	1252	704	988	1781	3048	508	1781	3554	1585
Grp Volume(v), veh/h	95	48	89	54	0	113	155	462	462	176	1036	185
Grp Sat Flow(s),veh/h/ln	1280	1870	1585	1252	0	1692	1781	1777	1779	1781	1777	1585
Q Serve(g_s), s	9.5	2.9	5.8	5.1	0.0	7.9	11.1	24.9	24.9	10.3	19.5	6.3
Cycle Q Clear(g_c), s	17.3	2.9	5.8	8.0	0.0	7.9	11.1	24.9	24.9	10.3	19.5	6.3
Prop In Lane	1.00		1.00	1.00		0.58	1.00		0.29	1.00		1.00
Lane Grp Cap(c), veh/h	174	287	406	220	0	260	183	806	807	494	2261	1008
V/C Ratio(X)	0.54	0.17	0.22	0.25	0.00	0.43	0.85	0.57	0.57	0.36	0.46	0.18
Avail Cap(c_a), veh/h	293	460	553	336	0	417	329	806	807	494	2261	1008
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	57.8	47.8	38.1	51.3	0.0	49.9	57.3	26.2	26.2	37.7	12.1	9.7
Incr Delay (d2), s/veh	2.6	0.3	0.3	0.6	0.0	1.1	10.3	2.9	2.9	0.4	0.7	0.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.2	1.4	2.3	1.6	0.0	3.4	5.4	10.6	10.6	4.5	7.2	2.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	60.4	48.1	38.4	51.8	0.0	51.0	67.7	29.1	29.1	38.1	12.8	10.1
LnGrp LOS	E	D	D	D	A	D	E	C	C	D	B	B
Approach Vol, veh/h		232			167			1079			1397	
Approach Delay, s/veh		49.4			51.3			34.7			15.7	
Approach LOS		D			D			C			B	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	41.0	64.0		25.0	17.3	87.7		25.0				
Change Period (Y+Rc), s	5.0	* 5		5.0	4.0	5.0		5.0				
Max Green Setting (Gmax), s	25.0	* 59		32.0	24.0	60.0		32.0				
Max Q Clear Time (g_c+I1), s	12.3	26.9		19.3	13.1	21.5		10.0				
Green Ext Time (p_c), s	0.3	5.8		0.6	0.3	9.1		0.8				

Intersection Summary			
HCM 6th Ctrl Delay		27.6	
HCM 6th LOS		C	

Notes

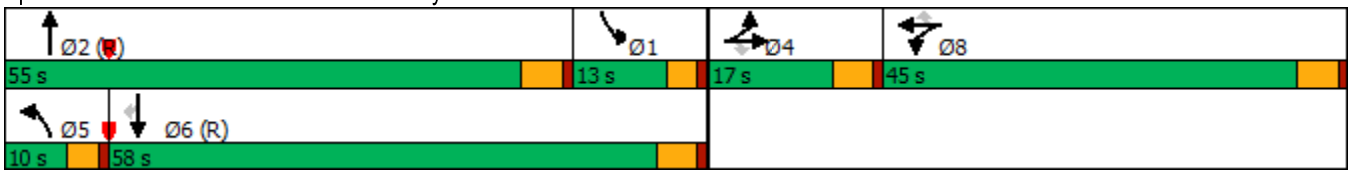
* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	51	84	9	539	116	166	22	920	382	57	975	80
Future Volume (vph)	51	84	9	539	116	166	22	920	382	57	975	80
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	85		0	0		0	95		0	140		140
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950	0.969		0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1681	1715	1583	1770	3539	1583	1770	3539	1583
Flt Permitted	0.950			0.950	0.969		0.950			0.950		
Satd. Flow (perm)	1770	1863	1583	1681	1715	1583	1770	3539	1583	1770	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			134			171			241			92
Link Speed (mph)		25			45			35			50	
Link Distance (ft)		871			1039			280			363	
Travel Time (s)		23.8			15.7			5.5			5.0	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	53	87	9	556	120	171	23	948	394	59	1005	82
Shared Lane Traffic (%)				40%								
Lane Group Flow (vph)	53	87	9	334	342	171	23	948	394	59	1005	82
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (ft)	20	100	20	20	100	20	20	100	20	20	100	20
Trailing Detector (ft)	0	0	0	0	0	0	0	0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0	0	0	0	0	0	0
Detector 1 Size(ft)	20	6	20	20	6	20	20	6	20	20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Split	NA	Perm	Split	NA	Perm	Prot	NA	Free	Prot	NA	Perm
Protected Phases	4	4		8	8		5	2		1	6	
Permitted Phases			4			8			Free			6

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	4	8	8	8	5	2		1	6	6
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0	6.0
Minimum Split (s)	11.0	11.0	11.0	36.0	36.0	36.0	10.0	23.0		10.0	30.0	30.0
Total Split (s)	17.0	17.0	17.0	45.0	45.0	45.0	10.0	55.0		13.0	58.0	58.0
Total Split (%)	13.1%	13.1%	13.1%	34.6%	34.6%	34.6%	7.7%	42.3%		10.0%	44.6%	44.6%
Maximum Green (s)	12.0	12.0	12.0	40.0	40.0	40.0	6.0	50.0		9.0	53.0	53.0
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	3.0	4.0		3.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	4.0	5.0		4.0	5.0	5.0
Lead/Lag							Lead	Lead		Lag	Lag	Lag
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max		None	C-Max	C-Max
Walk Time (s)				7.0	7.0	7.0		7.0			7.0	7.0
Flash Dont Walk (s)				24.0	24.0	24.0		11.0			18.0	18.0
Pedestrian Calls (#/hr)				5	5	5		5			5	5
Act Effct Green (s)	10.4	10.4	10.4	32.1	32.1	32.1	6.7	62.1	130.0	8.4	65.9	65.9
Actuated g/C Ratio	0.08	0.08	0.08	0.25	0.25	0.25	0.05	0.48	1.00	0.06	0.51	0.51
v/c Ratio	0.38	0.58	0.04	0.81	0.81	0.33	0.26	0.56	0.25	0.52	0.56	0.10
Control Delay	63.8	73.3	0.2	60.6	60.6	6.5	79.7	18.4	0.4	63.9	15.9	7.5
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	63.8	73.3	0.2	60.6	60.6	6.5	79.7	18.4	0.4	63.9	15.9	7.5
LOS	E	E	A	E	E	A	E	B	A	E	B	A
Approach Delay		65.5			49.7			14.3			17.8	
Approach LOS		E			D			B			B	
Queue Length 50th (ft)	43	71	0	277	284	0	20	209	0	51	77	0
Queue Length 95th (ft)	86	128	0	369	376	52	43	254	0	86	332	46
Internal Link Dist (ft)		791			959			200			283	
Turn Bay Length (ft)	85						95			140		140
Base Capacity (vph)	163	171	267	517	527	605	90	1691	1583	122	1793	847
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.33	0.51	0.03	0.65	0.65	0.28	0.26	0.56	0.25	0.48	0.56	0.10

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 90			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.81			
Intersection Signal Delay: 26.2		Intersection LOS: C	
Intersection Capacity Utilization 68.3%		ICU Level of Service C	
Analysis Period (min) 15			

Splits and Phases: 3: Harbor Blvd & Valley View/Brea Blvd



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	51	84	9	539	116	166	22	920	382	57	975	80
Future Volume (veh/h)	51	84	9	539	116	166	22	920	382	57	975	80
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	53	87	9	642	0	0	23	948	0	59	1005	82
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	111	117	99	734	0		46	1367		344	1988	887
Arrive On Green	0.06	0.06	0.06	0.21	0.00	0.00	0.03	0.38	0.00	0.19	0.56	0.56
Sat Flow, veh/h	1781	1870	1585	3563	0	1585	1781	3554	1585	1781	3554	1585
Grp Volume(v), veh/h	53	87	9	642	0	0	23	948	0	59	1005	82
Grp Sat Flow(s),veh/h/ln	1781	1870	1585	1781	0	1585	1781	1777	1585	1781	1777	1585
Q Serve(g_s), s	3.7	5.9	0.7	22.7	0.0	0.0	1.7	29.1	0.0	3.6	22.6	3.1
Cycle Q Clear(g_c), s	3.7	5.9	0.7	22.7	0.0	0.0	1.7	29.1	0.0	3.6	22.6	3.1
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	111	117	99	734	0		46	1367		344	1988	887
V/C Ratio(X)	0.48	0.75	0.09	0.87	0.00		0.50	0.69		0.17	0.51	0.09
Avail Cap(c_a), veh/h	164	173	146	1096	0		82	1367		344	1988	887
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	58.9	59.9	57.5	50.0	0.0	0.0	62.5	33.6	0.0	43.8	17.6	13.3
Incr Delay (d2), s/veh	3.2	9.3	0.4	5.5	0.0	0.0	8.0	2.9	0.0	0.2	0.9	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.8	3.1	0.3	10.4	0.0	0.0	0.9	12.9	0.0	1.6	8.7	1.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	62.1	69.3	57.9	55.5	0.0	0.0	70.4	36.5	0.0	44.0	18.5	13.5
LnGrp LOS	E	E	E	E	A		E	D		D	B	B
Approach Vol, veh/h		149			642			971			1146	
Approach Delay, s/veh		66.0			55.5			37.3			19.5	
Approach LOS		E			E			D			B	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	30.1	55.0		13.1	7.4	77.7		31.8				
Change Period (Y+Rc), s	5.0	* 5		5.0	4.0	5.0		5.0				
Max Green Setting (Gmax), s	9.0	* 50		12.0	6.0	53.0		40.0				
Max Q Clear Time (g_c+I1), s	5.6	31.1		7.9	3.7	24.6		24.7				
Green Ext Time (p_c), s	0.0	6.4		0.2	0.0	7.4		2.1				

Intersection Summary			
HCM 6th Ctrl Delay		35.7	
HCM 6th LOS		D	

Notes

User approved volume balancing among the lanes for turning movement.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

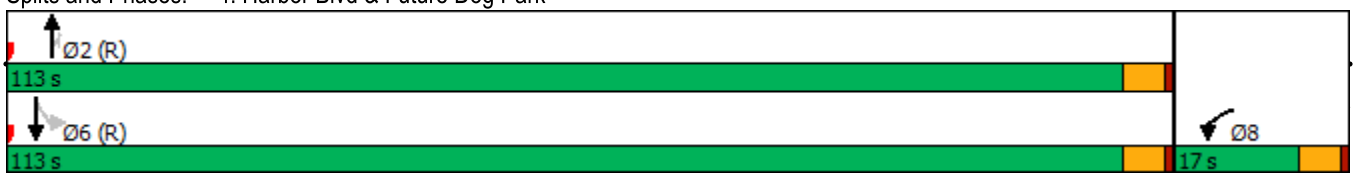


Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	5	5	1132	5	5	1115
Future Volume (vph)	5	5	1132	5	5	1115
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		200	250	
Storage Lanes	1	0		1	1	
Taper Length (ft)	60				60	
Lane Util. Factor	1.00	1.00	0.95	1.00	1.00	0.95
Frt	0.932			0.850		
Flt Protected	0.976				0.950	
Satd. Flow (prot)	1694	0	3539	1583	1770	3539
Flt Permitted	0.976				0.250	
Satd. Flow (perm)	1694	0	3539	1583	466	3539
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)	5			5		
Link Speed (mph)	30		50			50
Link Distance (ft)	484		960			1191
Travel Time (s)	11.0		13.1			16.2
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	5	5	1132	5	5	1115
Shared Lane Traffic (%)						
Lane Group Flow (vph)	10	0	1132	5	5	1115
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60	60		60	60	
Number of Detectors	1		2	1	1	2
Detector Template	Left		Thru	Right	Left	Thru
Leading Detector (ft)	20		100	20	20	100
Trailing Detector (ft)	0		0	0	0	0
Detector 1 Position(ft)	0		0	0	0	0
Detector 1 Size(ft)	20		6	20	20	6
Detector 1 Type	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0		0.0	0.0	0.0	0.0
Detector 2 Position(ft)			94			94
Detector 2 Size(ft)			6			6
Detector 2 Type			Cl+Ex			Cl+Ex
Detector 2 Channel						
Detector 2 Extend (s)			0.0			0.0
Turn Type	Prot		NA	Perm	Perm	NA
Protected Phases	8		2			6
Permitted Phases				2	6	

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Detector Phase	8		2	2	6	6
Switch Phase						
Minimum Initial (s)	6.0		6.0	6.0	6.0	6.0
Minimum Split (s)	11.0		11.0	11.0	11.0	11.0
Total Split (s)	17.0		113.0	113.0	113.0	113.0
Total Split (%)	13.1%		86.9%	86.9%	86.9%	86.9%
Maximum Green (s)	12.0		108.0	108.0	108.0	108.0
Yellow Time (s)	4.0		4.0	4.0	4.0	4.0
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0		5.0	5.0	5.0	5.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	None		C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	6.4		126.5	126.5	126.5	126.5
Actuated g/C Ratio	0.05		0.97	0.97	0.97	0.97
v/c Ratio	0.11		0.33	0.00	0.01	0.32
Control Delay	46.1		0.4	0.2	1.2	2.8
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	46.1		0.4	0.2	1.2	2.8
LOS	D		A	A	A	A
Approach Delay	46.1		0.4			2.8
Approach LOS	D		A			A
Queue Length 50th (ft)	4		3	0	0	0
Queue Length 95th (ft)	23		28	m0	m2	510
Internal Link Dist (ft)	404		880			1111
Turn Bay Length (ft)				200	250	
Base Capacity (vph)	160		3444	1541	453	3444
Starvation Cap Reductn	0		0	0	0	0
Spillback Cap Reductn	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	0.06		0.33	0.00	0.01	0.32

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Green			
Natural Cycle: 40			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.33			
Intersection Signal Delay: 1.8		Intersection LOS: A	
Intersection Capacity Utilization 44.6%		ICU Level of Service A	
Analysis Period (min) 15			
m Volume for 95th percentile queue is metered by upstream signal.			

Splits and Phases: 4: Harbor Blvd & Future Dog Park





Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	5	5	1132	5	5	1115
Future Volume (veh/h)	5	5	1132	5	5	1115
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	5	5	1132	5	5	1115
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	12	12	3227	1439	485	3227
Arrive On Green	0.02	0.02	0.91	0.91	0.91	0.91
Sat Flow, veh/h	770	770	3647	1585	497	3647
Grp Volume(v), veh/h	11	0	1132	5	5	1115
Grp Sat Flow(s),veh/h/ln	1693	0	1777	1585	497	1777
Q Serve(g_s), s	0.8	0.0	5.6	0.0	0.2	5.5
Cycle Q Clear(g_c), s	0.8	0.0	5.6	0.0	5.8	5.5
Prop In Lane	0.45	0.45		1.00	1.00	
Lane Grp Cap(c), veh/h	26	0	3227	1439	485	3227
V/C Ratio(X)	0.43	0.00	0.35	0.00	0.01	0.35
Avail Cap(c_a), veh/h	156	0	3227	1439	485	3227
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	63.5	0.0	0.8	0.6	1.2	0.8
Incr Delay (d2), s/veh	11.0	0.0	0.3	0.0	0.0	0.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	0.0	0.1	0.0	0.0	0.1
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	74.4	0.0	1.1	0.6	1.2	1.1
LnGrp LOS	E	A	A	A	A	A
Approach Vol, veh/h	11		1137			1120
Approach Delay, s/veh	74.4		1.1			1.1
Approach LOS	E		A			A

Timer - Assigned Phs		2			6	8
Phs Duration (G+Y+Rc), s		123.0			123.0	7.0
Change Period (Y+Rc), s		5.0			5.0	5.0
Max Green Setting (Gmax), s		108.0			108.0	12.0
Max Q Clear Time (g_c+I1), s		7.6			7.8	2.8
Green Ext Time (p_c), s		9.5			9.4	0.0

Intersection Summary	
HCM 6th Ctrl Delay	1.5
HCM 6th LOS	A

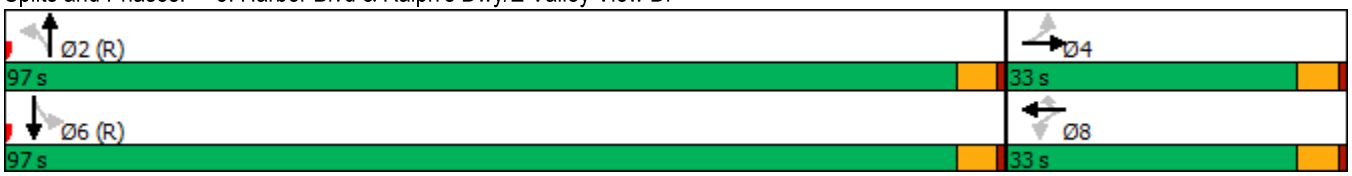
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	17	5	23	19	7	50	43	1242	26	36	1584	11
Future Volume (vph)	17	5	23	19	7	50	43	1242	26	36	1584	11
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		65	95		0	100		0
Storage Lanes	0		0	0		1	1		0	1		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.95	0.95
Frt		0.931				0.850		0.997			0.999	
Flt Protected		0.981			0.964		0.950			0.950		
Satd. Flow (prot)	0	1701	0	0	1796	1583	1770	5070	0	1770	3536	0
Flt Permitted		0.867			0.810		0.125			0.189		
Satd. Flow (perm)	0	1504	0	0	1509	1583	233	5070	0	352	3536	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		24				53		6			1	
Link Speed (mph)		25			25			35			35	
Link Distance (ft)		427			781			235			407	
Travel Time (s)		11.6			21.3			4.6			7.9	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	18	5	24	20	7	53	45	1307	27	38	1667	12
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	47	0	0	27	53	45	1334	0	38	1679	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100	20	20	100		20	100	
Trailing Detector (ft)	0	0		0	0	0	0	0		0	0	
Detector 1 Position(ft)	0	0		0	0	0	0	0		0	0	
Detector 1 Size(ft)	20	6		20	6	20	20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		

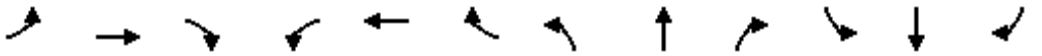
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4		8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0		6.0	6.0	
Minimum Split (s)	33.0	33.0		33.0	33.0	33.0	26.0	26.0		26.0	26.0	
Total Split (s)	33.0	33.0		33.0	33.0	33.0	97.0	97.0		97.0	97.0	
Total Split (%)	25.4%	25.4%		25.4%	25.4%	25.4%	74.6%	74.6%		74.6%	74.6%	
Maximum Green (s)	28.0	28.0		28.0	28.0	28.0	92.0	92.0		92.0	92.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	4.0	4.0		4.0	4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.0			5.0	5.0	5.0	5.0		5.0	5.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None	None	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	21.0	21.0		21.0	21.0	21.0	14.0	14.0		14.0	14.0	
Pedestrian Calls (#/hr)	5	5		5	5	5	5	5		5	5	
Act Effct Green (s)		11.4			11.4	11.4	111.8	111.8		111.8	111.8	
Actuated g/C Ratio		0.09			0.09	0.09	0.86	0.86		0.86	0.86	
v/c Ratio		0.31			0.20	0.28	0.23	0.31		0.13	0.55	
Control Delay		34.5			54.6	15.7	5.8	2.6		1.8	1.8	
Queue Delay		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		34.5			54.6	15.7	5.8	2.6		1.8	1.8	
LOS		C			D	B	A	A		A	A	
Approach Delay		34.5			28.8			2.7			1.8	
Approach LOS		C			C			A			A	
Queue Length 50th (ft)		19			22	0	6	65		0	6	
Queue Length 95th (ft)		50			45	35	m18	108		m9	298	
Internal Link Dist (ft)		347			701			155			327	
Turn Bay Length (ft)						65	95			100		
Base Capacity (vph)		342			325	382	200	4360		302	3040	
Starvation Cap Reductn		0			0	0	0	0		0	75	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.14			0.08	0.14	0.23	0.31		0.13	0.57	

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 105 (81%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green			
Natural Cycle: 75			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.55			
Intersection Signal Delay: 3.4		Intersection LOS: A	
Intersection Capacity Utilization 61.8%		ICU Level of Service B	
Analysis Period (min) 15			

m Volume for 95th percentile queue is metered by upstream signal.

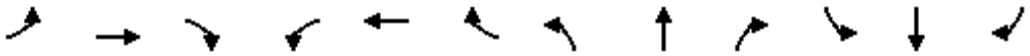
Splits and Phases: 5: Harbor Blvd & Ralph's Dwy/E Valley View Dr





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	17	5	23	19	7	50	43	1242	26	36	1584	11
Future Volume (veh/h)	17	5	23	19	7	50	43	1242	26	36	1584	11
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	18	5	24	20	7	53	45	1307	27	38	1667	12
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	59	14	36	103	29	79	281	4497	93	396	3159	23
Arrive On Green	0.05	0.05	0.05	0.05	0.05	0.05	0.87	0.87	0.87	0.87	0.87	0.87
Sat Flow, veh/h	411	289	731	1114	590	1585	294	5149	106	410	3617	26
Grp Volume(v), veh/h	47	0	0	27	0	53	45	864	470	38	818	861
Grp Sat Flow(s),veh/h/ln	1431	0	0	1704	0	1585	294	1702	1851	410	1777	1866
Q Serve(g_s), s	2.5	0.0	0.0	0.0	0.0	4.3	5.5	5.6	5.6	2.3	14.1	14.1
Cycle Q Clear(g_c), s	4.3	0.0	0.0	1.9	0.0	4.3	19.6	5.6	5.6	7.8	14.1	14.1
Prop In Lane	0.38		0.51	0.74		1.00	1.00		0.06	1.00		0.01
Lane Grp Cap(c), veh/h	109	0	0	133	0	79	281	2973	1617	396	1552	1630
V/C Ratio(X)	0.43	0.00	0.00	0.20	0.00	0.67	0.16	0.29	0.29	0.10	0.53	0.53
Avail Cap(c_a), veh/h	359	0	0	378	0	341	281	2973	1617	396	1552	1630
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	60.8	0.0	0.0	59.6	0.0	60.7	4.2	1.4	1.4	2.1	1.9	1.9
Incr Delay (d2), s/veh	2.7	0.0	0.0	0.7	0.0	9.6	1.2	0.2	0.5	0.5	1.3	1.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.6	0.0	0.0	0.9	0.0	1.9	0.4	1.0	1.1	0.2	2.8	2.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	63.4	0.0	0.0	60.3	0.0	70.3	5.5	1.6	1.9	2.5	3.2	3.2
LnGrp LOS	E	A	A	E	A	E	A	A	A	A	A	A
Approach Vol, veh/h		47			80			1379			1717	
Approach Delay, s/veh		63.4			66.9			1.8			3.2	
Approach LOS		E			E			A			A	
Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		118.5		11.5		118.5		11.5				
Change Period (Y+Rc), s		5.0		5.0		5.0		5.0				
Max Green Setting (Gmax), s		92.0		28.0		92.0		28.0				
Max Q Clear Time (g_c+I1), s		21.6		6.3		16.1		6.3				
Green Ext Time (p_c), s		15.0		0.2		23.2		0.2				
Intersection Summary												
HCM 6th Ctrl Delay			5.1									
HCM 6th LOS			A									

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	17	176	73	50	165	408	101	887	41	260	1290	19
Future Volume (vph)	17	176	73	50	165	408	101	887	41	260	1290	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	130		135	200		0	205		0	300		165
Storage Lanes	1		1	1		1	1		0	2		1
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.97	0.95	1.00
Frt			0.850			0.850		0.993				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1770	1863	1583	1770	3514	0	3433	3539	1583
Flt Permitted	0.435			0.400			0.950			0.950		
Satd. Flow (perm)	810	1863	1583	745	1863	1583	1770	3514	0	3433	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			92			80		5				50
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		647			786			659			504	
Travel Time (s)		12.6			15.3			12.8			9.8	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	17	180	74	51	168	416	103	905	42	265	1316	19
Shared Lane Traffic (%)												
Lane Group Flow (vph)	17	180	74	51	168	416	103	947	0	265	1316	19
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes										
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2	1	1	2	1	1	2		1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (ft)	20	100	20	20	100	20	20	100		20	100	20
Trailing Detector (ft)	0	0	0	0	0	0	0	0		0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0	0	0		0	0	0
Detector 1 Size(ft)	20	6	20	20	6	20	20	6		20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	Perm	Perm	NA	pm+ov	Prot	NA		Prot	NA	Perm
Protected Phases		4			8	1	5	2		1	6	
Permitted Phases	4		4	8		8						6

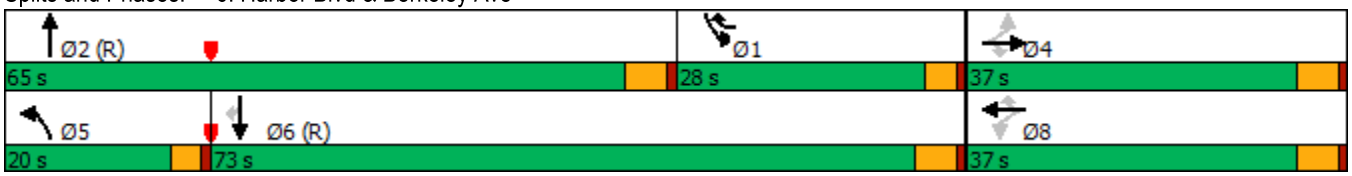


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	4	8	8	1	5	2		1	6	6
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0	6.0
Minimum Split (s)	30.0	30.0	30.0	36.0	36.0	10.0	10.0	30.0		10.0	30.0	30.0
Total Split (s)	37.0	37.0	37.0	37.0	37.0	28.0	20.0	65.0		28.0	73.0	73.0
Total Split (%)	28.5%	28.5%	28.5%	28.5%	28.5%	21.5%	15.4%	50.0%		21.5%	56.2%	56.2%
Maximum Green (s)	32.0	32.0	32.0	32.0	32.0	24.0	16.0	60.0		24.0	68.0	68.0
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	3.0	3.0	4.0		3.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	4.0	4.0	5.0		4.0	5.0	5.0
Lead/Lag						Lag	Lead	Lead		Lag	Lag	Lag
Lead-Lag Optimize?						Yes	Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max		None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0			7.0			7.0	7.0
Flash Dont Walk (s)	18.0	18.0	18.0	24.0	24.0			18.0			18.0	18.0
Pedestrian Calls (#/hr)	5	5	5	5	5			5			5	5
Act Effct Green (s)	19.3	19.3	19.3	19.3	19.3	48.3	12.7	72.7		24.0	84.0	84.0
Actuated g/C Ratio	0.15	0.15	0.15	0.15	0.15	0.37	0.10	0.56		0.18	0.65	0.65
v/c Ratio	0.14	0.65	0.24	0.46	0.61	0.65	0.60	0.48		0.42	0.58	0.02
Control Delay	47.1	62.3	6.7	61.9	60.1	31.3	69.8	19.2		41.0	9.8	0.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.1	0.0
Total Delay	47.1	62.3	6.7	61.9	60.1	31.3	69.8	19.2		41.0	9.9	0.4
LOS	D	E	A	E	E	C	E	B		D	A	A
Approach Delay		46.2			41.4			24.2			14.9	
Approach LOS		D			D			C			B	
Queue Length 50th (ft)	13	147	0	40	136	240	84	235		105	250	0
Queue Length 95th (ft)	33	200	27	76	188	300	142	363		142	386	m1
Internal Link Dist (ft)		567			706			579			424	
Turn Bay Length (ft)	130		135	200			205			300		165
Base Capacity (vph)	199	458	459	183	458	638	220	1968		633	2287	1040
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	168	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	0
Reduced v/c Ratio	0.09	0.39	0.16	0.28	0.37	0.65	0.47	0.48		0.42	0.62	0.02

Intersection Summary

Area Type: Other
Cycle Length: 130
Actuated Cycle Length: 130
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBT, Start of Green
Natural Cycle: 80
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.65
Intersection Signal Delay: 24.8 Intersection LOS: C
Intersection Capacity Utilization 71.4% ICU Level of Service C
Analysis Period (min) 15
m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Harbor Blvd & Berkeley Ave



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	17	176	73	50	165	408	101	887	41	260	1290	19
Future Volume (veh/h)	17	176	73	50	165	408	101	887	41	260	1290	19
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	17	180	74	51	168	416	103	905	42	265	1316	19
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	124	309	262	141	309	671	127	1596	74	891	2330	1039
Arrive On Green	0.17	0.17	0.17	0.17	0.17	0.17	0.07	0.46	0.46	0.26	0.66	0.66
Sat Flow, veh/h	831	1870	1585	1126	1870	1585	1781	3458	160	3456	3554	1585
Grp Volume(v), veh/h	17	180	74	51	168	416	103	465	482	265	1316	19
Grp Sat Flow(s),veh/h/ln	831	1870	1585	1126	1870	1585	1781	1777	1841	1728	1777	1585
Q Serve(g_s), s	2.5	11.6	5.3	5.7	10.7	0.0	7.4	24.8	24.8	8.0	26.3	0.5
Cycle Q Clear(g_c), s	13.2	11.6	5.3	17.3	10.7	0.0	7.4	24.8	24.8	8.0	26.3	0.5
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.09	1.00		1.00
Lane Grp Cap(c), veh/h	124	309	262	141	309	671	127	820	850	891	2330	1039
V/C Ratio(X)	0.14	0.58	0.28	0.36	0.54	0.62	0.81	0.57	0.57	0.30	0.56	0.02
Avail Cap(c_a), veh/h	191	460	390	232	460	799	219	820	850	891	2330	1039
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	55.8	50.1	47.5	58.1	49.8	29.3	59.5	25.5	25.5	38.8	12.2	7.8
Incr Delay (d2), s/veh	0.5	1.7	0.6	1.5	1.5	1.1	11.5	2.8	2.7	0.2	1.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.5	5.5	2.1	1.7	5.1	10.2	3.7	10.9	11.3	3.4	10.1	0.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	56.3	51.9	48.1	59.6	51.3	30.4	70.9	28.4	28.3	39.0	13.2	7.8
LnGrp LOS	E	D	D	E	D	C	E	C	C	D	B	A
Approach Vol, veh/h		271			635			1050			1600	
Approach Delay, s/veh		51.1			38.3			32.5			17.4	
Approach LOS		D			D			C			B	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	38.5	65.0		26.5	13.3	90.2		26.5				
Change Period (Y+Rc), s	5.0	* 5		5.0	4.0	5.0		5.0				
Max Green Setting (Gmax), s	24.0	* 60		32.0	16.0	68.0		32.0				
Max Q Clear Time (g_c+I1), s	10.0	26.8		15.2	9.4	28.3		19.3				
Green Ext Time (p_c), s	0.7	6.9		1.1	0.1	13.1		2.2				

Intersection Summary			
HCM 6th Ctrl Delay		28.2	
HCM 6th LOS		C	

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	293	1114	125	133	833	300	189	1079	203	321	796	338
Future Volume (vph)	293	1114	125	133	833	300	189	1079	203	321	796	338
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	325		0	193		145	230		150	275		0
Storage Lanes	1		0	2		1	2		1	2		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	0.91	0.91	0.97	0.91	1.00	0.97	0.91	1.00	0.97	0.91	0.91
Frt		0.985				0.850			0.850		0.955	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	5009	0	3433	5085	1583	3433	5085	1583	3433	4856	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	5009	0	3433	5085	1583	3433	5085	1583	3433	4856	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		18				248			141		84	
Link Speed (mph)		40			40			45			50	
Link Distance (ft)		1043			981			428			926	
Travel Time (s)		17.8			16.7			6.5			12.6	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	302	1148	129	137	859	309	195	1112	209	331	821	348
Shared Lane Traffic (%)												
Lane Group Flow (vph)	302	1277	0	137	859	309	195	1112	209	331	1169	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		24			24			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2	1	1	2	1	1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru	Right	Left	Thru	
Leading Detector (ft)	20	100		20	100	20	20	100	20	20	100	
Trailing Detector (ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Position(ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Size(ft)	20	6		20	6	20	20	6	20	20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases						8			2			

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	7	4		3	8	8	5	2	2	1	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Minimum Split (s)	10.0	43.0		10.0	43.0	43.0	10.0	43.0	43.0	10.0	36.0	
Total Split (s)	27.0	57.0		13.0	43.0	43.0	16.0	43.0	43.0	17.0	44.0	
Total Split (%)	20.8%	43.8%		10.0%	33.1%	33.1%	12.3%	33.1%	33.1%	13.1%	33.8%	
Maximum Green (s)	23.0	52.0		9.0	38.0	38.0	12.0	38.0	38.0	13.0	39.0	
Yellow Time (s)	3.0	4.0		3.0	4.0	4.0	3.0	4.0	4.0	3.0	4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	4.0	5.0		4.0	5.0	5.0	4.0	5.0	5.0	4.0	5.0	
Lead/Lag	Lag	Lag		Lead	Lead	Lead	Lag	Lag	Lag	Lead	Lead	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None		None	None	None	None	C-Max	C-Max	None	C-Max	
Walk Time (s)		7.0			7.0	7.0		7.0	7.0		7.0	
Flash Dont Walk (s)		31.0			31.0	31.0		31.0	31.0		24.0	
Pedestrian Calls (#/hr)		5			5	5		5	5		5	
Act Effct Green (s)	26.4	48.9		8.7	31.2	31.2	12.0	40.0	40.0	14.4	42.4	
Actuated g/C Ratio	0.20	0.38		0.07	0.24	0.24	0.09	0.31	0.31	0.11	0.33	
v/c Ratio	0.84	0.67		0.60	0.70	0.54	0.62	0.71	0.36	0.87	0.71	
Control Delay	70.5	35.1		70.1	48.2	12.7	73.0	51.2	24.3	80.0	39.2	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	70.5	35.1		70.1	48.2	12.7	73.0	51.2	24.3	80.0	39.2	
LOS	E	D		E	D	B	E	D	C	F	D	
Approach Delay		41.9			42.1			50.3			48.2	
Approach LOS		D			D			D			D	
Queue Length 50th (ft)	239	309		58	243	41	80	329	69	145	307	
Queue Length 95th (ft)	#438	360		93	272	121	m130	362	m110	#241	364	
Internal Link Dist (ft)		963			901			348			846	
Turn Bay Length (ft)	325			193		145	230		150	275		
Base Capacity (vph)	360	2014		237	1486	638	316	1563	584	380	1639	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.84	0.63		0.58	0.58	0.48	0.62	0.71	0.36	0.87	0.71	

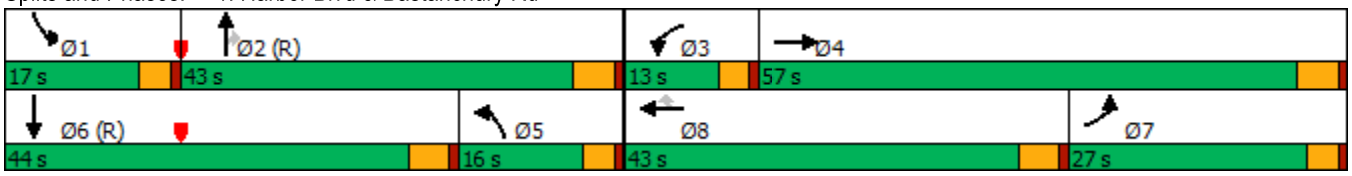
Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 5 (4%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 120			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.87			
Intersection Signal Delay: 45.7		Intersection LOS: D	
Intersection Capacity Utilization 77.3%		ICU Level of Service D	
Analysis Period (min) 15			

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Harbor Blvd & Bastanchury Rd



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	293	1114	125	133	833	300	189	1079	203	321	796	338
Future Volume (veh/h)	293	1114	125	133	833	300	189	1079	203	321	796	338
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	302	1148	129	137	859	309	195	1112	209	331	821	348
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	315	1629	183	188	1122	348	517	1824	566	346	1057	445
Arrive On Green	0.18	0.35	0.35	0.05	0.22	0.22	0.15	0.36	0.36	0.10	0.30	0.30
Sat Flow, veh/h	1781	4657	523	3456	5106	1585	3456	5106	1585	3456	3522	1485
Grp Volume(v), veh/h	302	839	438	137	859	309	195	1112	209	331	793	376
Grp Sat Flow(s),veh/h/ln	1781	1702	1776	1728	1702	1585	1728	1702	1585	1728	1702	1603
Q Serve(g_s), s	21.8	27.6	27.7	5.1	20.5	19.2	6.6	23.3	12.7	12.4	27.7	27.9
Cycle Q Clear(g_c), s	21.8	27.6	27.7	5.1	20.5	19.2	6.6	23.3	12.7	12.4	27.7	27.9
Prop In Lane	1.00		0.29	1.00		1.00	1.00		1.00	1.00		0.93
Lane Grp Cap(c), veh/h	315	1191	621	188	1122	348	517	1824	566	346	1021	481
V/C Ratio(X)	0.96	0.70	0.71	0.73	0.77	0.89	0.38	0.61	0.37	0.96	0.78	0.78
Avail Cap(c_a), veh/h	315	1362	710	239	1493	463	517	1824	566	346	1021	481
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	53.0	36.5	36.5	60.5	47.6	30.2	49.8	34.3	30.9	58.2	41.5	41.6
Incr Delay (d2), s/veh	39.5	1.4	2.7	7.9	1.7	15.0	0.5	1.5	1.8	37.3	5.8	11.9
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	13.0	11.5	12.2	2.4	8.7	8.6	2.8	9.6	5.1	7.0	11.9	12.1
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	92.5	37.9	39.2	68.4	49.3	45.2	50.3	35.9	32.8	95.5	47.3	53.5
LnGrp LOS	F	D	D	E	D	D	D	D	C	F	D	D
Approach Vol, veh/h		1579			1305			1516			1500	
Approach Delay, s/veh		48.7			50.3			37.3			59.5	
Approach LOS		D			D			D			E	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	17.0	51.4	11.1	50.5	24.4	44.0	28.0	33.6
Change Period (Y+Rc), s	4.0	5.0	4.0	5.0	5.0	* 5	5.0	* 5
Max Green Setting (Gmax), s	13.0	38.0	9.0	52.0	12.0	* 39	23.0	* 38
Max Q Clear Time (g_c+I1), s	14.4	25.3	7.1	29.7	8.6	29.9	23.8	22.5
Green Ext Time (p_c), s	0.0	6.3	0.1	8.8	0.2	4.6	0.0	6.0

Intersection Summary	
HCM 6th Ctrl Delay	48.9
HCM 6th LOS	D

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	119	22	132	71	41	199	48	1153	54	86	881	82
Future Volume (vph)	119	22	132	71	41	199	48	1153	54	86	881	82
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	55		85	0		0	185		0	230		0
Storage Lanes	1		1	1		0	1		0	1		1
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	1.00
Frt			0.850		0.876			0.993				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1770	1632	0	1770	3514	0	1770	3539	1583
Flt Permitted	0.314			0.742			0.950			0.950		
Satd. Flow (perm)	585	1863	1583	1382	1632	0	1770	3514	0	1770	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			112		169			5				86
Link Speed (mph)		30			30			50			45	
Link Distance (ft)		813			705			616			406	
Travel Time (s)		18.5			16.0			8.4			6.2	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Adj. Flow (vph)	125	23	139	75	43	209	51	1214	57	91	927	86
Shared Lane Traffic (%)												
Lane Group Flow (vph)	125	23	139	75	252	0	51	1271	0	91	927	86
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2	1	1	2		1	2		1	2	1
Detector Template	Left	Thru	Right	Left	Thru		Left	Thru		Left	Thru	Right
Leading Detector (ft)	20	100	20	20	100		20	100		20	100	20
Trailing Detector (ft)	0	0	0	0	0		0	0		0	0	0
Detector 1 Position(ft)	0	0	0	0	0		0	0		0	0	0
Detector 1 Size(ft)	20	6	20	20	6		20	6		20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	pm+ov	Perm	NA		Prot	NA		Prot	NA	Perm
Protected Phases		4	5		8		5	2		1	6	
Permitted Phases	4		4	8								6

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	5	8	8		5	2		1	6	6
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0		6.0	6.0		6.0	6.0	6.0
Minimum Split (s)	11.0	11.0	10.0	36.0	36.0		10.0	26.0		10.0	30.0	30.0
Total Split (s)	42.0	42.0	13.0	42.0	42.0		13.0	71.0		17.0	75.0	75.0
Total Split (%)	32.3%	32.3%	10.0%	32.3%	32.3%		10.0%	54.6%		13.1%	57.7%	57.7%
Maximum Green (s)	37.0	37.0	9.0	37.0	37.0		9.0	66.0		13.0	70.0	70.0
Yellow Time (s)	4.0	4.0	3.0	4.0	4.0		3.0	4.0		3.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0		1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	4.0	5.0	5.0		4.0	5.0		4.0	5.0	5.0
Lead/Lag			Lag				Lag	Lead		Lag	Lead	Lead
Lead-Lag Optimize?			Yes				Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0
Recall Mode	None	None	None	None	None		None	C-Max		None	C-Max	C-Max
Walk Time (s)				7.0	7.0			7.0			7.0	7.0
Flash Dont Walk (s)				24.0	24.0			14.0			18.0	18.0
Pedestrian Calls (#/hr)				5	5			5			5	5
Act Effct Green (s)	25.2	25.2	38.3	25.2	25.2		8.1	78.7		12.1	82.7	82.7
Actuated g/C Ratio	0.19	0.19	0.29	0.19	0.19		0.06	0.61		0.09	0.64	0.64
v/c Ratio	1.12	0.06	0.26	0.28	0.56		0.46	0.60		0.55	0.41	0.08
Control Delay	164.9	38.7	8.9	44.5	19.4		49.9	9.4		64.1	4.6	0.1
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Delay	164.9	38.7	8.9	44.5	19.4		49.9	9.4		64.1	4.6	0.1
LOS	F	D	A	D	B		D	A		E	A	A
Approach Delay		79.2			25.1			11.0			9.2	
Approach LOS		E			C			B			A	
Queue Length 50th (ft)	~119	16	16	54	60		43	91		79	58	0
Queue Length 95th (ft)	#206	37	57	91	131		88	215		m116	73	m0
Internal Link Dist (ft)		733			625			536			326	
Turn Bay Length (ft)	55		85				185			230		
Base Capacity (vph)	166	530	536	393	585		122	2129		177	2251	1038
Starvation Cap Reductn	0	0	0	0	0		0	0		0	0	0
Spillback Cap Reductn	0	0	0	0	0		0	0		0	0	0
Storage Cap Reductn	0	0	0	0	0		0	0		0	0	0
Reduced v/c Ratio	0.75	0.04	0.26	0.19	0.43		0.42	0.60		0.51	0.41	0.08

Intersection Summary

Area Type: Other
Cycle Length: 130
Actuated Cycle Length: 130
Offset: 2 (2%), Referenced to phase 2:NBT and 6:SBT, Start of Green
Natural Cycle: 80
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 1.12
Intersection Signal Delay: 18.3 Intersection LOS: B
Intersection Capacity Utilization 75.4% ICU Level of Service D
Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Harbor Blvd & Valencia Mesa

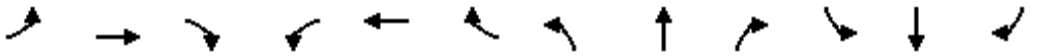


Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	119	22	132	71	41	199	48	1153	54	86	881	82
Future Volume (veh/h)	119	22	132	71	41	199	48	1153	54	86	881	82
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	125	23	139	75	43	209	51	1214	57	91	927	86
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	198	490	561	365	73	354	164	1755	82	218	1914	853
Arrive On Green	0.26	0.26	0.26	0.26	0.26	0.26	0.09	0.51	0.51	0.12	0.54	0.54
Sat Flow, veh/h	1128	1870	1585	1224	278	1350	1781	3456	162	1781	3554	1585
Grp Volume(v), veh/h	125	23	139	75	0	252	51	624	647	91	927	86
Grp Sat Flow(s),veh/h/ln	1128	1870	1585	1224	0	1627	1781	1777	1841	1781	1777	1585
Q Serve(g_s), s	14.2	1.2	0.0	6.3	0.0	17.6	3.5	34.6	34.7	6.1	21.2	3.4
Cycle Q Clear(g_c), s	31.7	1.2	0.0	7.5	0.0	17.6	3.5	34.6	34.7	6.1	21.2	3.4
Prop In Lane	1.00		1.00	1.00		0.83	1.00		0.09	1.00		1.00
Lane Grp Cap(c), veh/h	198	490	561	365	0	426	164	902	935	218	1914	853
V/C Ratio(X)	0.63	0.05	0.25	0.21	0.00	0.59	0.31	0.69	0.69	0.42	0.48	0.10
Avail Cap(c_a), veh/h	224	532	597	393	0	463	164	902	935	218	1914	853
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	55.7	35.8	29.7	38.7	0.0	41.9	55.2	24.3	24.3	52.7	18.7	14.6
Incr Delay (d2), s/veh	4.6	0.0	0.2	0.3	0.0	1.7	1.1	4.3	4.2	1.3	0.9	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	4.3	0.6	3.1	2.0	0.0	7.3	1.6	14.5	15.0	2.8	8.4	1.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	60.4	35.9	30.0	38.9	0.0	43.6	56.3	28.6	28.5	54.0	19.6	14.9
LnGrp LOS	E	D	C	D	A	D	E	C	C	D	B	B
Approach Vol, veh/h		287			327			1322			1104	
Approach Delay, s/veh		43.7			42.5			29.6			22.1	
Approach LOS		D			D			C			C	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	19.9	71.0		39.1	15.9	75.0		39.1				
Change Period (Y+Rc), s	4.0	5.0		5.0	4.0	5.0		5.0				
Max Green Setting (Gmax), s	13.0	66.0		37.0	9.0	70.0		37.0				
Max Q Clear Time (g_c+I1), s	8.1	36.7		33.7	5.5	23.2		19.6				
Green Ext Time (p_c), s	0.1	8.9		0.3	0.0	7.6		1.7				

Intersection Summary												
HCM 6th Ctrl Delay			29.6									
HCM 6th LOS			C									

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	68	179	26	469	97	111	9	1024	605	97	1000	50
Future Volume (vph)	68	179	26	469	97	111	9	1024	605	97	1000	50
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	85		0	0		0	95		0	140		140
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950	0.968		0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1681	1713	1583	1770	3539	1583	1770	3539	1583
Flt Permitted	0.950			0.950	0.968		0.950			0.950		
Satd. Flow (perm)	1770	1863	1583	1681	1713	1583	1770	3539	1583	1770	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			134			134			342			101
Link Speed (mph)		25			45			35			50	
Link Distance (ft)		871			1039			280			363	
Travel Time (s)		23.8			15.7			5.5			5.0	
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Adj. Flow (vph)	70	185	27	484	100	114	9	1056	624	100	1031	52
Shared Lane Traffic (%)				40%								
Lane Group Flow (vph)	70	185	27	290	294	114	9	1056	624	100	1031	52
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (ft)	20	100	20	20	100	20	20	100	20	20	100	20
Trailing Detector (ft)	0	0	0	0	0	0	0	0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0	0	0	0	0	0	0
Detector 1 Size(ft)	20	6	20	20	6	20	20	6	20	20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Split	NA	Perm	Split	NA	Perm	Prot	NA	Free	Prot	NA	Perm
Protected Phases	4	4		8	8		5	2		1	6	
Permitted Phases			4			8			Free			6



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	4	8	8	8	5	2		1	6	6
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0	6.0
Minimum Split (s)	11.0	11.0	11.0	36.0	36.0	36.0	10.0	23.0		10.0	30.0	30.0
Total Split (s)	24.0	24.0	24.0	37.0	37.0	37.0	10.0	53.0		16.0	59.0	59.0
Total Split (%)	18.5%	18.5%	18.5%	28.5%	28.5%	28.5%	7.7%	40.8%		12.3%	45.4%	45.4%
Maximum Green (s)	19.0	19.0	19.0	32.0	32.0	32.0	6.0	48.0		12.0	54.0	54.0
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	3.0	4.0		3.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	4.0	5.0		4.0	5.0	5.0
Lead/Lag							Lag	Lag		Lead	Lead	Lead
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max		None	C-Max	C-Max
Walk Time (s)				7.0	7.0	7.0		7.0			7.0	7.0
Flash Dont Walk (s)				24.0	24.0	24.0		11.0			18.0	18.0
Pedestrian Calls (#/hr)				5	5	5		5			5	5
Act Effct Green (s)	16.7	16.7	16.7	27.2	27.2	27.2	6.0	56.0	130.0	11.1	69.0	69.0
Actuated g/C Ratio	0.13	0.13	0.13	0.21	0.21	0.21	0.05	0.43	1.00	0.09	0.53	0.53
v/c Ratio	0.31	0.77	0.08	0.82	0.82	0.26	0.11	0.69	0.39	0.67	0.55	0.06
Control Delay	54.2	75.8	0.5	68.0	67.2	5.5	50.4	25.0	0.8	89.8	15.5	1.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	54.2	75.8	0.5	68.0	67.2	5.5	50.4	25.0	0.8	89.8	15.5	1.6
LOS	D	E	A	E	E	A	D	C	A	F	B	A
Approach Delay		63.3			57.4			16.2			21.2	
Approach LOS		E			E			B			C	
Queue Length 50th (ft)	54	151	0	245	248	0	7	433	1	89	151	0
Queue Length 95th (ft)	101	#234	0	345	348	34	m19	447	10	#155	316	9
Internal Link Dist (ft)		791			959			200			283	
Turn Bay Length (ft)	85						95			140		140
Base Capacity (vph)	258	272	345	413	421	490	81	1523	1583	165	1879	887
Starvation Cap Reductn	0	0	0	0	0	0	0	15	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.27	0.68	0.08	0.70	0.70	0.23	0.11	0.70	0.39	0.61	0.55	0.06

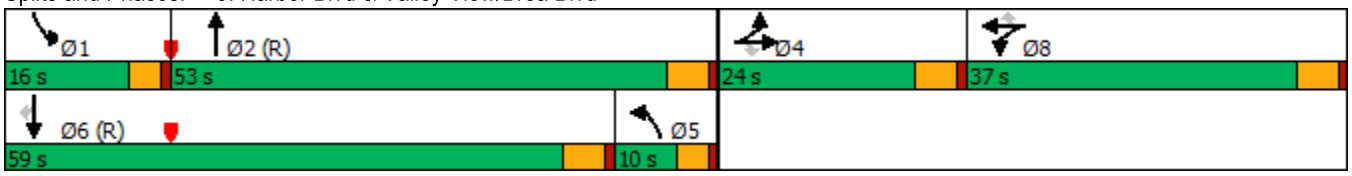
Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 72 (55%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 90			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.82			
Intersection Signal Delay: 28.7		Intersection LOS: C	
Intersection Capacity Utilization 74.5%		ICU Level of Service D	
Analysis Period (min) 15			

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Harbor Blvd & Valley View/Brea Blvd



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	68	179	26	469	97	111	9	1024	605	97	1000	50
Future Volume (veh/h)	68	179	26	469	97	111	9	1024	605	97	1000	50
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	70	185	27	555	0	0	9	1056	0	100	1031	52
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	207	218	185	636	0		242	1740		123	1476	658
Arrive On Green	0.12	0.12	0.12	0.18	0.00	0.00	0.14	0.49	0.00	0.07	0.42	0.42
Sat Flow, veh/h	1781	1870	1585	3563	0	1585	1781	3554	1585	1781	3554	1585
Grp Volume(v), veh/h	70	185	27	555	0	0	9	1056	0	100	1031	52
Grp Sat Flow(s),veh/h/ln	1781	1870	1585	1781	0	1585	1781	1777	1585	1781	1777	1585
Q Serve(g_s), s	4.7	12.6	2.0	19.7	0.0	0.0	0.6	28.1	0.0	7.2	31.1	2.6
Cycle Q Clear(g_c), s	4.7	12.6	2.0	19.7	0.0	0.0	0.6	28.1	0.0	7.2	31.1	2.6
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	207	218	185	636	0		242	1740		123	1476	658
V/C Ratio(X)	0.34	0.85	0.15	0.87	0.00		0.04	0.61		0.81	0.70	0.08
Avail Cap(c_a), veh/h	260	273	232	877	0		242	1740		164	1476	658
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	52.8	56.3	51.6	51.9	0.0	0.0	48.8	24.1	0.0	59.7	31.3	23.0
Incr Delay (d2), s/veh	1.0	18.2	0.4	7.3	0.0	0.0	0.1	1.6	0.0	19.7	2.8	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.2	7.1	0.8	9.2	0.0	0.0	0.3	11.9	0.0	3.8	13.2	1.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	53.8	74.5	52.0	59.2	0.0	0.0	48.9	25.7	0.0	79.3	34.1	23.2
LnGrp LOS	D	E	D	E	A		D	C		E	C	C
Approach Vol, veh/h		282			555			1065			1183	
Approach Delay, s/veh		67.2			59.2			25.9			37.4	
Approach LOS		E			E			C			D	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	13.0	68.7		20.1	22.6	59.0		28.2				
Change Period (Y+Rc), s	4.0	5.0		5.0	5.0	* 5		5.0				
Max Green Setting (Gmax), s	12.0	48.0		19.0	6.0	* 54		32.0				
Max Q Clear Time (g_c+I1), s	9.2	30.1		14.6	2.6	33.1		21.7				
Green Ext Time (p_c), s	0.0	7.1		0.5	0.0	6.9		1.5				

Intersection Summary	
HCM 6th Ctrl Delay	40.1
HCM 6th LOS	D

Notes

User approved volume balancing among the lanes for turning movement.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

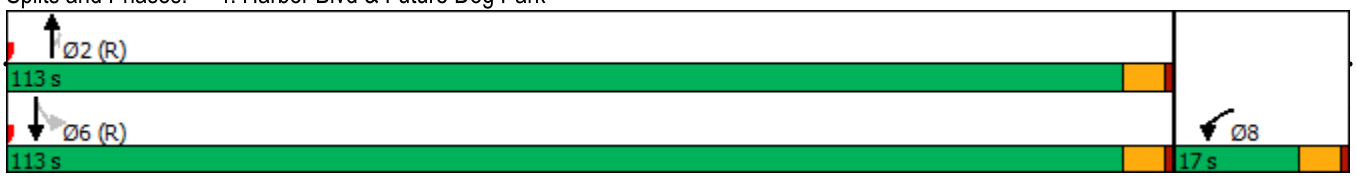


Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	5	5	1198	5	5	1079
Future Volume (vph)	5	5	1198	5	5	1079
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		200	250	
Storage Lanes	1	0		1	1	
Taper Length (ft)	60				60	
Lane Util. Factor	1.00	1.00	0.95	1.00	1.00	0.95
Frt	0.932			0.850		
Flt Protected	0.976				0.950	
Satd. Flow (prot)	1694	0	3539	1583	1770	3539
Flt Permitted	0.976				0.233	
Satd. Flow (perm)	1694	0	3539	1583	434	3539
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)	5			5		
Link Speed (mph)	30		30			30
Link Distance (ft)	484		960			1191
Travel Time (s)	11.0		21.8			27.1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	5	5	1198	5	5	1079
Shared Lane Traffic (%)						
Lane Group Flow (vph)	10	0	1198	5	5	1079
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60	60		60	60	
Number of Detectors	1		2	1	1	2
Detector Template	Left		Thru	Right	Left	Thru
Leading Detector (ft)	20		100	20	20	100
Trailing Detector (ft)	0		0	0	0	0
Detector 1 Position(ft)	0		0	0	0	0
Detector 1 Size(ft)	20		6	20	20	6
Detector 1 Type	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0		0.0	0.0	0.0	0.0
Detector 2 Position(ft)			94			94
Detector 2 Size(ft)			6			6
Detector 2 Type			Cl+Ex			Cl+Ex
Detector 2 Channel						
Detector 2 Extend (s)			0.0			0.0
Turn Type	Prot		NA	Perm	Perm	NA
Protected Phases	8		2			6
Permitted Phases				2	6	

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Detector Phase	8		2	2	6	6
Switch Phase						
Minimum Initial (s)	6.0		6.0	6.0	6.0	6.0
Minimum Split (s)	11.0		11.0	11.0	11.0	11.0
Total Split (s)	17.0		113.0	113.0	113.0	113.0
Total Split (%)	13.1%		86.9%	86.9%	86.9%	86.9%
Maximum Green (s)	12.0		108.0	108.0	108.0	108.0
Yellow Time (s)	4.0		4.0	4.0	4.0	4.0
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0		5.0	5.0	5.0	5.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	None		C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	6.4		126.5	126.5	126.5	126.5
Actuated g/C Ratio	0.05		0.97	0.97	0.97	0.97
v/c Ratio	0.11		0.35	0.00	0.01	0.31
Control Delay	46.1		1.2	0.0	0.8	0.6
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	46.1		1.2	0.0	0.8	0.6
LOS	D		A	A	A	A
Approach Delay	46.1		1.2			0.6
Approach LOS	D		A			A
Queue Length 50th (ft)	4		0	0	0	0
Queue Length 95th (ft)	23		3	m1	m1	65
Internal Link Dist (ft)	404		880			1111
Turn Bay Length (ft)				200	250	
Base Capacity (vph)	160		3444	1541	422	3444
Starvation Cap Reductn	0		0	0	0	0
Spillback Cap Reductn	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	0.06		0.35	0.00	0.01	0.31

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Green			
Natural Cycle: 40			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.35			
Intersection Signal Delay: 1.1		Intersection LOS: A	
Intersection Capacity Utilization 46.4%		ICU Level of Service A	
Analysis Period (min) 15			
m Volume for 95th percentile queue is metered by upstream signal.			

Splits and Phases: 4: Harbor Blvd & Future Dog Park





Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	5	5	1198	5	5	1079
Future Volume (veh/h)	5	5	1198	5	5	1079
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	5	5	1198	5	5	1079
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	12	12	3227	1439	458	3227
Arrive On Green	0.02	0.02	0.91	0.91	0.91	0.91
Sat Flow, veh/h	770	770	3647	1585	467	3647
Grp Volume(v), veh/h	11	0	1198	5	5	1079
Grp Sat Flow(s),veh/h/ln	1693	0	1777	1585	467	1777
Q Serve(g_s), s	0.8	0.0	6.1	0.0	0.2	5.2
Cycle Q Clear(g_c), s	0.8	0.0	6.1	0.0	6.3	5.2
Prop In Lane	0.45	0.45		1.00	1.00	
Lane Grp Cap(c), veh/h	26	0	3227	1439	458	3227
V/C Ratio(X)	0.43	0.00	0.37	0.00	0.01	0.33
Avail Cap(c_a), veh/h	156	0	3227	1439	458	3227
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	63.5	0.0	0.8	0.6	1.3	0.8
Incr Delay (d2), s/veh	11.0	0.0	0.3	0.0	0.0	0.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	0.0	0.5	0.0	0.0	0.5
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	74.4	0.0	1.2	0.6	1.3	1.1
LnGrp LOS	E	A	A	A	A	A
Approach Vol, veh/h	11		1203			1084
Approach Delay, s/veh	74.4		1.2			1.1
Approach LOS	E		A			A

Timer - Assigned Phs		2			6	8
Phs Duration (G+Y+Rc), s		123.0			123.0	7.0
Change Period (Y+Rc), s		5.0			5.0	5.0
Max Green Setting (Gmax), s		108.0			108.0	12.0
Max Q Clear Time (g_c+I1), s		8.1			8.3	2.8
Green Ext Time (p_c), s		13.1			11.1	0.0

Intersection Summary	
HCM 6th Ctrl Delay	1.5
HCM 6th LOS	A

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	47	21	65	14	11	43	60	1608	24	48	1455	42
Future Volume (vph)	47	21	65	14	11	43	60	1608	24	48	1455	42
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		65	95		0	100		0
Storage Lanes	0		0	0		1	1		0	1		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.95	0.95
Frt		0.934				0.850		0.998			0.996	
Flt Protected		0.983			0.973		0.950			0.950		
Satd. Flow (prot)	0	1710	0	0	1812	1583	1770	5075	0	1770	3525	0
Flt Permitted		0.872			0.775		0.143			0.125		
Satd. Flow (perm)	0	1517	0	0	1444	1583	266	5075	0	233	3525	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		34				44		4			5	
Link Speed (mph)		25			25			35			35	
Link Distance (ft)		427			781			235			407	
Travel Time (s)		11.6			21.3			4.6			7.9	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	48	21	66	14	11	44	61	1641	24	49	1485	43
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	135	0	0	25	44	61	1665	0	49	1528	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60		60	60		60	60		60	60		60
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100	20	20	100		20	100	
Trailing Detector (ft)	0	0		0	0	0	0	0		0	0	
Detector 1 Position(ft)	0	0		0	0	0	0	0		0	0	
Detector 1 Size(ft)	20	6		20	6	20	20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		

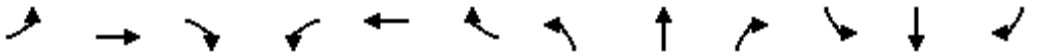
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4		8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0		6.0	6.0	
Minimum Split (s)	33.0	33.0		33.0	33.0	33.0	26.0	26.0		26.0	26.0	
Total Split (s)	35.0	35.0		35.0	35.0	35.0	95.0	95.0		95.0	95.0	
Total Split (%)	26.9%	26.9%		26.9%	26.9%	26.9%	73.1%	73.1%		73.1%	73.1%	
Maximum Green (s)	30.0	30.0		30.0	30.0	30.0	90.0	90.0		90.0	90.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	4.0	4.0		4.0	4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.0			5.0	5.0	5.0	5.0		5.0	5.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None	None	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	21.0	21.0		21.0	21.0	21.0	14.0	14.0		14.0	14.0	
Pedestrian Calls (#/hr)	5	5		5	5	5	5	5		5	5	
Act Effct Green (s)		15.5			15.5	15.5	104.5	104.5		104.5	104.5	
Actuated g/C Ratio		0.12			0.12	0.12	0.80	0.80		0.80	0.80	
v/c Ratio		0.64			0.15	0.19	0.29	0.41		0.26	0.54	
Control Delay		53.0			49.2	14.4	5.8	2.9		3.5	1.3	
Queue Delay		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		53.0			49.2	14.4	5.8	2.9		3.5	1.3	
LOS		D			D	B	A	A		A	A	
Approach Delay		53.0			27.0			3.0			1.4	
Approach LOS		D			C			A			A	
Queue Length 50th (ft)		83			19	0	8	79		1	16	
Queue Length 95th (ft)		136			42	32	m15	112		m1	14	
Internal Link Dist (ft)		347			701			155			327	
Turn Bay Length (ft)						65	95			100		
Base Capacity (vph)		376			333	399	213	4081		187	2835	
Starvation Cap Reductn		0			0	0	0	0		0	86	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.36			0.08	0.11	0.29	0.41		0.26	0.56	

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 41 (32%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green			
Natural Cycle: 70			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.64			
Intersection Signal Delay: 4.7		Intersection LOS: A	
Intersection Capacity Utilization 72.6%		ICU Level of Service C	
Analysis Period (min) 15			

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Harbor Blvd & Ralph's Dwy/E Valley View Dr



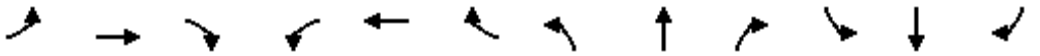


Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	47	21	65	14	11	43	60	1608	24	48	1455	42
Future Volume (veh/h)	47	21	65	14	11	43	60	1608	24	48	1455	42
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	48	21	66	14	11	44	61	1641	24	49	1485	43
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	86	35	80	110	75	165	289	4247	62	275	2889	84
Arrive On Green	0.10	0.10	0.10	0.10	0.10	0.10	0.82	0.82	0.82	0.82	0.82	0.82
Sat Flow, veh/h	469	336	770	643	722	1585	341	5185	76	298	3527	102
Grp Volume(v), veh/h	135	0	0	25	0	44	61	1077	588	49	747	781
Grp Sat Flow(s),veh/h/ln	1576	0	0	1365	0	1585	341	1702	1857	298	1777	1852
Q Serve(g_s), s	8.9	0.0	0.0	0.0	0.0	3.3	8.9	10.9	10.9	6.8	17.1	17.1
Cycle Q Clear(g_c), s	10.9	0.0	0.0	1.6	0.0	3.3	26.0	10.9	10.9	17.6	17.1	17.1
Prop In Lane	0.36		0.49	0.56		1.00	1.00		0.04	1.00		0.06
Lane Grp Cap(c), veh/h	201	0	0	185	0	165	289	2788	1521	275	1455	1517
V/C Ratio(X)	0.67	0.00	0.00	0.14	0.00	0.27	0.21	0.39	0.39	0.18	0.51	0.51
Avail Cap(c_a), veh/h	398	0	0	376	0	366	289	2788	1521	275	1455	1517
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	57.0	0.0	0.0	52.9	0.0	53.7	7.7	3.1	3.1	5.4	3.7	3.7
Incr Delay (d2), s/veh	3.8	0.0	0.0	0.3	0.0	0.9	1.7	0.4	0.7	1.4	1.3	1.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	4.6	0.0	0.0	0.8	0.0	1.4	0.7	2.9	3.3	0.5	4.9	5.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	60.8	0.0	0.0	53.2	0.0	54.5	9.4	3.5	3.9	6.9	5.0	4.9
LnGrp LOS	E	A	A	D	A	D	A	A	A	A	A	A
Approach Vol, veh/h		135			69			1726			1577	
Approach Delay, s/veh		60.8			54.1			3.8			5.0	
Approach LOS		E			D			A			A	

Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		111.5		18.5		111.5		18.5				
Change Period (Y+Rc), s		5.0		5.0		5.0		5.0				
Max Green Setting (Gmax), s		90.0		30.0		90.0		30.0				
Max Q Clear Time (g_c+I1), s		28.0		12.9		19.6		5.3				
Green Ext Time (p_c), s		21.6		0.7		20.1		0.2				

Intersection Summary												
HCM 6th Ctrl Delay			7.5									
HCM 6th LOS			A									

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	28	147	81	56	168	460	87	1191	25	330	1252	21
Future Volume (vph)	28	147	81	56	168	460	87	1191	25	330	1252	21
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	130		135	200		0	205		0	300		165
Storage Lanes	1		1	1		1	1		0	2		1
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.97	0.95	1.00
Frt			0.850			0.850		0.997				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1770	1863	1583	1770	3529	0	3433	3539	1583
Flt Permitted	0.400			0.467			0.950			0.950		
Satd. Flow (perm)	745	1863	1583	870	1863	1583	1770	3529	0	3433	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			92			50		2				50
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		647			786			659			504	
Travel Time (s)		12.6			15.3			12.8			9.8	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Adj. Flow (vph)	29	153	84	58	175	479	91	1241	26	344	1304	22
Shared Lane Traffic (%)												
Lane Group Flow (vph)	29	153	84	58	175	479	91	1267	0	344	1304	22
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes										
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60		60	60		60	60		60	60		60
Number of Detectors	1	2	1	1	2	1	1	2		1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (ft)	20	100	20	20	100	20	20	100		20	100	20
Trailing Detector (ft)	0	0	0	0	0	0	0	0		0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0	0	0		0	0	0
Detector 1 Size(ft)	20	6	20	20	6	20	20	6		20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	Perm	Perm	NA	pm+ov	Prot	NA		Prot	NA	Perm
Protected Phases		4			8	1	5	2		1	6	
Permitted Phases	4		4	8		8						6

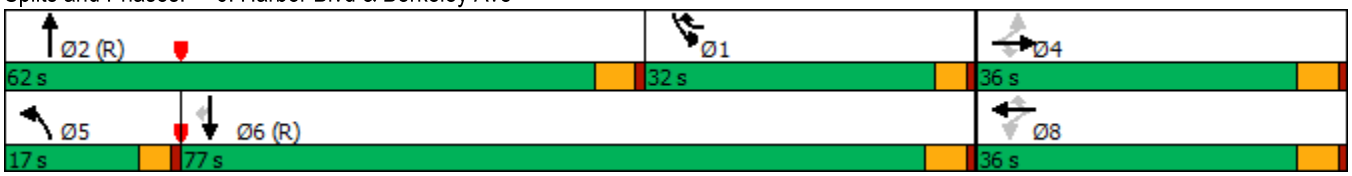


Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	4	8	8	1	5	2		1	6	6
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0	6.0
Minimum Split (s)	30.0	30.0	30.0	33.0	33.0	10.0	10.0	30.0		10.0	30.0	30.0
Total Split (s)	36.0	36.0	36.0	36.0	36.0	32.0	17.0	62.0		32.0	77.0	77.0
Total Split (%)	27.7%	27.7%	27.7%	27.7%	27.7%	24.6%	13.1%	47.7%		24.6%	59.2%	59.2%
Maximum Green (s)	31.0	31.0	31.0	31.0	31.0	28.0	13.0	57.0		28.0	72.0	72.0
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	3.0	3.0	4.0		3.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	4.0	4.0	5.0		4.0	5.0	5.0
Lead/Lag						Lag	Lead	Lead		Lag	Lag	Lag
Lead-Lag Optimize?						Yes	Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max		None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0			7.0			7.0	7.0
Flash Dont Walk (s)	18.0	18.0	18.0	21.0	21.0			18.0			18.0	18.0
Pedestrian Calls (#/hr)	5	5	5	5	5			5			5	5
Act Effct Green (s)	18.4	18.4	18.4	18.4	18.4	51.4	11.9	69.6		28.0	85.7	85.7
Actuated g/C Ratio	0.14	0.14	0.14	0.14	0.14	0.40	0.09	0.54		0.22	0.66	0.66
v/c Ratio	0.28	0.58	0.28	0.47	0.67	0.73	0.57	0.67		0.47	0.56	0.02
Control Delay	53.9	60.0	9.5	62.1	64.3	36.2	69.6	25.1		36.6	8.0	0.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.1	0.0
Total Delay	53.9	60.0	9.5	62.1	64.3	36.2	69.6	25.1		36.6	8.0	0.3
LOS	D	E	A	E	E	D	E	C		D	A	A
Approach Delay		43.4			45.2			28.1			13.8	
Approach LOS		D			D			C			B	
Queue Length 50th (ft)	22	123	0	46	143	311	75	385		116	100	0
Queue Length 95th (ft)	50	178	38	86	202	386	129	553		172	313	m1
Internal Link Dist (ft)		567			706			579			424	
Turn Bay Length (ft)	130		135	200			205			300		165
Base Capacity (vph)	177	444	447	207	444	656	187	1890		739	2334	1061
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	165	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	0
Reduced v/c Ratio	0.16	0.34	0.19	0.28	0.39	0.73	0.49	0.67		0.47	0.60	0.02

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 55 (42%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 80			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.73			
Intersection Signal Delay: 26.2		Intersection LOS: C	
Intersection Capacity Utilization 78.9%		ICU Level of Service D	
Analysis Period (min) 15			

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Harbor Blvd & Berkeley Ave



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	28	147	81	56	168	460	87	1191	25	330	1252	21
Future Volume (veh/h)	28	147	81	56	168	460	87	1191	25	330	1252	21
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	29	153	84	58	175	479	91	1241	26	344	1304	22
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	138	291	247	155	291	707	114	1561	33	1004	2391	1067
Arrive On Green	0.16	0.16	0.16	0.16	0.16	0.16	0.06	0.44	0.44	0.29	0.67	0.67
Sat Flow, veh/h	1210	1870	1585	1234	1870	1585	1781	3559	75	3456	3554	1585
Grp Volume(v), veh/h	29	153	84	58	175	479	91	619	648	344	1304	22
Grp Sat Flow(s),veh/h/ln	1210	1870	1585	1234	1870	1585	1781	1777	1857	1728	1777	1585
Q Serve(g_s), s	3.0	9.8	6.1	5.9	11.3	0.0	6.6	39.1	39.1	10.2	24.6	0.6
Cycle Q Clear(g_c), s	14.3	9.8	6.1	15.7	11.3	0.0	6.6	39.1	39.1	10.2	24.6	0.6
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.04	1.00		1.00
Lane Grp Cap(c), veh/h	138	291	247	155	291	707	114	779	814	1004	2391	1067
V/C Ratio(X)	0.21	0.53	0.34	0.38	0.60	0.68	0.80	0.79	0.80	0.34	0.55	0.02
Avail Cap(c_a), veh/h	238	446	378	257	446	839	178	779	814	1004	2391	1067
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	57.8	50.5	48.9	57.7	51.1	28.6	60.0	31.5	31.5	36.3	11.0	7.1
Incr Delay (d2), s/veh	0.7	1.5	0.8	1.5	2.0	1.7	13.0	8.2	7.9	0.2	0.9	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.9	4.7	2.5	1.9	5.4	11.9	3.4	18.1	18.9	4.3	9.3	0.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	58.5	51.9	49.8	59.2	53.1	30.3	73.0	39.7	39.4	36.5	11.9	7.1
LnGrp LOS	E	D	D	E	D	C	E	D	D	D	B	A
Approach Vol, veh/h		266			712			1358			1670	
Approach Delay, s/veh		52.0			38.3			41.8			16.9	
Approach LOS		D			D			D			B	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	42.8	62.0		25.2	12.3	92.5		25.2				
Change Period (Y+Rc), s	5.0	* 5		5.0	4.0	5.0		5.0				
Max Green Setting (Gmax), s	28.0	* 57		31.0	13.0	72.0		31.0				
Max Q Clear Time (g_c+I1), s	12.2	41.1		16.3	8.6	26.6		17.7				
Green Ext Time (p_c), s	1.1	7.6		1.0	0.1	13.3		2.6				

Intersection Summary			
HCM 6th Ctrl Delay		31.5	
HCM 6th LOS		C	

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

APPENDIX B-III

YEAR 2045 BUILDOUT TRAFFIC CONDITIONS

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	327	1211	171	276	973	221	198	714	150	450	1511	341
Future Volume (vph)	327	1211	171	276	973	221	198	714	150	450	1511	341
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	325		0	193		145	230		150	275		0
Storage Lanes	1		0	2		1	2		1	2		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	0.91	0.91	0.97	0.91	1.00	0.97	0.91	1.00	0.97	0.91	0.91
Frt		0.981				0.850			0.850		0.972	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	4989	0	3433	5085	1583	3433	5085	1583	3433	4943	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	4989	0	3433	5085	1583	3433	5085	1583	3433	4943	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		21				165			150		43	
Link Speed (mph)		40			40			45			50	
Link Distance (ft)		1043			981			833			926	
Travel Time (s)		17.8			16.7			12.6			12.6	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	327	1211	171	276	973	221	198	714	150	450	1511	341
Shared Lane Traffic (%)												
Lane Group Flow (vph)	327	1382	0	276	973	221	198	714	150	450	1852	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		24			24			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2	1	1	2	1	1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru	Right	Left	Thru	
Leading Detector (ft)	20	100		20	100	20	20	100	20	20	100	
Trailing Detector (ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Position(ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Size(ft)	20	6		20	6	20	20	6	20	20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases						8			2			

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	7	4		3	8	8	5	2	2	1	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Minimum Split (s)	10.0	43.0		10.0	43.0	43.0	10.0	43.0	43.0	10.0	36.0	
Total Split (s)	25.0	48.0		20.0	43.0	43.0	12.0	43.0	43.0	19.0	50.0	
Total Split (%)	19.2%	36.9%		15.4%	33.1%	33.1%	9.2%	33.1%	33.1%	14.6%	38.5%	
Maximum Green (s)	21.0	43.0		16.0	38.0	38.0	8.0	38.0	38.0	15.0	45.0	
Yellow Time (s)	3.0	4.0		3.0	4.0	4.0	3.0	4.0	4.0	3.0	4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	4.0	5.0		4.0	5.0	5.0	4.0	5.0	5.0	4.0	5.0	
Lead/Lag	Lead	Lead		Lag	Lag	Lag	Lag	Lag	Lag	Lead	Lead	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None		None	None	None	None	C-Max	C-Max	None	C-Max	
Walk Time (s)		7.0			7.0	7.0		7.0	7.0		7.0	
Flash Dont Walk (s)		31.0			31.0	31.0		31.0	31.0		24.0	
Pedestrian Calls (#/hr)		5			5	5		5	5		5	
Act Effct Green (s)	21.0	41.7		14.6	35.4	35.4	8.0	38.0	38.0	17.6	47.6	
Actuated g/C Ratio	0.16	0.32		0.11	0.27	0.27	0.06	0.29	0.29	0.14	0.37	
v/c Ratio	1.15	0.86		0.72	0.70	0.40	0.94	0.48	0.26	0.97	1.01	
Control Delay	147.2	46.8		66.4	45.4	12.6	88.4	18.7	3.2	90.0	63.0	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	147.2	46.8		66.4	45.4	12.6	88.4	18.7	3.2	90.0	63.0	
LOS	F	D		E	D	B	F	B	A	F	E	
Approach Delay		66.0			44.4			29.5			68.3	
Approach LOS		E			D			C			E	
Queue Length 50th (ft)	~322	387		116	264	35	89	79	5	~223	~618	
Queue Length 95th (ft)	#513	449		164	313	103	#165	154	30	#336	#720	
Internal Link Dist (ft)		963			901			753			846	
Turn Bay Length (ft)	325			193		145	230		150	275		
Base Capacity (vph)	285	1664		422	1486	579	211	1486	568	466	1838	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	1.15	0.83		0.65	0.65	0.38	0.94	0.48	0.26	0.97	1.01	

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 55 (42%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 120			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 1.15			
Intersection Signal Delay: 56.0		Intersection LOS: E	
Intersection Capacity Utilization 94.4%		ICU Level of Service F	
Analysis Period (min) 15			

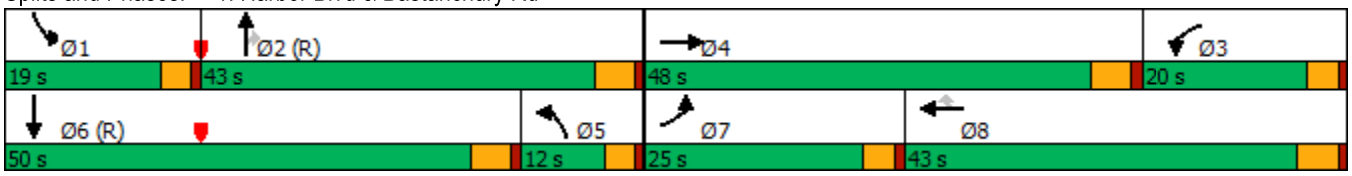
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Harbor Blvd & Bastanchury Rd



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	327	1211	171	276	973	221	198	714	150	450	1511	341
Future Volume (veh/h)	327	1211	171	276	973	221	198	714	150	450	1511	341
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	327	1211	171	276	973	221	198	714	150	450	1511	341
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	288	1391	196	333	1277	396	332	1708	530	399	1443	324
Arrive On Green	0.16	0.31	0.31	0.10	0.25	0.25	0.19	0.67	0.67	0.12	0.35	0.35
Sat Flow, veh/h	1781	4521	638	3456	5106	1585	3456	5106	1585	3456	4170	936
Grp Volume(v), veh/h	327	912	470	276	973	221	198	714	150	450	1232	620
Grp Sat Flow(s),veh/h/ln	1781	1702	1755	1728	1702	1585	1728	1702	1585	1728	1702	1702
Q Serve(g_s), s	21.0	32.9	32.9	10.2	23.0	15.8	6.8	8.4	3.7	15.0	45.0	45.0
Cycle Q Clear(g_c), s	21.0	32.9	32.9	10.2	23.0	15.8	6.8	8.4	3.7	15.0	45.0	45.0
Prop In Lane	1.00		0.36	1.00		1.00	1.00		1.00	1.00		0.55
Lane Grp Cap(c), veh/h	288	1047	540	333	1277	396	332	1708	530	399	1178	589
V/C Ratio(X)	1.14	0.87	0.87	0.83	0.76	0.56	0.60	0.42	0.28	1.13	1.05	1.05
Avail Cap(c_a), veh/h	288	1126	581	425	1493	463	332	1708	530	399	1178	589
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	2.00	2.00	2.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	0.93	0.93	0.93	1.00	1.00	1.00
Uniform Delay (d), s/veh	54.5	42.6	42.6	57.7	45.2	42.5	50.2	15.7	8.3	57.5	42.5	42.5
Incr Delay (d2), s/veh	95.0	7.2	12.9	10.4	2.0	1.2	2.7	0.7	1.2	84.9	39.0	51.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	16.8	14.5	15.9	4.9	9.8	6.2	2.8	2.7	1.8	11.0	24.2	26.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	149.5	49.8	55.5	68.0	47.2	43.7	52.9	16.4	9.5	142.4	81.5	94.1
LnGrp LOS	F	D	E	E	D	D	D	B	A	F	F	F
Approach Vol, veh/h		1709			1470			1062			2302	
Approach Delay, s/veh		70.5			50.6			22.2			96.8	
Approach LOS		E			D			C			F	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	19.0	48.5	17.5	45.0	17.5	50.0	25.0	37.5
Change Period (Y+Rc), s	4.0	5.0	5.0	* 5	5.0	* 5	4.0	5.0
Max Green Setting (Gmax), s	15.0	38.0	16.0	* 43	8.0	* 45	21.0	38.0
Max Q Clear Time (g_c+I1), s	17.0	10.4	12.2	34.9	8.8	47.0	23.0	25.0
Green Ext Time (p_c), s	0.0	5.3	0.3	5.0	0.0	0.0	0.0	5.9

Intersection Summary	
HCM 6th Ctrl Delay	67.4
HCM 6th LOS	E

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

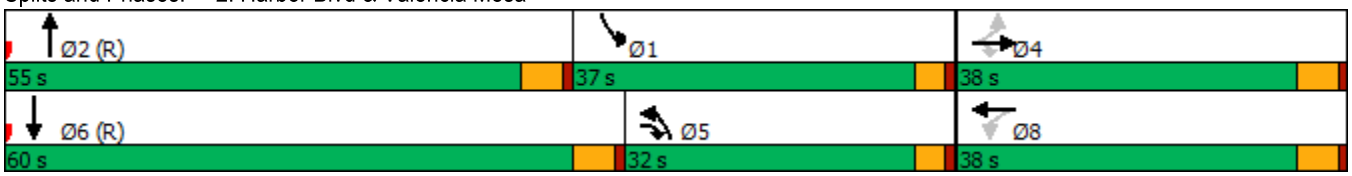
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	101	48	87	59	47	65	154	889	127	244	1448	185
Future Volume (vph)	101	48	87	59	47	65	154	889	127	244	1448	185
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	55		85	0		0	175		0	230		0
Storage Lanes	1		1	1		0	1		0	1		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.91	0.91
Frt			0.850		0.913			0.981			0.983	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1770	1701	0	1770	4989	0	1770	4999	0
Flt Permitted	0.589			0.726			0.950			0.950		
Satd. Flow (perm)	1097	1863	1583	1352	1701	0	1770	4989	0	1770	4999	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			17		51			23			22	
Link Speed (mph)		30			30			50			45	
Link Distance (ft)		813			705			3958			833	
Travel Time (s)		18.5			16.0			54.0			12.6	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	101	48	87	59	47	65	154	889	127	244	1448	185
Shared Lane Traffic (%)												
Lane Group Flow (vph)	101	48	87	59	112	0	154	1016	0	244	1633	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2	1	1	2		1	2		1	2	
Detector Template	Left	Thru	Right	Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100	20	20	100		20	100		20	100	
Trailing Detector (ft)	0	0	0	0	0		0	0		0	0	
Detector 1 Position(ft)	0	0	0	0	0		0	0		0	0	
Detector 1 Size(ft)	20	6	20	20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	pm+ov	Perm	NA		Prot	NA		Prot	NA	
Protected Phases		4	5		8		5	2		1	6	
Permitted Phases	4		4	8								

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	5	8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0		6.0	6.0		6.0	6.0	
Minimum Split (s)	11.0	11.0	10.0	36.0	36.0		10.0	26.0		10.0	30.0	
Total Split (s)	38.0	38.0	32.0	38.0	38.0		32.0	55.0		37.0	60.0	
Total Split (%)	29.2%	29.2%	24.6%	29.2%	29.2%		24.6%	42.3%		28.5%	46.2%	
Maximum Green (s)	33.0	33.0	28.0	33.0	33.0		28.0	50.0		33.0	55.0	
Yellow Time (s)	4.0	4.0	3.0	4.0	4.0		3.0	4.0		3.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.0	5.0	4.0	5.0	5.0		4.0	5.0		4.0	5.0	
Lead/Lag			Lag				Lag	Lead		Lag	Lead	
Lead-Lag Optimize?			Yes				Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None	None	None	None		None	C-Max		None	C-Max	
Walk Time (s)				7.0	7.0			7.0			7.0	
Flash Dont Walk (s)				24.0	24.0			14.0			18.0	
Pedestrian Calls (#/hr)				5	5			5			5	
Act Effct Green (s)	17.6	17.6	40.8	17.6	17.6		18.1	75.2		23.1	80.2	
Actuated g/C Ratio	0.14	0.14	0.31	0.14	0.14		0.14	0.58		0.18	0.62	
v/c Ratio	0.68	0.19	0.17	0.32	0.41		0.63	0.35		0.77	0.53	
Control Delay	73.9	48.0	23.0	52.4	30.8		50.9	9.2		45.4	5.9	
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	73.9	48.0	23.0	52.4	30.8		50.9	9.2		45.4	5.9	
LOS	E	D	C	D	C		D	A		D	A	
Approach Delay		49.9			38.3			14.7			11.0	
Approach LOS		D			D			B			B	
Queue Length 50th (ft)	83	37	43	46	47		131	53		220	85	
Queue Length 95th (ft)	129	66	64	80	94		199	116		m231	m118	
Internal Link Dist (ft)		733			625			3878			753	
Turn Bay Length (ft)	55		85				175			230		
Base Capacity (vph)	278	472	512	343	469		381	2896		449	3093	
Starvation Cap Reductn	0	0	0	0	0		0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0		0	0		0	0	
Storage Cap Reductn	0	0	0	0	0		0	0		0	0	
Reduced v/c Ratio	0.36	0.10	0.17	0.17	0.24		0.40	0.35		0.54	0.53	

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 33 (25%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 80			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.77			
Intersection Signal Delay: 16.3		Intersection LOS: B	
Intersection Capacity Utilization 64.6%		ICU Level of Service C	
Analysis Period (min) 15			

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Harbor Blvd & Valencia Mesa

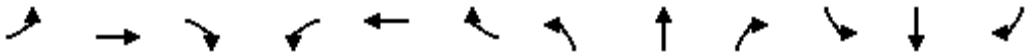


Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	101	48	87	59	47	65	154	889	127	244	1448	185
Future Volume (veh/h)	101	48	87	59	47	65	154	889	127	244	1448	185
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	101	48	87	59	47	65	154	889	127	244	1448	185
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	181	294	744	225	112	155	555	1737	247	624	1939	248
Arrive On Green	0.16	0.16	0.16	0.16	0.16	0.16	0.31	0.38	0.38	0.70	0.85	0.85
Sat Flow, veh/h	1281	1870	1585	1254	711	983	1781	4516	642	1781	4584	585
Grp Volume(v), veh/h	101	48	87	59	0	112	154	669	347	244	1075	558
Grp Sat Flow(s),veh/h/ln	1281	1870	1585	1254	0	1693	1781	1702	1755	1781	1702	1765
Q Serve(g_s), s	10.0	2.9	0.0	5.5	0.0	7.8	8.5	19.6	19.7	7.3	17.1	17.2
Cycle Q Clear(g_c), s	17.8	2.9	0.0	8.4	0.0	7.8	8.5	19.6	19.7	7.3	17.1	17.2
Prop In Lane	1.00		1.00	1.00		0.58	1.00		0.37	1.00		0.33
Lane Grp Cap(c), veh/h	181	294	744	225	0	267	555	1309	675	624	1440	747
V/C Ratio(X)	0.56	0.16	0.12	0.26	0.00	0.42	0.28	0.51	0.51	0.39	0.75	0.75
Avail Cap(c_a), veh/h	304	475	897	346	0	430	555	1309	675	624	1440	747
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	2.00	2.00	2.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	0.90	0.90	0.90	0.16	0.16	0.16
Uniform Delay (d), s/veh	57.4	47.4	19.4	51.0	0.0	49.4	33.7	30.6	30.7	13.7	7.1	7.1
Incr Delay (d2), s/veh	2.7	0.3	0.1	0.6	0.0	1.1	0.2	1.3	2.5	0.1	0.6	1.1
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.4	1.4	1.5	1.8	0.0	3.4	3.6	7.9	8.4	2.4	2.8	3.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	60.1	47.6	19.4	51.6	0.0	50.5	33.9	31.9	33.2	13.8	7.7	8.2
LnGrp LOS	E	D	B	D	A	D	C	C	C	B	A	A
Approach Vol, veh/h		236			171			1170			1877	
Approach Delay, s/veh		42.6			50.9			32.6			8.6	
Approach LOS		D			D			C			A	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	49.5	55.0		25.5	44.5	60.0		25.5				
Change Period (Y+Rc), s	4.0	5.0		5.0	4.0	5.0		5.0				
Max Green Setting (Gmax), s	33.0	50.0		33.0	28.0	55.0		33.0				
Max Q Clear Time (g_c+I1), s	9.3	21.7		19.8	10.5	19.2		10.4				
Green Ext Time (p_c), s	0.6	6.6		0.7	0.3	14.2		0.8				

Intersection Summary			
HCM 6th Ctrl Delay		21.2	
HCM 6th LOS		C	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	51	84	9	773	116	174	22	1059	494	60	1464	80
Future Volume (vph)	51	84	9	773	116	174	22	1059	494	60	1464	80
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	85		0	0		0	95		0	150		0
Storage Lanes	1		1	1		1	1		1	1		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00	1.00	0.91	1.00	1.00	0.91	0.91
Frt			0.850			0.850			0.850		0.992	
Flt Protected	0.950			0.950	0.964		0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1681	1706	1583	1770	5085	1583	1770	5045	0
Flt Permitted	0.950			0.950	0.964		0.950			0.950		
Satd. Flow (perm)	1770	1863	1583	1681	1706	1583	1770	5085	1583	1770	5045	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			134			169			388		8	
Link Speed (mph)		25			45			35			50	
Link Distance (ft)		871			1039			280			3958	
Travel Time (s)		23.8			15.7			5.5			54.0	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	51	84	9	773	116	174	22	1059	494	60	1464	80
Shared Lane Traffic (%)				43%								
Lane Group Flow (vph)	51	84	9	441	448	174	22	1059	494	60	1544	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	
Leading Detector (ft)	20	100	20	20	100	20	20	100	20	20	100	
Trailing Detector (ft)	0	0	0	0	0	0	0	0	0	0	0	
Detector 1 Position(ft)	0	0	0	0	0	0	0	0	0	0	0	
Detector 1 Size(ft)	20	6	20	20	6	20	20	6	20	20	6	
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Split	NA	Perm	Split	NA	Perm	Prot	NA	Free	Prot	NA	
Protected Phases	4	4		8	8		5	2		1	6	
Permitted Phases			4			8			Free			



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	4	8	8	8	5	2		1	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0	
Minimum Split (s)	11.0	11.0	11.0	36.0	36.0	36.0	10.0	23.0		10.0	30.0	
Total Split (s)	12.0	12.0	12.0	44.0	44.0	44.0	10.0	60.0		14.0	64.0	
Total Split (%)	9.2%	9.2%	9.2%	33.8%	33.8%	33.8%	7.7%	46.2%		10.8%	49.2%	
Maximum Green (s)	7.0	7.0	7.0	39.0	39.0	39.0	6.0	55.0		10.0	59.0	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	3.0	4.0		3.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	4.0	5.0		4.0	5.0	
Lead/Lag							Lag	Lag		Lead	Lead	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None	None	None	None	None	None	C-Max		None	C-Max	
Walk Time (s)				7.0	7.0	7.0		7.0			7.0	
Flash Dont Walk (s)				24.0	24.0	24.0		11.0			18.0	
Pedestrian Calls (#/hr)				5	5	5		5			5	
Act Effct Green (s)	7.0	7.0	7.0	37.3	37.3	37.3	6.0	59.9	130.0	8.8	64.7	
Actuated g/C Ratio	0.05	0.05	0.05	0.29	0.29	0.29	0.05	0.46	1.00	0.07	0.50	
v/c Ratio	0.54	0.84	0.04	0.92	0.92	0.30	0.27	0.45	0.31	0.50	0.61	
Control Delay	80.6	114.8	0.3	69.6	69.6	6.8	55.7	16.0	0.5	72.8	18.9	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	80.6	114.8	0.3	69.6	69.6	6.8	55.7	16.0	0.5	72.8	18.9	
LOS	F	F	A	E	E	A	E	B	A	E	B	
Approach Delay		95.6			59.3			11.7			20.9	
Approach LOS		F			E			B			C	
Queue Length 50th (ft)	43	72	0	371	377	3	19	216	0	44	409	
Queue Length 95th (ft)	#95	#170	0	#569	#574	56	45	143	2	m89	539	
Internal Link Dist (ft)		791			959			200			3878	
Turn Bay Length (ft)	85						95			150		
Base Capacity (vph)	95	100	212	504	511	593	81	2343	1583	136	2516	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	59	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.54	0.84	0.04	0.88	0.88	0.29	0.27	0.45	0.31	0.44	0.63	

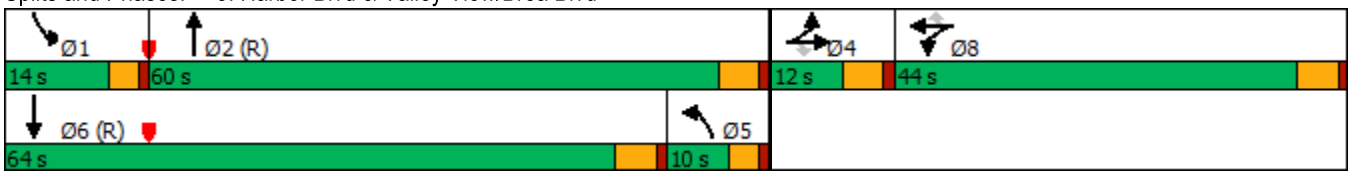
Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 120 (92%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 90			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.92			
Intersection Signal Delay: 29.4		Intersection LOS: C	
Intersection Capacity Utilization 77.9%		ICU Level of Service D	
Analysis Period (min) 15			

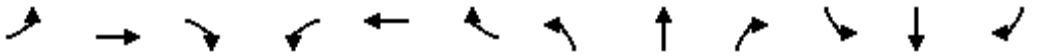
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Harbor Blvd & Valley View/Brea Blvd





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	51	84	9	773	116	174	22	1059	494	60	1464	80
Future Volume (veh/h)	51	84	9	773	116	174	22	1059	494	60	1464	80
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	51	84	9	856	0	0	22	1059	0	60	1464	80
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	96	101	85	939	0		134	2518		77	2249	123
Arrive On Green	0.05	0.05	0.05	0.26	0.00	0.00	0.07	0.49	0.00	0.04	0.45	0.45
Sat Flow, veh/h	1781	1870	1585	3563	0	1585	1781	5106	1585	1781	4955	271
Grp Volume(v), veh/h	51	84	9	856	0	0	22	1059	0	60	1006	538
Grp Sat Flow(s),veh/h/ln	1781	1870	1585	1781	0	1585	1781	1702	1585	1781	1702	1822
Q Serve(g_s), s	3.6	5.8	0.7	30.3	0.0	0.0	1.5	17.2	0.0	4.3	29.8	29.8
Cycle Q Clear(g_c), s	3.6	5.8	0.7	30.3	0.0	0.0	1.5	17.2	0.0	4.3	29.8	29.8
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		0.15
Lane Grp Cap(c), veh/h	96	101	85	939	0		134	2518		77	1545	827
V/C Ratio(X)	0.53	0.83	0.11	0.91	0.00		0.16	0.42		0.78	0.65	0.65
Avail Cap(c_a), veh/h	96	101	85	1069	0		134	2518		137	1545	827
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00	0.00	0.86	0.86	0.86
Uniform Delay (d), s/veh	59.9	60.9	58.5	46.4	0.0	0.0	56.3	21.1	0.0	61.6	27.5	27.5
Incr Delay (d2), s/veh	5.5	42.4	0.5	10.7	0.0	0.0	0.6	0.5	0.0	13.4	1.8	3.4
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.8	4.0	0.3	14.4	0.0	0.0	0.7	6.9	0.0	2.2	11.8	13.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	65.4	103.3	59.1	57.1	0.0	0.0	56.9	21.6	0.0	74.9	29.4	30.9
LnGrp LOS	E	F	E	E	A		E	C		E	C	C
Approach Vol, veh/h		144			856			1081			1604	
Approach Delay, s/veh		87.1			57.1			22.3			31.6	
Approach LOS		F			E			C			C	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	9.6	69.1		12.0	14.7	64.0		39.3				
Change Period (Y+Rc), s	4.0	5.0		5.0	5.0	* 5		5.0				
Max Green Setting (Gmax), s	10.0	55.0		7.0	6.0	* 59		39.0				
Max Q Clear Time (g_c+I1), s	6.3	19.2		7.8	3.5	31.8		32.3				
Green Ext Time (p_c), s	0.0	8.9		0.0	0.0	11.2		2.0				

Intersection Summary			
HCM 6th Ctrl Delay		37.0	
HCM 6th LOS		D	

Notes

User approved volume balancing among the lanes for turning movement.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	48	22	42	23	12	53	80	1461	33	63	2242	49
Future Volume (vph)	48	22	42	23	12	53	80	1461	33	63	2242	49
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		65	95		0	100		0
Storage Lanes	0		0	0		1	1		0	1		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.95	0.95
Frt		0.949				0.850		0.997			0.997	
Flt Protected		0.979			0.968		0.950			0.950		
Satd. Flow (prot)	0	1731	0	0	1803	1583	1770	5070	0	1770	3529	0
Flt Permitted		0.845			0.743		0.043			0.154		
Satd. Flow (perm)	0	1494	0	0	1384	1583	80	5070	0	287	3529	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		14				53		6			4	
Link Speed (mph)		25			25			35			35	
Link Distance (ft)		390			645			190			410	
Travel Time (s)		10.6			17.6			3.7			8.0	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	48	22	42	23	12	53	80	1461	33	63	2242	49
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	112	0	0	35	53	80	1494	0	63	2291	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60		60	60		60	60		60	60		60
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100	20	20	100		20	100	
Trailing Detector (ft)	0	0		0	0	0	0	0		0	0	
Detector 1 Position(ft)	0	0		0	0	0	0	0		0	0	
Detector 1 Size(ft)	20	6		20	6	20	20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4		8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0		6.0	6.0	
Minimum Split (s)	33.0	33.0		33.0	33.0	33.0	26.0	26.0		26.0	26.0	
Total Split (s)	33.0	33.0		33.0	33.0	33.0	97.0	97.0		97.0	97.0	
Total Split (%)	25.4%	25.4%		25.4%	25.4%	25.4%	74.6%	74.6%		74.6%	74.6%	
Maximum Green (s)	28.0	28.0		28.0	28.0	28.0	92.0	92.0		92.0	92.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	4.0	4.0		4.0	4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.0			5.0	5.0	5.0	5.0		5.0	5.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None	None	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	21.0	21.0		21.0	21.0	21.0	14.0	14.0		14.0	14.0	
Pedestrian Calls (#/hr)	5	5		5	5	5	5	5		5	5	
Act Effct Green (s)		15.4			15.4	15.4	104.6	104.6		104.6	104.6	
Actuated g/C Ratio		0.12			0.12	0.12	0.80	0.80		0.80	0.80	
v/c Ratio		0.59			0.21	0.23	1.25	0.37		0.27	0.81	
Control Delay		58.5			51.4	13.7	204.0	3.2		2.8	7.4	
Queue Delay		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		58.5			51.4	13.7	204.0	3.2		2.8	7.4	
LOS		E			D	B	F	A		A	A	
Approach Delay		58.5			28.7			13.4			7.3	
Approach LOS		E			C			B			A	
Queue Length 50th (ft)		80			28	0	~84	78		0	200	
Queue Length 95th (ft)		129			54	35	m#168	107		m7	286	
Internal Link Dist (ft)		310			565			110			330	
Turn Bay Length (ft)						65	95			100		
Base Capacity (vph)		332			298	382	64	4080		230	2840	
Starvation Cap Reductn		0			0	0	0	0		0	0	
Spillback Cap Reductn		0			0	0	0	0		0	28	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.34			0.12	0.14	1.25	0.37		0.27	0.81	

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 87 (67%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green			
Natural Cycle: 120			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 1.25			
Intersection Signal Delay: 11.5		Intersection LOS: B	
Intersection Capacity Utilization 87.9%		ICU Level of Service E	
Analysis Period (min) 15			

~ Volume exceeds capacity, queue is theoretically infinite.

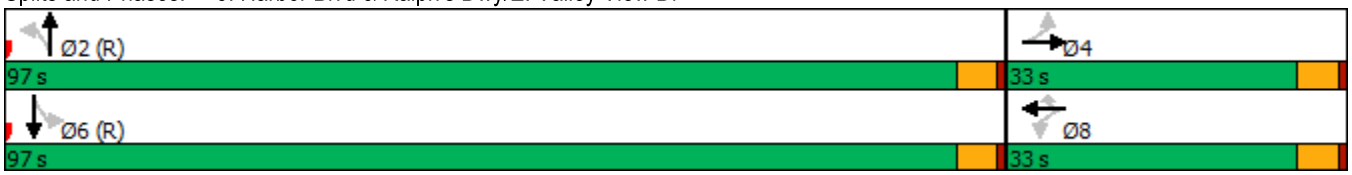
Queue shown is maximum after two cycles.

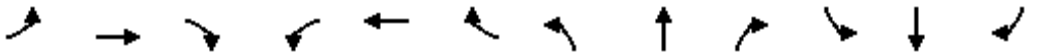
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Harbor Blvd & Ralph's Dwy/E. Valley View Dr



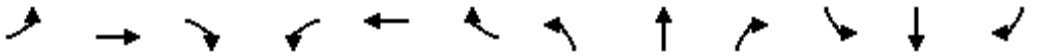


Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	48	22	42	23	12	53	80	1461	33	63	2242	49
Future Volume (veh/h)	48	22	42	23	12	53	80	1461	33	63	2242	49
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	48	22	42	23	12	53	80	1461	33	63	2242	49
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	92	35	52	116	52	138	145	4296	97	327	2973	65
Arrive On Green	0.09	0.09	0.09	0.09	0.09	0.09	0.84	0.84	0.84	0.84	0.84	0.84
Sat Flow, veh/h	599	405	602	811	596	1585	162	5137	116	352	3556	77
Grp Volume(v), veh/h	112	0	0	35	0	53	80	968	526	63	1116	1175
Grp Sat Flow(s),veh/h/ln	1606	0	0	1407	0	1585	162	1702	1849	352	1777	1856
Q Serve(g_s), s	6.0	0.0	0.0	0.0	0.0	4.1	56.9	8.5	8.5	6.5	36.0	36.7
Cycle Q Clear(g_c), s	8.8	0.0	0.0	2.8	0.0	4.1	93.6	8.5	8.5	14.9	36.0	36.7
Prop In Lane	0.43		0.37	0.66		1.00	1.00		0.06	1.00		0.04
Lane Grp Cap(c), veh/h	179	0	0	168	0	138	145	2847	1547	327	1486	1552
V/C Ratio(X)	0.63	0.00	0.00	0.21	0.00	0.38	0.55	0.34	0.34	0.19	0.75	0.76
Avail Cap(c_a), veh/h	378	0	0	359	0	341	145	2847	1547	327	1486	1552
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	58.1	0.0	0.0	55.4	0.0	56.1	25.6	2.4	2.4	4.1	4.7	4.7
Incr Delay (d2), s/veh	3.6	0.0	0.0	0.6	0.0	1.8	14.3	0.3	0.6	1.3	3.5	3.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.8	0.0	0.0	1.1	0.0	1.7	2.6	2.0	2.3	0.5	9.9	10.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	61.6	0.0	0.0	56.0	0.0	57.8	39.9	2.8	3.0	5.5	8.2	8.2
LnGrp LOS	E	A	A	E	A	E	D	A	A	A	A	A
Approach Vol, veh/h		112			88			1574			2354	
Approach Delay, s/veh		61.6			57.1			4.7			8.2	
Approach LOS		E			E			A			A	

Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		113.7		16.3		113.7		16.3				
Change Period (Y+Rc), s		5.0		5.0		5.0		5.0				
Max Green Setting (Gmax), s		92.0		28.0		92.0		28.0				
Max Q Clear Time (g_c+I1), s		95.6		10.8		38.7		6.1				
Green Ext Time (p_c), s		0.0		0.5		39.1		0.3				

Intersection Summary												
HCM 6th Ctrl Delay			9.4									
HCM 6th LOS			A									

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	17	176	73	53	165	466	101	1092	43	639	1592	19
Future Volume (vph)	17	176	73	53	165	466	101	1092	43	639	1592	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	130		135	200		0	205		0	300		165
Storage Lanes	1		1	1		1	1		0	2		1
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.97	0.95	1.00
Frt			0.850			0.850		0.994				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1770	1863	1583	1770	3518	0	3433	3539	1583
Flt Permitted	0.440			0.407			0.950			0.950		
Satd. Flow (perm)	820	1863	1583	758	1863	1583	1770	3518	0	3433	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			92			50		4				50
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		538			720			654			541	
Travel Time (s)		10.5			14.0			12.7			10.5	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	17	176	73	53	165	466	101	1092	43	639	1592	19
Shared Lane Traffic (%)												
Lane Group Flow (vph)	17	176	73	53	165	466	101	1135	0	639	1592	19
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes										
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60		60	60		60	60		60	60		60
Number of Detectors	1	2	1	1	2	1	1	2		1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (ft)	20	100	20	20	100	20	20	100		20	100	20
Trailing Detector (ft)	0	0	0	0	0	0	0	0		0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0	0	0		0	0	0
Detector 1 Size(ft)	20	6	20	20	6	20	20	6		20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	Perm	Perm	NA	pm+ov	Prot	NA		Prot	NA	Perm
Protected Phases		4			8	1	5	2		1	6	
Permitted Phases	4		4	8		8						6

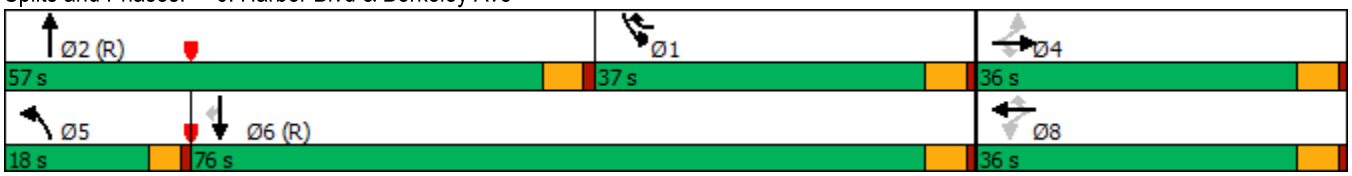


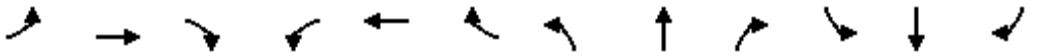
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	4	8	8	1	5	2		1	6	6
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0	6.0
Minimum Split (s)	30.0	30.0	30.0	36.0	36.0	11.0	10.0	30.0		11.0	30.0	30.0
Total Split (s)	36.0	36.0	36.0	36.0	36.0	37.0	18.0	57.0		37.0	76.0	76.0
Total Split (%)	27.7%	27.7%	27.7%	27.7%	27.7%	28.5%	13.8%	43.8%		28.5%	58.5%	58.5%
Maximum Green (s)	31.0	31.0	31.0	31.0	31.0	32.0	14.0	52.0		32.0	71.0	71.0
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	3.0	4.0		4.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	4.0	5.0		5.0	5.0	5.0
Lead/Lag						Lag	Lead	Lead		Lag	Lag	Lag
Lead-Lag Optimize?						Yes	Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max		None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0			7.0			7.0	7.0
Flash Dont Walk (s)	18.0	18.0	18.0	24.0	24.0			18.0			18.0	18.0
Pedestrian Calls (#/hr)	5	5	5	5	5			5			5	5
Act Effct Green (s)	19.0	19.0	19.0	19.0	19.0	56.0	12.0	64.0		32.0	85.0	85.0
Actuated g/C Ratio	0.15	0.15	0.15	0.15	0.15	0.43	0.09	0.49		0.25	0.65	0.65
v/c Ratio	0.14	0.65	0.24	0.48	0.61	0.66	0.62	0.66		0.76	0.69	0.02
Control Delay	47.1	62.2	6.5	63.0	60.1	30.0	73.1	28.2		35.9	7.4	0.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.3	0.0
Total Delay	47.1	62.2	6.5	63.0	60.1	30.0	73.1	28.2		35.9	7.8	0.2
LOS	D	E	A	E	E	C	E	C		D	A	A
Approach Delay		45.9			39.8			31.8			15.7	
Approach LOS		D			D			C			B	
Queue Length 50th (ft)	13	144	0	42	134	281	83	357		230	178	0
Queue Length 95th (ft)	33	197	27	78	185	330	143	530		310	468	m0
Internal Link Dist (ft)		458			640			574			461	
Turn Bay Length (ft)	130		135	200			205			300		165
Base Capacity (vph)	195	444	447	180	444	710	192	1732		845	2313	1052
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	240	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	0
Reduced v/c Ratio	0.09	0.40	0.16	0.29	0.37	0.66	0.53	0.66		0.76	0.77	0.02

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 115 (88%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 90			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.76			
Intersection Signal Delay: 25.7		Intersection LOS: C	
Intersection Capacity Utilization 80.7%		ICU Level of Service D	
Analysis Period (min) 15			

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Harbor Blvd & Berkeley Ave





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	17	176	73	53	165	466	101	1092	43	639	1592	19
Future Volume (veh/h)	17	176	73	53	165	466	101	1092	43	639	1592	19
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	17	176	73	53	165	466	101	1092	43	639	1592	19
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	155	304	258	147	304	768	125	1394	55	1112	2344	1045
Arrive On Green	0.16	0.16	0.16	0.16	0.16	0.16	0.07	0.40	0.40	0.32	0.66	0.66
Sat Flow, veh/h	1221	1870	1585	1209	1870	1585	1781	3485	137	3456	3554	1585
Grp Volume(v), veh/h	17	176	73	53	165	466	101	557	578	639	1592	19
Grp Sat Flow(s),veh/h/ln	1221	1870	1585	1209	1870	1585	1781	1777	1846	1728	1777	1585
Q Serve(g_s), s	1.7	11.3	5.3	5.5	10.5	0.0	7.3	35.6	35.6	20.0	35.9	0.5
Cycle Q Clear(g_c), s	12.2	11.3	5.3	16.8	10.5	0.0	7.3	35.6	35.6	20.0	35.9	0.5
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.07	1.00		1.00
Lane Grp Cap(c), veh/h	155	304	258	147	304	768	125	711	738	1112	2344	1045
V/C Ratio(X)	0.11	0.58	0.28	0.36	0.54	0.61	0.81	0.78	0.78	0.57	0.68	0.02
Avail Cap(c_a), veh/h	248	446	378	238	446	888	192	711	738	1112	2344	1045
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	55.6	50.3	47.8	58.1	50.0	24.5	59.6	34.1	34.1	36.7	13.7	7.6
Incr Delay (d2), s/veh	0.3	1.7	0.6	1.5	1.5	0.9	13.6	8.4	8.1	0.7	1.6	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.5	5.4	2.1	1.7	5.0	10.4	3.7	16.7	17.3	8.5	13.8	0.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	55.9	52.0	48.4	59.5	51.5	25.4	73.2	42.5	42.2	37.4	15.3	7.7
LnGrp LOS	E	D	D	E	D	C	E	D	D	D	B	A
Approach Vol, veh/h		266			684			1236			2250	
Approach Delay, s/veh		51.3			34.3			44.9			21.5	
Approach LOS		D			C			D			C	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	46.8	57.0		26.2	13.1	90.7		26.2				
Change Period (Y+Rc), s	5.0	5.0		5.0	4.0	5.0		5.0				
Max Green Setting (Gmax), s	32.0	52.0		31.0	14.0	71.0		31.0				
Max Q Clear Time (g_c+I1), s	22.0	37.6		14.2	9.3	37.9		18.8				
Green Ext Time (p_c), s	1.8	6.3		1.1	0.1	16.2		2.3				

Intersection Summary			
HCM 6th Ctrl Delay		31.8	
HCM 6th LOS		C	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	373	1194	131	169	1048	338	198	1564	229	379	979	382
Future Volume (vph)	373	1194	131	169	1048	338	198	1564	229	379	979	382
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	325		0	193		145	230		150	275		0
Storage Lanes	1		0	2		1	2		1	2		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	0.91	0.91	0.97	0.91	1.00	0.97	0.91	1.00	0.97	0.91	0.91
Frt		0.985				0.850			0.850		0.958	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	5009	0	3433	5085	1583	3433	5085	1583	3433	4872	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	5009	0	3433	5085	1583	3433	5085	1583	3433	4872	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		17				206			126		79	
Link Speed (mph)		40			40			45			50	
Link Distance (ft)		1043			981			833			926	
Travel Time (s)		17.8			16.7			12.6			12.6	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	373	1194	131	169	1048	338	198	1564	229	379	979	382
Shared Lane Traffic (%)												
Lane Group Flow (vph)	373	1325	0	169	1048	338	198	1564	229	379	1361	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		24			24			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2	1	1	2	1	1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru	Right	Left	Thru	
Leading Detector (ft)	20	100		20	100	20	20	100	20	20	100	
Trailing Detector (ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Position(ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Size(ft)	20	6		20	6	20	20	6	20	20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases						8			2			

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	7	4		3	8	8	5	2	2	1	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Minimum Split (s)	10.0	43.0		10.0	43.0	43.0	10.0	43.0	43.0	10.0	36.0	
Total Split (s)	27.0	55.0		15.0	43.0	43.0	14.0	45.0	45.0	15.0	46.0	
Total Split (%)	20.8%	42.3%		11.5%	33.1%	33.1%	10.8%	34.6%	34.6%	11.5%	35.4%	
Maximum Green (s)	23.0	50.0		11.0	38.0	38.0	10.0	40.0	40.0	11.0	41.0	
Yellow Time (s)	3.0	4.0		3.0	4.0	4.0	3.0	4.0	4.0	3.0	4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	4.0	5.0		4.0	5.0	5.0	4.0	5.0	5.0	4.0	5.0	
Lead/Lag	Lag	Lead		Lag	Lead	Lead	Lag	Lag	Lag	Lead	Lead	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None		None	None	None	None	C-Max	C-Max	None	C-Max	
Walk Time (s)		7.0			7.0	7.0		7.0	7.0		7.0	
Flash Dont Walk (s)		31.0			31.0	31.0		31.0	31.0		24.0	
Pedestrian Calls (#/hr)		5			5	5		5	5		5	
Act Effct Green (s)	25.5	43.8		17.2	35.5	35.5	10.0	40.0	40.0	11.0	41.0	
Actuated g/C Ratio	0.20	0.34		0.13	0.27	0.27	0.08	0.31	0.31	0.08	0.32	
v/c Ratio	1.07	0.78		0.37	0.75	0.58	0.75	1.00	0.40	1.31	0.86	
Control Delay	119.2	41.5		55.9	46.9	19.1	52.6	45.1	3.7	206.3	45.6	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	119.2	41.5		55.9	46.9	19.1	52.6	45.1	3.7	206.3	45.6	
LOS	F	D		E	D	B	D	D	A	F	D	
Approach Delay		58.6			41.9			41.1			80.6	
Approach LOS		E			D			D			F	
Queue Length 50th (ft)	~369	361		68	291	90	88	~500	1	~210	373	
Queue Length 95th (ft)	#577	389		110	341	189	m#135	m#593	m9	#313	436	
Internal Link Dist (ft)		963			901			753			846	
Turn Bay Length (ft)	325			193		145	230		150	275		
Base Capacity (vph)	347	1937		453	1486	608	264	1564	574	290	1590	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	1.07	0.68		0.37	0.71	0.56	0.75	1.00	0.40	1.31	0.86	

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 85 (65%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 120			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 1.31			
Intersection Signal Delay: 55.4		Intersection LOS: E	
Intersection Capacity Utilization 96.9%		ICU Level of Service F	
Analysis Period (min) 15			

~ Volume exceeds capacity, queue is theoretically infinite.

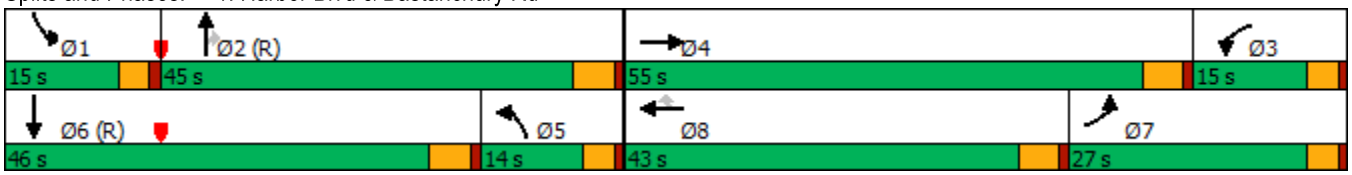
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Harbor Blvd & Bastanchury Rd



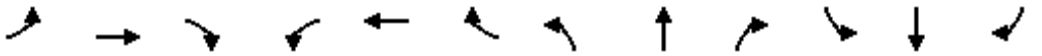
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	373	1194	131	169	1048	338	198	1564	229	379	979	382
Future Volume (veh/h)	373	1194	131	169	1048	338	198	1564	229	379	979	382
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	373	1194	131	169	1048	338	198	1564	229	379	979	382
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	315	1464	161	404	1294	402	373	1769	549	292	1139	444
Arrive On Green	0.18	0.31	0.31	0.12	0.25	0.25	0.22	0.69	0.69	0.08	0.32	0.32
Sat Flow, veh/h	1781	4670	512	3456	5106	1585	3456	5106	1585	3456	3612	1409
Grp Volume(v), veh/h	373	870	455	169	1048	338	198	1564	229	379	923	438
Grp Sat Flow(s),veh/h/ln	1781	1702	1778	1728	1702	1585	1728	1702	1585	1728	1702	1617
Q Serve(g_s), s	23.0	30.6	30.7	5.9	25.1	20.9	6.6	31.6	5.8	11.0	33.1	33.1
Cycle Q Clear(g_c), s	23.0	30.6	30.7	5.9	25.1	20.9	6.6	31.6	5.8	11.0	33.1	33.1
Prop In Lane	1.00		0.29	1.00		1.00	1.00		1.00	1.00		0.87
Lane Grp Cap(c), veh/h	315	1067	558	404	1294	402	373	1769	549	292	1074	510
V/C Ratio(X)	1.18	0.82	0.82	0.42	0.81	0.84	0.53	0.88	0.42	1.30	0.86	0.86
Avail Cap(c_a), veh/h	315	1309	684	404	1493	463	373	1769	549	292	1074	510
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	2.00	2.00	2.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	0.70	0.70	0.70	1.00	1.00	1.00
Uniform Delay (d), s/veh	53.5	41.1	41.2	53.3	45.6	29.0	48.0	17.9	7.3	59.5	41.8	41.8
Incr Delay (d2), s/veh	110.2	3.4	6.3	0.7	3.0	11.7	1.0	5.0	1.6	156.2	9.0	17.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	19.7	13.0	14.1	2.6	10.7	9.1	2.7	6.5	2.5	11.0	14.6	15.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	163.7	44.5	47.5	54.0	48.6	40.8	49.0	22.8	8.9	215.7	50.8	58.8
LnGrp LOS	F	D	D	D	D	D	D	C	A	F	D	E
Approach Vol, veh/h		1698			1555			1991			1740	
Approach Delay, s/veh		71.5			47.5			23.8			88.7	
Approach LOS		E			D			C			F	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	15.0	50.0	19.2	45.8	19.0	46.0	27.0	38.0
Change Period (Y+Rc), s	4.0	5.0	4.0	5.0	5.0	* 5	4.0	5.0
Max Green Setting (Gmax), s	11.0	40.0	11.0	50.0	10.0	* 41	23.0	38.0
Max Q Clear Time (g_c+I1), s	13.0	33.6	7.9	32.7	8.6	35.1	25.0	27.1
Green Ext Time (p_c), s	0.0	4.9	0.1	8.1	0.1	3.8	0.0	5.9

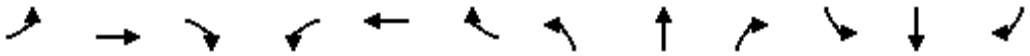
Intersection Summary	
HCM 6th Ctrl Delay	56.9
HCM 6th LOS	E

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	125	23	139	76	44	305	50	1569	56	119	1020	95
Future Volume (vph)	125	23	139	76	44	305	50	1569	56	119	1020	95
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	55		85	0		0	175		0	230		0
Storage Lanes	1		1	1		0	1		0	1		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.91	0.91
Frt			0.850		0.869			0.995			0.987	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1770	1619	0	1770	5060	0	1770	5019	0
Flt Permitted	0.195			0.742			0.950			0.950		
Satd. Flow (perm)	363	1863	1583	1382	1619	0	1770	5060	0	1770	5019	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			63		253			5			16	
Link Speed (mph)		30			30			50			45	
Link Distance (ft)		813			705			3958			833	
Travel Time (s)		18.5			16.0			54.0			12.6	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	125	23	139	76	44	305	50	1569	56	119	1020	95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	125	23	139	76	349	0	50	1625	0	119	1115	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2	1	1	2		1	2		1	2	
Detector Template	Left	Thru	Right	Left	Thru		Left	Thru		Left	Thru	
Leading Detector (ft)	20	100	20	20	100		20	100		20	100	
Trailing Detector (ft)	0	0	0	0	0		0	0		0	0	
Detector 1 Position(ft)	0	0	0	0	0		0	0		0	0	
Detector 1 Size(ft)	20	6	20	20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	pm+ov	Perm	NA		Prot	NA		Prot	NA	
Protected Phases		4	5		8		5	2		1	6	
Permitted Phases	4		4	8								



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	5	8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0		6.0	6.0		6.0	6.0	
Minimum Split (s)	11.0	11.0	10.0	36.0	36.0		10.0	26.0		10.0	30.0	
Total Split (s)	50.0	50.0	13.0	50.0	50.0		13.0	60.0		20.0	67.0	
Total Split (%)	38.5%	38.5%	10.0%	38.5%	38.5%		10.0%	46.2%		15.4%	51.5%	
Maximum Green (s)	45.0	45.0	9.0	45.0	45.0		9.0	55.0		16.0	62.0	
Yellow Time (s)	4.0	4.0	3.0	4.0	4.0		3.0	4.0		3.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0		1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.0	5.0	4.0	5.0	5.0		4.0	5.0		4.0	5.0	
Lead/Lag			Lead				Lead	Lead		Lag	Lag	
Lead-Lag Optimize?			Yes				Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None	None	None	None		None	C-Max		None	C-Max	
Walk Time (s)				7.0	7.0			7.0			7.0	
Flash Dont Walk (s)				24.0	24.0			14.0			18.0	
Pedestrian Calls (#/hr)				5	5			5			5	
Act Effct Green (s)	29.6	29.6	43.3	29.6	29.6		8.7	70.4		16.0	77.7	
Actuated g/C Ratio	0.23	0.23	0.33	0.23	0.23		0.07	0.54		0.12	0.60	
v/c Ratio	1.52	0.05	0.24	0.24	0.62		0.42	0.59		0.55	0.37	
Control Delay	321.1	34.3	15.3	39.5	16.1		96.9	6.5		50.3	5.6	
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay	321.1	34.3	15.3	39.5	16.1		96.9	6.5		50.3	5.6	
LOS	F	C	B	D	B		F	A		D	A	
Approach Delay		150.0			20.3			9.2			9.9	
Approach LOS		F			C			A			A	
Queue Length 50th (ft)	~146	15	44	53	67		44	53		106	69	
Queue Length 95th (ft)	#244	34	78	84	147		m63	82		m134	112	
Internal Link Dist (ft)		733			625			3878			753	
Turn Bay Length (ft)	55		85				175			230		
Base Capacity (vph)	125	644	579	478	725		130	2743		217	3006	
Starvation Cap Reductn	0	0	0	0	0		0	0		0	0	
Spillback Cap Reductn	0	0	0	0	0		0	0		0	0	
Storage Cap Reductn	0	0	0	0	0		0	0		0	0	
Reduced v/c Ratio	1.00	0.04	0.24	0.16	0.48		0.38	0.59		0.55	0.37	

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 81 (62%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 80			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 1.52			
Intersection Signal Delay: 21.9		Intersection LOS: C	
Intersection Capacity Utilization 82.1%		ICU Level of Service E	
Analysis Period (min) 15			

~ Volume exceeds capacity, queue is theoretically infinite.

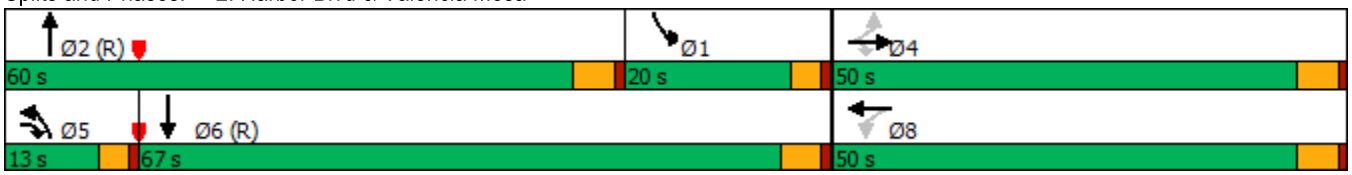
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Harbor Blvd & Valencia Mesa



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	125	23	139	76	44	305	50	1569	56	119	1020	95
Future Volume (veh/h)	125	23	139	76	44	305	50	1569	56	119	1020	95
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	125	23	139	76	44	305	50	1569	56	119	1020	95
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	197	605	574	441	66	457	69	2141	76	246	2521	234
Arrive On Green	0.32	0.32	0.32	0.32	0.32	0.32	0.04	0.42	0.42	0.28	1.00	1.00
Sat Flow, veh/h	1032	1870	1585	1224	204	1412	1781	5061	181	1781	4753	442
Grp Volume(v), veh/h	125	23	139	76	0	349	50	1055	570	119	730	385
Grp Sat Flow(s),veh/h/ln	1032	1870	1585	1224	0	1616	1781	1702	1838	1781	1702	1791
Q Serve(g_s), s	15.5	1.1	8.0	5.9	0.0	24.2	3.6	33.7	33.7	7.3	0.0	0.0
Cycle Q Clear(g_c), s	39.7	1.1	8.0	7.0	0.0	24.2	3.6	33.7	33.7	7.3	0.0	0.0
Prop In Lane	1.00		1.00	1.00		0.87	1.00		0.10	1.00		0.25
Lane Grp Cap(c), veh/h	197	605	574	441	0	523	69	1440	778	246	1805	950
V/C Ratio(X)	0.64	0.04	0.24	0.17	0.00	0.67	0.73	0.73	0.73	0.48	0.40	0.41
Avail Cap(c_a), veh/h	220	647	610	469	0	559	123	1440	778	246	1805	950
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	2.00	2.00	2.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	0.77	0.77	0.77	0.50	0.50	0.50
Uniform Delay (d), s/veh	55.1	30.1	29.0	32.5	0.0	37.9	61.8	31.4	31.4	43.2	0.0	0.0
Incr Delay (d2), s/veh	5.0	0.0	0.2	0.2	0.0	2.8	10.7	2.6	4.7	0.7	0.3	0.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	4.3	0.5	3.1	1.8	0.0	10.0	1.8	13.5	15.1	3.0	0.1	0.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	60.0	30.1	29.2	32.7	0.0	40.7	72.5	33.9	36.1	43.9	0.3	0.6
LnGrp LOS	E	C	C	C	A	D	E	C	D	D	A	A
Approach Vol, veh/h		287			425			1675			1234	
Approach Delay, s/veh		42.7			39.3			35.8			4.6	
Approach LOS		D			D			D			A	

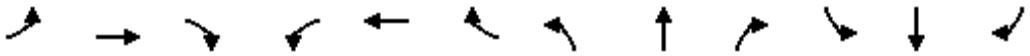
Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	23.0	60.0		47.0	9.0	73.9		47.0				
Change Period (Y+Rc), s	5.0	* 5		5.0	4.0	5.0		5.0				
Max Green Setting (Gmax), s	16.0	* 55		45.0	9.0	62.0		45.0				
Max Q Clear Time (g_c+I1), s	9.3	35.7		41.7	5.6	2.0		26.2				
Green Ext Time (p_c), s	0.1	10.1		0.4	0.0	8.6		2.5				

Intersection Summary			
HCM 6th Ctrl Delay		26.1	
HCM 6th LOS		C	

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	68	179	26	587	97	117	9	1447	788	102	1151	50
Future Volume (vph)	68	179	26	587	97	117	9	1447	788	102	1151	50
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	85		0	0		0	95		0	150		0
Storage Lanes	1		1	1		1	1		1	1		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00	1.00	0.91	1.00	1.00	0.91	0.91
Frt			0.850			0.850			0.850		0.994	
Flt Protected	0.950			0.950	0.965		0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1681	1708	1583	1770	5085	1583	1770	5055	0
Flt Permitted	0.950			0.950	0.965		0.950			0.950		
Satd. Flow (perm)	1770	1863	1583	1681	1708	1583	1770	5085	1583	1770	5055	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			134			134			453		7	
Link Speed (mph)		25			45			35			50	
Link Distance (ft)		871			1039			280			3958	
Travel Time (s)		23.8			15.7			5.5			54.0	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	68	179	26	587	97	117	9	1447	788	102	1151	50
Shared Lane Traffic (%)				42%								
Lane Group Flow (vph)	68	179	26	340	344	117	9	1447	788	102	1201	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	
Leading Detector (ft)	20	100	20	20	100	20	20	100	20	20	100	
Trailing Detector (ft)	0	0	0	0	0	0	0	0	0	0	0	
Detector 1 Position(ft)	0	0	0	0	0	0	0	0	0	0	0	
Detector 1 Size(ft)	20	6	20	20	6	20	20	6	20	20	6	
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Split	NA	Perm	Split	NA	Perm	Prot	NA	Free	Prot	NA	
Protected Phases	4	4		8	8		5	2		1	6	
Permitted Phases			4			8			Free			



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	4	8	8	8	5	2		1	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0	
Minimum Split (s)	11.0	11.0	11.0	36.0	36.0	36.0	10.0	23.0		10.0	30.0	
Total Split (s)	19.0	19.0	19.0	36.0	36.0	36.0	10.0	62.0		13.0	65.0	
Total Split (%)	14.6%	14.6%	14.6%	27.7%	27.7%	27.7%	7.7%	47.7%		10.0%	50.0%	
Maximum Green (s)	14.0	14.0	14.0	31.0	31.0	31.0	6.0	57.0		9.0	60.0	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	3.0	4.0		3.0	4.0	
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0		1.0	1.0	
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	4.0	5.0		4.0	5.0	
Lead/Lag							Lag	Lag		Lead	Lead	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None	None	None	None	None	None	C-Max		None	C-Max	
Walk Time (s)				7.0	7.0	7.0		7.0			7.0	
Flash Dont Walk (s)				24.0	24.0	24.0		11.0			18.0	
Pedestrian Calls (#/hr)				5	5	5		5			5	
Act Effct Green (s)	14.0	14.0	14.0	29.2	29.2	29.2	6.0	58.5	130.0	9.3	69.8	
Actuated g/C Ratio	0.11	0.11	0.11	0.22	0.22	0.22	0.05	0.45	1.00	0.07	0.54	
v/c Ratio	0.36	0.90	0.09	0.90	0.90	0.25	0.11	0.63	0.50	0.80	0.44	
Control Delay	59.8	99.0	0.6	76.2	75.2	5.9	50.8	19.1	1.3	110.5	10.7	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	59.8	99.0	0.6	76.2	75.2	5.9	50.8	19.1	1.3	110.5	10.7	
LOS	E	F	A	E	E	A	D	B	A	F	B	
Approach Delay		79.8			65.5			13.0			18.5	
Approach LOS		E			E			B			B	
Queue Length 50th (ft)	54	151	0	289	292	0	8	363	0	83	154	
Queue Length 95th (ft)	103	#290	0	#460	#463	38	m12	288	0	#183	82	
Internal Link Dist (ft)		791			959			200			3878	
Turn Bay Length (ft)	85						95			150		
Base Capacity (vph)	190	200	290	400	407	479	81	2288	1583	127	2719	
Starvation Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	
Reduced v/c Ratio	0.36	0.90	0.09	0.85	0.85	0.24	0.11	0.63	0.50	0.80	0.44	

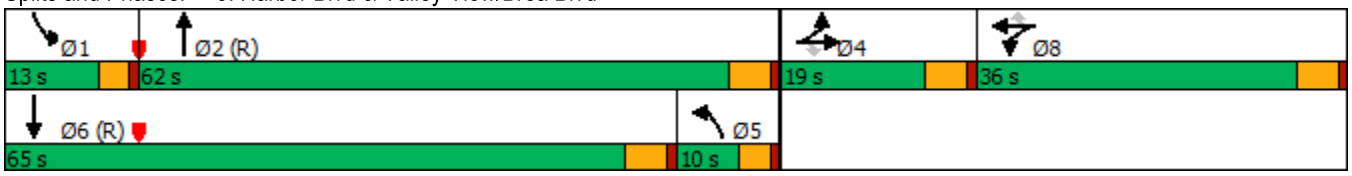
Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 16 (12%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 90			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.90			
Intersection Signal Delay: 27.6		Intersection LOS: C	
Intersection Capacity Utilization 77.7%		ICU Level of Service D	
Analysis Period (min) 15			

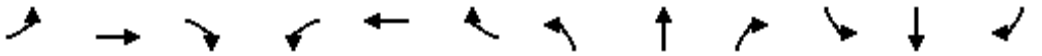
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Harbor Blvd & Valley View/Brea Blvd





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	68	179	26	587	97	117	9	1447	788	102	1151	50
Future Volume (veh/h)	68	179	26	587	97	117	9	1447	788	102	1151	50
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	68	179	26	656	0	0	9	1447	0	102	1151	50
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	192	201	171	730	0		128	2410		123	2316	101
Arrive On Green	0.11	0.11	0.11	0.20	0.00	0.00	0.07	0.47	0.00	0.07	0.46	0.46
Sat Flow, veh/h	1781	1870	1585	3563	0	1585	1781	5106	1585	1781	5017	218
Grp Volume(v), veh/h	68	179	26	656	0	0	9	1447	0	102	781	420
Grp Sat Flow(s),veh/h/ln	1781	1870	1585	1781	0	1585	1781	1702	1585	1781	1702	1831
Q Serve(g_s), s	4.6	12.3	1.9	23.3	0.0	0.0	0.6	27.1	0.0	7.3	20.8	20.9
Cycle Q Clear(g_c), s	4.6	12.3	1.9	23.3	0.0	0.0	0.6	27.1	0.0	7.3	20.8	20.9
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		0.12
Lane Grp Cap(c), veh/h	192	201	171	730	0		128	2410		123	1571	845
V/C Ratio(X)	0.35	0.89	0.15	0.90	0.00		0.07	0.60		0.83	0.50	0.50
Avail Cap(c_a), veh/h	192	201	171	850	0		128	2410		123	1571	845
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00	0.00	0.95	0.95	0.95
Uniform Delay (d), s/veh	53.8	57.2	52.6	50.4	0.0	0.0	56.3	25.3	0.0	59.7	24.5	24.5
Incr Delay (d2), s/veh	1.1	34.8	0.4	11.3	0.0	0.0	0.2	1.1	0.0	33.7	1.1	2.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.1	7.8	0.8	11.3	0.0	0.0	0.3	11.0	0.0	4.4	8.1	9.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	54.9	92.0	53.0	61.6	0.0	0.0	56.5	26.4	0.0	93.5	25.5	26.4
LnGrp LOS	D	F	D	E	A		E	C		F	C	C
Approach Vol, veh/h		273			656			1456			1303	
Approach Delay, s/veh		79.0			61.6			26.6			31.1	
Approach LOS		E			E			C			C	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	13.0	66.4		19.0	14.4	65.0		31.6				
Change Period (Y+Rc), s	4.0	5.0		5.0	5.0	* 5		5.0				
Max Green Setting (Gmax), s	9.0	57.0		14.0	6.0	* 60		31.0				
Max Q Clear Time (g_c+I1), s	9.3	29.1		14.3	2.6	22.9		25.3				
Green Ext Time (p_c), s	0.0	12.4		0.0	0.0	8.6		1.3				

Intersection Summary		
HCM 6th Ctrl Delay		38.3
HCM 6th LOS		D

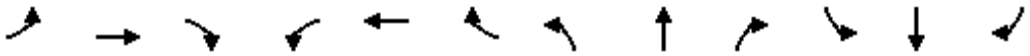
Notes

User approved volume balancing among the lanes for turning movement.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	137	29	86	31	35	62	96	2104	26	50	1713	53
Future Volume (vph)	137	29	86	31	35	62	96	2104	26	50	1713	53
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		65	95		0	100		0
Storage Lanes	0		0	0		1	1		0	1		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.95	0.95
Frt		0.954				0.850		0.998			0.995	
Flt Protected		0.974			0.977		0.950			0.950		
Satd. Flow (prot)	0	1731	0	0	1820	1583	1770	5075	0	1770	3522	0
Flt Permitted		0.795			0.780		0.087			0.063		
Satd. Flow (perm)	0	1413	0	0	1453	1583	162	5075	0	117	3522	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		18				19		3			6	
Link Speed (mph)		25			25			35			35	
Link Distance (ft)		390			645			190			410	
Travel Time (s)		10.6			17.6			3.7			8.0	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	137	29	86	31	35	62	96	2104	26	50	1713	53
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	252	0	0	66	62	96	2130	0	50	1766	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60		60	60		60	60		60	60		60
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100	20	20	100		20	100	
Trailing Detector (ft)	0	0		0	0	0	0	0		0	0	
Detector 1 Position(ft)	0	0		0	0	0	0	0		0	0	
Detector 1 Size(ft)	20	6		20	6	20	20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4		8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0		6.0	6.0	
Minimum Split (s)	33.0	33.0		33.0	33.0	33.0	26.0	26.0		26.0	26.0	
Total Split (s)	33.0	33.0		33.0	33.0	33.0	97.0	97.0		97.0	97.0	
Total Split (%)	25.4%	25.4%		25.4%	25.4%	25.4%	74.6%	74.6%		74.6%	74.6%	
Maximum Green (s)	28.0	28.0		28.0	28.0	28.0	92.0	92.0		92.0	92.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	4.0	4.0		4.0	4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.0			5.0	5.0	5.0	5.0		5.0	5.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None	None	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	21.0	21.0		21.0	21.0	21.0	14.0	14.0		14.0	14.0	
Pedestrian Calls (#/hr)	5	5		5	5	5	5	5		5	5	
Act Effct Green (s)		25.1			25.1	25.1	94.9	94.9		94.9	94.9	
Actuated g/C Ratio		0.19			0.19	0.19	0.73	0.73		0.73	0.73	
v/c Ratio		0.88			0.24	0.19	0.81	0.58		0.59	0.69	
Control Delay		76.3			45.3	32.0	40.5	6.1		31.7	4.1	
Queue Delay		0.0			0.0	0.0	0.0	0.3		0.0	0.0	
Total Delay		76.3			45.3	32.0	40.5	6.4		31.7	4.1	
LOS		E			D	C	D	A		C	A	
Approach Delay		76.3			38.9			7.9			4.8	
Approach LOS		E			D			A			A	
Queue Length 50th (ft)		190			46	30	73	203		2	116	
Queue Length 95th (ft)		#322			89	70	m29	m167		m#34	91	
Internal Link Dist (ft)		310			565			110			330	
Turn Bay Length (ft)						65	95			100		
Base Capacity (vph)		318			312	355	118	3704		85	2572	
Starvation Cap Reductn		0			0	0	0	731		0	0	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.79			0.21	0.17	0.81	0.72		0.59	0.69	

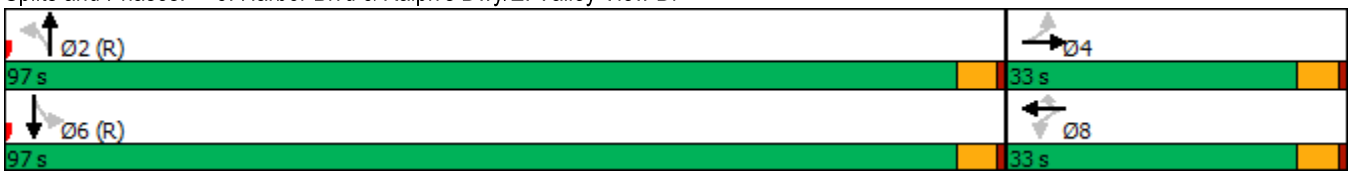
Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 111 (85%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green			
Natural Cycle: 110			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.88			
Intersection Signal Delay: 11.4		Intersection LOS: B	
Intersection Capacity Utilization 87.9%		ICU Level of Service E	
Analysis Period (min) 15			

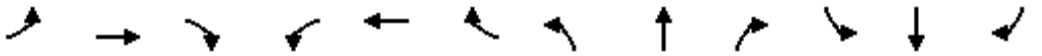
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Harbor Blvd & Ralph's Dwy/E. Valley View Dr



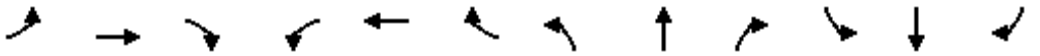


Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	137	29	86	31	35	62	96	2104	26	50	1713	53
Future Volume (veh/h)	137	29	86	31	35	62	96	2104	26	50	1713	53
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	137	29	86	31	35	62	96	2104	26	50	1713	53
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	192	34	95	146	152	286	190	3861	48	163	2613	81
Arrive On Green	0.18	0.18	0.18	0.18	0.18	0.18	0.74	0.74	0.74	0.74	0.74	0.74
Sat Flow, veh/h	826	186	524	585	841	1585	271	5199	64	189	3519	109
Grp Volume(v), veh/h	252	0	0	66	0	62	96	1377	753	50	862	904
Grp Sat Flow(s),veh/h/ln	1537	0	0	1426	0	1585	271	1702	1859	189	1777	1851
Q Serve(g_s), s	16.2	0.0	0.0	0.0	0.0	4.3	36.0	22.7	22.8	20.2	31.5	31.9
Cycle Q Clear(g_c), s	20.8	0.0	0.0	4.6	0.0	4.3	67.9	22.7	22.8	42.9	31.5	31.9
Prop In Lane	0.54		0.34	0.47		1.00	1.00		0.03	1.00		0.06
Lane Grp Cap(c), veh/h	320	0	0	298	0	286	190	2528	1380	163	1320	1375
V/C Ratio(X)	0.79	0.00	0.00	0.22	0.00	0.22	0.51	0.54	0.55	0.31	0.65	0.66
Avail Cap(c_a), veh/h	372	0	0	352	0	341	190	2528	1380	163	1320	1375
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	51.9	0.0	0.0	45.4	0.0	45.4	25.5	7.2	7.2	16.5	8.4	8.4
Incr Delay (d2), s/veh	9.4	0.0	0.0	0.4	0.0	0.4	9.3	0.8	1.6	4.8	2.5	2.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	8.9	0.0	0.0	1.9	0.0	1.8	2.7	7.5	8.4	1.1	11.3	11.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	61.3	0.0	0.0	45.7	0.0	45.8	34.8	8.1	8.8	21.3	10.9	10.9
LnGrp LOS	E	A	A	D	A	D	C	A	A	C	B	B
Approach Vol, veh/h		252			128			2226			1816	
Approach Delay, s/veh		61.3			45.8			9.5			11.2	
Approach LOS		E			D			A			B	

Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		101.5		28.5		101.5		28.5				
Change Period (Y+Rc), s		5.0		5.0		5.0		5.0				
Max Green Setting (Gmax), s		92.0		28.0		92.0		28.0				
Max Q Clear Time (g_c+I1), s		69.9		22.8		44.9		6.6				
Green Ext Time (p_c), s		17.5		0.7		24.9		0.5				

Intersection Summary												
HCM 6th Ctrl Delay			14.2									
HCM 6th LOS			B									

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	28	147	81	59	168	750	87	1438	26	412	1467	21
Future Volume (vph)	28	147	81	59	168	750	87	1438	26	412	1467	21
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	130		135	200		0	205		0	300		165
Storage Lanes	1		1	1		1	1		0	2		1
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.97	0.95	1.00
Frt			0.850			0.850		0.997				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1770	1863	1583	1770	3529	0	3433	3539	1583
Flt Permitted	0.425			0.488			0.950			0.950		
Satd. Flow (perm)	792	1863	1583	909	1863	1583	1770	3529	0	3433	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			92			50		2				50
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		538			720			654			541	
Travel Time (s)		10.5			14.0			12.7			10.5	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	28	147	81	59	168	750	87	1438	26	412	1467	21
Shared Lane Traffic (%)												
Lane Group Flow (vph)	28	147	81	59	168	750	87	1464	0	412	1467	21
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes										
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60		60	60		60	60		60	60		60
Number of Detectors	1	2	1	1	2	1	1	2		1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (ft)	20	100	20	20	100	20	20	100		20	100	20
Trailing Detector (ft)	0	0	0	0	0	0	0	0		0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0	0	0		0	0	0
Detector 1 Size(ft)	20	6	20	20	6	20	20	6		20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	Perm	Perm	NA	pm+ov	Prot	NA		Prot	NA	Perm
Protected Phases		4			8	1	5	2		1	6	
Permitted Phases	4		4	8		8						6



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	4	8	8	1	5	2		1	6	6
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0	6.0
Minimum Split (s)	30.0	30.0	30.0	36.0	36.0	10.0	10.0	30.0		10.0	30.0	30.0
Total Split (s)	36.0	36.0	36.0	36.0	36.0	35.0	16.0	59.0		35.0	78.0	78.0
Total Split (%)	27.7%	27.7%	27.7%	27.7%	27.7%	26.9%	12.3%	45.4%		26.9%	60.0%	60.0%
Maximum Green (s)	31.0	31.0	31.0	31.0	31.0	31.0	12.0	54.0		31.0	73.0	73.0
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	3.0	3.0	4.0		3.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	4.0	4.0	5.0		4.0	5.0	5.0
Lead/Lag						Lag	Lead	Lead		Lag	Lag	Lag
Lead-Lag Optimize?						Yes	Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max		None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0			7.0			7.0	7.0
Flash Dont Walk (s)	18.0	18.0	18.0	24.0	24.0			18.0			18.0	18.0
Pedestrian Calls (#/hr)	5	5	5	5	5			5			5	5
Act Effct Green (s)	18.6	18.6	18.6	18.6	18.6	54.6	10.9	66.4		31.0	86.5	86.5
Actuated g/C Ratio	0.14	0.14	0.14	0.14	0.14	0.42	0.08	0.51		0.24	0.67	0.67
v/c Ratio	0.25	0.55	0.27	0.46	0.63	1.08	0.59	0.81		0.50	0.62	0.02
Control Delay	51.6	58.3	8.6	59.9	61.9	92.1	73.4	32.1		28.9	4.3	0.1
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.1	0.0
Total Delay	51.6	58.3	8.6	59.9	61.9	92.1	73.4	32.1		28.9	4.4	0.1
LOS	D	E	A	E	E	F	E	C		C	A	A
Approach Delay		41.8			84.9			34.4			9.7	
Approach LOS		D			F			C			A	
Queue Length 50th (ft)	22	119	0	47	137	~698	71	511		141	55	0
Queue Length 95th (ft)	47	167	34	83	188	#799	128	#814		m178	291	m0
Internal Link Dist (ft)		458			640			574			461	
Turn Bay Length (ft)	130		135	200			205			300		165
Base Capacity (vph)	188	444	447	216	444	693	167	1803		818	2355	1070
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	190	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	0
Reduced v/c Ratio	0.15	0.33	0.18	0.27	0.38	1.08	0.52	0.81		0.50	0.68	0.02

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 6 (5%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 100			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 1.08			
Intersection Signal Delay: 35.3		Intersection LOS: D	
Intersection Capacity Utilization 103.7%		ICU Level of Service G	
Analysis Period (min) 15			

~ Volume exceeds capacity, queue is theoretically infinite.

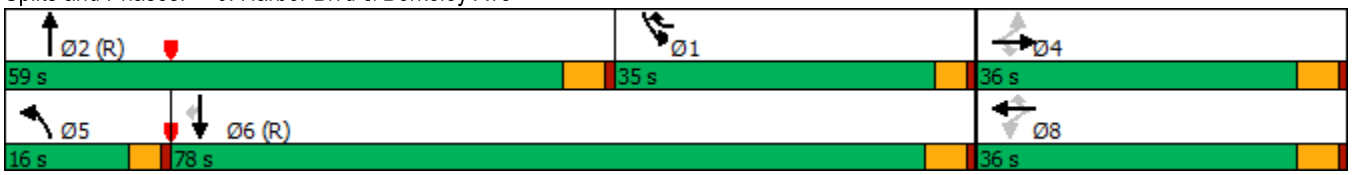
Queue shown is maximum after two cycles.

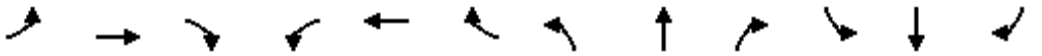
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Harbor Blvd & Berkeley Ave





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	28	147	81	59	168	750	87	1438	26	412	1467	21
Future Volume (veh/h)	28	147	81	59	168	750	87	1438	26	412	1467	21
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	28	147	81	59	168	750	87	1438	26	412	1467	21
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	254	446	378	271	446	744	109	1483	27	797	2106	939
Arrive On Green	0.24	0.24	0.24	0.24	0.24	0.24	0.06	0.42	0.42	0.23	0.59	0.59
Sat Flow, veh/h	1217	1870	1585	1241	1870	1585	1781	3571	65	3456	3554	1585
Grp Volume(v), veh/h	28	147	81	59	168	750	87	715	749	412	1467	21
Grp Sat Flow(s),veh/h/ln	1217	1870	1585	1241	1870	1585	1781	1777	1859	1728	1777	1585
Q Serve(g_s), s	2.6	8.4	5.3	5.4	9.8	31.0	6.3	51.2	51.3	13.5	37.2	0.7
Cycle Q Clear(g_c), s	12.3	8.4	5.3	13.8	9.8	31.0	6.3	51.2	51.3	13.5	37.2	0.7
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.03	1.00		1.00
Lane Grp Cap(c), veh/h	254	446	378	271	446	744	109	738	772	797	2106	939
V/C Ratio(X)	0.11	0.33	0.21	0.22	0.38	1.01	0.80	0.97	0.97	0.52	0.70	0.02
Avail Cap(c_a), veh/h	254	446	378	271	446	744	164	738	772	824	2106	939
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	46.6	40.9	39.7	46.6	41.4	34.5	60.2	37.2	37.2	43.7	18.4	10.9
Incr Delay (d2), s/veh	0.2	0.4	0.3	0.4	0.5	35.1	14.7	26.3	25.9	0.5	1.9	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.8	3.9	2.1	1.7	4.6	16.5	3.3	26.9	28.1	5.8	15.1	0.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	46.8	41.3	40.0	47.0	41.9	69.6	74.9	63.5	63.1	44.2	20.3	11.0
LnGrp LOS	D	D	D	D	D	F	E	E	E	D	C	B
Approach Vol, veh/h		256			977			1551			1900	
Approach Delay, s/veh		41.5			63.5			63.9			25.4	
Approach LOS		D			E			E			C	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	35.0	59.0		36.0	12.0	82.0		36.0				
Change Period (Y+Rc), s	5.0	* 5		5.0	4.0	5.0		5.0				
Max Green Setting (Gmax), s	31.0	* 54		31.0	12.0	73.0		31.0				
Max Q Clear Time (g_c+I1), s	15.5	53.3		14.3	8.3	39.2		33.0				
Green Ext Time (p_c), s	1.3	0.6		1.0	0.1	14.5		0.0				

Intersection Summary			
HCM 6th Ctrl Delay		47.0	
HCM 6th LOS		D	

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

APPENDIX B-IV

YEAR 2045 BUILDOUT PLUS PROJECT TRAFFIC CONDITIONS

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	327	1211	171	276	973	221	198	714	150	450	1511	341
Future Volume (vph)	327	1211	171	276	973	221	198	714	150	450	1511	341
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	325		0	193		145	230		150	275		0
Storage Lanes	1		0	2		1	2		1	2		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	0.91	0.91	0.97	0.91	1.00	0.97	0.91	1.00	0.97	0.91	0.91
Frt		0.981				0.850			0.850		0.972	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	4989	0	3433	5085	1583	3433	5085	1583	3433	4943	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	4989	0	3433	5085	1583	3433	5085	1583	3433	4943	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		21				165			150		43	
Link Speed (mph)		40			40			45			50	
Link Distance (ft)		1043			981			428			926	
Travel Time (s)		17.8			16.7			6.5			12.6	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	327	1211	171	276	973	221	198	714	150	450	1511	341
Shared Lane Traffic (%)												
Lane Group Flow (vph)	327	1382	0	276	973	221	198	714	150	450	1852	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		24			24			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2	1	1	2	1	1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru	Right	Left	Thru	
Leading Detector (ft)	20	100		20	100	20	20	100	20	20	100	
Trailing Detector (ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Position(ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Size(ft)	20	6		20	6	20	20	6	20	20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases						8			2			

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	7	4		3	8	8	5	2	2	1	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Minimum Split (s)	10.0	43.0		10.0	43.0	43.0	10.0	43.0	43.0	10.0	36.0	
Total Split (s)	25.0	48.0		20.0	43.0	43.0	12.0	43.0	43.0	19.0	50.0	
Total Split (%)	19.2%	36.9%		15.4%	33.1%	33.1%	9.2%	33.1%	33.1%	14.6%	38.5%	
Maximum Green (s)	21.0	43.0		16.0	38.0	38.0	8.0	38.0	38.0	15.0	45.0	
Yellow Time (s)	3.0	4.0		3.0	4.0	4.0	3.0	4.0	4.0	3.0	4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	4.0	5.0		4.0	5.0	5.0	4.0	5.0	5.0	4.0	5.0	
Lead/Lag	Lead	Lead		Lag	Lag	Lag	Lag	Lag	Lag	Lead	Lead	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None		None	None	None	None	C-Max	C-Max	None	C-Max	
Walk Time (s)		7.0			7.0	7.0		7.0	7.0		7.0	
Flash Dont Walk (s)		31.0			31.0	31.0		31.0	31.0		24.0	
Pedestrian Calls (#/hr)		5			5	5		5	5		5	
Act Effct Green (s)	21.0	41.7		14.6	35.4	35.4	8.0	38.0	38.0	17.6	47.6	
Actuated g/C Ratio	0.16	0.32		0.11	0.27	0.27	0.06	0.29	0.29	0.14	0.37	
v/c Ratio	1.15	0.86		0.72	0.70	0.40	0.94	0.48	0.26	0.97	1.01	
Control Delay	147.2	46.8		66.4	45.4	12.6	100.9	35.2	6.4	90.0	63.0	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	147.2	46.8		66.4	45.4	12.6	100.9	35.2	6.4	90.0	63.0	
LOS	F	D		E	D	B	F	D	A	F	E	
Approach Delay		66.0			44.4			43.4			68.3	
Approach LOS		E			D			D			E	
Queue Length 50th (ft)	~322	387		116	264	35	84	216	42	~223	~618	
Queue Length 95th (ft)	#513	449		164	313	103	#162	167	18	#336	#720	
Internal Link Dist (ft)		963			901			348			846	
Turn Bay Length (ft)	325			193		145	230		150	275		
Base Capacity (vph)	285	1664		422	1486	579	211	1486	568	466	1838	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	1.15	0.83		0.65	0.65	0.38	0.94	0.48	0.26	0.97	1.01	

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 55 (42%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 120			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 1.15			
Intersection Signal Delay: 58.3		Intersection LOS: E	
Intersection Capacity Utilization 94.4%		ICU Level of Service F	
Analysis Period (min) 15			

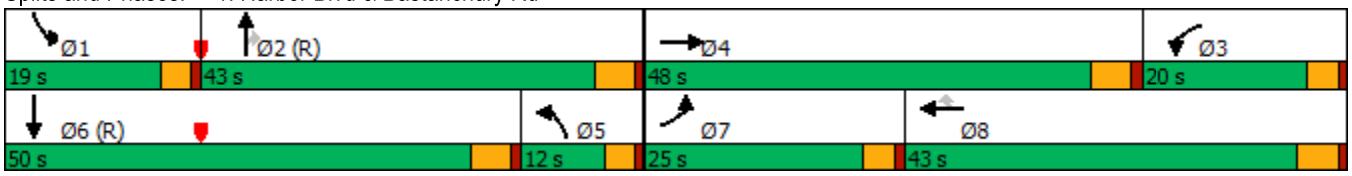
~ Volume exceeds capacity, queue is theoretically infinite.

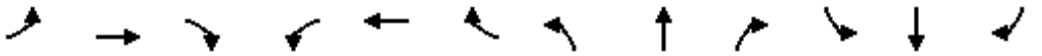
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: Harbor Blvd & Bastanchury Rd





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	327	1211	171	276	973	221	198	714	150	450	1511	341
Future Volume (veh/h)	327	1211	171	276	973	221	198	714	150	450	1511	341
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	327	1211	171	276	973	221	198	714	150	450	1511	341
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	288	1391	196	333	1277	396	332	1708	530	399	1443	324
Arrive On Green	0.16	0.31	0.31	0.10	0.25	0.25	0.10	0.33	0.33	0.12	0.35	0.35
Sat Flow, veh/h	1781	4521	638	3456	5106	1585	3456	5106	1585	3456	4170	936
Grp Volume(v), veh/h	327	912	470	276	973	221	198	714	150	450	1232	620
Grp Sat Flow(s),veh/h/ln	1781	1702	1755	1728	1702	1585	1728	1702	1585	1728	1702	1702
Q Serve(g_s), s	21.0	32.9	32.9	10.2	23.0	15.8	7.1	14.1	6.7	15.0	45.0	45.0
Cycle Q Clear(g_c), s	21.0	32.9	32.9	10.2	23.0	15.8	7.1	14.1	6.7	15.0	45.0	45.0
Prop In Lane	1.00		0.36	1.00		1.00	1.00		1.00	1.00		0.55
Lane Grp Cap(c), veh/h	288	1047	540	333	1277	396	332	1708	530	399	1178	589
V/C Ratio(X)	1.14	0.87	0.87	0.83	0.76	0.56	0.60	0.42	0.28	1.13	1.05	1.05
Avail Cap(c_a), veh/h	288	1126	581	425	1493	463	332	1708	530	399	1178	589
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	54.5	42.6	42.6	57.7	45.2	42.5	56.3	33.5	17.4	57.5	42.5	42.5
Incr Delay (d2), s/veh	95.0	7.2	12.9	10.4	2.0	1.2	2.9	0.8	1.3	84.9	39.0	51.6
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	16.8	14.5	15.9	4.9	9.8	6.2	3.2	5.8	3.5	11.0	24.2	26.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	149.5	49.8	55.5	68.0	47.2	43.7	59.2	34.2	18.7	142.4	81.5	94.1
LnGrp LOS	F	D	E	E	D	D	E	C	B	F	F	F
Approach Vol, veh/h		1709			1470			1062			2302	
Approach Delay, s/veh		70.5			50.6			36.7			96.8	
Approach LOS		E			D			D			F	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	19.0	48.5	17.5	45.0	17.5	50.0	25.0	37.5
Change Period (Y+Rc), s	4.0	5.0	5.0	* 5	5.0	* 5	4.0	5.0
Max Green Setting (Gmax), s	15.0	38.0	16.0	* 43	8.0	* 45	21.0	38.0
Max Q Clear Time (g_c+I1), s	17.0	16.1	12.2	34.9	9.1	47.0	23.0	25.0
Green Ext Time (p_c), s	0.0	5.0	0.3	5.0	0.0	0.0	0.0	5.9

Intersection Summary	
HCM 6th Ctrl Delay	69.8
HCM 6th LOS	E

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

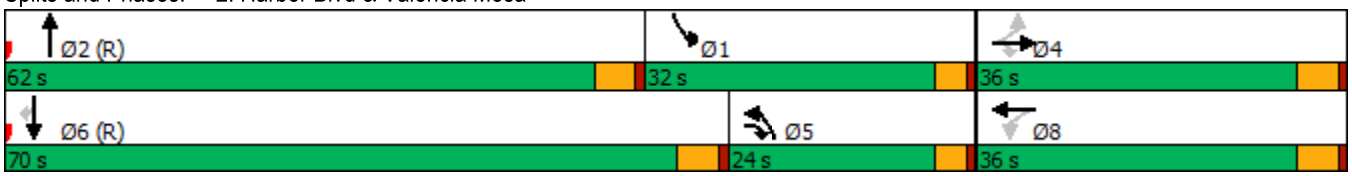
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	101	48	87	59	47	65	154	889	127	244	1448	185
Future Volume (vph)	101	48	87	59	47	65	154	889	127	244	1448	185
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	55		85	0		0	185		0	230		0
Storage Lanes	1		1	1		0	1		0	1		1
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	1.00
Frt			0.850		0.913			0.981				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1770	1701	0	1770	3472	0	1770	3539	1583
Flt Permitted	0.590			0.726			0.950			0.950		
Satd. Flow (perm)	1099	1863	1583	1352	1701	0	1770	3472	0	1770	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			59		50			15				148
Link Speed (mph)		30			30			50			45	
Link Distance (ft)		813			705			616			406	
Travel Time (s)		18.5			16.0			8.4			6.2	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	101	48	87	59	47	65	154	889	127	244	1448	185
Shared Lane Traffic (%)												
Lane Group Flow (vph)	101	48	87	59	112	0	154	1016	0	244	1448	185
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2	1	1	2		1	2		1	2	1
Detector Template	Left	Thru	Right	Left	Thru		Left	Thru		Left	Thru	Right
Leading Detector (ft)	20	100	20	20	100		20	100		20	100	20
Trailing Detector (ft)	0	0	0	0	0		0	0		0	0	0
Detector 1 Position(ft)	0	0	0	0	0		0	0		0	0	0
Detector 1 Size(ft)	20	6	20	20	6		20	6		20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	pm+ov	Perm	NA		Prot	NA		Prot	NA	Perm
Protected Phases		4	5		8		5	2		1	6	
Permitted Phases	4		4	8								6

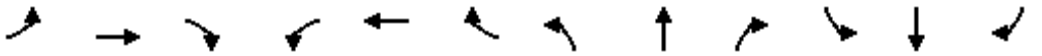
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	5	8	8		5	2		1	6	6
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0		6.0	6.0		6.0	6.0	6.0
Minimum Split (s)	11.0	11.0	10.0	36.0	36.0		10.0	26.0		10.0	30.0	30.0
Total Split (s)	36.0	36.0	24.0	36.0	36.0		24.0	62.0		32.0	70.0	70.0
Total Split (%)	27.7%	27.7%	18.5%	27.7%	27.7%		18.5%	47.7%		24.6%	53.8%	53.8%
Maximum Green (s)	31.0	31.0	20.0	31.0	31.0		20.0	57.0		28.0	65.0	65.0
Yellow Time (s)	4.0	4.0	3.0	4.0	4.0		3.0	4.0		3.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0		1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	4.0	5.0	5.0		4.0	5.0		4.0	5.0	5.0
Lead/Lag			Lag				Lag	Lead		Lag	Lead	Lead
Lead-Lag Optimize?			Yes				Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0
Recall Mode	None	None	None	None	None		None	C-Max		None	C-Max	C-Max
Walk Time (s)				7.0	7.0			7.0			7.0	7.0
Flash Dont Walk (s)				24.0	24.0			14.0			18.0	18.0
Pedestrian Calls (#/hr)				5	5			5			5	5
Act Effct Green (s)	17.7	17.7	38.7	17.7	17.7		16.0	74.3		24.0	82.3	82.3
Actuated g/C Ratio	0.14	0.14	0.30	0.14	0.14		0.12	0.57		0.18	0.63	0.63
v/c Ratio	0.68	0.19	0.17	0.32	0.41		0.71	0.51		0.75	0.65	0.18
Control Delay	73.5	48.0	11.4	52.4	31.2		48.5	29.8		47.2	3.1	0.1
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.1	0.0
Total Delay	73.5	48.0	11.4	52.4	31.2		48.5	29.8		47.2	3.2	0.1
LOS	E	D	B	D	C		D	C		D	A	A
Approach Delay		45.4			38.5			32.3			8.6	
Approach LOS		D			D			C			A	
Queue Length 50th (ft)	83	37	17	46	48		121	296		217	53	0
Queue Length 95th (ft)	129	66	46	80	95		185	380		m231	m72	m1
Internal Link Dist (ft)		733			625			536			326	
Turn Bay Length (ft)	55		85				185			230		
Base Capacity (vph)	262	444	504	322	443		272	1990		381	2240	1056
Starvation Cap Reductn	0	0	0	0	0		0	0		0	94	0
Spillback Cap Reductn	0	0	0	0	0		0	0		0	0	0
Storage Cap Reductn	0	0	0	0	0		0	0		0	0	0
Reduced v/c Ratio	0.39	0.11	0.17	0.18	0.25		0.57	0.51		0.64	0.67	0.18

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 33 (25%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 90			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.75			
Intersection Signal Delay: 20.6		Intersection LOS: C	
Intersection Capacity Utilization 72.5%		ICU Level of Service C	
Analysis Period (min) 15			

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Harbor Blvd & Valencia Mesa



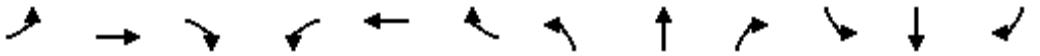


Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	101	48	87	59	47	65	154	889	127	244	1448	185
Future Volume (veh/h)	101	48	87	59	47	65	154	889	127	244	1448	185
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	101	48	87	59	47	65	154	889	127	244	1448	185
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	180	294	622	225	112	154	419	1368	195	529	1777	793
Arrive On Green	0.16	0.16	0.16	0.16	0.16	0.16	0.24	0.44	0.44	0.30	0.50	0.50
Sat Flow, veh/h	1281	1870	1585	1254	711	983	1781	3121	446	1781	3554	1585
Grp Volume(v), veh/h	101	48	87	59	0	112	154	506	510	244	1448	185
Grp Sat Flow(s),veh/h/ln	1281	1870	1585	1254	0	1693	1781	1777	1790	1781	1777	1585
Q Serve(g_s), s	10.0	2.9	0.0	5.6	0.0	7.8	9.4	29.1	29.1	14.5	44.7	8.6
Cycle Q Clear(g_c), s	17.8	2.9	0.0	8.4	0.0	7.8	9.4	29.1	29.1	14.5	44.7	8.6
Prop In Lane	1.00		1.00	1.00		0.58	1.00		0.25	1.00		1.00
Lane Grp Cap(c), veh/h	180	294	622	225	0	266	419	779	785	529	1777	793
V/C Ratio(X)	0.56	0.16	0.14	0.26	0.00	0.42	0.37	0.65	0.65	0.46	0.81	0.23
Avail Cap(c_a), veh/h	284	446	751	327	0	404	419	779	785	529	1777	793
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	57.5	47.4	25.4	51.0	0.0	49.5	41.6	28.7	28.7	37.2	27.4	18.4
Incr Delay (d2), s/veh	2.7	0.3	0.1	0.6	0.0	1.1	0.5	4.2	4.1	0.6	4.2	0.7
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.4	1.4	1.8	1.8	0.0	3.4	4.1	12.6	12.6	6.3	18.7	3.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	60.2	47.7	25.5	51.7	0.0	50.5	42.2	32.8	32.8	37.9	31.7	19.1
LnGrp LOS	E	D	C	D	A	D	D	C	C	D	C	B
Approach Vol, veh/h		236			171			1170			1877	
Approach Delay, s/veh		44.9			50.9			34.0			31.2	
Approach LOS		D			D			C			C	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	42.6	62.0		25.4	34.6	70.0		25.4				
Change Period (Y+Rc), s	4.0	5.0		5.0	4.0	5.0		5.0				
Max Green Setting (Gmax), s	28.0	57.0		31.0	20.0	65.0		31.0				
Max Q Clear Time (g_c+I1), s	16.5	31.1		19.8	11.4	46.7		10.4				
Green Ext Time (p_c), s	0.5	6.3		0.6	0.2	10.3		0.8				

Intersection Summary			
HCM 6th Ctrl Delay		34.1	
HCM 6th LOS		C	

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	51	84	9	773	116	174	22	1059	494	60	1464	80
Future Volume (vph)	51	84	9	773	116	174	22	1059	494	60	1464	80
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	85		0	0		0	95		0	140		140
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950	0.964		0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1681	1706	1583	1770	3539	1583	1770	3539	1583
Flt Permitted	0.950			0.950	0.964		0.950			0.950		
Satd. Flow (perm)	1770	1863	1583	1681	1706	1583	1770	3539	1583	1770	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			134			169			270			101
Link Speed (mph)		25			45			35			50	
Link Distance (ft)		871			1039			280			363	
Travel Time (s)		23.8			15.7			5.5			5.0	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	51	84	9	773	116	174	22	1059	494	60	1464	80
Shared Lane Traffic (%)				43%								
Lane Group Flow (vph)	51	84	9	441	448	174	22	1059	494	60	1464	80
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (ft)	20	100	20	20	100	20	20	100	20	20	100	20
Trailing Detector (ft)	0	0	0	0	0	0	0	0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0	0	0	0	0	0	0
Detector 1 Size(ft)	20	6	20	20	6	20	20	6	20	20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Split	NA	Perm	Split	NA	Perm	Prot	NA	Free	Prot	NA	Perm
Protected Phases	4	4		8	8		5	2		1	6	
Permitted Phases			4			8			Free			6



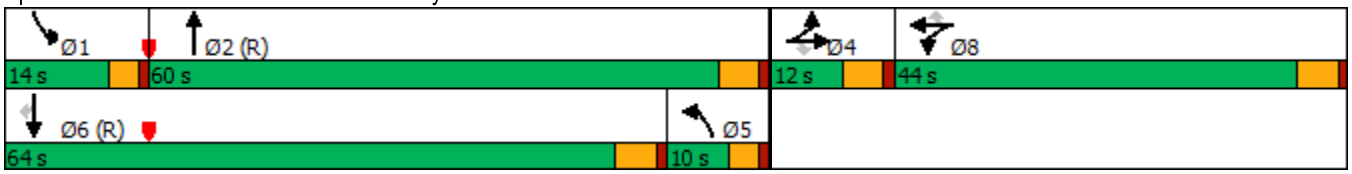
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	4	8	8	8	5	2		1	6	6
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0	6.0
Minimum Split (s)	11.0	11.0	11.0	36.0	36.0	36.0	10.0	23.0		10.0	30.0	30.0
Total Split (s)	12.0	12.0	12.0	44.0	44.0	44.0	10.0	60.0		14.0	64.0	64.0
Total Split (%)	9.2%	9.2%	9.2%	33.8%	33.8%	33.8%	7.7%	46.2%		10.8%	49.2%	49.2%
Maximum Green (s)	7.0	7.0	7.0	39.0	39.0	39.0	6.0	55.0		10.0	59.0	59.0
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	3.0	4.0		3.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	4.0	5.0		4.0	5.0	5.0
Lead/Lag							Lag	Lag		Lead	Lead	Lead
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max		None	C-Max	C-Max
Walk Time (s)				7.0	7.0	7.0		7.0			7.0	7.0
Flash Dont Walk (s)				24.0	24.0	24.0		11.0			18.0	18.0
Pedestrian Calls (#/hr)				5	5	5		5			5	5
Act Effct Green (s)	7.0	7.0	7.0	37.3	37.3	37.3	6.0	59.9	130.0	8.8	64.7	64.7
Actuated g/C Ratio	0.05	0.05	0.05	0.29	0.29	0.29	0.05	0.46	1.00	0.07	0.50	0.50
v/c Ratio	0.54	0.84	0.04	0.92	0.92	0.30	0.27	0.65	0.31	0.50	0.83	0.10
Control Delay	80.6	114.8	0.3	69.6	69.6	6.8	55.6	20.5	0.5	87.3	26.1	5.0
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.1	0.0	0.0	0.1	0.0
Total Delay	80.6	114.8	0.3	69.6	69.6	6.8	55.6	20.6	0.5	87.3	26.2	5.0
LOS	F	F	A	E	E	A	E	C	A	F	C	A
Approach Delay		95.6			59.3			14.8			27.4	
Approach LOS		F			E			B			C	
Queue Length 50th (ft)	43	72	0	371	377	3	19	424	0	52	403	5
Queue Length 95th (ft)	#95	#170	0	#569	#574	56	46	272	2	102	#773	29
Internal Link Dist (ft)		791			959			200			283	
Turn Bay Length (ft)	85						95			140		140
Base Capacity (vph)	95	100	212	504	511	593	81	1631	1583	136	1762	839
Starvation Cap Reductn	0	0	0	0	0	0	0	47	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	17	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.54	0.84	0.04	0.88	0.88	0.29	0.27	0.67	0.31	0.44	0.84	0.10

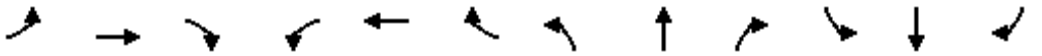
Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 120 (92%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 100			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.92			
Intersection Signal Delay: 32.9		Intersection LOS: C	
Intersection Capacity Utilization 88.3%		ICU Level of Service E	
Analysis Period (min) 15			

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: Harbor Blvd & Valley View/Brea Blvd





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	51	84	9	773	116	174	22	1059	494	60	1464	80
Future Volume (veh/h)	51	84	9	773	116	174	22	1059	494	60	1464	80
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	51	84	9	856	0	0	22	1059	0	60	1464	80
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	96	101	85	939	0		134	1753		77	1613	719
Arrive On Green	0.05	0.05	0.05	0.26	0.00	0.00	0.07	0.49	0.00	0.04	0.45	0.45
Sat Flow, veh/h	1781	1870	1585	3563	0	1585	1781	3554	1585	1781	3554	1585
Grp Volume(v), veh/h	51	84	9	856	0	0	22	1059	0	60	1464	80
Grp Sat Flow(s),veh/h/ln	1781	1870	1585	1781	0	1585	1781	1777	1585	1781	1777	1585
Q Serve(g_s), s	3.6	5.8	0.7	30.3	0.0	0.0	1.5	28.0	0.0	4.3	49.7	3.8
Cycle Q Clear(g_c), s	3.6	5.8	0.7	30.3	0.0	0.0	1.5	28.0	0.0	4.3	49.7	3.8
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	96	101	85	939	0		134	1753		77	1613	719
V/C Ratio(X)	0.53	0.83	0.11	0.91	0.00		0.16	0.60		0.78	0.91	0.11
Avail Cap(c_a), veh/h	96	101	85	1069	0		134	1753		137	1613	719
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	59.9	60.9	58.5	46.4	0.0	0.0	56.3	23.8	0.0	61.6	33.0	20.4
Incr Delay (d2), s/veh	5.5	42.4	0.5	10.7	0.0	0.0	0.6	1.6	0.0	15.3	9.0	0.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.8	4.0	0.3	14.4	0.0	0.0	0.7	11.8	0.0	2.2	21.9	1.4
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	65.4	103.3	59.1	57.1	0.0	0.0	56.9	25.3	0.0	76.8	42.0	20.7
LnGrp LOS	E	F	E	E	A		E	C		E	D	C
Approach Vol, veh/h		144			856			1081			1604	
Approach Delay, s/veh		87.1			57.1			26.0			42.2	
Approach LOS		F			E			C			D	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	9.6	69.1		12.0	14.7	64.0		39.3				
Change Period (Y+Rc), s	4.0	5.0		5.0	5.0	* 5		5.0				
Max Green Setting (Gmax), s	10.0	55.0		7.0	6.0	* 59		39.0				
Max Q Clear Time (g_c+I1), s	6.3	30.0		7.8	3.5	51.7		32.3				
Green Ext Time (p_c), s	0.0	8.2		0.0	0.0	5.0		2.0				

Intersection Summary			
HCM 6th Ctrl Delay		42.7	
HCM 6th LOS		D	

Notes

User approved volume balancing among the lanes for turning movement.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

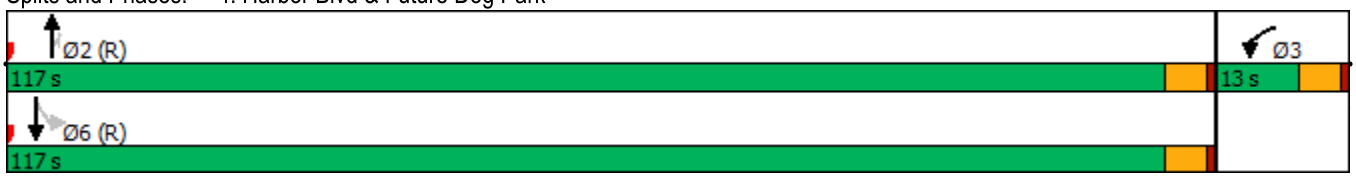
Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	5	5	1279	5	5	1589
Future Volume (vph)	5	5	1279	5	5	1589
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		200	250	
Storage Lanes	1	0		1	1	
Taper Length (ft)	60				60	
Lane Util. Factor	1.00	1.00	0.95	1.00	1.00	0.95
Frt	0.932			0.850		
Flt Protected	0.976				0.950	
Satd. Flow (prot)	1694	0	3539	1583	1770	3539
Flt Permitted	0.976				0.213	
Satd. Flow (perm)	1694	0	3539	1583	397	3539
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)	5			5		
Link Speed (mph)	30		30			30
Link Distance (ft)	484		960			1191
Travel Time (s)	11.0		21.8			27.1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	5	5	1279	5	5	1589
Shared Lane Traffic (%)						
Lane Group Flow (vph)	10	0	1279	5	5	1589
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Number of Detectors	1		2	1	1	2
Detector Template	Left		Thru	Right	Left	Thru
Leading Detector (ft)	20		100	20	20	100
Trailing Detector (ft)	0		0	0	0	0
Detector 1 Position(ft)	0		0	0	0	0
Detector 1 Size(ft)	20		6	20	20	6
Detector 1 Type	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0		0.0	0.0	0.0	0.0
Detector 2 Position(ft)			94			94
Detector 2 Size(ft)			6			6
Detector 2 Type			Cl+Ex			Cl+Ex
Detector 2 Channel						
Detector 2 Extend (s)			0.0			0.0
Turn Type	Prot		NA	Perm	Perm	NA
Protected Phases	3		2			6
Permitted Phases				2	6	

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Detector Phase	3		2	2	6	6
Switch Phase						
Minimum Initial (s)	6.0		6.0	6.0	6.0	6.0
Minimum Split (s)	11.0		11.0	11.0	11.0	11.0
Total Split (s)	13.0		117.0	117.0	117.0	117.0
Total Split (%)	10.0%		90.0%	90.0%	90.0%	90.0%
Maximum Green (s)	8.0		112.0	112.0	112.0	112.0
Yellow Time (s)	4.0		4.0	4.0	4.0	4.0
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0		5.0	5.0	5.0	5.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	None		C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	6.4		126.5	126.5	126.5	126.5
Actuated g/C Ratio	0.05		0.97	0.97	0.97	0.97
v/c Ratio	0.11		0.37	0.00	0.01	0.46
Control Delay	46.1		0.8	0.2	1.6	2.1
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	46.1		0.8	0.2	1.6	2.1
LOS	D		A	A	A	A
Approach Delay	46.1		0.8			2.0
Approach LOS	D		A			A
Queue Length 50th (ft)	4		17	0	0	1
Queue Length 95th (ft)	23		28	m0	m1	347
Internal Link Dist (ft)	404		880			1111
Turn Bay Length (ft)				200	250	
Base Capacity (vph)	108		3444	1541	386	3444
Starvation Cap Reductn	0		0	0	0	0
Spillback Cap Reductn	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	0.09		0.37	0.00	0.01	0.46

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Green			
Natural Cycle: 40			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.46			
Intersection Signal Delay: 1.6		Intersection LOS: A	
Intersection Capacity Utilization 57.3%		ICU Level of Service B	
Analysis Period (min) 15			
m Volume for 95th percentile queue is metered by upstream signal.			

Splits and Phases: 4: Harbor Blvd & Future Dog Park



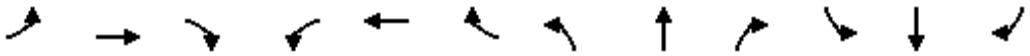


Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	5	5	1279	5	5	1589
Future Volume (veh/h)	5	5	1279	5	5	1589
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	5	5	1279	5	5	1589
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	12	12	3227	1439	424	3227
Arrive On Green	0.02	0.02	0.91	0.91	0.91	0.91
Sat Flow, veh/h	770	770	3647	1585	430	3647
Grp Volume(v), veh/h	11	0	1279	5	5	1589
Grp Sat Flow(s),veh/h/ln	1693	0	1777	1585	430	1777
Q Serve(g_s), s	0.8	0.0	6.7	0.0	0.2	9.7
Cycle Q Clear(g_c), s	0.8	0.0	6.7	0.0	6.9	9.7
Prop In Lane	0.45	0.45		1.00	1.00	
Lane Grp Cap(c), veh/h	26	0	3227	1439	424	3227
V/C Ratio(X)	0.43	0.00	0.40	0.00	0.01	0.49
Avail Cap(c_a), veh/h	104	0	3227	1439	424	3227
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	63.5	0.0	0.9	0.6	1.4	1.0
Incr Delay (d2), s/veh	11.0	0.0	0.4	0.0	0.1	0.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	0.0	0.6	0.0	0.0	0.9
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	74.4	0.0	1.2	0.6	1.4	1.5
LnGrp LOS	E	A	A	A	A	A
Approach Vol, veh/h	11		1284			1594
Approach Delay, s/veh	74.4		1.2			1.5
Approach LOS	E		A			A

Timer - Assigned Phs		2			6	8
Phs Duration (G+Y+Rc), s		123.0			123.0	7.0
Change Period (Y+Rc), s		5.0			5.0	5.0
Max Green Setting (Gmax), s		112.0			112.0	8.0
Max Q Clear Time (g_c+I1), s		8.7			11.7	2.8
Green Ext Time (p_c), s		14.8			23.3	0.0

Intersection Summary	
HCM 6th Ctrl Delay	1.7
HCM 6th LOS	A

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	48	22	42	23	12	53	80	1461	33	63	2242	49
Future Volume (vph)	48	22	42	23	12	53	80	1461	33	63	2242	49
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		65	95		0	100		0
Storage Lanes	0		0	0		1	1		0	1		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.95	0.95
Frt		0.949				0.850		0.997			0.997	
Flt Protected		0.979			0.968		0.950			0.950		
Satd. Flow (prot)	0	1731	0	0	1803	1583	1770	5070	0	1770	3529	0
Flt Permitted		0.845			0.743		0.043			0.154		
Satd. Flow (perm)	0	1494	0	0	1384	1583	80	5070	0	287	3529	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		14				53		6			4	
Link Speed (mph)		25			25			35			35	
Link Distance (ft)		427			781			235			407	
Travel Time (s)		11.6			21.3			4.6			7.9	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	48	22	42	23	12	53	80	1461	33	63	2242	49
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	112	0	0	35	53	80	1494	0	63	2291	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60		60	60		60	60		60	60		60
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100	20	20	100		20	100	
Trailing Detector (ft)	0	0		0	0	0	0	0		0	0	
Detector 1 Position(ft)	0	0		0	0	0	0	0		0	0	
Detector 1 Size(ft)	20	6		20	6	20	20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4		8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0		6.0	6.0	
Minimum Split (s)	33.0	33.0		33.0	33.0	33.0	26.0	26.0		26.0	26.0	
Total Split (s)	33.0	33.0		33.0	33.0	33.0	97.0	97.0		97.0	97.0	
Total Split (%)	25.4%	25.4%		25.4%	25.4%	25.4%	74.6%	74.6%		74.6%	74.6%	
Maximum Green (s)	28.0	28.0		28.0	28.0	28.0	92.0	92.0		92.0	92.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	4.0	4.0		4.0	4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.0			5.0	5.0	5.0	5.0		5.0	5.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None	None	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	21.0	21.0		21.0	21.0	21.0	14.0	14.0		14.0	14.0	
Pedestrian Calls (#/hr)	5	5		5	5	5	5	5		5	5	
Act Effct Green (s)		15.4			15.4	15.4	104.6	104.6		104.6	104.6	
Actuated g/C Ratio		0.12			0.12	0.12	0.80	0.80		0.80	0.80	
v/c Ratio		0.59			0.21	0.23	1.25	0.37		0.27	0.81	
Control Delay		58.5			51.4	13.7	204.2	3.2		2.5	3.6	
Queue Delay		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		58.5			51.4	13.7	204.2	3.2		2.5	3.7	
LOS		E			D	B	F	A		A	A	
Approach Delay		58.5			28.7			13.4			3.6	
Approach LOS		E			C			B			A	
Queue Length 50th (ft)		80			28	0	~84	77		1	10	
Queue Length 95th (ft)		129			54	35	m#170	106		m5	298	
Internal Link Dist (ft)		347			701			155			327	
Turn Bay Length (ft)						65	95			100		
Base Capacity (vph)		332			298	382	64	4080		230	2840	
Starvation Cap Reductn		0			0	0	0	0		0	6	
Spillback Cap Reductn		0			0	0	0	0		0	28	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.34			0.12	0.14	1.25	0.37		0.27	0.81	

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 87 (67%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green			
Natural Cycle: 120			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 1.25			
Intersection Signal Delay: 9.4		Intersection LOS: A	
Intersection Capacity Utilization 87.9%		ICU Level of Service E	
Analysis Period (min) 15			

~ Volume exceeds capacity, queue is theoretically infinite.

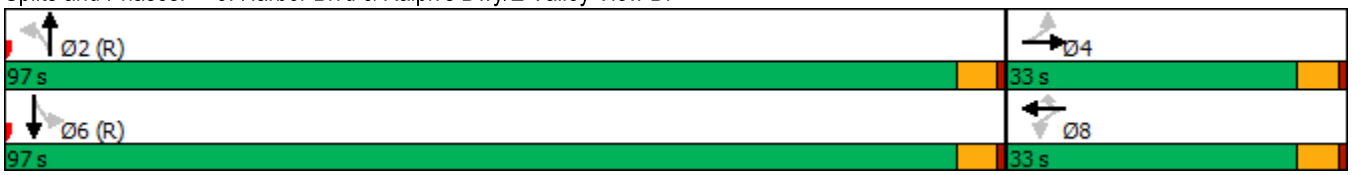
Queue shown is maximum after two cycles.

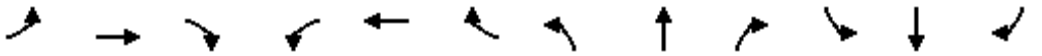
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Harbor Blvd & Ralph's Dwy/E Valley View Dr





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	48	22	42	23	12	53	80	1461	33	63	2242	49
Future Volume (veh/h)	48	22	42	23	12	53	80	1461	33	63	2242	49
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	48	22	42	23	12	53	80	1461	33	63	2242	49
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	92	35	52	116	52	138	145	4296	97	327	2973	65
Arrive On Green	0.09	0.09	0.09	0.09	0.09	0.09	0.84	0.84	0.84	0.84	0.84	0.84
Sat Flow, veh/h	599	405	602	811	596	1585	162	5137	116	352	3556	77
Grp Volume(v), veh/h	112	0	0	35	0	53	80	968	526	63	1116	1175
Grp Sat Flow(s),veh/h/ln	1606	0	0	1407	0	1585	162	1702	1849	352	1777	1856
Q Serve(g_s), s	6.0	0.0	0.0	0.0	0.0	4.1	56.9	8.5	8.5	6.5	36.0	36.7
Cycle Q Clear(g_c), s	8.8	0.0	0.0	2.8	0.0	4.1	93.6	8.5	8.5	14.9	36.0	36.7
Prop In Lane	0.43		0.37	0.66		1.00	1.00		0.06	1.00		0.04
Lane Grp Cap(c), veh/h	179	0	0	168	0	138	145	2847	1547	327	1486	1552
V/C Ratio(X)	0.63	0.00	0.00	0.21	0.00	0.38	0.55	0.34	0.34	0.19	0.75	0.76
Avail Cap(c_a), veh/h	378	0	0	359	0	341	145	2847	1547	327	1486	1552
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	58.1	0.0	0.0	55.4	0.0	56.1	25.6	2.4	2.4	4.1	4.7	4.7
Incr Delay (d2), s/veh	3.6	0.0	0.0	0.6	0.0	1.8	14.3	0.3	0.6	1.3	3.5	3.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.8	0.0	0.0	1.1	0.0	1.7	2.6	2.0	2.3	0.5	9.9	10.5
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	61.6	0.0	0.0	56.0	0.0	57.8	39.9	2.8	3.0	5.5	8.2	8.2
LnGrp LOS	E	A	A	E	A	E	D	A	A	A	A	A
Approach Vol, veh/h		112			88			1574			2354	
Approach Delay, s/veh		61.6			57.1			4.7			8.2	
Approach LOS		E			E			A			A	

Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		113.7		16.3		113.7		16.3				
Change Period (Y+Rc), s		5.0		5.0		5.0		5.0				
Max Green Setting (Gmax), s		92.0		28.0		92.0		28.0				
Max Q Clear Time (g_c+I1), s		95.6		10.8		38.7		6.1				
Green Ext Time (p_c), s		0.0		0.5		39.1		0.3				

Intersection Summary												
HCM 6th Ctrl Delay			9.4									
HCM 6th LOS			A									

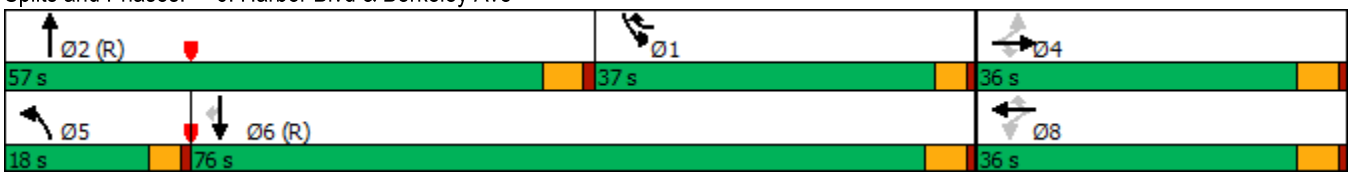
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	17	176	73	53	165	466	101	1092	43	639	1592	19
Future Volume (vph)	17	176	73	53	165	466	101	1092	43	639	1592	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	130		135	200		0	205		0	300		165
Storage Lanes	1		1	1		1	1		0	2		1
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.97	0.95	1.00
Frt			0.850			0.850		0.994				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1770	1863	1583	1770	3518	0	3433	3539	1583
Flt Permitted	0.440			0.407			0.950			0.950		
Satd. Flow (perm)	820	1863	1583	758	1863	1583	1770	3518	0	3433	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			92			50		4				50
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		647			786			659			504	
Travel Time (s)		12.6			15.3			12.8			9.8	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	17	176	73	53	165	466	101	1092	43	639	1592	19
Shared Lane Traffic (%)												
Lane Group Flow (vph)	17	176	73	53	165	466	101	1135	0	639	1592	19
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes										
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60		60	60		60	60		60	60		60
Number of Detectors	1	2	1	1	2	1	1	2		1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (ft)	20	100	20	20	100	20	20	100		20	100	20
Trailing Detector (ft)	0	0	0	0	0	0	0	0		0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0	0	0		0	0	0
Detector 1 Size(ft)	20	6	20	20	6	20	20	6		20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	Perm	Perm	NA	pm+ov	Prot	NA		Prot	NA	Perm
Protected Phases		4			8	1	5	2		1	6	
Permitted Phases	4		4	8		8						6

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	4	8	8	1	5	2		1	6	6
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0	6.0
Minimum Split (s)	30.0	30.0	30.0	36.0	36.0	10.0	10.0	30.0		10.0	30.0	30.0
Total Split (s)	36.0	36.0	36.0	36.0	36.0	37.0	18.0	57.0		37.0	76.0	76.0
Total Split (%)	27.7%	27.7%	27.7%	27.7%	27.7%	28.5%	13.8%	43.8%		28.5%	58.5%	58.5%
Maximum Green (s)	31.0	31.0	31.0	31.0	31.0	33.0	14.0	52.0		33.0	71.0	71.0
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	3.0	3.0	4.0		3.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	4.0	4.0	5.0		4.0	5.0	5.0
Lead/Lag						Lag	Lead	Lead		Lag	Lag	Lag
Lead-Lag Optimize?						Yes	Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max		None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0			7.0			7.0	7.0
Flash Dont Walk (s)	18.0	18.0	18.0	24.0	24.0			18.0			18.0	18.0
Pedestrian Calls (#/hr)	5	5	5	5	5			5			5	5
Act Effct Green (s)	19.0	19.0	19.0	19.0	19.0	57.0	12.0	64.0		33.0	85.0	85.0
Actuated g/C Ratio	0.15	0.15	0.15	0.15	0.15	0.44	0.09	0.49		0.25	0.65	0.65
v/c Ratio	0.14	0.65	0.24	0.48	0.61	0.65	0.62	0.66		0.73	0.69	0.02
Control Delay	47.1	62.2	6.5	63.0	60.1	29.0	73.1	28.2		36.0	8.1	0.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.3	0.0
Total Delay	47.1	62.2	6.5	63.0	60.1	29.0	73.1	28.2		36.0	8.5	0.2
LOS	D	E	A	E	E	C	E	C		D	A	A
Approach Delay		45.9			39.1			31.8			16.2	
Approach LOS		D			D			C			B	
Queue Length 50th (ft)	13	144	0	42	134	277	83	357		227	180	0
Queue Length 95th (ft)	33	197	27	78	185	324	143	530		305	470	m0
Internal Link Dist (ft)		567			706			579			424	
Turn Bay Length (ft)	130		135	200			205			300		165
Base Capacity (vph)	195	444	447	180	444	722	192	1732		871	2313	1052
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	240	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	0
Reduced v/c Ratio	0.09	0.40	0.16	0.29	0.37	0.65	0.53	0.66		0.73	0.77	0.02

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 115 (88%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 90			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.73			
Intersection Signal Delay: 25.9		Intersection LOS: C	
Intersection Capacity Utilization 79.9%		ICU Level of Service D	
Analysis Period (min) 15			

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Harbor Blvd & Berkeley Ave



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	17	176	73	53	165	466	101	1092	43	639	1592	19
Future Volume (veh/h)	17	176	73	53	165	466	101	1092	43	639	1592	19
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	17	176	73	53	165	466	101	1092	43	639	1592	19
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	155	304	258	147	304	768	125	1394	55	1112	2344	1045
Arrive On Green	0.16	0.16	0.16	0.16	0.16	0.16	0.07	0.40	0.40	0.32	0.66	0.66
Sat Flow, veh/h	1221	1870	1585	1209	1870	1585	1781	3485	137	3456	3554	1585
Grp Volume(v), veh/h	17	176	73	53	165	466	101	557	578	639	1592	19
Grp Sat Flow(s),veh/h/ln	1221	1870	1585	1209	1870	1585	1781	1777	1846	1728	1777	1585
Q Serve(g_s), s	1.7	11.3	5.3	5.5	10.5	0.0	7.3	35.6	35.6	20.0	35.9	0.5
Cycle Q Clear(g_c), s	12.2	11.3	5.3	16.8	10.5	0.0	7.3	35.6	35.6	20.0	35.9	0.5
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.07	1.00		1.00
Lane Grp Cap(c), veh/h	155	304	258	147	304	768	125	711	738	1112	2344	1045
V/C Ratio(X)	0.11	0.58	0.28	0.36	0.54	0.61	0.81	0.78	0.78	0.57	0.68	0.02
Avail Cap(c_a), veh/h	248	446	378	238	446	888	192	711	738	1112	2344	1045
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	55.6	50.3	47.8	58.1	50.0	24.5	59.6	34.1	34.1	36.7	13.7	7.6
Incr Delay (d2), s/veh	0.3	1.7	0.6	1.5	1.5	0.9	13.6	8.4	8.1	0.7	1.6	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.5	5.4	2.1	1.7	5.0	10.4	3.7	16.7	17.3	8.5	13.8	0.2
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	55.9	52.0	48.4	59.5	51.5	25.4	73.2	42.5	42.2	37.4	15.3	7.7
LnGrp LOS	E	D	D	E	D	C	E	D	D	D	B	A
Approach Vol, veh/h		266			684			1236			2250	
Approach Delay, s/veh		51.3			34.3			44.9			21.5	
Approach LOS		D			C			D			C	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	46.8	57.0		26.2	13.1	90.7		26.2				
Change Period (Y+Rc), s	5.0	* 5		5.0	4.0	5.0		5.0				
Max Green Setting (Gmax), s	33.0	* 52		31.0	14.0	71.0		31.0				
Max Q Clear Time (g_c+I1), s	22.0	37.6		14.2	9.3	37.9		18.8				
Green Ext Time (p_c), s	1.9	6.3		1.1	0.1	16.2		2.3				

Intersection Summary			
HCM 6th Ctrl Delay		31.8	
HCM 6th LOS		C	

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	373	1194	131	169	1048	338	198	1564	229	379	979	382
Future Volume (vph)	373	1194	131	169	1048	338	198	1564	229	379	979	382
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	325		0	193		145	230		150	275		0
Storage Lanes	1		0	2		1	2		1	2		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	0.91	0.91	0.97	0.91	1.00	0.97	0.91	1.00	0.97	0.91	0.91
Frt		0.985				0.850			0.850		0.958	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	5009	0	3433	5085	1583	3433	5085	1583	3433	4872	0
Flt Permitted	0.950			0.950			0.950			0.950		
Satd. Flow (perm)	1770	5009	0	3433	5085	1583	3433	5085	1583	3433	4872	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		17				223			126		79	
Link Speed (mph)		40			40			45			50	
Link Distance (ft)		1043			981			428			926	
Travel Time (s)		17.8			16.7			6.5			12.6	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	373	1194	131	169	1048	338	198	1564	229	379	979	382
Shared Lane Traffic (%)												
Lane Group Flow (vph)	373	1325	0	169	1048	338	198	1564	229	379	1361	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		24			24			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2	1	1	2	1	1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru	Right	Left	Thru	
Leading Detector (ft)	20	100		20	100	20	20	100	20	20	100	
Trailing Detector (ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Position(ft)	0	0		0	0	0	0	0	0	0	0	
Detector 1 Size(ft)	20	6		20	6	20	20	6	20	20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Prot	NA		Prot	NA	Perm	Prot	NA	Perm	Prot	NA	
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases						8			2			

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	7	4		3	8	8	5	2	2	1	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0	
Minimum Split (s)	10.0	43.0		10.0	43.0	43.0	10.0	43.0	43.0	10.0	36.0	
Total Split (s)	27.0	55.0		15.0	43.0	43.0	14.0	44.0	44.0	16.0	46.0	
Total Split (%)	20.8%	42.3%		11.5%	33.1%	33.1%	10.8%	33.8%	33.8%	12.3%	35.4%	
Maximum Green (s)	23.0	50.0		11.0	38.0	38.0	10.0	39.0	39.0	12.0	41.0	
Yellow Time (s)	3.0	4.0		3.0	4.0	4.0	3.0	4.0	4.0	3.0	4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Lost Time (s)	4.0	5.0		4.0	5.0	5.0	4.0	5.0	5.0	4.0	5.0	
Lead/Lag	Lag	Lead		Lag	Lead	Lead	Lag	Lag	Lag	Lead	Lead	
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes	Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Recall Mode	None	None		None	None	None	None	C-Max	C-Max	None	C-Max	
Walk Time (s)		7.0			7.0	7.0		7.0	7.0		7.0	
Flash Dont Walk (s)		31.0			31.0	31.0		31.0	31.0		24.0	
Pedestrian Calls (#/hr)		5			5	5		5	5		5	
Act Effct Green (s)	25.5	43.8		17.2	35.5	35.5	10.0	39.0	39.0	12.0	41.0	
Actuated g/C Ratio	0.20	0.34		0.13	0.27	0.27	0.08	0.30	0.30	0.09	0.32	
v/c Ratio	1.07	0.78		0.37	0.75	0.57	0.75	1.03	0.41	1.20	0.86	
Control Delay	119.2	41.5		55.9	46.9	17.0	63.9	63.5	15.9	165.4	45.6	
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	119.2	41.5		55.9	46.9	17.0	63.9	63.5	15.9	165.4	45.6	
LOS	F	D		E	D	B	E	E	B	F	D	
Approach Delay		58.6			41.4			58.0			71.7	
Approach LOS		E			D			E			E	
Queue Length 50th (ft)	~369	361		68	291	76	79	~471	46	~198	373	
Queue Length 95th (ft)	#577	389		110	341	174	m102	m#598	m55	#302	436	
Internal Link Dist (ft)		963			901			348			846	
Turn Bay Length (ft)	325			193		145	230		150	275		
Base Capacity (vph)	347	1937		453	1486	620	264	1525	563	316	1590	
Starvation Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Spillback Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Storage Cap Reductn	0	0		0	0	0	0	0	0	0	0	
Reduced v/c Ratio	1.07	0.68		0.37	0.71	0.55	0.75	1.03	0.41	1.20	0.86	

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 85 (65%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 120			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 1.20			
Intersection Signal Delay: 57.9		Intersection LOS: E	
Intersection Capacity Utilization 96.9%		ICU Level of Service F	
Analysis Period (min) 15			

~ Volume exceeds capacity, queue is theoretically infinite.

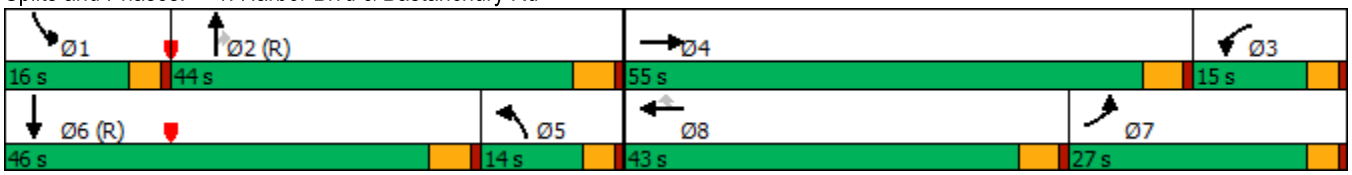
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Harbor Blvd & Bastanchury Rd



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	373	1194	131	169	1048	338	198	1564	229	379	979	382
Future Volume (veh/h)	373	1194	131	169	1048	338	198	1564	229	379	979	382
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	373	1194	131	169	1048	338	198	1564	229	379	979	382
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	315	1464	161	404	1294	402	373	1730	537	319	1139	444
Arrive On Green	0.18	0.31	0.31	0.12	0.25	0.25	0.11	0.34	0.34	0.09	0.32	0.32
Sat Flow, veh/h	1781	4670	512	3456	5106	1585	3456	5106	1585	3456	3612	1409
Grp Volume(v), veh/h	373	870	455	169	1048	338	198	1564	229	379	923	438
Grp Sat Flow(s),veh/h/ln	1781	1702	1778	1728	1702	1585	1728	1702	1585	1728	1702	1617
Q Serve(g_s), s	23.0	30.6	30.7	5.9	25.1	20.6	7.0	38.0	10.4	12.0	33.1	33.1
Cycle Q Clear(g_c), s	23.0	30.6	30.7	5.9	25.1	20.6	7.0	38.0	10.4	12.0	33.1	33.1
Prop In Lane	1.00		0.29	1.00		1.00	1.00		1.00	1.00		0.87
Lane Grp Cap(c), veh/h	315	1067	558	404	1294	402	373	1730	537	319	1074	510
V/C Ratio(X)	1.18	0.82	0.82	0.42	0.81	0.84	0.53	0.90	0.43	1.19	0.86	0.86
Avail Cap(c_a), veh/h	315	1309	684	404	1493	463	373	1730	537	319	1074	510
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	53.5	41.1	41.2	53.3	45.6	28.3	54.9	41.0	17.1	59.0	41.8	41.8
Incr Delay (d2), s/veh	110.2	3.4	6.3	0.7	3.0	11.7	1.4	8.2	2.5	111.7	9.0	17.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	19.7	13.0	14.1	2.6	10.7	8.9	3.1	16.6	4.1	10.0	14.6	15.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	163.7	44.5	47.5	54.0	48.6	40.0	56.3	49.2	19.6	170.7	50.8	58.8
LnGrp LOS	F	D	D	D	D	D	E	D	B	F	D	E
Approach Vol, veh/h		1698			1555			1991			1740	
Approach Delay, s/veh		71.5			47.3			46.5			78.9	
Approach LOS		E			D			D			E	

Timer - Assigned Phs	1	2	3	4	5	6	7	8
Phs Duration (G+Y+Rc), s	16.0	49.0	19.2	45.8	19.0	46.0	27.0	38.0
Change Period (Y+Rc), s	4.0	5.0	4.0	5.0	5.0	* 5	4.0	5.0
Max Green Setting (Gmax), s	12.0	39.0	11.0	50.0	10.0	* 41	23.0	38.0
Max Q Clear Time (g_c+I1), s	14.0	40.0	7.9	32.7	9.0	35.1	25.0	27.1
Green Ext Time (p_c), s	0.0	0.0	0.1	8.1	0.1	3.8	0.0	5.9

Intersection Summary	
HCM 6th Ctrl Delay	60.8
HCM 6th LOS	E

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	125	23	139	76	44	305	50	1569	56	119	1020	95
Future Volume (vph)	125	23	139	76	44	305	50	1569	56	119	1020	95
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	55		85	0		0	185		0	230		0
Storage Lanes	1		1	1		0	1		0	1		1
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	1.00
Frt			0.850		0.869			0.995				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1770	1619	0	1770	3522	0	1770	3539	1583
Flt Permitted	0.232			0.742			0.950			0.950		
Satd. Flow (perm)	432	1863	1583	1382	1619	0	1770	3522	0	1770	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			85		178			4				95
Link Speed (mph)		30			30			50			45	
Link Distance (ft)		813			705			616			406	
Travel Time (s)		18.5			16.0			8.4			6.2	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	125	23	139	76	44	305	50	1569	56	119	1020	95
Shared Lane Traffic (%)												
Lane Group Flow (vph)	125	23	139	76	349	0	50	1625	0	119	1020	95
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2	1	1	2		1	2		1	2	1
Detector Template	Left	Thru	Right	Left	Thru		Left	Thru		Left	Thru	Right
Leading Detector (ft)	20	100	20	20	100		20	100		20	100	20
Trailing Detector (ft)	0	0	0	0	0		0	0		0	0	0
Detector 1 Position(ft)	0	0	0	0	0		0	0		0	0	0
Detector 1 Size(ft)	20	6	20	20	6		20	6		20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	pm+ov	Perm	NA		Prot	NA		Prot	NA	Perm
Protected Phases		4	5		8		5	2		1	6	
Permitted Phases	4		4	8								6

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	5	8	8		5	2		1	6	6
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0		6.0	6.0		6.0	6.0	6.0
Minimum Split (s)	11.0	11.0	10.0	36.0	36.0		10.0	26.0		10.0	30.0	30.0
Total Split (s)	43.0	43.0	13.0	43.0	43.0		13.0	71.0		16.0	74.0	74.0
Total Split (%)	33.1%	33.1%	10.0%	33.1%	33.1%		10.0%	54.6%		12.3%	56.9%	56.9%
Maximum Green (s)	38.0	38.0	9.0	38.0	38.0		9.0	66.0		12.0	69.0	69.0
Yellow Time (s)	4.0	4.0	3.0	4.0	4.0		3.0	4.0		3.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0		1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	4.0	5.0	5.0		4.0	5.0		4.0	5.0	5.0
Lead/Lag			Lead				Lead	Lead		Lag	Lag	Lag
Lead-Lag Optimize?			Yes				Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0
Recall Mode	None	None	None	None	None		None	C-Max		None	C-Max	C-Max
Walk Time (s)				7.0	7.0			7.0			7.0	7.0
Flash Dont Walk (s)				24.0	24.0			14.0			18.0	18.0
Pedestrian Calls (#/hr)				5	5			5			5	5
Act Effct Green (s)	32.0	32.0	45.1	32.0	32.0		8.1	72.0		12.0	75.9	75.9
Actuated g/C Ratio	0.25	0.25	0.35	0.25	0.25		0.06	0.55		0.09	0.58	0.58
v/c Ratio	1.18	0.05	0.23	0.22	0.66		0.45	0.83		0.73	0.49	0.10
Control Delay	184.9	34.3	11.8	38.5	26.0		94.5	12.7		67.3	7.6	0.7
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Delay	184.9	34.3	11.8	38.5	26.0		94.5	12.7		67.3	7.6	0.7
LOS	F	C	B	D	C		F	B		E	A	A
Approach Delay		89.0			28.3			15.1			12.8	
Approach LOS		F			C			B			B	
Queue Length 50th (ft)	~114	14	29	49	124		44	102		105	116	0
Queue Length 95th (ft)	#240	36	73	90	225		90	221		m132	152	m3
Internal Link Dist (ft)		733			625			536			326	
Turn Bay Length (ft)	55		85				185			230		
Base Capacity (vph)	126	544	615	403	599		122	1952		163	2065	963
Starvation Cap Reductn	0	0	0	0	0		0	0		0	0	0
Spillback Cap Reductn	0	0	0	0	0		0	0		0	0	0
Storage Cap Reductn	0	0	0	0	0		0	0		0	0	0
Reduced v/c Ratio	0.99	0.04	0.23	0.19	0.58		0.41	0.83		0.73	0.49	0.10

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 81 (62%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 90			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 1.18			
Intersection Signal Delay: 21.7		Intersection LOS: C	
Intersection Capacity Utilization 95.6%		ICU Level of Service F	
Analysis Period (min) 15			

~ Volume exceeds capacity, queue is theoretically infinite.

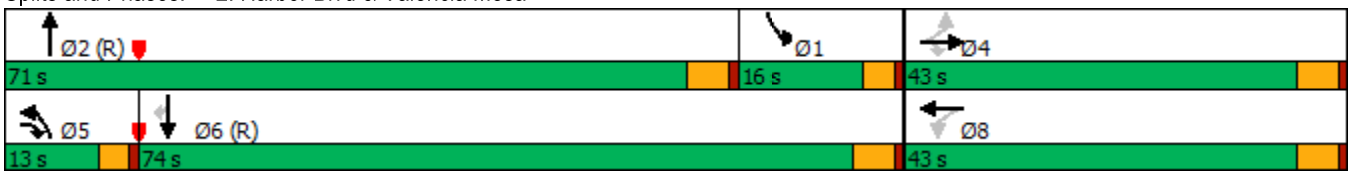
Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Harbor Blvd & Valencia Mesa



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	125	23	139	76	44	305	50	1569	56	119	1020	95
Future Volume (veh/h)	125	23	139	76	44	305	50	1569	56	119	1020	95
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	125	23	139	76	44	305	50	1569	56	119	1020	95
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	156	547	524	402	60	413	69	1777	63	151	1995	890
Arrive On Green	0.29	0.29	0.29	0.29	0.29	0.29	0.04	0.51	0.51	0.08	0.56	0.56
Sat Flow, veh/h	1032	1870	1585	1224	204	1412	1781	3500	125	1781	3554	1585
Grp Volume(v), veh/h	125	23	139	76	0	349	50	794	831	119	1020	95
Grp Sat Flow(s),veh/h/ln	1032	1870	1585	1224	0	1616	1781	1777	1848	1781	1777	1585
Q Serve(g_s), s	12.7	1.1	8.4	6.2	0.0	25.3	3.6	51.7	52.3	8.5	23.0	3.6
Cycle Q Clear(g_c), s	38.0	1.1	8.4	7.3	0.0	25.3	3.6	51.7	52.3	8.5	23.0	3.6
Prop In Lane	1.00		1.00	1.00		0.87	1.00		0.07	1.00		1.00
Lane Grp Cap(c), veh/h	156	547	524	402	0	472	69	902	938	151	1995	890
V/C Ratio(X)	0.80	0.04	0.27	0.19	0.00	0.74	0.73	0.88	0.89	0.79	0.51	0.11
Avail Cap(c_a), veh/h	156	547	524	402	0	472	123	902	938	164	1995	890
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	59.9	33.0	31.9	35.6	0.0	41.5	61.8	28.5	28.6	58.4	17.5	13.3
Incr Delay (d2), s/veh	25.1	0.0	0.3	0.2	0.0	6.1	13.6	12.0	12.0	20.8	0.9	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	5.2	0.5	3.3	1.9	0.0	10.9	1.9	23.1	24.2	4.6	9.0	1.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	85.0	33.0	32.2	35.8	0.0	47.6	75.4	40.5	40.6	79.2	18.5	13.5
LnGrp LOS	F	C	C	D	A	D	E	D	D	E	B	B
Approach Vol, veh/h		287			425			1675			1234	
Approach Delay, s/veh		55.3			45.5			41.6			23.9	
Approach LOS		E			D			D			C	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	16.0	71.0		43.0	9.0	78.0		43.0				
Change Period (Y+Rc), s	5.0	* 5		5.0	4.0	5.0		5.0				
Max Green Setting (Gmax), s	12.0	* 66		38.0	9.0	69.0		38.0				
Max Q Clear Time (g_c+I1), s	10.5	54.3		40.0	5.6	25.0		27.3				
Green Ext Time (p_c), s	0.0	7.5		0.0	0.0	8.6		1.9				

Intersection Summary			
HCM 6th Ctrl Delay		37.1	
HCM 6th LOS		D	

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	68	179	26	587	97	117	9	1447	788	102	1151	50
Future Volume (vph)	68	179	26	587	97	117	9	1447	788	102	1151	50
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	85		0	0		0	95		0	140		140
Storage Lanes	1		1	1		1	1		1	1		1
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	0.95	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt			0.850			0.850			0.850			0.850
Flt Protected	0.950			0.950	0.965		0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1681	1708	1583	1770	3539	1583	1770	3539	1583
Flt Permitted	0.950			0.950	0.965		0.950			0.950		
Satd. Flow (perm)	1770	1863	1583	1681	1708	1583	1770	3539	1583	1770	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			134			134			315			101
Link Speed (mph)		25			45			35			50	
Link Distance (ft)		871			1039			280			363	
Travel Time (s)		23.8			15.7			5.5			5.0	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	68	179	26	587	97	117	9	1447	788	102	1151	50
Shared Lane Traffic (%)				42%								
Lane Group Flow (vph)	68	179	26	340	344	117	9	1447	788	102	1151	50
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2	1	1	2	1	1	2	1	1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right
Leading Detector (ft)	20	100	20	20	100	20	20	100	20	20	100	20
Trailing Detector (ft)	0	0	0	0	0	0	0	0	0	0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0	0	0	0	0	0	0
Detector 1 Size(ft)	20	6	20	20	6	20	20	6	20	20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Split	NA	Perm	Split	NA	Perm	Prot	NA	Free	Prot	NA	Perm
Protected Phases	4	4		8	8		5	2		1	6	
Permitted Phases			4			8			Free			6

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	4	8	8	8	5	2		1	6	6
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0	6.0
Minimum Split (s)	11.0	11.0	11.0	36.0	36.0	36.0	10.0	23.0		10.0	30.0	30.0
Total Split (s)	19.0	19.0	19.0	36.0	36.0	36.0	10.0	62.0		13.0	65.0	65.0
Total Split (%)	14.6%	14.6%	14.6%	27.7%	27.7%	27.7%	7.7%	47.7%		10.0%	50.0%	50.0%
Maximum Green (s)	14.0	14.0	14.0	31.0	31.0	31.0	6.0	57.0		9.0	60.0	60.0
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	3.0	4.0		3.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	5.0	4.0	5.0		4.0	5.0	5.0
Lead/Lag							Lag	Lag		Lead	Lead	Lead
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max		None	C-Max	C-Max
Walk Time (s)				7.0	7.0	7.0		7.0			7.0	7.0
Flash Dont Walk (s)				24.0	24.0	24.0		11.0			18.0	18.0
Pedestrian Calls (#/hr)				5	5	5		5			5	5
Act Effct Green (s)	14.0	14.0	14.0	29.2	29.2	29.2	6.0	58.5	130.0	9.3	69.8	69.8
Actuated g/C Ratio	0.11	0.11	0.11	0.22	0.22	0.22	0.05	0.45	1.00	0.07	0.54	0.54
v/c Ratio	0.36	0.90	0.09	0.90	0.90	0.25	0.11	0.91	0.50	0.80	0.61	0.06
Control Delay	59.8	99.0	0.6	76.2	75.2	5.9	51.0	31.6	1.2	108.1	17.2	4.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.5	0.0	0.0	0.0	0.0
Total Delay	59.8	99.0	0.6	76.2	75.2	5.9	51.0	32.1	1.2	108.1	17.2	4.3
LOS	E	F	A	E	E	A	D	C	A	F	B	A
Approach Delay		79.8			65.5			21.3			23.9	
Approach LOS		E			E			C			C	
Queue Length 50th (ft)	54	151	0	289	292	0	8	643	16	91	120	0
Queue Length 95th (ft)	103	#290	0	#460	#463	38	m12	#747	18	#196	414	21
Internal Link Dist (ft)		791			959			200			283	
Turn Bay Length (ft)	85						95			140		140
Base Capacity (vph)	190	200	290	400	407	479	81	1592	1583	127	1901	897
Starvation Cap Reductn	0	0	0	0	0	0	0	22	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0	0	0	0	0
Reduced v/c Ratio	0.36	0.90	0.09	0.85	0.85	0.24	0.11	0.92	0.50	0.80	0.61	0.06

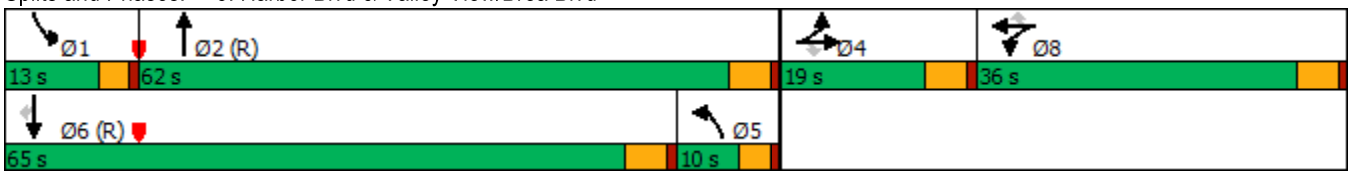
Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 16 (12%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 110			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.91			
Intersection Signal Delay: 33.2		Intersection LOS: C	
Intersection Capacity Utilization 89.7%		ICU Level of Service E	
Analysis Period (min) 15			

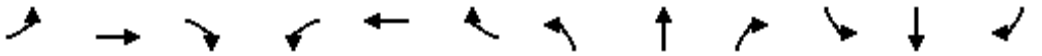
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 3: Harbor Blvd & Valley View/Brea Blvd





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	68	179	26	587	97	117	9	1447	788	102	1151	50
Future Volume (veh/h)	68	179	26	587	97	117	9	1447	788	102	1151	50
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	68	179	26	656	0	0	9	1447	0	102	1151	50
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	192	201	171	730	0		128	1677		123	1640	732
Arrive On Green	0.11	0.11	0.11	0.20	0.00	0.00	0.07	0.47	0.00	0.07	0.46	0.46
Sat Flow, veh/h	1781	1870	1585	3563	0	1585	1781	3554	1585	1781	3554	1585
Grp Volume(v), veh/h	68	179	26	656	0	0	9	1447	0	102	1151	50
Grp Sat Flow(s),veh/h/ln	1781	1870	1585	1781	0	1585	1781	1777	1585	1781	1777	1585
Q Serve(g_s), s	4.6	12.3	1.9	23.3	0.0	0.0	0.6	47.1	0.0	7.3	33.5	2.3
Cycle Q Clear(g_c), s	4.6	12.3	1.9	23.3	0.0	0.0	0.6	47.1	0.0	7.3	33.5	2.3
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	192	201	171	730	0		128	1677		123	1640	732
V/C Ratio(X)	0.35	0.89	0.15	0.90	0.00		0.07	0.86		0.83	0.70	0.07
Avail Cap(c_a), veh/h	192	201	171	850	0		128	1677		123	1640	732
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	0.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	53.8	57.2	52.6	50.4	0.0	0.0	56.3	30.6	0.0	59.7	27.9	19.5
Incr Delay (d2), s/veh	1.1	34.8	0.4	11.3	0.0	0.0	0.2	6.1	0.0	35.1	2.5	0.2
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.1	7.8	0.8	11.3	0.0	0.0	0.3	20.9	0.0	4.4	13.9	0.8
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	54.9	92.0	53.0	61.6	0.0	0.0	56.5	36.7	0.0	94.8	30.4	19.6
LnGrp LOS	D	F	D	E	A		E	D		F	C	B
Approach Vol, veh/h		273			656			1456			1303	
Approach Delay, s/veh		79.0			61.6			36.8			35.0	
Approach LOS		E			E			D			D	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	13.0	66.4		19.0	14.4	65.0		31.6				
Change Period (Y+Rc), s	4.0	5.0		5.0	5.0	* 5		5.0				
Max Green Setting (Gmax), s	9.0	57.0		14.0	6.0	* 60		31.0				
Max Q Clear Time (g_c+I1), s	9.3	49.1		14.3	2.6	35.5		25.3				
Green Ext Time (p_c), s	0.0	5.5		0.0	0.0	8.3		1.3				

Intersection Summary		
HCM 6th Ctrl Delay		43.7
HCM 6th LOS		D

Notes

User approved volume balancing among the lanes for turning movement.

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

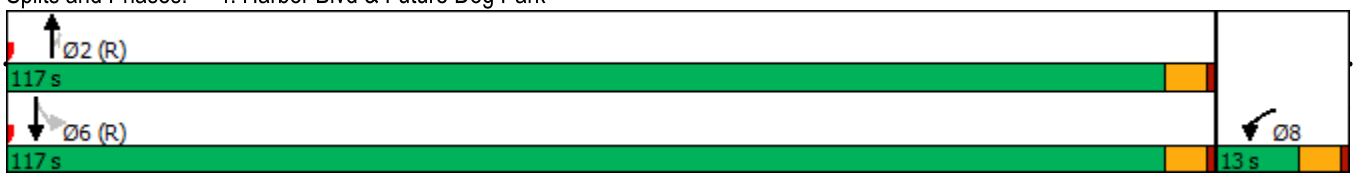
Unsignalized Delay for [NBR, WBR] is excluded from calculations of the approach delay and intersection delay.

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	5	5	1627	5	5	1230
Future Volume (vph)	5	5	1627	5	5	1230
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0		200	250	
Storage Lanes	1	0		1	1	
Taper Length (ft)	60				60	
Lane Util. Factor	1.00	1.00	0.95	1.00	1.00	0.95
Frt	0.932			0.850		
Flt Protected	0.976				0.950	
Satd. Flow (prot)	1694	0	3539	1583	1770	3539
Flt Permitted	0.976				0.145	
Satd. Flow (perm)	1694	0	3539	1583	270	3539
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)	5			5		
Link Speed (mph)	30		30			30
Link Distance (ft)	484		960			1191
Travel Time (s)	11.0		21.8			27.1
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	5	5	1627	5	5	1230
Shared Lane Traffic (%)						
Lane Group Flow (vph)	10	0	1627	5	5	1230
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60	60		60	60	
Number of Detectors	1		2	1	1	2
Detector Template	Left		Thru	Right	Left	Thru
Leading Detector (ft)	20		100	20	20	100
Trailing Detector (ft)	0		0	0	0	0
Detector 1 Position(ft)	0		0	0	0	0
Detector 1 Size(ft)	20		6	20	20	6
Detector 1 Type	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0		0.0	0.0	0.0	0.0
Detector 2 Position(ft)			94			94
Detector 2 Size(ft)			6			6
Detector 2 Type			Cl+Ex			Cl+Ex
Detector 2 Channel						
Detector 2 Extend (s)			0.0			0.0
Turn Type	Prot		NA	Perm	Perm	NA
Protected Phases	8		2			6
Permitted Phases				2	6	

Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Detector Phase	8		2	2	6	6
Switch Phase						
Minimum Initial (s)	6.0		6.0	6.0	6.0	6.0
Minimum Split (s)	11.0		11.0	11.0	11.0	11.0
Total Split (s)	13.0		117.0	117.0	117.0	117.0
Total Split (%)	10.0%		90.0%	90.0%	90.0%	90.0%
Maximum Green (s)	8.0		112.0	112.0	112.0	112.0
Yellow Time (s)	4.0		4.0	4.0	4.0	4.0
All-Red Time (s)	1.0		1.0	1.0	1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0		5.0	5.0	5.0	5.0
Lead/Lag						
Lead-Lag Optimize?						
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	None		C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)	6.4		126.5	126.5	126.5	126.5
Actuated g/C Ratio	0.05		0.97	0.97	0.97	0.97
v/c Ratio	0.11		0.47	0.00	0.02	0.36
Control Delay	46.1		2.1	0.4	0.6	1.9
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	46.1		2.1	0.4	0.6	1.9
LOS	D		A	A	A	A
Approach Delay	46.1		2.1			1.8
Approach LOS	D		A			A
Queue Length 50th (ft)	4		18	0	0	4
Queue Length 95th (ft)	23		101	m0	m0	355
Internal Link Dist (ft)	404		880			1111
Turn Bay Length (ft)				200	250	
Base Capacity (vph)	108		3444	1541	263	3444
Starvation Cap Reductn	0		0	0	0	0
Spillback Cap Reductn	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	0.09		0.47	0.00	0.02	0.36

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Green			
Natural Cycle: 40			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.47			
Intersection Signal Delay: 2.2		Intersection LOS: A	
Intersection Capacity Utilization 58.3%		ICU Level of Service B	
Analysis Period (min) 15			
m Volume for 95th percentile queue is metered by upstream signal.			

Splits and Phases: 4: Harbor Blvd & Future Dog Park



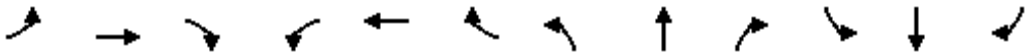


Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (veh/h)	5	5	1627	5	5	1230
Future Volume (veh/h)	5	5	1627	5	5	1230
Initial Q (Qb), veh	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00	1.00		1.00	1.00	
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach	No		No			No
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	5	5	1627	5	5	1230
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2
Cap, veh/h	12	12	3227	1439	312	3227
Arrive On Green	0.02	0.02	0.91	0.91	0.91	0.91
Sat Flow, veh/h	770	770	3647	1585	310	3647
Grp Volume(v), veh/h	11	0	1627	5	5	1230
Grp Sat Flow(s),veh/h/ln	1693	0	1777	1585	310	1777
Q Serve(g_s), s	0.8	0.0	10.1	0.0	0.4	6.3
Cycle Q Clear(g_c), s	0.8	0.0	10.1	0.0	10.5	6.3
Prop In Lane	0.45	0.45		1.00	1.00	
Lane Grp Cap(c), veh/h	26	0	3227	1439	312	3227
V/C Ratio(X)	0.43	0.00	0.50	0.00	0.02	0.38
Avail Cap(c_a), veh/h	104	0	3227	1439	312	3227
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	63.5	0.0	1.0	0.6	1.9	0.8
Incr Delay (d2), s/veh	11.0	0.0	0.6	0.0	0.1	0.3
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.4	0.0	0.9	0.0	0.0	0.6
Unsig. Movement Delay, s/veh						
LnGrp Delay(d),s/veh	74.4	0.0	1.6	0.6	2.0	1.2
LnGrp LOS	E	A	A	A	A	A
Approach Vol, veh/h	11		1632			1235
Approach Delay, s/veh	74.4		1.6			1.2
Approach LOS	E		A			A

Timer - Assigned Phs		2			6	8
Phs Duration (G+Y+Rc), s		123.0			123.0	7.0
Change Period (Y+Rc), s		5.0			5.0	5.0
Max Green Setting (Gmax), s		112.0			112.0	8.0
Max Q Clear Time (g_c+I1), s		12.1			12.5	2.8
Green Ext Time (p_c), s		24.4			14.0	0.0

Intersection Summary	
HCM 6th Ctrl Delay	1.7
HCM 6th LOS	A

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	137	29	86	31	35	62	96	2104	26	50	1713	53
Future Volume (vph)	137	29	86	31	35	62	96	2104	26	50	1713	53
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		65	95		0	100		0
Storage Lanes	0		0	0		1	1		0	1		0
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.95	0.95
Frt		0.954				0.850		0.998			0.995	
Flt Protected		0.974			0.977		0.950			0.950		
Satd. Flow (prot)	0	1731	0	0	1820	1583	1770	5075	0	1770	3522	0
Flt Permitted		0.795			0.780		0.087			0.063		
Satd. Flow (perm)	0	1413	0	0	1453	1583	162	5075	0	117	3522	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		18				19		3			6	
Link Speed (mph)		25			25			35			35	
Link Distance (ft)		427			781			235			407	
Travel Time (s)		11.6			21.3			4.6			7.9	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	137	29	86	31	35	62	96	2104	26	50	1713	53
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	252	0	0	66	62	96	2130	0	50	1766	0
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60		60	60		60	60		60	60		60
Number of Detectors	1	2		1	2	1	1	2		1	2	
Detector Template	Left	Thru		Left	Thru	Right	Left	Thru		Left	Thru	
Leading Detector (ft)	20	100		20	100	20	20	100		20	100	
Trailing Detector (ft)	0	0		0	0	0	0	0		0	0	
Detector 1 Position(ft)	0	0		0	0	0	0	0		0	0	
Detector 1 Size(ft)	20	6		20	6	20	20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		Perm	NA	Perm	Perm	NA		Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4			8		8	2			6		



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4		8	8	8	2	2		6	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0	6.0	6.0	6.0		6.0	6.0	
Minimum Split (s)	33.0	33.0		33.0	33.0	33.0	26.0	26.0		26.0	26.0	
Total Split (s)	33.0	33.0		33.0	33.0	33.0	97.0	97.0		97.0	97.0	
Total Split (%)	25.4%	25.4%		25.4%	25.4%	25.4%	74.6%	74.6%		74.6%	74.6%	
Maximum Green (s)	28.0	28.0		28.0	28.0	28.0	92.0	92.0		92.0	92.0	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	4.0	4.0		4.0	4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.0	1.0		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		5.0			5.0	5.0	5.0	5.0		5.0	5.0	
Lead/Lag												
Lead-Lag Optimize?												
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None	None	C-Max	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0	7.0	7.0	7.0		7.0	7.0	
Flash Dont Walk (s)	21.0	21.0		21.0	21.0	21.0	14.0	14.0		14.0	14.0	
Pedestrian Calls (#/hr)	5	5		5	5	5	5	5		5	5	
Act Effct Green (s)		25.1			25.1	25.1	94.9	94.9		94.9	94.9	
Actuated g/C Ratio		0.19			0.19	0.19	0.73	0.73		0.73	0.73	
v/c Ratio		0.88			0.24	0.19	0.81	0.58		0.59	0.69	
Control Delay		76.3			45.3	32.0	39.7	6.1		30.5	6.1	
Queue Delay		0.0			0.0	0.0	0.0	0.2		0.0	0.0	
Total Delay		76.3			45.3	32.0	39.7	6.3		30.5	6.1	
LOS		E			D	C	D	A		C	A	
Approach Delay		76.3			38.9			7.7			6.7	
Approach LOS		E			D			A			A	
Queue Length 50th (ft)		190			46	30	73	219		14	373	
Queue Length 95th (ft)		#322			89	70	m29	m167		m2	12	
Internal Link Dist (ft)		347			701			155			327	
Turn Bay Length (ft)						65	95			100		
Base Capacity (vph)		318			312	355	118	3704		85	2572	
Starvation Cap Reductn		0			0	0	0	673		0	14	
Spillback Cap Reductn		0			0	0	0	238		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.79			0.21	0.17	0.81	0.70		0.59	0.69	

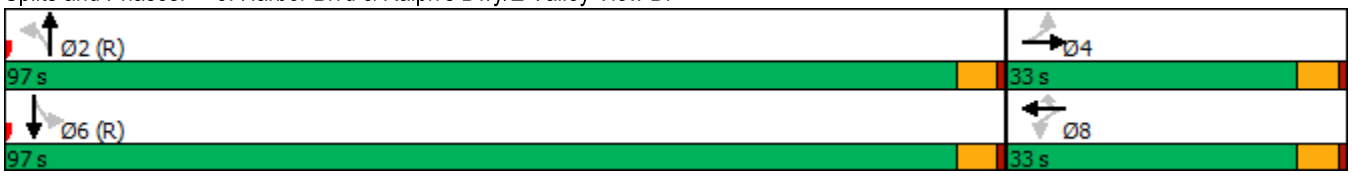
Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 115 (88%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green			
Natural Cycle: 110			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 0.88			
Intersection Signal Delay: 12.1		Intersection LOS: B	
Intersection Capacity Utilization 87.9%		ICU Level of Service E	
Analysis Period (min) 15			

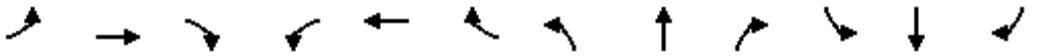
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 5: Harbor Blvd & Ralph's Dwy/E Valley View Dr





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	137	29	86	31	35	62	96	2104	26	50	1713	53
Future Volume (veh/h)	137	29	86	31	35	62	96	2104	26	50	1713	53
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	137	29	86	31	35	62	96	2104	26	50	1713	53
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	192	34	95	146	152	286	190	3861	48	163	2613	81
Arrive On Green	0.18	0.18	0.18	0.18	0.18	0.18	0.74	0.74	0.74	0.74	0.74	0.74
Sat Flow, veh/h	826	186	524	585	841	1585	271	5199	64	189	3519	109
Grp Volume(v), veh/h	252	0	0	66	0	62	96	1377	753	50	862	904
Grp Sat Flow(s),veh/h/ln	1537	0	0	1426	0	1585	271	1702	1859	189	1777	1851
Q Serve(g_s), s	16.2	0.0	0.0	0.0	0.0	4.3	36.0	22.7	22.8	20.2	31.5	31.9
Cycle Q Clear(g_c), s	20.8	0.0	0.0	4.6	0.0	4.3	67.9	22.7	22.8	42.9	31.5	31.9
Prop In Lane	0.54		0.34	0.47		1.00	1.00		0.03	1.00		0.06
Lane Grp Cap(c), veh/h	320	0	0	298	0	286	190	2528	1380	163	1320	1375
V/C Ratio(X)	0.79	0.00	0.00	0.22	0.00	0.22	0.51	0.54	0.55	0.31	0.65	0.66
Avail Cap(c_a), veh/h	372	0	0	352	0	341	190	2528	1380	163	1320	1375
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	51.9	0.0	0.0	45.4	0.0	45.4	25.5	7.2	7.2	16.5	8.4	8.4
Incr Delay (d2), s/veh	9.4	0.0	0.0	0.4	0.0	0.4	9.3	0.8	1.6	4.8	2.5	2.5
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	8.9	0.0	0.0	1.9	0.0	1.8	2.7	7.5	8.4	1.1	11.3	11.9
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	61.3	0.0	0.0	45.7	0.0	45.8	34.8	8.1	8.8	21.3	10.9	10.9
LnGrp LOS	E	A	A	D	A	D	C	A	A	C	B	B
Approach Vol, veh/h		252			128			2226			1816	
Approach Delay, s/veh		61.3			45.8			9.5			11.2	
Approach LOS		E			D			A			B	

Timer - Assigned Phs		2		4		6		8				
Phs Duration (G+Y+Rc), s		101.5		28.5		101.5		28.5				
Change Period (Y+Rc), s		5.0		5.0		5.0		5.0				
Max Green Setting (Gmax), s		92.0		28.0		92.0		28.0				
Max Q Clear Time (g_c+I1), s		69.9		22.8		44.9		6.6				
Green Ext Time (p_c), s		17.5		0.7		24.9		0.5				

Intersection Summary												
HCM 6th Ctrl Delay			14.2									
HCM 6th LOS			B									

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	28	147	81	59	168	750	87	1438	26	412	1467	21
Future Volume (vph)	28	147	81	59	168	750	87	1438	26	412	1467	21
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	130		135	200		0	205		0	300		165
Storage Lanes	1		1	1		1	1		0	2		1
Taper Length (ft)	60			60			60			60		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	0.97	0.95	1.00
Frt			0.850			0.850		0.997				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1863	1583	1770	1863	1583	1770	3529	0	3433	3539	1583
Flt Permitted	0.425			0.488			0.950			0.950		
Satd. Flow (perm)	792	1863	1583	909	1863	1583	1770	3529	0	3433	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			92			50		2				50
Link Speed (mph)		35			35			35			35	
Link Distance (ft)		647			786			659			504	
Travel Time (s)		12.6			15.3			12.8			9.8	
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	28	147	81	59	168	750	87	1438	26	412	1467	21
Shared Lane Traffic (%)												
Lane Group Flow (vph)	28	147	81	59	168	750	87	1464	0	412	1467	21
Enter Blocked Intersection	No	No	No	No	No	No	No	No	No	No	No	No
Lane Alignment	Left	Left	Right	Left	Left	Right	Left	Left	Right	Left	Left	Right
Median Width(ft)		12			12			24			24	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane		Yes										
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	60		60	60		60	60		60	60		60
Number of Detectors	1	2	1	1	2	1	1	2		1	2	1
Detector Template	Left	Thru	Right	Left	Thru	Right	Left	Thru		Left	Thru	Right
Leading Detector (ft)	20	100	20	20	100	20	20	100		20	100	20
Trailing Detector (ft)	0	0	0	0	0	0	0	0		0	0	0
Detector 1 Position(ft)	0	0	0	0	0	0	0	0		0	0	0
Detector 1 Size(ft)	20	6	20	20	6	20	20	6		20	6	20
Detector 1 Type	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Queue (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 1 Delay (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA	Perm	Perm	NA	pm+ov	Prot	NA		Prot	NA	Perm
Protected Phases		4			8	1	5	2		1	6	
Permitted Phases	4		4	8		8						6

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4	4	8	8	1	5	2		1	6	6
Switch Phase												
Minimum Initial (s)	6.0	6.0	6.0	6.0	6.0	6.0	6.0	6.0		6.0	6.0	6.0
Minimum Split (s)	30.0	30.0	30.0	36.0	36.0	10.0	10.0	30.0		10.0	30.0	30.0
Total Split (s)	36.0	36.0	36.0	36.0	36.0	35.0	16.0	59.0		35.0	78.0	78.0
Total Split (%)	27.7%	27.7%	27.7%	27.7%	27.7%	26.9%	12.3%	45.4%		26.9%	60.0%	60.0%
Maximum Green (s)	31.0	31.0	31.0	31.0	31.0	31.0	12.0	54.0		31.0	73.0	73.0
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	3.0	3.0	4.0		3.0	4.0	4.0
All-Red Time (s)	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0		1.0	1.0	1.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	5.0	5.0	5.0	5.0	5.0	4.0	4.0	5.0		4.0	5.0	5.0
Lead/Lag						Lag	Lead	Lead		Lag	Lag	Lag
Lead-Lag Optimize?						Yes	Yes	Yes		Yes	Yes	Yes
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Recall Mode	None	None	None	None	None	None	None	C-Max		None	C-Max	C-Max
Walk Time (s)	7.0	7.0	7.0	7.0	7.0			7.0			7.0	7.0
Flash Dont Walk (s)	18.0	18.0	18.0	24.0	24.0			18.0			18.0	18.0
Pedestrian Calls (#/hr)	5	5	5	5	5			5			5	5
Act Effct Green (s)	18.6	18.6	18.6	18.6	18.6	54.6	10.9	66.4		31.0	86.5	86.5
Actuated g/C Ratio	0.14	0.14	0.14	0.14	0.14	0.42	0.08	0.51		0.24	0.67	0.67
v/c Ratio	0.25	0.55	0.27	0.46	0.63	1.08	0.59	0.81		0.50	0.62	0.02
Control Delay	51.6	58.3	8.6	59.9	61.9	92.1	73.4	32.1		36.1	8.0	0.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.1	0.0
Total Delay	51.6	58.3	8.6	59.9	61.9	92.1	73.4	32.1		36.1	8.1	0.3
LOS	D	E	A	E	E	F	E	C		D	A	A
Approach Delay		41.8			84.9			34.4			14.1	
Approach LOS		D			F			C			B	
Queue Length 50th (ft)	22	119	0	47	137	~698	71	511		134	85	0
Queue Length 95th (ft)	47	167	34	83	188	#799	128	#814		m208	425	m0
Internal Link Dist (ft)		567			706			579			424	
Turn Bay Length (ft)	130		135	200			205			300		165
Base Capacity (vph)	188	444	447	216	444	693	167	1803		818	2355	1070
Starvation Cap Reductn	0	0	0	0	0	0	0	0		0	190	0
Spillback Cap Reductn	0	0	0	0	0	0	0	0		0	0	0
Storage Cap Reductn	0	0	0	0	0	0	0	0		0	0	0
Reduced v/c Ratio	0.15	0.33	0.18	0.27	0.38	1.08	0.52	0.81		0.50	0.68	0.02

Intersection Summary			
Area Type:	Other		
Cycle Length: 130			
Actuated Cycle Length: 130			
Offset: 14 (11%), Referenced to phase 2:NBT and 6:SBT, Start of Green			
Natural Cycle: 100			
Control Type: Actuated-Coordinated			
Maximum v/c Ratio: 1.08			
Intersection Signal Delay: 37.1		Intersection LOS: D	
Intersection Capacity Utilization 103.7%		ICU Level of Service G	
Analysis Period (min) 15			

~ Volume exceeds capacity, queue is theoretically infinite.

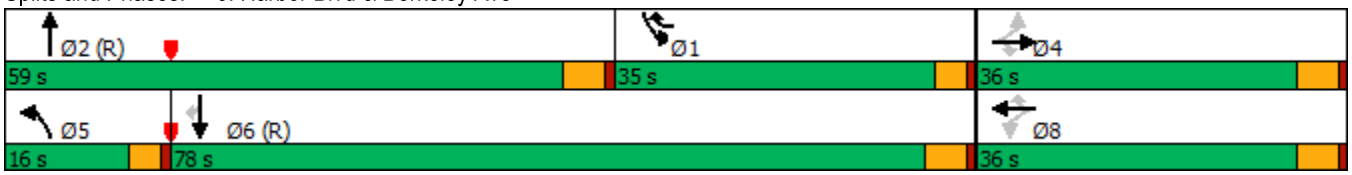
Queue shown is maximum after two cycles.

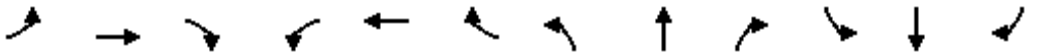
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 6: Harbor Blvd & Berkeley Ave





Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	28	147	81	59	168	750	87	1438	26	412	1467	21
Future Volume (veh/h)	28	147	81	59	168	750	87	1438	26	412	1467	21
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870	1870
Adj Flow Rate, veh/h	28	147	81	59	168	750	87	1438	26	412	1467	21
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Percent Heavy Veh, %	2	2	2	2	2	2	2	2	2	2	2	2
Cap, veh/h	254	446	378	271	446	744	109	1483	27	797	2106	939
Arrive On Green	0.24	0.24	0.24	0.24	0.24	0.24	0.06	0.42	0.42	0.23	0.59	0.59
Sat Flow, veh/h	1217	1870	1585	1241	1870	1585	1781	3571	65	3456	3554	1585
Grp Volume(v), veh/h	28	147	81	59	168	750	87	715	749	412	1467	21
Grp Sat Flow(s),veh/h/ln	1217	1870	1585	1241	1870	1585	1781	1777	1859	1728	1777	1585
Q Serve(g_s), s	2.6	8.4	5.3	5.4	9.8	31.0	6.3	51.2	51.3	13.5	37.2	0.7
Cycle Q Clear(g_c), s	12.3	8.4	5.3	13.8	9.8	31.0	6.3	51.2	51.3	13.5	37.2	0.7
Prop In Lane	1.00		1.00	1.00		1.00	1.00		0.03	1.00		1.00
Lane Grp Cap(c), veh/h	254	446	378	271	446	744	109	738	772	797	2106	939
V/C Ratio(X)	0.11	0.33	0.21	0.22	0.38	1.01	0.80	0.97	0.97	0.52	0.70	0.02
Avail Cap(c_a), veh/h	254	446	378	271	446	744	164	738	772	824	2106	939
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Uniform Delay (d), s/veh	46.6	40.9	39.7	46.6	41.4	34.5	60.2	37.2	37.2	43.7	18.4	10.9
Incr Delay (d2), s/veh	0.2	0.4	0.3	0.4	0.5	35.1	14.7	26.3	25.9	0.5	1.9	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.8	3.9	2.1	1.7	4.6	16.5	3.3	26.9	28.1	5.8	15.1	0.3
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	46.8	41.3	40.0	47.0	41.9	69.6	74.9	63.5	63.1	44.2	20.3	11.0
LnGrp LOS	D	D	D	D	D	F	E	E	E	D	C	B
Approach Vol, veh/h		256			977			1551			1900	
Approach Delay, s/veh		41.5			63.5			63.9			25.4	
Approach LOS		D			E			E			C	

Timer - Assigned Phs	1	2		4	5	6		8				
Phs Duration (G+Y+Rc), s	35.0	59.0		36.0	12.0	82.0		36.0				
Change Period (Y+Rc), s	5.0	* 5		5.0	4.0	5.0		5.0				
Max Green Setting (Gmax), s	31.0	* 54		31.0	12.0	73.0		31.0				
Max Q Clear Time (g_c+I1), s	15.5	53.3		14.3	8.3	39.2		33.0				
Green Ext Time (p_c), s	1.3	0.6		1.0	0.1	14.5		0.0				

Intersection Summary			
HCM 6th Ctrl Delay		47.0	
HCM 6th LOS		D	

Notes

* HCM 6th computational engine requires equal clearance times for the phases crossing the barrier.

APPENDIX C

SIMTRAFFIC QUEUEING WORKSHEETS

APPENDIX C-I

EXISTING TRAFFIC CONDITIONS

Queuing and Blocking Report

Intersection: 1: Harbor Blvd & Bastanchury Rd

Movement	EB	EB	EB	EB	WB	WB	WB	WB	WB	WB	NB	NB
Directions Served	L	T	T	TR	L	L	T	T	T	R	L	L
Maximum Queue (ft)	385	868	830	416	190	252	435	403	261	203	129	142
Average Queue (ft)	338	543	469	197	76	166	246	218	155	51	60	88
95th Queue (ft)	453	1033	950	324	157	281	358	318	239	126	110	134
Link Distance (ft)		976	976	976			902	902	902			
Upstream Blk Time (%)		12	0									
Queuing Penalty (veh)		0	0									
Storage Bay Dist (ft)	325				193	193				145	230	230
Storage Blk Time (%)	50	1			0	1	21		6	0		
Queuing Penalty (veh)	142	2			0	2	54		12	0		

Intersection: 1: Harbor Blvd & Bastanchury Rd

Movement	NB	NB	NB	NB	SB	SB	SB	SB	SB
Directions Served	T	T	T	R	L	L	T	T	TR
Maximum Queue (ft)	113	124	139	88	294	332	490	468	506
Average Queue (ft)	45	65	72	30	192	237	262	251	298
95th Queue (ft)	92	109	125	68	284	333	397	380	451
Link Distance (ft)	722	722	722				854	854	854
Upstream Blk Time (%)									
Queuing Penalty (veh)									
Storage Bay Dist (ft)				150	275	275			
Storage Blk Time (%)			0		1	6	4		
Queuing Penalty (veh)			0		2	18	14		

Queuing and Blocking Report

Intersection: 2: Harbor Blvd & Valencia Mesa

Movement	EB	EB	EB	WB	WB	NB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	R	L	TR	L	T	T	TR	L	T	T
Maximum Queue (ft)	114	188	126	119	150	201	198	116	145	245	169	167
Average Queue (ft)	64	57	28	47	66	115	36	35	58	125	34	52
95th Queue (ft)	108	136	74	100	128	192	107	86	119	212	103	123
Link Distance (ft)		743		640	640		3866	3866	3866		722	722
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	55		85			175				230		
Storage Blk Time (%)	32	8	0			4	0			0		
Queuing Penalty (veh)	42	14	0			11	0			2		

Intersection: 2: Harbor Blvd & Valencia Mesa

Movement	SB
Directions Served	TR
Maximum Queue (ft)	202
Average Queue (ft)	82
95th Queue (ft)	162
Link Distance (ft)	722
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 3: Harbor Blvd & Valley View/Brea Blvd

Movement	EB	EB	EB	WB	WB	NB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	R	L	LT	L	T	T	T	L	T	T
Maximum Queue (ft)	127	163	42	337	329	66	151	122	116	147	272	293
Average Queue (ft)	39	72	8	229	203	7	59	52	41	46	88	104
95th Queue (ft)	89	137	30	317	302	35	118	106	92	108	195	219
Link Distance (ft)		793	793	974	974		178	178	178		3866	3866
Upstream Blk Time (%)							0		0			
Queuing Penalty (veh)							0		0			
Storage Bay Dist (ft)	85					95				150		
Storage Blk Time (%)	1	12					3			0	3	
Queuing Penalty (veh)	1	6					1			0	1	

Intersection: 3: Harbor Blvd & Valley View/Brea Blvd

Movement	SB
Directions Served	TR
Maximum Queue (ft)	203
Average Queue (ft)	24
95th Queue (ft)	114
Link Distance (ft)	3866
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 5: Harbor Blvd & Ralph's Dwy/E. Valley View Dr

Movement	EB	WB	WB	NB	NB	NB	NB	SB	SB	SB
Directions Served	LTR	LT	R	L	T	T	TR	L	T	TR
Maximum Queue (ft)	80	77	58	64	73	71	108	139	210	221
Average Queue (ft)	30	25	24	27	15	25	38	25	56	56
95th Queue (ft)	66	62	49	59	49	62	93	76	153	146
Link Distance (ft)	343	584			128	128	128		343	343
Upstream Blk Time (%)							0			
Queuing Penalty (veh)							1			
Storage Bay Dist (ft)			65	95				100		
Storage Blk Time (%)		1	0	0	0			0	2	
Queuing Penalty (veh)		0	0	0	0			1	1	

Queuing and Blocking Report

Intersection: 6: Harbor Blvd & Berkeley Ave

Movement	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	R	L	T	R	L	T	TR	L	L	T
Maximum Queue (ft)	89	221	149	136	231	284	232	336	397	137	199	334
Average Queue (ft)	14	103	30	41	108	130	88	186	212	67	90	156
95th Queue (ft)	51	191	97	96	184	234	173	287	328	126	153	284
Link Distance (ft)		460			660	660		600	600			458
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	130		135	200			205			300	300	
Storage Blk Time (%)		9			1		0	4				0
Queuing Penalty (veh)		8			0		0	4				1

Intersection: 6: Harbor Blvd & Berkeley Ave

Movement	SB	SB
Directions Served	T	R
Maximum Queue (ft)	358	108
Average Queue (ft)	171	8
95th Queue (ft)	303	54
Link Distance (ft)	458	
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		165
Storage Blk Time (%)	7	
Queuing Penalty (veh)	1	

Network Summary

Network wide Queuing Penalty: 344

Queuing and Blocking Report

Intersection: 1: Harbor Blvd & Bastanchury Rd

Movement	EB	EB	EB	EB	WB	WB	WB	WB	WB	WB	NB	NB
Directions Served	L	T	T	TR	L	L	T	T	T	R	L	L
Maximum Queue (ft)	383	519	477	292	118	252	363	327	227	201	149	226
Average Queue (ft)	281	288	246	190	35	107	235	200	129	80	55	88
95th Queue (ft)	426	461	385	267	88	226	324	290	226	151	111	156
Link Distance (ft)		976	976	976			902	902	902			
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	325				193	193				145	230	230
Storage Blk Time (%)	13	1				0	18		3	1		
Queuing Penalty (veh)	46	3				0	24		7	2		

Intersection: 1: Harbor Blvd & Bastanchury Rd

Movement	NB	NB	NB	NB	SB	SB	SB	SB	SB
Directions Served	T	T	T	R	L	L	T	T	TR
Maximum Queue (ft)	269	288	316	210	304	330	553	525	446
Average Queue (ft)	154	175	185	100	226	262	321	278	249
95th Queue (ft)	236	259	285	213	338	364	662	584	405
Link Distance (ft)	722	722	722				854	854	854
Upstream Blk Time (%)							5	0	
Queuing Penalty (veh)							0	0	
Storage Bay Dist (ft)				150	275	275			
Storage Blk Time (%)	1		17	0	9	23	2		
Queuing Penalty (veh)	1		35	1	23	61	7		

Queuing and Blocking Report

Intersection: 2: Harbor Blvd & Valencia Mesa

Movement	EB	EB	EB	WB	WB	NB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	R	L	TR	L	T	T	TR	L	T	T
Maximum Queue (ft)	113	170	124	133	274	104	147	180	181	136	78	95
Average Queue (ft)	69	38	38	58	125	38	25	29	42	66	26	49
95th Queue (ft)	114	120	88	117	226	84	89	124	126	125	64	87
Link Distance (ft)		743		640	640		3866	3866	3866		722	722
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	55		85			175				230		
Storage Blk Time (%)	34	1	0				0					
Queuing Penalty (veh)	53	2	0				0					

Intersection: 2: Harbor Blvd & Valencia Mesa

Movement	SB
Directions Served	TR
Maximum Queue (ft)	121
Average Queue (ft)	64
95th Queue (ft)	106
Link Distance (ft)	722
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Queuing and Blocking Report

Intersection: 3: Harbor Blvd & Valley View/Brea Blvd

Movement	EB	EB	EB	WB	WB	NB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	R	L	LT	L	T	T	T	L	T	T
Maximum Queue (ft)	144	316	59	325	310	65	184	188	160	184	241	319
Average Queue (ft)	68	162	19	211	183	3	82	82	64	87	100	114
95th Queue (ft)	147	280	50	300	274	32	150	151	128	156	192	236
Link Distance (ft)		793	793	974	974		178	178	178		3866	3866
Upstream Blk Time (%)							0	0	0			
Queuing Penalty (veh)							1	1	0			
Storage Bay Dist (ft)	85					95				150		
Storage Blk Time (%)	5	37					5			3	2	
Queuing Penalty (veh)	9	25					0			11	2	

Intersection: 3: Harbor Blvd & Valley View/Brea Blvd

Movement	SB
Directions Served	TR
Maximum Queue (ft)	143
Average Queue (ft)	19
95th Queue (ft)	112
Link Distance (ft)	3866
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 5: Harbor Blvd & Ralph's Dwy/E. Valley View Dr

Movement	EB	WB	WB	NB	NB	NB	NB	B19	SB	SB	SB
Directions Served	LTR	LT	R	L	T	T	TR	T	L	T	TR
Maximum Queue (ft)	189	72	60	82	120	119	173	34	102	95	92
Average Queue (ft)	85	24	21	35	32	46	82	1	33	22	20
95th Queue (ft)	154	58	51	70	83	100	152	17	75	64	63
Link Distance (ft)	343	584			128	128	128	458		343	343
Upstream Blk Time (%)					0	0	2				
Queuing Penalty (veh)					0	0	9				
Storage Bay Dist (ft)			65	95					100		
Storage Blk Time (%)		1	0	0	0				1	0	
Queuing Penalty (veh)		0	0	2	0				5	0	

Queuing and Blocking Report

Intersection: 6: Harbor Blvd & Berkeley Ave

Movement	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	R	L	T	R	L	T	TR	L	L	T
Maximum Queue (ft)	139	213	146	104	210	349	264	464	540	176	199	221
Average Queue (ft)	23	91	30	42	115	187	89	265	309	81	104	93
95th Queue (ft)	75	172	86	84	190	310	204	395	460	147	168	178
Link Distance (ft)		460			660	660		600	600			458
Upstream Blk Time (%)								0	0			
Queuing Penalty (veh)								0	0			
Storage Bay Dist (ft)	130		135	200			205			300	300	
Storage Blk Time (%)	0	5			1		0	15				
Queuing Penalty (veh)	0	6			0		0	13				

Intersection: 6: Harbor Blvd & Berkeley Ave

Movement	SB	SB
Directions Served	T	R
Maximum Queue (ft)	237	24
Average Queue (ft)	103	2
95th Queue (ft)	190	14
Link Distance (ft)	458	
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		165
Storage Blk Time (%)	1	
Queuing Penalty (veh)	0	

Network Summary

Network wide Queuing Penalty: 351

APPENDIX C-II

EXISTING PLUS PROJECT TRAFFIC CONDITIONS

Intersection: 1: Harbor Blvd & Bastanchury Rd

Movement	EB	EB	EB	EB	WB	WB	WB	WB	WB	WB	NB	NB
Directions Served	L	T	T	TR	L	L	T	T	T	R	L	L
Maximum Queue (ft)	385	748	669	354	188	253	384	348	258	166	152	157
Average Queue (ft)	331	438	357	182	79	173	250	217	153	45	68	92
95th Queue (ft)	441	859	748	293	166	287	355	307	234	111	132	145
Link Distance (ft)		976	976	976			902	902	902			
Upstream Blk Time (%)		1										
Queuing Penalty (veh)		0										
Storage Bay Dist (ft)	325				193	193				145	230	230
Storage Blk Time (%)	38	0			0	1	21		4			
Queuing Penalty (veh)	106	1			0	4	55		9			

Intersection: 1: Harbor Blvd & Bastanchury Rd

Movement	NB	NB	NB	NB	SB	SB	SB	SB	SB
Directions Served	T	T	T	R	L	L	T	T	TR
Maximum Queue (ft)	173	175	163	112	298	334	604	580	530
Average Queue (ft)	93	104	98	38	213	256	347	307	252
95th Queue (ft)	150	155	156	80	326	364	666	589	432
Link Distance (ft)	333	333	333				854	854	854
Upstream Blk Time (%)							4	0	
Queuing Penalty (veh)							0	0	
Storage Bay Dist (ft)				150	275	275			
Storage Blk Time (%)			1		7	17	5		
Queuing Penalty (veh)			1		21	54	18		

Intersection: 2: Harbor Blvd & Valencia Mesa

Movement	EB	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	SB
Directions Served	L	T	R	L	TR	L	T	TR	L	T	T	R
Maximum Queue (ft)	109	126	95	103	162	212	241	240	244	210	192	86
Average Queue (ft)	59	45	34	38	67	120	112	131	129	66	76	22
95th Queue (ft)	104	104	74	83	128	198	207	219	214	154	151	55
Link Distance (ft)		749		645	645		542	542		334	334	334
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	55		85			185			230			
Storage Blk Time (%)	18	5	1			4	1		0	0		
Queuing Penalty (veh)	24	9	1			14	2		2	0		

Intersection: 2: Harbor Blvd & Valencia Mesa

Movement	B13	B13	B13
Directions Served	T	T	T
Maximum Queue (ft)	71	10	6
Average Queue (ft)	3	0	0
95th Queue (ft)	50	7	4
Link Distance (ft)	333	333	333
Upstream Blk Time (%)	0		
Queuing Penalty (veh)	0		
Storage Bay Dist (ft)			
Storage Blk Time (%)			
Queuing Penalty (veh)			

Intersection: 3: Harbor Blvd & Valley View/Brea Blvd

Movement	EB	EB	EB	WB	WB	WB	NB	NB	NB	B12	B12	SB
Directions Served	L	T	R	L	LT	R	L	T	T	T	T	L
Maximum Queue (ft)	100	157	38	349	334	34	66	220	233	3	10	160
Average Queue (ft)	38	74	8	226	200	3	7	105	104	0	0	53
95th Queue (ft)	83	134	31	313	291	39	34	195	198	2	7	119
Link Distance (ft)		793	793	984	984	984		178	178	340	340	
Upstream Blk Time (%)								1	2			
Queuing Penalty (veh)								6	7			
Storage Bay Dist (ft)	85						95					140
Storage Blk Time (%)	2	12						12				0
Queuing Penalty (veh)	1	6						3				0

Intersection: 3: Harbor Blvd & Valley View/Brea Blvd

Movement	SB	SB	SB	B14	B14
Directions Served	T	T	R	T	T
Maximum Queue (ft)	312	326	200	18	15
Average Queue (ft)	171	176	52	1	1
95th Queue (ft)	282	284	164	17	18
Link Distance (ft)	290	290		772	772
Upstream Blk Time (%)	2	1			
Queuing Penalty (veh)	10	8			
Storage Bay Dist (ft)			140		
Storage Blk Time (%)	20	23	0		
Queuing Penalty (veh)	12	18	0		

Intersection: 4: Harbor Blvd & Future Dog Park

Movement	WB	NB	NB	B16	B14	SB	SB	SB	B15
Directions Served	LR	T	T	T	T	L	T	T	T
Maximum Queue (ft)	34	21	22	10	4	31	108	116	7
Average Queue (ft)	7	1	1	0	0	2	9	11	0
95th Queue (ft)	25	10	10	7	2	13	50	60	5
Link Distance (ft)	423	896	896	772	290		1143	1143	542
Upstream Blk Time (%)									
Queuing Penalty (veh)									
Storage Bay Dist (ft)						250			
Storage Blk Time (%)									
Queuing Penalty (veh)									

Intersection: 5: Harbor Blvd & Ralph's Dwy/E Valley View Dr

Movement	EB	WB	WB	NB	NB	NB	NB	SB	SB	SB
Directions Served	LTR	LT	R	L	T	T	TR	L	T	TR
Maximum Queue (ft)	94	81	86	64	92	107	90	80	215	215
Average Queue (ft)	35	23	27	23	22	35	23	26	51	51
95th Queue (ft)	77	60	59	52	69	88	68	69	154	152
Link Distance (ft)	380	720			173	173	173		340	340
Upstream Blk Time (%)										
Queuing Penalty (veh)										
Storage Bay Dist (ft)			65	95				100		
Storage Blk Time (%)		1	1	0	0			0	2	
Queuing Penalty (veh)		1	0	0	0			0	1	

Intersection: 6: Harbor Blvd & Berkeley Ave

Movement	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	R	L	T	R	L	T	TR	L	L	T
Maximum Queue (ft)	75	235	158	116	213	307	203	310	332	149	203	338
Average Queue (ft)	16	116	33	45	107	131	92	198	201	70	99	157
95th Queue (ft)	51	205	98	96	187	241	172	292	297	129	161	292
Link Distance (ft)		572			726	726		605	605			425
Upstream Blk Time (%)												0
Queuing Penalty (veh)												0
Storage Bay Dist (ft)	130		135	200			205			300	300	
Storage Blk Time (%)		9			1		0	6				0
Queuing Penalty (veh)		8			0		0	6				1

Intersection: 6: Harbor Blvd & Berkeley Ave

Movement	SB	SB
Directions Served	T	R
Maximum Queue (ft)	373	146
Average Queue (ft)	178	9
95th Queue (ft)	321	69
Link Distance (ft)	425	
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		165
Storage Blk Time (%)	7	
Queuing Penalty (veh)	1	

Network Summary

Network wide Queuing Penalty: 412

Queuing and Blocking Report

Intersection: 1: Harbor Blvd & Bastanchury Rd

Movement	EB	EB	EB	EB	WB	WB	WB	WB	WB	WB	NB	NB
Directions Served	L	T	T	TR	L	L	T	T	T	R	L	L
Maximum Queue (ft)	364	462	404	296	130	252	362	327	243	167	137	282
Average Queue (ft)	248	264	232	189	41	107	232	201	133	77	67	117
95th Queue (ft)	377	401	334	263	105	214	318	279	221	150	114	216
Link Distance (ft)		976	976	976			902	902	902			
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	325				193	193				145	230	230
Storage Blk Time (%)	4	1				0	18		2	1		0
Queuing Penalty (veh)	16	2				0	23		6	4		0

Intersection: 1: Harbor Blvd & Bastanchury Rd

Movement	NB	NB	NB	NB	B13	SB	SB	SB	SB	SB
Directions Served	T	T	T	R	T	L	L	T	T	TR
Maximum Queue (ft)	346	358	375	210	9	280	323	525	490	388
Average Queue (ft)	209	228	241	140	0	197	233	280	238	210
95th Queue (ft)	305	325	355	264	8	299	334	519	456	351
Link Distance (ft)	333	333	333		334			854	854	854
Upstream Blk Time (%)	0	0	1					0		
Queuing Penalty (veh)	1	2	7					0		
Storage Bay Dist (ft)				150		275	275			
Storage Blk Time (%)	5		32	0		3	12	3		
Queuing Penalty (veh)	9		64	0		9	32	10		

Queuing and Blocking Report

Intersection: 2: Harbor Blvd & Valencia Mesa

Movement	EB	EB	EB	WB	WB	NB	NB	NB	B15	SB	SB	SB
Directions Served	L	T	R	L	TR	L	T	TR	T	L	T	T
Maximum Queue (ft)	114	219	136	124	279	192	317	334	11	170	112	130
Average Queue (ft)	70	46	51	54	123	51	179	200	0	75	46	58
95th Queue (ft)	114	150	104	107	227	139	308	327	8	143	102	108
Link Distance (ft)		749		645	645		542	542	1143		334	334
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	55		85			185				230		
Storage Blk Time (%)	28	1	2				12			0		
Queuing Penalty (veh)	42	4	3				6			0		

Intersection: 2: Harbor Blvd & Valencia Mesa

Movement	SB	B13	B13	B13
Directions Served	R	T	T	T
Maximum Queue (ft)	54	4	8	9
Average Queue (ft)	13	0	0	0
95th Queue (ft)	40	4	6	6
Link Distance (ft)	334	333	333	333
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Queuing and Blocking Report

Intersection: 3: Harbor Blvd & Valley View/Brea Blvd

Movement	EB	EB	EB	WB	WB	WB	NB	NB	NB	B12	B12	SB
Directions Served	L	T	R	L	LT	R	L	T	T	T	T	L
Maximum Queue (ft)	145	282	54	311	284	28	72	244	254	86	98	200
Average Queue (ft)	59	140	19	206	177	2	3	124	135	5	5	105
95th Queue (ft)	133	249	47	289	264	25	33	226	244	40	43	196
Link Distance (ft)		793	793	984	984	984		178	178	340	340	
Upstream Blk Time (%)								2	3			
Queuing Penalty (veh)								14	18			
Storage Bay Dist (ft)	85						95					140
Storage Blk Time (%)	4	33						14				5
Queuing Penalty (veh)	7	23						1				23

Intersection: 3: Harbor Blvd & Valley View/Brea Blvd

Movement	SB	SB	SB	B14	B14
Directions Served	T	T	R	T	T
Maximum Queue (ft)	361	358	200	58	45
Average Queue (ft)	234	238	58	3	2
95th Queue (ft)	350	349	190	27	23
Link Distance (ft)	290	290		772	772
Upstream Blk Time (%)	5	6			
Queuing Penalty (veh)	27	32			
Storage Bay Dist (ft)			140		
Storage Blk Time (%)	31	32			
Queuing Penalty (veh)	30	16			

Intersection: 4: Harbor Blvd & Future Dog Park

Movement	WB	NB	NB	NB	SB	SB	SB
Directions Served	LR	T	T	R	L	T	T
Maximum Queue (ft)	30	73	80	12	19	39	58
Average Queue (ft)	6	5	6	0	1	2	5
95th Queue (ft)	22	33	36	6	11	16	32
Link Distance (ft)	423	896	896			1143	1143
Upstream Blk Time (%)							
Queuing Penalty (veh)							
Storage Bay Dist (ft)				200	250		
Storage Blk Time (%)							
Queuing Penalty (veh)							

Queuing and Blocking Report

Intersection: 5: Harbor Blvd & Ralph's Dwy/E Valley View Dr

Movement	EB	WB	WB	NB	NB	NB	NB	B25	SB	SB	SB	B12
Directions Served	LTR	LT	R	L	T	T	TR	T	L	T	TR	T
Maximum Queue (ft)	189	68	64	97	114	133	137	14	75	124	126	8
Average Queue (ft)	85	21	24	33	29	48	42	0	29	34	31	0
95th Queue (ft)	153	55	53	72	86	107	105	7	62	91	87	6
Link Distance (ft)	380	720			173	173	173	425		340	340	178
Upstream Blk Time (%)					0	0	0					
Queuing Penalty (veh)					0	0	1					
Storage Bay Dist (ft)			65	95					100			
Storage Blk Time (%)		1	1	1	0				0	1		
Queuing Penalty (veh)		0	0	3	0				0	0		

Intersection: 6: Harbor Blvd & Berkeley Ave

Movement	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	R	L	T	R	L	T	TR	L	L	T
Maximum Queue (ft)	100	181	148	153	221	346	264	508	499	156	186	250
Average Queue (ft)	26	93	34	47	108	168	96	271	296	79	105	103
95th Queue (ft)	68	167	94	105	192	284	219	413	440	137	160	194
Link Distance (ft)		572			726	726		605	605			425
Upstream Blk Time (%)								0	0			
Queuing Penalty (veh)								0	0			
Storage Bay Dist (ft)	130		135	200			205			300	300	
Storage Blk Time (%)		5			1		0	16				0
Queuing Penalty (veh)		6			0		1	14				0

Intersection: 6: Harbor Blvd & Berkeley Ave

Movement	SB	SB
Directions Served	T	R
Maximum Queue (ft)	248	35
Average Queue (ft)	118	4
95th Queue (ft)	200	20
Link Distance (ft)	425	
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		165
Storage Blk Time (%)	3	
Queuing Penalty (veh)	1	

Network Summary

Network wide Queuing Penalty: 459

APPENDIX C-III

YEAR 2045 BUILDOUT TRAFFIC CONDITIONS

Intersection: 1: Harbor Blvd & Bastanchury Rd

Movement	EB	EB	EB	EB	WB	WB	WB	WB	WB	WB	NB	NB
Directions Served	L	T	T	TR	L	L	T	T	T	R	L	L
Maximum Queue (ft)	385	1026	991	988	181	252	382	333	254	203	164	168
Average Queue (ft)	370	812	758	484	82	174	262	223	165	60	76	103
95th Queue (ft)	433	1261	1212	939	162	287	362	309	250	147	140	162
Link Distance (ft)		976	976	976			902	902	902			
Upstream Blk Time (%)		44	6	0								
Queuing Penalty (veh)		0	0	0								
Storage Bay Dist (ft)	325				193	193				145	230	230
Storage Blk Time (%)	73	4			0	1	23		7	0		
Queuing Penalty (veh)	296	12			0	2	64		16	1		

Intersection: 1: Harbor Blvd & Bastanchury Rd

Movement	NB	NB	NB	NB	SB	SB	SB	SB	SB
Directions Served	T	T	T	R	L	L	T	T	TR
Maximum Queue (ft)	167	196	192	151	305	335	903	885	882
Average Queue (ft)	81	106	110	51	273	328	821	777	717
95th Queue (ft)	138	163	169	109	359	370	1059	1046	1040
Link Distance (ft)	722	722	722				854	854	854
Upstream Blk Time (%)							55	22	18
Queuing Penalty (veh)							0	0	0
Storage Bay Dist (ft)				150	275	275			
Storage Blk Time (%)			2	0	21	47	36		
Queuing Penalty (veh)			2	0	107	236	161		

Intersection: 2: Harbor Blvd & Valencia Mesa

Movement	EB	EB	EB	WB	WB	NB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	R	L	TR	L	T	T	TR	L	T	T
Maximum Queue (ft)	113	199	106	119	161	188	154	181	235	276	222	188
Average Queue (ft)	66	54	27	54	66	105	66	83	125	156	93	106
95th Queue (ft)	112	135	67	108	121	177	130	150	206	260	176	173
Link Distance (ft)		743		640	640		3866	3866	3866		722	722
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	55		85			175				230		
Storage Blk Time (%)	34	5	0			1	0			2	0	
Queuing Penalty (veh)	45	10	0			2	0			8	0	

Intersection: 2: Harbor Blvd & Valencia Mesa

Movement	SB
Directions Served	TR
Maximum Queue (ft)	210
Average Queue (ft)	124
95th Queue (ft)	192
Link Distance (ft)	722
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 3: Harbor Blvd & Valley View/Brea Blvd

Movement	EB	EB	EB	WB	WB	NB	NB	NB	NB	B12	SB	SB
Directions Served	L	T	R	L	LT	L	T	T	T	T	L	T
Maximum Queue (ft)	133	187	34	542	503	48	202	152	146	7	210	491
Average Queue (ft)	46	82	8	347	311	5	77	67	51	0	67	217
95th Queue (ft)	109	156	31	478	451	25	158	140	121	5	157	377
Link Distance (ft)		793	793	974	974		178	178	178	343		3866
Upstream Blk Time (%)							0	0	0			
Queuing Penalty (veh)							1	0	0			
Storage Bay Dist (ft)	85					95					150	
Storage Blk Time (%)	3	19					5				0	11
Queuing Penalty (veh)	3	10					1				2	7

Intersection: 3: Harbor Blvd & Valley View/Brea Blvd

Movement	SB	SB
Directions Served	T	TR
Maximum Queue (ft)	608	439
Average Queue (ft)	237	90
95th Queue (ft)	418	279
Link Distance (ft)	3866	3866
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 5: Harbor Blvd & Ralph's Dwy/E. Valley View Dr

Movement	EB	WB	WB	NB	NB	NB	NB	B19	B19	SB	SB	SB
Directions Served	LTR	LT	R	L	T	T	TR	T	T	L	T	TR
Maximum Queue (ft)	182	94	98	128	212	172	180	306	303	104	288	290
Average Queue (ft)	77	35	28	82	112	82	84	103	100	36	63	52
95th Queue (ft)	147	76	65	144	246	163	168	372	371	75	181	171
Link Distance (ft)	343	584			128	128	128	458	458		343	343
Upstream Blk Time (%)				16	27	2	2	2	2		0	0
Queuing Penalty (veh)				0	144	10	11	16	14		2	2
Storage Bay Dist (ft)			65	95						100		
Storage Blk Time (%)		4	1	32	8					0	3	
Queuing Penalty (veh)		2	0	154	6					1	2	

Intersection: 5: Harbor Blvd & Ralph's Dwy/E. Valley View Dr

Movement	B12	B12
Directions Served	T	T
Maximum Queue (ft)	75	69
Average Queue (ft)	3	3
95th Queue (ft)	31	28
Link Distance (ft)	178	178
Upstream Blk Time (%)	0	
Queuing Penalty (veh)	0	
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Intersection: 6: Harbor Blvd & Berkeley Ave

Movement	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	R	L	T	R	L	T	TR	L	L	T
Maximum Queue (ft)	77	303	175	106	225	379	264	512	578	240	263	323
Average Queue (ft)	12	115	36	47	106	171	116	296	330	141	165	132
95th Queue (ft)	48	217	104	93	185	311	242	491	523	214	235	253
Link Distance (ft)		460			660	660		600	600			458
Upstream Blk Time (%)								3	5			
Queuing Penalty (veh)								0	0			
Storage Bay Dist (ft)	130		135	200			205			300	300	
Storage Blk Time (%)		9	0		1		0	21		0	0	0
Queuing Penalty (veh)		8	0		0		1	21		0	0	1

Intersection: 6: Harbor Blvd & Berkeley Ave

Movement	SB	SB	B19
Directions Served	T	R	T
Maximum Queue (ft)	342	150	12
Average Queue (ft)	150	9	0
95th Queue (ft)	278	76	6
Link Distance (ft)	458		128
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)		165	
Storage Blk Time (%)	5		
Queuing Penalty (veh)	1		

Network Summary

Network wide Queuing Penalty: 1385

Queuing and Blocking Report

Intersection: 1: Harbor Blvd & Bastanchury Rd

Movement	EB	EB	EB	EB	WB	WB	WB	WB	WB	WB	NB	NB
Directions Served	L	T	T	TR	L	L	T	T	T	R	L	L
Maximum Queue (ft)	385	1022	1002	988	152	253	403	370	333	205	121	290
Average Queue (ft)	380	892	838	508	42	143	284	244	183	110	59	183
95th Queue (ft)	419	1225	1215	1029	104	286	381	336	275	203	108	368
Link Distance (ft)		976	976	976			902	902	902			
Upstream Blk Time (%)		65	10	1								
Queuing Penalty (veh)		0	0	0								
Storage Bay Dist (ft)	325				193	193				145	230	230
Storage Blk Time (%)	79	1			0	0	29		8	3		0
Queuing Penalty (veh)	316	4			0	0	48		27	11		0

Intersection: 1: Harbor Blvd & Bastanchury Rd

Movement	NB	NB	NB	NB	SB	SB	SB	SB	SB
Directions Served	T	T	T	R	L	L	T	T	TR
Maximum Queue (ft)	745	747	759	210	305	335	893	866	656
Average Queue (ft)	570	581	596	172	289	321	741	598	330
95th Queue (ft)	896	892	907	278	350	380	1115	1038	565
Link Distance (ft)	722	722	722				854	854	854
Upstream Blk Time (%)	5	5	7				60	0	0
Queuing Penalty (veh)	32	36	47				0	0	0
Storage Bay Dist (ft)				150	275	275			
Storage Blk Time (%)	52		62	4	40	79	3		
Queuing Penalty (veh)	104		142	20	131	257	13		

Intersection: 2: Harbor Blvd & Valencia Mesa

Movement	EB	EB	EB	WB	WB	NB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	R	L	TR	L	T	T	TR	L	T	T
Maximum Queue (ft)	114	241	145	222	390	209	896	932	957	181	73	82
Average Queue (ft)	70	47	43	61	193	70	371	396	418	81	17	32
95th Queue (ft)	120	158	108	145	322	191	1029	1062	1095	146	53	70
Link Distance (ft)		743		640	640		3866	3866	3866		722	722
Upstream Blk Time (%)												
Queuing Penalty (veh)												
Storage Bay Dist (ft)	55		85			175				230		
Storage Blk Time (%)	33	1	1				31			0		
Queuing Penalty (veh)	53	3	1				15			0		

Intersection: 2: Harbor Blvd & Valencia Mesa

Movement	SB
Directions Served	TR
Maximum Queue (ft)	112
Average Queue (ft)	54
95th Queue (ft)	100
Link Distance (ft)	722
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Queuing and Blocking Report

Intersection: 3: Harbor Blvd & Valley View/Brea Blvd

Movement	EB	EB	EB	WB	WB	NB	NB	NB	NB	B12	B12	B12
Directions Served	L	T	R	L	LT	L	T	T	T	T	T	T
Maximum Queue (ft)	145	444	50	421	380	95	257	252	220	61	52	20
Average Queue (ft)	80	235	17	276	244	6	152	145	122	3	2	1
95th Queue (ft)	161	472	44	385	349	50	234	226	202	25	24	14
Link Distance (ft)		793	793	974	974		178	178	178	343	343	343
Upstream Blk Time (%)							3	2	1			
Queuing Penalty (veh)							16	11	4			
Storage Bay Dist (ft)	85					95						
Storage Blk Time (%)	8	57					13					
Queuing Penalty (veh)	15	39					1					

Intersection: 3: Harbor Blvd & Valley View/Brea Blvd

Movement	SB	SB	SB	SB
Directions Served	L	T	T	TR
Maximum Queue (ft)	170	271	339	253
Average Queue (ft)	91	99	108	26
95th Queue (ft)	163	199	237	134
Link Distance (ft)		3866	3866	3866
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)	150			
Storage Blk Time (%)	5	1		
Queuing Penalty (veh)	20	2		

Intersection: 5: Harbor Blvd & Ralph's Dwy/E. Valley View Dr

Movement	EB	WB	WB	NB	NB	NB	NB	B19	B19	SB	SB	SB
Directions Served	LTR	LT	R	L	T	T	TR	T	T	L	T	TR
Maximum Queue (ft)	326	179	125	128	225	179	213	360	381	113	157	160
Average Queue (ft)	193	58	42	74	125	110	163	57	74	44	52	47
95th Queue (ft)	304	130	93	130	223	167	218	257	281	96	135	124
Link Distance (ft)	343	584			128	128	128	458	458		343	343
Upstream Blk Time (%)	0			7	13	4	13	0	0			
Queuing Penalty (veh)	0			0	96	26	96	1	3			
Storage Bay Dist (ft)			65	95						100		
Storage Blk Time (%)		12	3	18	11					2	2	
Queuing Penalty (veh)		7	2	124	11					17	1	

Intersection: 5: Harbor Blvd & Ralph's Dwy/E. Valley View Dr

Movement	B12	B12
Directions Served	T	T
Maximum Queue (ft)	4	29
Average Queue (ft)	0	1
95th Queue (ft)	3	17
Link Distance (ft)	178	178
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Queuing and Blocking Report

Intersection: 6: Harbor Blvd & Berkeley Ave

Movement	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	R	L	T	R	L	T	TR	L	L	T
Maximum Queue (ft)	112	208	123	259	605	673	264	634	641	160	195	206
Average Queue (ft)	31	92	30	56	219	456	129	552	572	93	119	93
95th Queue (ft)	77	172	77	142	561	702	286	722	717	156	180	170
Link Distance (ft)		460			660	660		600	600			458
Upstream Blk Time (%)					2	7		22	39			
Queuing Penalty (veh)					0	0		0	0			
Storage Bay Dist (ft)	130		135	200			205			300	300	
Storage Blk Time (%)		5	0		4		0	48				
Queuing Penalty (veh)		5	0		3		2	42				

Intersection: 6: Harbor Blvd & Berkeley Ave

Movement	SB	SB	B19
Directions Served	T	R	T
Maximum Queue (ft)	226	63	6
Average Queue (ft)	110	4	0
95th Queue (ft)	191	37	5
Link Distance (ft)	458		128
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)		165	
Storage Blk Time (%)	1		
Queuing Penalty (veh)	0		

Network Summary

Network wide Queuing Penalty: 1807

APPENDIX C-IV

YEAR 2045 BUILDOUT PLUS PROJECT TRAFFIC CONDITIONS

Intersection: 1: Harbor Blvd & Bastanchury Rd

Movement	EB	EB	EB	EB	WB	WB	WB	WB	WB	WB	NB	NB
Directions Served	L	T	T	TR	L	L	T	T	T	R	L	L
Maximum Queue (ft)	385	1004	971	878	196	252	415	353	277	205	191	210
Average Queue (ft)	379	815	754	389	90	174	259	220	170	56	94	122
95th Queue (ft)	417	1186	1120	750	170	287	372	305	252	140	198	226
Link Distance (ft)		976	976	976			902	902	902			
Upstream Blk Time (%)		28	2	0								
Queuing Penalty (veh)		0	0	0								
Storage Bay Dist (ft)	325				193	193				145	230	230
Storage Blk Time (%)	80	1			0	1	22		7	0	4	7
Queuing Penalty (veh)	323	5			0	2	60		16	0	10	16

Intersection: 1: Harbor Blvd & Bastanchury Rd

Movement	NB	NB	NB	NB	B13	SB	SB	SB	SB	SB
Directions Served	T	T	T	R	T	L	L	T	T	TR
Maximum Queue (ft)	249	231	234	183	18	298	335	900	899	894
Average Queue (ft)	136	136	118	56	1	200	317	839	816	802
95th Queue (ft)	268	233	198	130	17	316	387	1009	1030	1055
Link Distance (ft)	333	333	333		334			854	854	854
Upstream Blk Time (%)	2	0						52	40	33
Queuing Penalty (veh)	7	0						0	0	0
Storage Bay Dist (ft)				150		275	275			
Storage Blk Time (%)	0		2	0		7	11	56		
Queuing Penalty (veh)	1		3	0		33	55	252		

Intersection: 2: Harbor Blvd & Valencia Mesa

Movement	EB	EB	EB	WB	WB	NB	NB	NB	SB	SB	SB	SB
Directions Served	L	T	R	L	TR	L	T	TR	L	T	T	R
Maximum Queue (ft)	114	233	142	119	174	245	470	508	280	211	219	78
Average Queue (ft)	69	59	43	48	68	148	310	340	151	86	95	22
95th Queue (ft)	112	144	97	95	135	278	454	475	247	166	172	55
Link Distance (ft)		749		645	645		542	542		334	334	334
Upstream Blk Time (%)								0		0	0	
Queuing Penalty (veh)								0		2	1	
Storage Bay Dist (ft)	55		85			185			230			
Storage Blk Time (%)	25	5	1			1	25		1	0		
Queuing Penalty (veh)	34	9	2			3	38		8	1		

Intersection: 3: Harbor Blvd & Valley View/Brea Blvd

Movement	EB	EB	EB	WB	WB	WB	NB	NB	NB	B12	B12	SB
Directions Served	L	T	R	L	LT	R	L	T	T	T	T	L
Maximum Queue (ft)	134	237	38	448	404	80	100	216	227	8	12	199
Average Queue (ft)	54	121	10	305	274	3	8	116	121	0	1	69
95th Queue (ft)	134	245	32	404	374	34	48	200	212	5	7	165
Link Distance (ft)		793	793	984	984	984		178	178	340	340	
Upstream Blk Time (%)								1	2			
Queuing Penalty (veh)								8	12			
Storage Bay Dist (ft)	85						95					140
Storage Blk Time (%)	4	43					0	13				0
Queuing Penalty (veh)	3	22					0	3				1

Intersection: 3: Harbor Blvd & Valley View/Brea Blvd

Movement	SB	SB	SB	B14	B14
Directions Served	T	T	R	T	T
Maximum Queue (ft)	362	365	200	81	70
Average Queue (ft)	272	273	55	8	7
95th Queue (ft)	378	382	180	42	41
Link Distance (ft)	290	290		772	772
Upstream Blk Time (%)	10	12			
Queuing Penalty (veh)	81	96			
Storage Bay Dist (ft)			140		
Storage Blk Time (%)	37	37			
Queuing Penalty (veh)	22	29			

Intersection: 4: Harbor Blvd & Future Dog Park

Movement	WB	NB	NB	B16	B16	SB	SB	SB	B15
Directions Served	LR	T	T	T	T	L	T	T	T
Maximum Queue (ft)	30	18	43	8	8	63	142	149	4
Average Queue (ft)	8	1	3	0	0	3	17	20	0
95th Queue (ft)	26	10	19	4	6	33	87	94	3
Link Distance (ft)	423	896	896	772	772		1143	1143	542
Upstream Blk Time (%)									
Queuing Penalty (veh)									
Storage Bay Dist (ft)						250			
Storage Blk Time (%)							0		
Queuing Penalty (veh)							0		

Intersection: 5: Harbor Blvd & Ralph's Dwy/E Valley View Dr

Movement	EB	WB	WB	NB	NB	NB	NB	B25	B25	SB	SB	SB
Directions Served	LTR	LT	R	L	T	T	TR	T	T	L	T	TR
Maximum Queue (ft)	203	72	64	147	205	178	146	105	110	84	228	250
Average Queue (ft)	88	27	26	71	62	66	53	6	5	31	63	60
95th Queue (ft)	161	64	52	131	158	141	118	68	66	65	159	157
Link Distance (ft)	380	720			173	173	173	425	425		340	340
Upstream Blk Time (%)				0	2	1	0				0	0
Queuing Penalty (veh)				0	11	3	0				1	1
Storage Bay Dist (ft)			65	95						100		
Storage Blk Time (%)		2	0	14	2					0	2	
Queuing Penalty (veh)		1	0	70	2					3	2	

Intersection: 5: Harbor Blvd & Ralph's Dwy/E Valley View Dr

Movement	B12
Directions Served	T
Maximum Queue (ft)	9
Average Queue (ft)	0
95th Queue (ft)	6
Link Distance (ft)	178
Upstream Blk Time (%)	
Queuing Penalty (veh)	
Storage Bay Dist (ft)	
Storage Blk Time (%)	
Queuing Penalty (veh)	

Intersection: 6: Harbor Blvd & Berkeley Ave

Movement	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	R	L	T	R	L	T	TR	L	L	T
Maximum Queue (ft)	83	239	172	131	229	347	264	512	501	256	257	280
Average Queue (ft)	16	116	43	46	112	156	117	291	297	145	168	125
95th Queue (ft)	52	200	118	99	193	280	246	444	437	229	243	236
Link Distance (ft)		572			726	726		605	605			425
Upstream Blk Time (%)								0				
Queuing Penalty (veh)								0				
Storage Bay Dist (ft)	130		135	200			205			300	300	
Storage Blk Time (%)		10	0		1			20			0	0
Queuing Penalty (veh)		9	0		0			20			0	0

Intersection: 6: Harbor Blvd & Berkeley Ave

Movement	SB	SB	B25
Directions Served	T	R	T
Maximum Queue (ft)	317	112	9
Average Queue (ft)	147	9	1
95th Queue (ft)	264	63	9
Link Distance (ft)	425		173
Upstream Blk Time (%)			
Queuing Penalty (veh)			
Storage Bay Dist (ft)		165	
Storage Blk Time (%)	4		
Queuing Penalty (veh)	1		

Network Summary

Network wide Queuing Penalty: 1282

Queuing and Blocking Report

Intersection: 1: Harbor Blvd & Bastanchury Rd

Movement	EB	EB	EB	EB	WB	WB	WB	WB	WB	WB	NB	NB
Directions Served	L	T	T	TR	L	L	T	T	T	R	L	L
Maximum Queue (ft)	385	971	950	816	132	253	384	362	295	199	132	290
Average Queue (ft)	359	677	618	287	50	150	274	228	171	102	57	204
95th Queue (ft)	452	1140	1067	568	115	289	358	310	256	184	111	387
Link Distance (ft)		976	976	976			902	902	902			
Upstream Blk Time (%)		17	1	0								
Queuing Penalty (veh)		0	0	0								
Storage Bay Dist (ft)	325				193	193				145	230	230
Storage Blk Time (%)	62	2				0	26		6	2		0
Queuing Penalty (veh)	249	9				0	43		22	8		0

Intersection: 1: Harbor Blvd & Bastanchury Rd

Movement	NB	NB	NB	NB	B13	B13	B13	SB	SB	SB	SB	SB
Directions Served	T	T	T	R	T	T	T	L	L	T	T	TR
Maximum Queue (ft)	418	430	428	210	370	392	397	305	335	902	869	846
Average Queue (ft)	380	384	388	177	248	254	261	295	325	786	645	279
95th Queue (ft)	489	491	481	274	480	481	478	346	377	1111	1067	541
Link Distance (ft)	333	333	333		334	334	334			854	854	854
Upstream Blk Time (%)	52	55	57		5	5	6			71	0	0
Queuing Penalty (veh)	347	368	379		33	30	43			0	0	0
Storage Bay Dist (ft)				150				275	275			
Storage Blk Time (%)	58		65	2				45	83	2		
Queuing Penalty (veh)	114		148	9				146	272	7		

Intersection: 2: Harbor Blvd & Valencia Mesa

Movement	EB	EB	EB	WB	WB	NB	NB	NB	B15	B15	SB	SB
Directions Served	L	T	R	L	TR	L	T	TR	T	T	L	T
Maximum Queue (ft)	114	285	144	124	420	245	648	647	1038	1071	169	124
Average Queue (ft)	77	56	52	49	193	113	489	502	347	364	86	42
95th Queue (ft)	123	182	116	99	344	278	774	767	1009	1044	155	93
Link Distance (ft)		749		645	645		542	542	1143	1143		334
Upstream Blk Time (%)							42	45	1	2		
Queuing Penalty (veh)							346	368	11	13		
Storage Bay Dist (ft)	55		85			185					230	
Storage Blk Time (%)	30	1	1			0	53					
Queuing Penalty (veh)	49	2	2			0	27					

Intersection: 2: Harbor Blvd & Valencia Mesa

Movement	SB	SB	B13	B13
Directions Served	T	R	T	T
Maximum Queue (ft)	131	34	7	11
Average Queue (ft)	57	7	0	0
95th Queue (ft)	108	28	4	7
Link Distance (ft)	334	334	333	333
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)				
Storage Blk Time (%)				
Queuing Penalty (veh)				

Intersection: 3: Harbor Blvd & Valley View/Brea Blvd

Movement	EB	EB	EB	WB	WB	WB	NB	NB	NB	B12	B12	B12
Directions Served	L	T	R	L	LT	R	L	T	T	T	T	T
Maximum Queue (ft)	145	354	43	429	392	113	96	266	280	230	249	37
Average Queue (ft)	78	185	18	279	245	7	7	233	239	69	81	2
95th Queue (ft)	161	317	42	396	368	63	57	292	293	203	222	19
Link Distance (ft)		793	793	984	984	984		178	178	340	340	340
Upstream Blk Time (%)								19	22	0	0	
Queuing Penalty (veh)								146	168	1	3	
Storage Bay Dist (ft)	85						95					
Storage Blk Time (%)	7	53						34				
Queuing Penalty (veh)	13	36						3				

Intersection: 3: Harbor Blvd & Valley View/Brea Blvd

Movement	SB	SB	SB	SB	B14	B14
Directions Served	L	T	T	R	T	T
Maximum Queue (ft)	200	361	356	200	30	38
Average Queue (ft)	99	212	220	33	2	2
95th Queue (ft)	196	345	356	128	15	17
Link Distance (ft)		290	290		772	772
Upstream Blk Time (%)		5	6			
Queuing Penalty (veh)		28	36			
Storage Bay Dist (ft)	140			140		
Storage Blk Time (%)	5	25	28	0		
Queuing Penalty (veh)	30	25	14	0		

Intersection: 4: Harbor Blvd & Future Dog Park

Movement	WB	NB	NB	B16	B16	B14	B14	SB	SB	SB
Directions Served	LR	T	T	T	T	T	T	L	T	T
Maximum Queue (ft)	38	430	434	5	10	4	19	33	62	74
Average Queue (ft)	8	71	76	0	0	0	1	3	5	8
95th Queue (ft)	28	369	377	6	8	2	11	18	29	39
Link Distance (ft)	423	896	896	772	772	290	290		1143	1143
Upstream Blk Time (%)		1	1							
Queuing Penalty (veh)		5	6							
Storage Bay Dist (ft)								250		
Storage Blk Time (%)			5							
Queuing Penalty (veh)			0							

Intersection: 5: Harbor Blvd & Ralph's Dwy/E Valley View Dr

Movement	EB	WB	WB	NB	NB	NB	NB	B25	B25	SB	SB	SB
Directions Served	LTR	LT	R	L	T	T	TR	T	T	L	T	TR
Maximum Queue (ft)	321	108	91	152	222	213	234	177	191	92	149	136
Average Queue (ft)	183	50	31	61	114	132	129	16	18	40	63	58
95th Queue (ft)	295	99	70	125	198	199	211	141	147	84	134	120
Link Distance (ft)	380	720			173	173	173	425	425		340	340
Upstream Blk Time (%)	0			0	3	1	2	0	0			
Queuing Penalty (veh)	0			0	25	10	16	0	1			
Storage Bay Dist (ft)			65	95						100		
Storage Blk Time (%)		8	2	9	7					1	4	
Queuing Penalty (veh)		5	1	65	7					7	2	

Intersection: 5: Harbor Blvd & Ralph's Dwy/E Valley View Dr

Movement	B12	B12
Directions Served	T	T
Maximum Queue (ft)	10	10
Average Queue (ft)	0	0
95th Queue (ft)	7	7
Link Distance (ft)	178	178
Upstream Blk Time (%)		
Queuing Penalty (veh)		
Storage Bay Dist (ft)		
Storage Blk Time (%)		
Queuing Penalty (veh)		

Queuing and Blocking Report

Intersection: 6: Harbor Blvd & Berkeley Ave

Movement	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB
Directions Served	L	T	R	L	T	R	L	T	TR	L	L	T
Maximum Queue (ft)	100	206	90	116	671	738	264	644	653	191	204	265
Average Queue (ft)	26	93	33	49	163	439	140	564	577	108	129	107
95th Queue (ft)	71	165	72	98	440	703	300	716	722	169	188	213
Link Distance (ft)		572			726	726		605	605			425
Upstream Blk Time (%)					0	2		25	33			
Queuing Penalty (veh)					0	0		0	0			
Storage Bay Dist (ft)	130		135	200			205			300	300	
Storage Blk Time (%)		5			1		1	50				0
Queuing Penalty (veh)		5			1		5	43				0

Intersection: 6: Harbor Blvd & Berkeley Ave

Movement	SB	SB	B25	B25
Directions Served	T	R	T	T
Maximum Queue (ft)	295	99	10	16
Average Queue (ft)	125	6	0	1
95th Queue (ft)	230	49	7	8
Link Distance (ft)	425		173	173
Upstream Blk Time (%)				
Queuing Penalty (veh)				
Storage Bay Dist (ft)		165		
Storage Blk Time (%)	2			
Queuing Penalty (veh)	0			

Network Summary

Network wide Queuing Penalty: 3753

ATTACHMENT F

CITY OF LAGUNA HILLS

City Manager

June 11, 2024

Orange County Transportation Authority
ATTN: Kia Mortazavi, Executive Director, Planning Division
550 South Main Street
P.O. Box 14184
Orange, CA 92863

Subject: Master Plan of Arterial Highways Amendment – Paseo De Valencia and Cabot Road

Dear Mr. Mortazavi:

The City of Laguna Hills is requesting an amendment to the Master Plan of Arterial Highways (MPAH). Paseo De Valencia is identified on the MPAH as a 6-lane divided Major Arterial between Alicia Parkway and La Paz Road but currently built as a 4-lane undivided arterial, and as a 4-lane undivided Secondary Arterial between La Paz Road and Cabot Road within the City of Laguna Hills. Cabot Road is identified on the MPAH as a 4-lane undivided Secondary Arterial between Paseo De Valencia and Oso Parkway. The City is recommending the segment of Paseo De Valencia between Alicia Parkway and La Paz Road to be reclassified as a 2-lane Divided Collector, and the segment between La Paz Road and Cabot Road to be reclassified as a 2-lane Undivided Collector. The City is also recommending the segment of Cabot Road between Paseo De Valencia and Oso Parkway to be reclassified as a 2-lane Divided Collector.

This reclassification will support the City's objective to provide traffic safety and circulation improvements in front of Valencia Elementary School along Paseo De Valencia. School improvements may include all-way stop installations, curb extensions at intersection crossings, and a student pick-up/drop-off lane for queuing vehicles during school loading times. Reclassification will also support the City's objective to accommodate multi-modal improvements along both Paseo De Valencia and Cabot Road. Attachment 'A' illustrates the project limits and proposed improvements.

The reclassification requires lane reduction, which was determined feasible based on Annual Daily Traffic (ADT) data collected in 2022, per the 2023 OCTA Traffic Volume Map (Attachment B). The segment of Paseo De Valencia between Alicia Parkway and La Paz Road has approximately 11,000 ADT, and the segment of Paseo De Valencia between La Paz Road and Cabot Road has approximately 6,000 ADT. Both of these segments are well within the ranges of 9,000 to 15,000 ADT for a Divided Collector and 7,500 to 10,000 ADT for an Undivided Collector identified in the MPAH, respectively. The segment of Cabot Road between Paseo De Valencia and El Paseo has approximately 13,000 to 14,000 ADT and is also within the range of 9,000 to 15,000 ADT for a Divided Collector identified in the MPAH.

Thank you for your attention to this important matter.

If you have any questions about this project, please contact me at jhildenbrand@lagunahillsca.gov or 949/707-2610, or Public Works Director Joe Ames at james@lagunahillsca.gov or 949/707-2655.

Sincerely,



Jarad Hildenbrand
City Manager



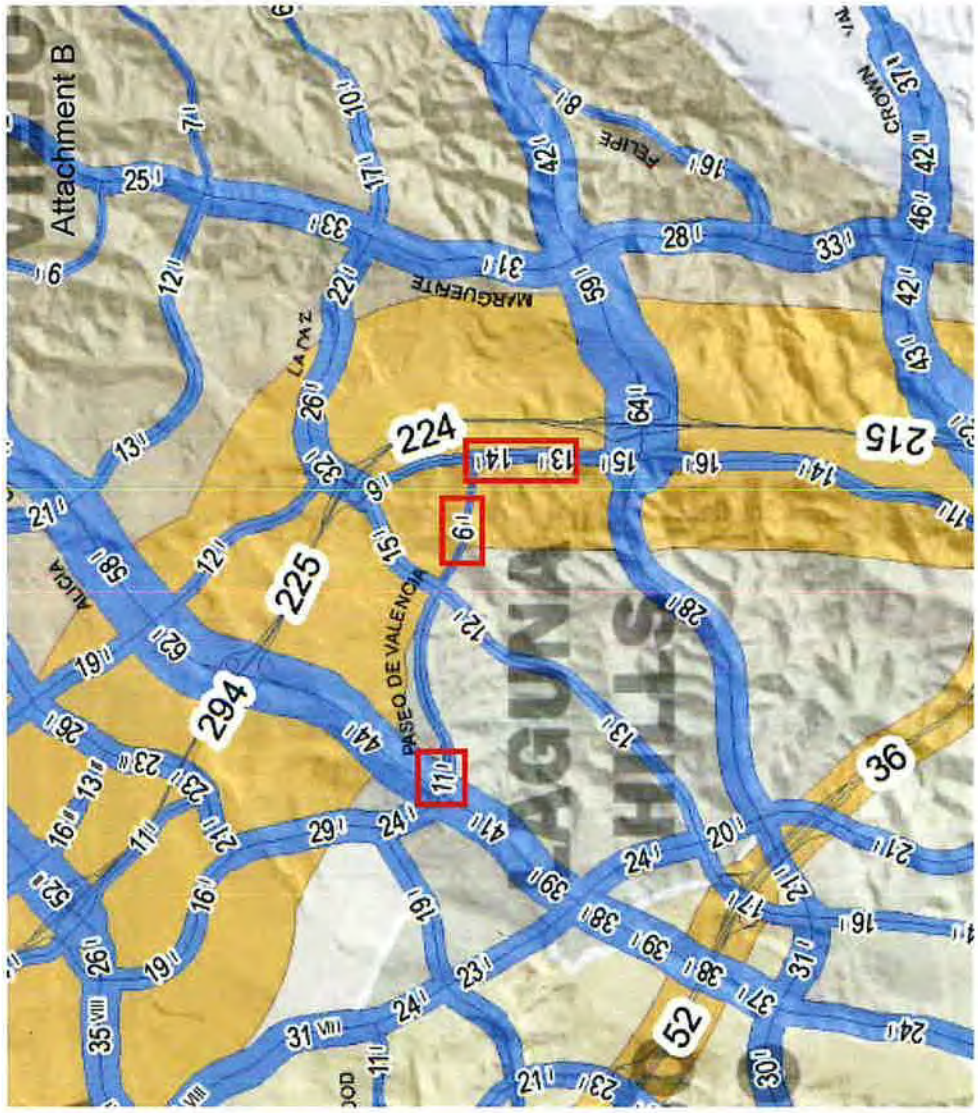
Paseo De Valencia & Cabot Road MPAH Amendment

Project Location Map

MARK THOMAS

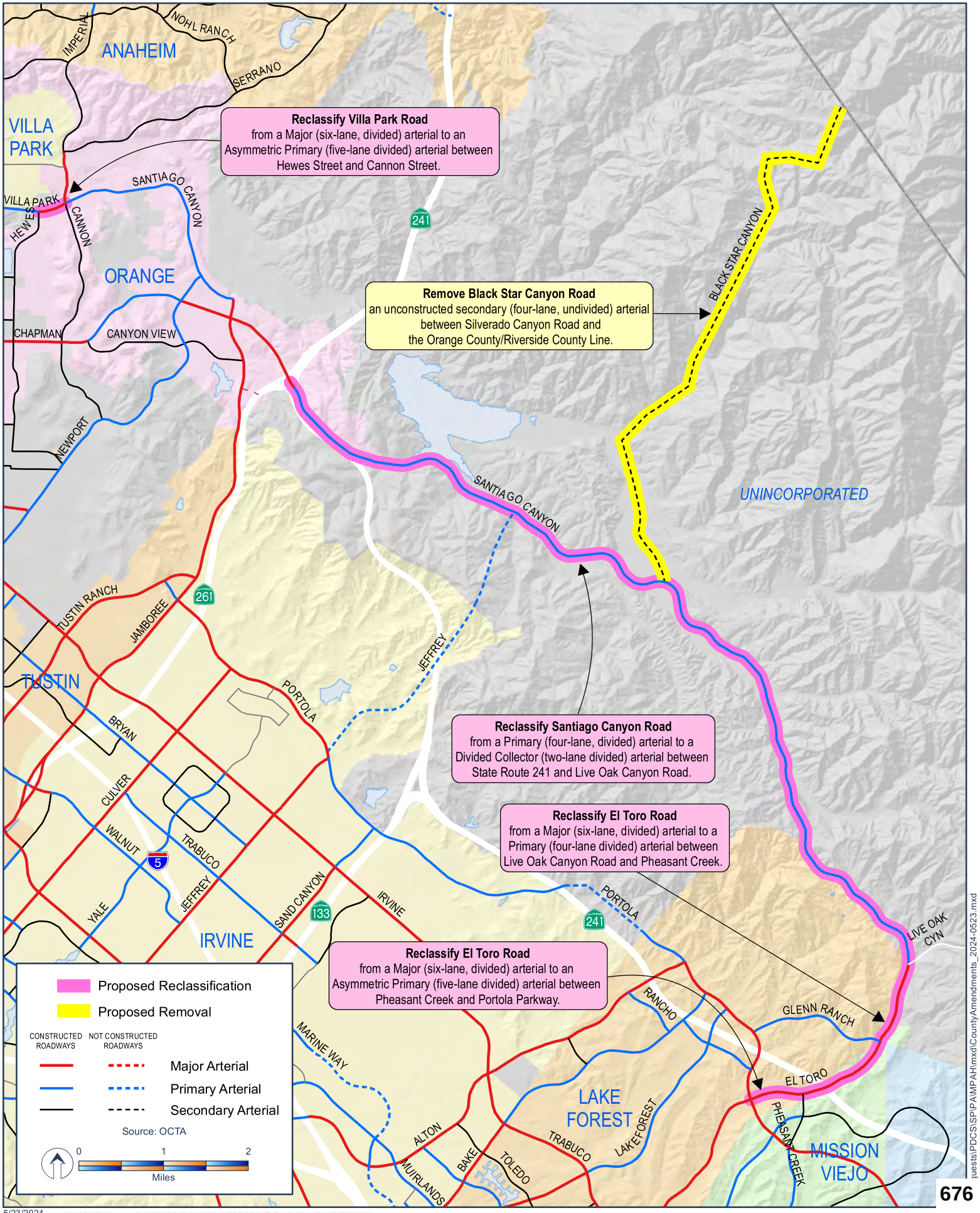
Legend

- MPAH Amendment Limits
- Laguna Hills City Boundary


Attachment B
MARGUERITE
PASEO DE VALENCIA
ALICIA
LAGUNA
HILLS
224
225
294
215
36
52

County of Orange MPAH Amendments

ATTACHMENT G



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March 12, 2024

Consent Form

Project: MPAH Amendment Request for Villa Park Road/Santiago Canyon Road

Lead: Orange County Public Works

Amendment: Reclassification of Villa Park Road/Santiago Canyon Road between Hewes Street and Cannon Street from a 6-lane Major Arterial Highway to a 5-lane Asymmetric Primary Highway

Please select your response below:

☒ We, City of Orange, grant consent to the subject MPAH Amendment Request, and are in support of OCPW being the lead agency in the Amendment efforts.

☐ We, _____, **do not** grant consent to the subject MPAH Amendment Request. Our reasons are attached as a separate page to this form.

_____ 3/12/2024

Signature Date

Frank Sun

Name (Print)

Assistant PW Director/City Engineer

Title

March 4, 2024

Consent Form

Project: MPAH Amendment Request for Villa Park Road/Santiago Canyon Road

Lead: Orange County Public Works

Amendment: Reclassification of Villa Park Road/Santiago Canyon Road between Hewes Street and Cannon Street from a 6-lane Major Arterial Highway to a 5-lane Asymmetric Primary Highway

Please select your response below:

☒ We, CITY OF VILLA PARK, grant consent to the subject MPAH Amendment Request, and are in support of OCPW being the lead agency in the Amendment efforts.

☐ We, _____, **do not** grant consent to the subject MPAH Amendment Request. Our reasons are attached as a separate page to this form.

Hamid Reza Torkamandeh
Signature

3/13/2024
Date

HAMID TORKAMANHA
Name (Print)

CITY ENGINEER
Title

March 25, 2024

Consent Form

Project: MPAH Amendment Request for Santiago Canyon Road, El Toro Road, and Black Star Canyon Road

Lead: Orange County Public Works

Amendment:

- Reclassify Santiago Canyon Road (SCR) between SR-241 and Live Oak Canyon Road from its currently planned designation as a Primary Arterial (four-lane divided roadway) to a Divided Collector (two-lane divided roadway with a center turn-lane)
- Reclassify El Toro Road (ETR) between Live Oak Canyon Road and Portola Parkway from its currently planned designation as a Major Arterial (six-lane divided roadway) to a Primary Arterial (four-lane divided roadway) with the exception that the southern end segment between Pheasant Creek and Portola Parkway remain at its current condition as an asymmetric Primary Arterial (five-lane divided roadway)
- Delete Black Star Canyon Road (BSCR) between Silverado Canyon Road and the Orange County/Riverside County line.

Please select your response below:

☒ We, City of Lake Forest, grant consent to the subject MPAH Amendment Request, and are in support of OCPW being the lead agency in the Amendment efforts.

☐ We, _____, **do not** grant consent to the subject MPAH Amendment Request. Our reasons are attached as a separate page to this form.

Tom Wheeler
_____
Signature

April 9, 2024
Date

Thomas E. Wheeler
Name (Print)

Public Works Director/City Engineer
Title

April 11, 2024

Consent Form

Project: MPAH Amendment Request for Santiago Canyon Road, El Toro Road, and Black Star Canyon Road

Lead: Orange County Public Works

Amendment:

- Reclassify Santiago Canyon Road (SCR) between SR-241 and Live Oak Canyon Road from its currently planned designation as a Primary Arterial (four-lane divided roadway) to a Divided Collector (two-lane divided roadway with a center turn-lane)
- Reclassify El Toro Road (ETR) between Live Oak Canyon Road and Portola Parkway from its currently planned designation as a Major Arterial (six-lane divided roadway) to a Primary Arterial (four-lane divided roadway) with the exception that the southern end segment between Pheasant Creek and Portola Parkway remain at its current condition as an asymmetric Primary Arterial (five-lane divided roadway)
- Delete Black Star Canyon Road (BSCR) between Silverado Canyon Road and the Orange County/Riverside County line.

Please select your response below:

☒ We, The City of Mission Viejo, grant consent to the subject MPAH Amendment Request, and are in support of OCPW being the lead agency in the Amendment efforts.

☐ We, _____, **do not** grant consent to the subject MPAH Amendment Request. Our reasons are attached as a separate page to this form.

_____ April 11, 2024

Signature Date

Mark Chagnon

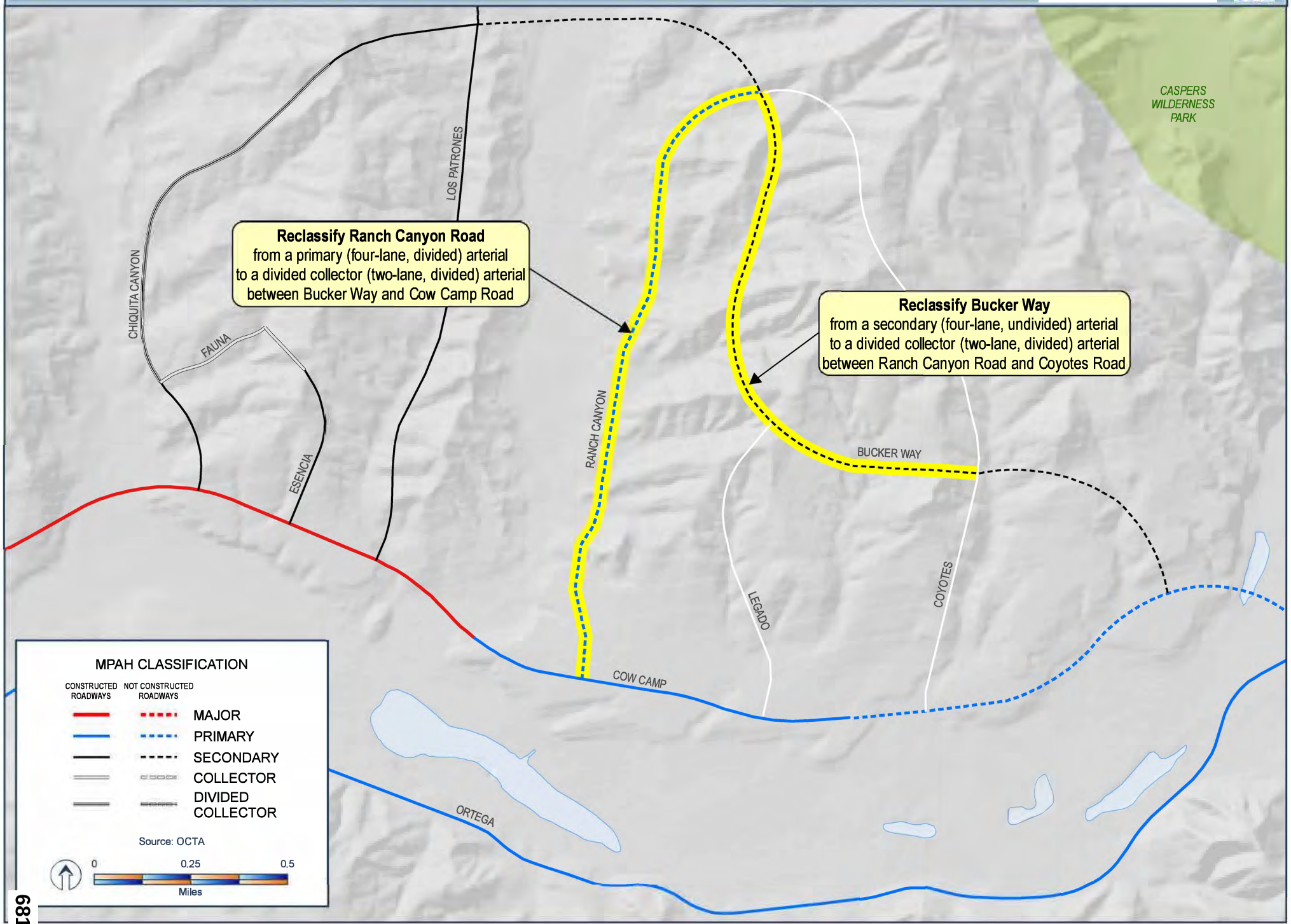
Name (Print)

Director of Public Works

Title

County of Orange MPAH Amendment – Bucker Way and Ranch Canyon Road

ATTACHMENT J

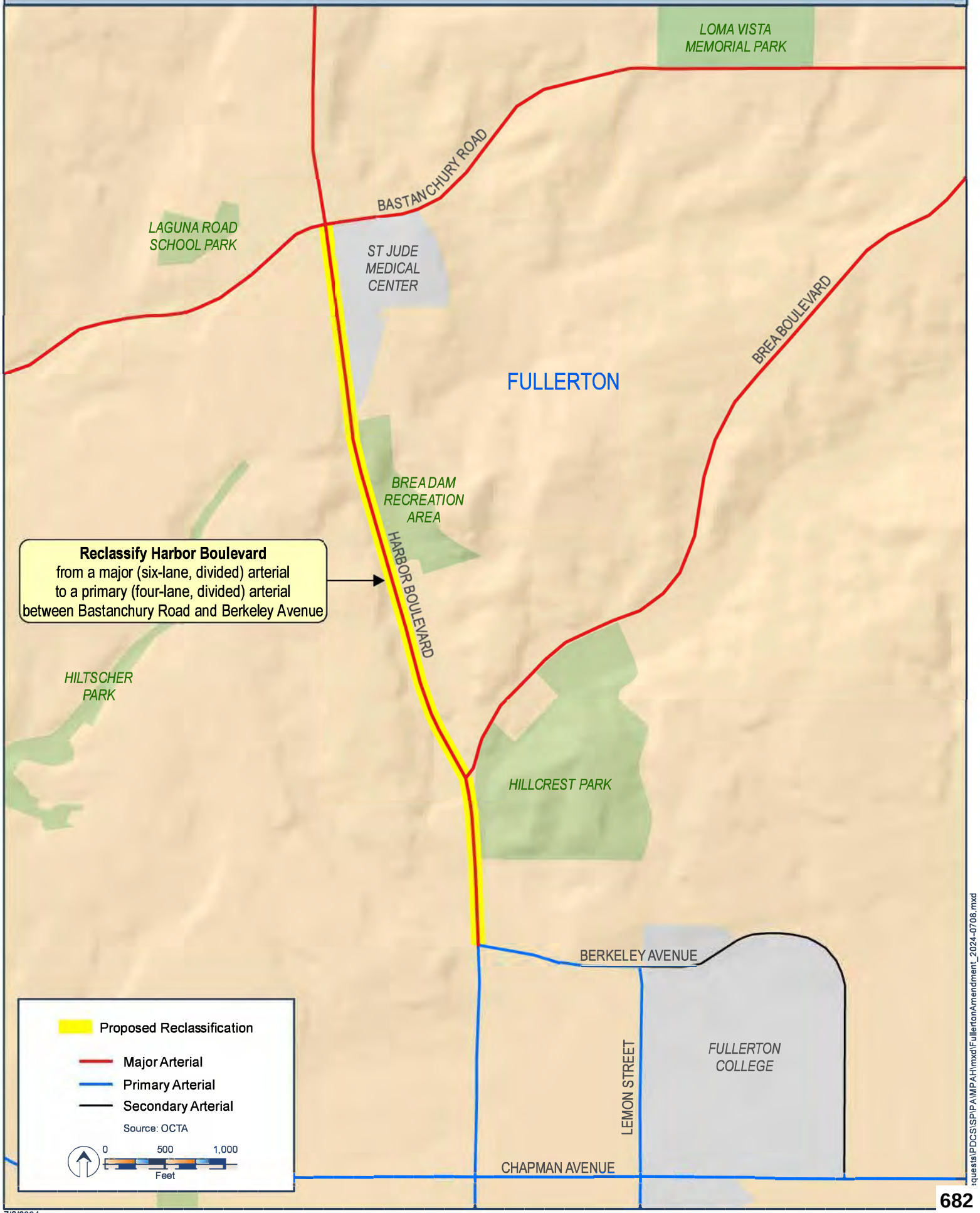


MPAH – Master Plan of Arterial Highways

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City of Fullerton MPAH Amendment

ATTACHMENT K



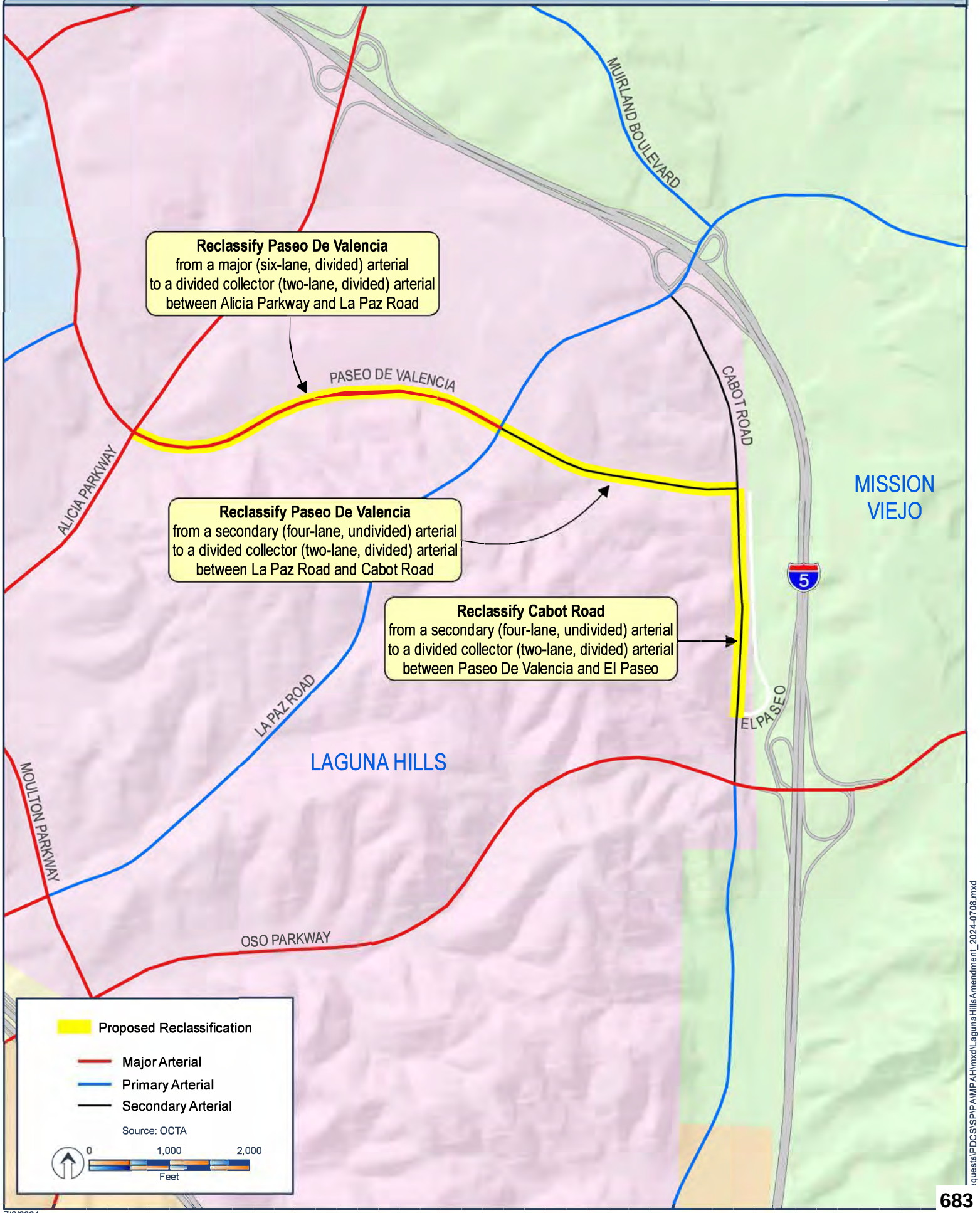
7/8/2024

MPAH – Master Plan of Arterial Highways

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City of Laguna Hills MPAH Amendment

ATTACHMENT L



7/8/2024

MPAH – Master Plan of Arterial Highways

ATTACHMENT M

Status Report on Pending Master Plan of Arterial Highways Amendments

#	City	Street	From	To	Type of Amendment	Status	Board Approval Date
1	Brea / County of Orange	Tonner Canyon Road	Brea Canyon Road	Planned Valencia Avenue	Delete	Amendment expired, requires City to submit a new request.	7/12/2021
2	Brea / County of Orange	Valencia Avenue	Carbon Canyon Road	Planned Tonner Canyon Road	Delete	Amendment expired, requires City to submit a new request.	7/12/2021
3	Costa Mesa	Bluff Road	19th Street	Victoria Street	Delete	On hold pending final consensus between Costa Mesa and Newport Beach on circulation plans.	
4	Costa Mesa	19th Street	Placentia Avenue	west city limit	Reclassify from primary to divided collector	On hold pending final consensus between the cities of Costa Mesa and Newport Beach on circulation plans.	
5	County of Orange / Irvine	Jeffrey Road	SR-241	Santiago Canyon Road	Delete	The amendment was conditionally approved by the Board. Waiting for documentation confirming completion of CEQA and general plan change.	5/8/2017
6	Santa Ana/Orange	Fairhaven Avenue	Grand Avenue	Tustin Avenue	Reclassify from secondary to divided collector	The amendment was conditionally approved by the Board. Waiting for documentation confirming completion of CEQA and general plan change.	11/9/2020
7	Irvine	Red Hill Avenue	MacArthur Boulevard	Main Street	Reclassify from major to primary	The amendment was conditionally approved by the Board. Waiting for documentation confirming completion of CEQA and general plan change.	4/10/2023
8	Fullerton	Associated Road	Bastanchury Road	Imperial Highway	Reclassify from a secondary to a collector	The amendment was conditionally approved by the Board. Waiting for documentation confirming completion of CEQA and general plan change.	4/10/2023
9	County	Villa Park Road	Hewes Street	Cannon Street	Reclassify from a major to a asymmetric primary	**Amendment will be being presented to the Board for consideration 9/9/2024.**	
10	County	Santiago Canyon Road	SR-241	Live Oak Canyon Road	Reclassify	**Amendment will be being presented to the Board for consideration 9/9/2024.**	
11	County	El Toro Road	Live Oak Canyon Road	Portola Parkway	Reclassify	**Amendment will be being presented to the Board for consideration 9/9/2024.**	

Status Report on Pending Master Plan of Arterial Highways Amendments

#	City	Street	From	To	Type of Amendment	Status	Board Approval Date
12	County	Black Star Canyon Road	Silverado Canyon Road	Orange County/Riverside County Line	Delete	**Amendment will be being presented to the Board for consideration 9/9/2024.**	
13	County	Bucker Way	Ranch Canyon Road	Coyotes Road	Reclassify	**Amendment will be being presented to the Board for consideration 9/9/2024.**	
14	County	Ranch Canyon Road	Bucker Way	Cow Camp Road	Reclassify	**Amendment will be being presented to the Board for consideration 9/9/2024.**	
15	Fullerton	Harbor Boulevard	Bastanchury Road	Berkeley Avenue	Reclassify	**Amendment will be being presented to the Board for consideration 9/9/2024.**	
16	Laguna Hills	Paseo De Valencia	Alicia Parkway	Cabot Road	Reclassify	**Amendment will be being presented to the Board for consideration 9/9/2024.**	
17	Laguna Hills	Cabot Road	Paseo De Valencia	El Paseo	Reclassify	**Amendment will be being presented to the Board for consideration 9/9/2024.**	
MOU	Costa Mesa/ Fountain Valley/ Huntington Beach	Garfield Avenue/ Gisler Avenue Crossing over the Santa Ana River	Santa Ana River Westbank	Santa Ana River Eastbank	Reclassify from secondary to right-of-way reserve status	The cities of Costa Mesa, Fountain Valley, Huntington Beach, and OCTA entered a Memorandum of Understanding (C-6-0834). Reasonable progress has been made on implementation of 19 of the 25 mitigation measures that were specified. All improvements are required to be completed by 2025, at which time OCTA will revisit the designation of the Garfield Avenue/Gisler Avenue Bridge.	

Board – Board of Directors
CEQA – California Environmental Quality Act
MOU – Memorandum of understanding
MPAH – Master Plan of Arterial Highways
OCTA - Orange County Transportation Authority
SR-241 – State Route 241

Appendix K Transportation Impact Study

Appendices

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Memorandum

To/Attention	Placeworks	**Date**	October 7, 2020
From	IBI Group	**Project No**	119772
cc			
Subject	Santa Ana General Plan Traffic Analysis Updates		

This memo summarizes changes made to the Traffic Analysis prepared in support of the Santa Ana General Plan DEIR. The traffic analysis was circulated among relevant stakeholders in order to solicit input regarding the proposed reclassifications and analysis methodology. The major changes incorporated into the traffic analysis based on the comments received are summarized below:

- Adjustment of volumes in northeast corner for select intersections along Grand Avenue and Tustin Avenue. Volume changes at the intersection of Tustin Avenue and Santa Clara Avenue were highlighted by the City of Tustin as being excessive. Volumes were adjusted at intersections 74-76 and 91-93 in order to improve trip balance.
- Language for LOS impacts and mitigations updated to better address improvements.
- Fair –share added for non-City of Santa Ana intersections.
- LOS worksheets for the 2045 WP AM were added; they were missing in original appendix.
- V/C and LOS updated for intersection #98 at Red Hill and Warner. An impact was caused as part of the update. Mitigation was developed.
- Additional roadway segments added to analysis.
- Mitigation measure for intersection #3 at Euclid Street and Edinger Avenue expanded.
- Mitigation measure for intersection #62 at Santa Ana Boulevard and Santiago Street modified.
- Mitigation at intersection #100 at Red Hill Avenue and Alton Parkway modified.
- Updates to Figure 2.1 through 2.7.

Santa Ana General Plan Update
Traffic Impact Study
FINAL Report

Prepared for:

City of Santa Ana
Public Works Agency
20 Civic Center Plaza, M-36
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By:

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October 2020

DOCUMENT HISTORY

Version	Date	Originator	Revisions By	Status
1	06/05/20	Y. Esquivel		Initial draft.
2	07/21/20	Y. Esquivel	Y. Esquivel	Draft revision.
3	08/24/20	Y. Esquivel	Y. Esquivel	Draft revision.
4	10/20/20	Y. Esquivel	Y. Esquivel	Final Draft.
5				
6				
7				
8				

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1.0 Introduction

1.1 STUDY PURPOSE

This traffic analysis report has been prepared in support of the General Plan Update (GPU) Draft Program Environmental Impact Report (PEIR) for the City of Santa Ana and to assess requested changes to the Orange County Transportation Authority's (OCTA) Master Plan of Arterial Highways (MPAH) by the City of Santa Ana. This traffic analysis report reviews existing and future traffic conditions under the current General Plan and with the proposed Land Use Element and Circulation Element Updates.

This report is used as part of the Program EIR prepared for the General Plan Update. The Program EIR includes the results and findings of this traffic study report and identifies the significant impacts of the proposed project. The draft Program EIR will be available to the public for review and comment.

1.2 REPORT ORGANIZATION

This report consists of 11 sections.

1.0 Introduction
2.0 Project Description
3.0 Circulation Element Roadway Classifications
4.0 Analysis Methodology
5.0 Existing Year (2020) Conditions
6.0 Future Year 2045 Forecast Modeling
7.0 Future Year (2045) No Project
8.0 Future Year (2045) With Project
9.0 Significant Impacts and Mitigation Measures
10.0 Conclusions
11.0 Appendices

Section 1 provides a brief introduction to the study area and report organization. Section 2 provides a summary of the project description, background history, and project alternatives. Section 3 describes the various roadway classifications within the Circulation Element. Section 4 includes the methodology utilized in the analysis and the referenced standards. The study area roadways and Existing Year (2020) intersection geometry, turning movement volumes, and level of service are presented in Section 5. The Future Year (2045) forecast modeling methodology is provided in Section 6. Section 7 describes the Future Year (2045) No Project condition and provides the roadway segment and intersection level of service results. Section 8 describes the Future Year (2045) With Project Network Alternative condition and provides the roadway segment and intersection level of service results. Section 9 presents the mitigation measures to address facilities operating at an unacceptable level of service. Findings and conclusions from this study are presented in Section 10. Appendices are provided in Section 11.

The City boundaries/project study area is shown in Exhibits 1.1 and 1.2.

Exhibit 1.1 Project Location

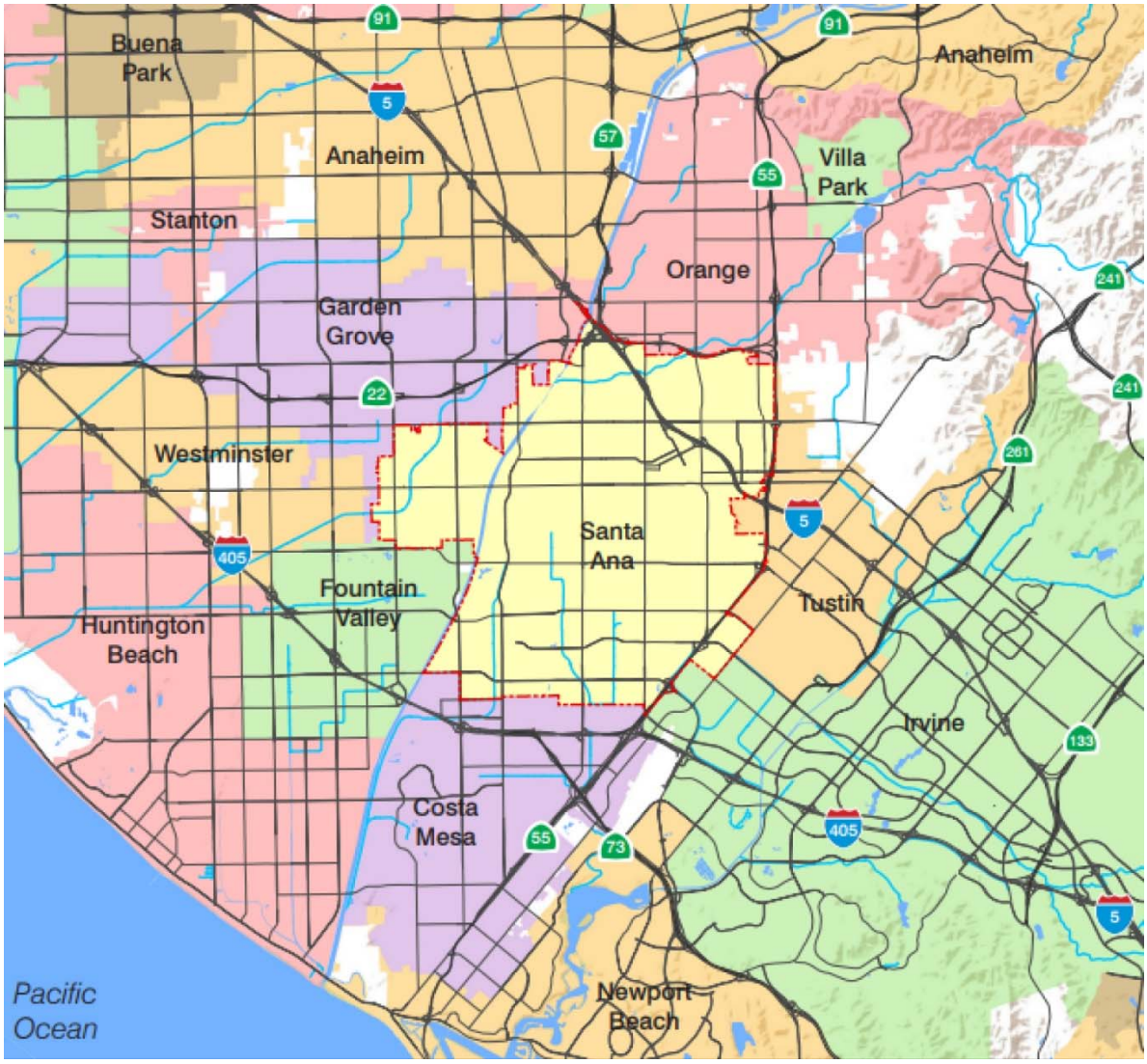
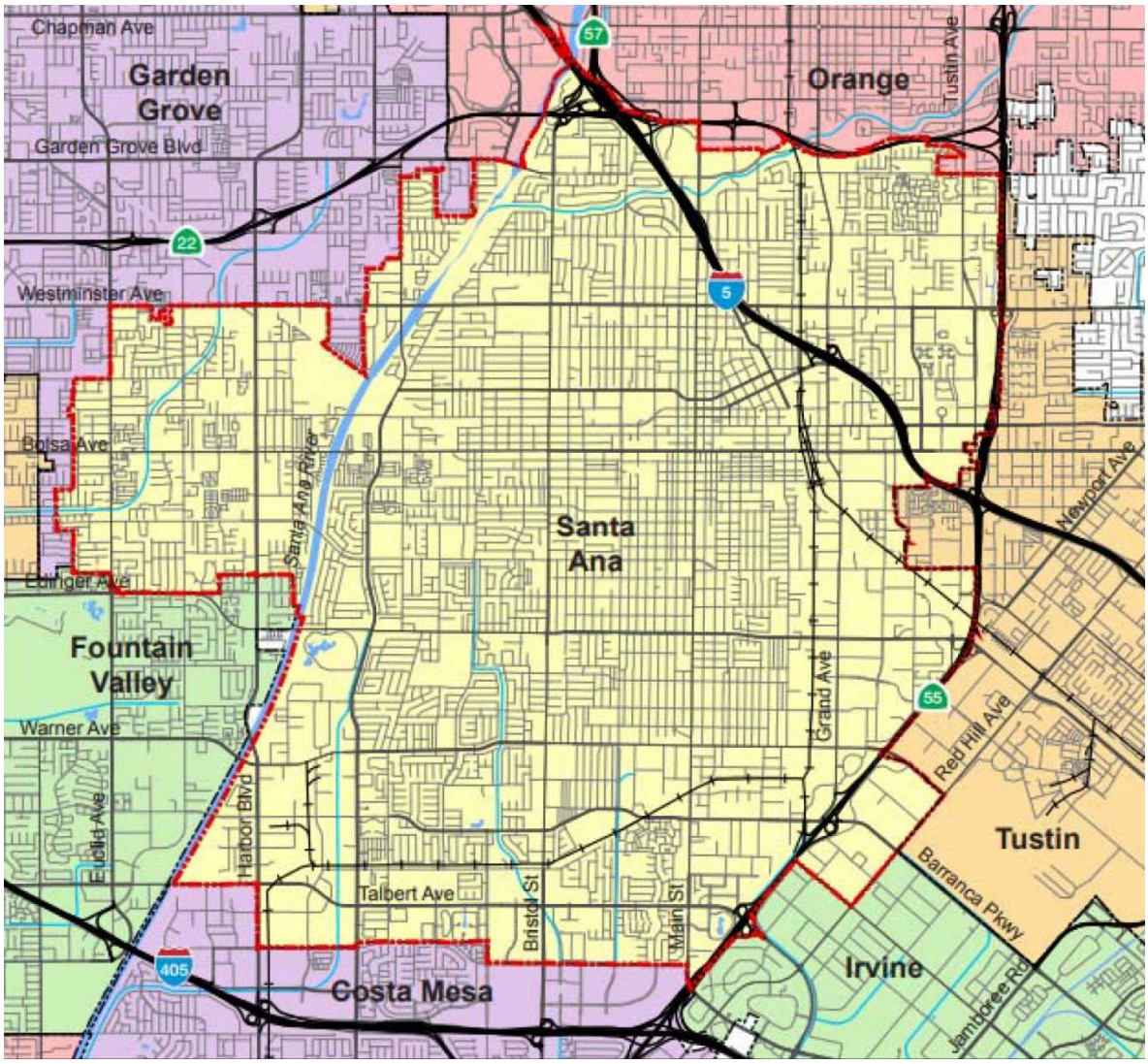


Exhibit 1.2 Project Study Area



2.0 Project Description

This section provides an overview of the project background and traffic study for the General Plan Update. This section also includes a description of the land use and Circulation Element network derived throughout the public outreach and planning process.

2.1 BACKGROUND

The City of Santa Ana's General Plan is comprised of 16 elements, serving as the principal long-range policy and planning document guiding the development, conservation, and enhancement of the City. Each element contains goals and policies related to the physical development of the City. The General Plan was adopted by City Council in 1998 and has been partially amended several times since then. For this analysis, the Land Use and Circulation Elements are of particular interest.

The Land Use Element serves as a long-range guide for land use and development within the City. The primary objective is to manage future growth, minimize land use conflicts, and facilitate growth and development as part of the City's overall vision.

The Circulation Element of the Santa Ana General Plan serves as the City's primary guide for transportation planning. The Circulation Element focuses on roadways and other transportation modes including public transit, railroads, pedestrian and bicycle paths that provide a full range of travel options.

The most recent update to the City of Santa Ana Circulation Element was authorized by City Council in 2011. The project was initiated to update the Circulation Element in its entirety to include the following key components:

- A Complete Streets Policy compliant with the requirements of Assembly Bill (AB) 1358
- OC Streetcar – a proposed streetcar service that would link the Santa Ana Regional Transportation Center (SARTC) with the Civic Center and Garden Grove. The project is currently under construction (at the time this report was written).

Over the last six years, the draft Circulation Element has gone through significant changes, including a new vision, completion of the Santa Ana Safe Mobility Plan, changes to the policy framework and roadway designations, OCTA modeling updates, work by the City on two separate Complete Street studies, advancement of a fixed-guideway/streetcar system and BRT routes, and incorporation of Complete Street Principals.

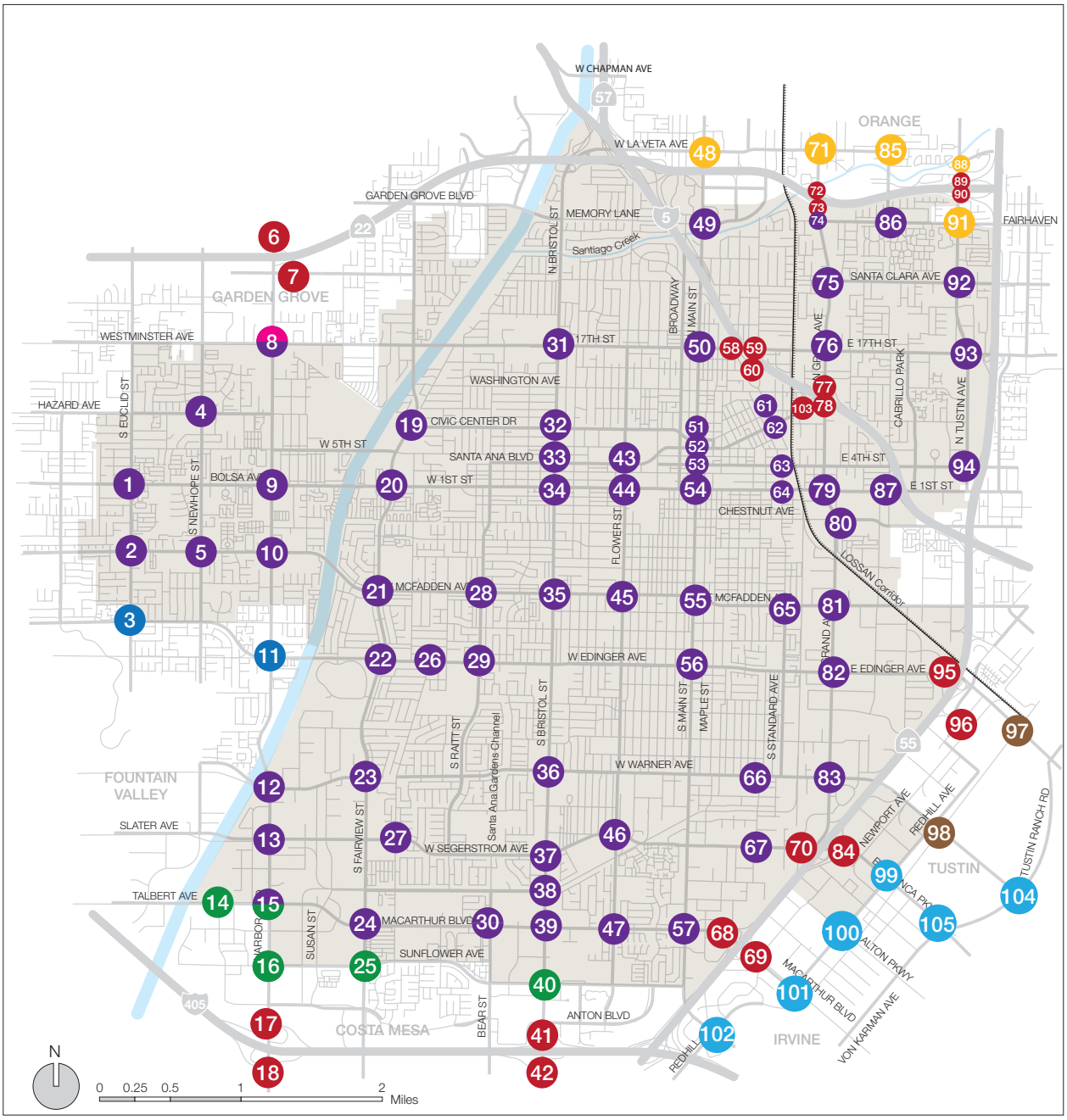
The updates to both the Circulation Element and the EIR are based on a proposed new roadway network scenario, updated OCTA modeling and new modal split, work on the Bikeway Master Plan and Pedestrian Opportunity Area Plan, regulatory changes (e.g., air quality/GHG requirements per 2015 Climate Action Plan, SB743, etc.), integrating related plans (Safe Mobility Santa Ana Plan, and complete street plans (Downtown and Central Santa Ana Transit Zone plans).

2.2 STUDY AREA

As noted, the project consists of changes to both land uses and the transportation network within the City of Santa Ana. The study area was chosen such that the direct and indirect effects of the project could be assessed, within the City of Santa Ana as well as in the adjacent cities. Study intersections were developed based on the locations of the proposed changes as well as in consideration of input provided by OCTA, Caltrans, and neighboring cities during outreach for the Circulation Element Update. Prior to merging with the GPU, the CEU was in the process of evaluating various transportation network

scenarios. The transportation network proposed in this study is a result of those efforts. Study roadway segments and intersections are shown in Exhibit 2.1 and 2.2, respectively.

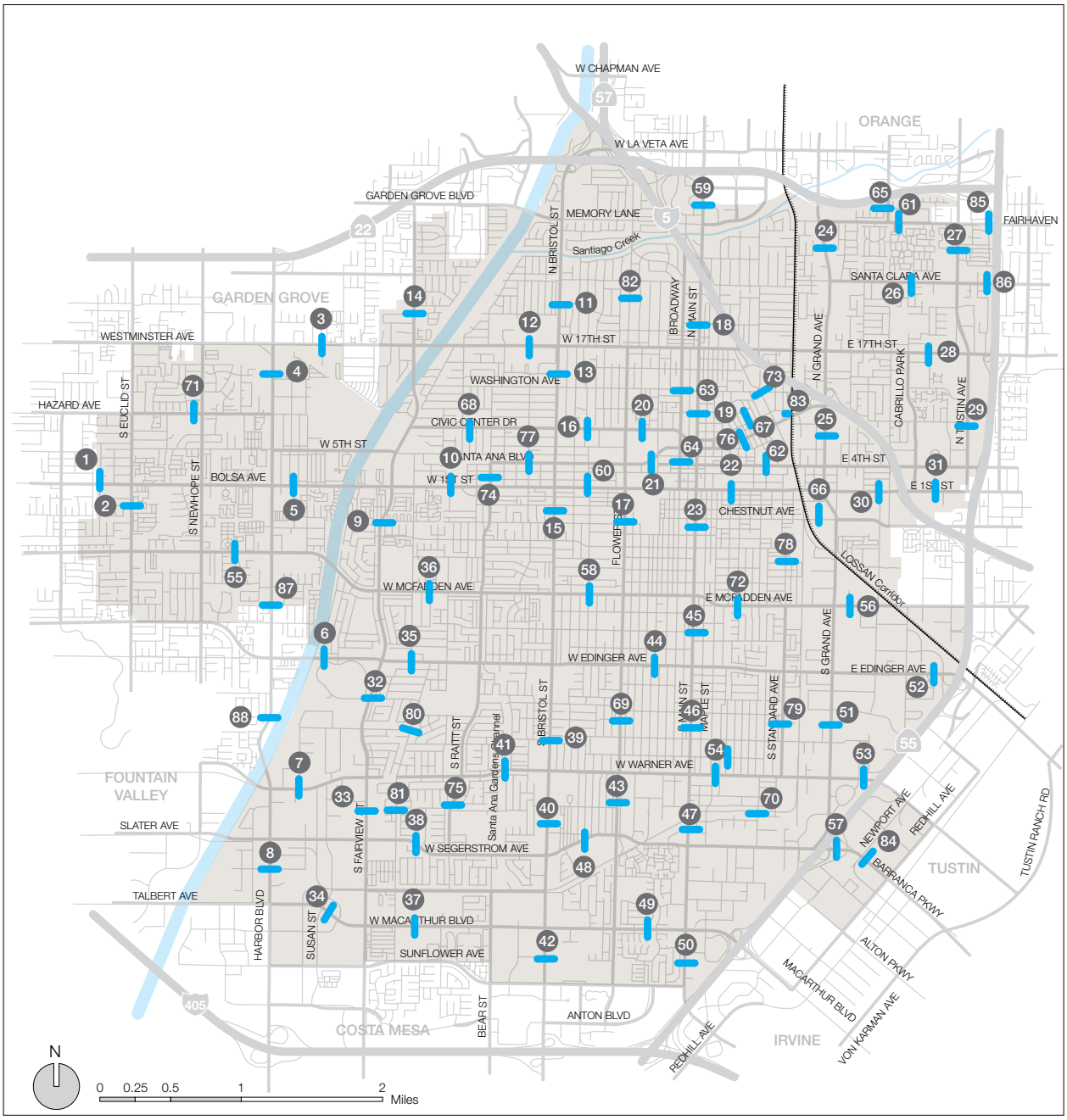
Exhibit 2.1 Study Intersections



LEGEND

- Freeway
- Santa Ana
- Costa Mesa
- Fountain Valley
- Caltrans
- Irvine
- Tustin
- Orange
- Garden Grove

Exhibit 2.2 Study Roadway Segments



LEGEND

Study Segment

2.3 CIRCULATION ELEMENT ROADWAY CLASSIFICATIONS

The Circulation Element contains a Master Plan of Streets and Highways (MPSH) that uses a hierarchy of roadway classifications. Each roadway classification is described by size and function and has specific physical dimensions and characteristics. The MPSH contains regional arterials that connect to the countywide arterial network. OCTA's Master Plan of Arterial Highways (MPAH) is the County's planning document to ensure that the regional arterial highway network is maintained and developed in a coordinated manner between local jurisdictions to support mobility and future growth in the region. Consistency between the MPSH and MPAH is essential in order to maintain a functional regional arterial network and to receive funding from Measure M2 for street improvement projects.

Freeways

Freeways are multi-lane, limited-access, high-volume, high-speed roadways constructed for regional and interregional vehicular travel. Access to these facilities is restricted to interchange ramps at selected roadways along their route. Freeways are under the jurisdiction of the California Department of Transportation (Caltrans).

Principal Arterial

Typically, an eight-lane, divided roadway designed to accommodate between 45,000 to 67,500 vehicle trips daily. The typical right of way width is 144 feet.

Major Arterial

Generally consists of six-travel lanes, and is also divided. Typically, the right-of-way width for this type of roadway is 120 feet. A major arterial is designed to accommodate between 33,900 and 50,600 vehicle trips daily.

Primary Arterial

Generally consists of a four-lane, divided roadway. Typically, the right-of-way width is 100 feet. A primary arterial is designed to accommodate between 22,500 and 33,800 vehicle trips daily.

Secondary Arterial

Generally a four-lane, undivided roadway. The typical right-of-way width for this category of roadway is 80 feet. A secondary arterial is typically designed to accommodate between 15,000 and 22,500 vehicle trips daily.

Divided Collector Arterial

Generally a two-lane roadway with a continuous center two-way left-turn lane. The typical right-of-way width is 80 feet, for the purpose of allocating right-of-way to bicycle and pedestrian use. A divided collector arterial is designed to accommodate up to 22,000 vehicle trips per day.

Collector Street

A two-lane, undivided roadway. The right-of way width for this roadway classification is 60 feet. Collector Streets are also two-lane undivided roadways with a right-of-way width of 56 feet. A divided collector arterial is designed to accommodate up to 11,300 vehicle trips per day.

2.4 SCENARIO SUMMARY

This report presents the baseline analysis results for the existing year (2020) and future year (2045) baseline scenarios as well as future year (2045) project alternative scenario. These scenarios are listed below and described in the following sections:

- Existing Year (2020)
- Future Year (2045) No Project
- Future Year (2045) With Project – Proposed GPU

2.5 EXISTING YEAR (2020)

This scenario is representative of on-the-ground conditions for the transportation network and socio-economic makeup of the city for the year 2020. This scenario serves as a point of reference for future baseline and project alternative scenarios. The network and land use assumptions are as follows:

- The transportation network is reflective of configurations as of April 2020.
- Socio-economic data is representative of December 2019 conditions.

2.6 FUTURE YEAR (2045) NO PROJECT

This scenario serves as the baseline for future year (2045) analysis and consists of the following key assumptions:

- Transportation network and socio-economic data for OCTAM 5.0 Year 2045 Baseline scenario.
- Buildout of roadways consistent with the City of Santa Ana Master Plan of Streets and Highways map (MPSH) as shown in the Circulation Element (1998).
- Buildout of the Orange County Transportation Authority's (OCTA) Master Plan of Arterial Highways (MPAH)
- Freeway and transit improvements considered in the Preferred Alternative of OCTA's LRTP
    - OC Streetcar – The OC Streetcar Project is a proposed streetcar service that would travel between the Santa Ana Regional Transportation Center (SARTC) and Garden Grove. The streetcar is proposed to travel along an exclusive guideway or path on rails that are embedded in the pavement. The streetcar is proposed to travel primarily along Santa Ana Boulevard and along the Pacific Electric Right-of-Way.
    - Bus Rapid Transit (BRT) - These improvements include the Bus Rapid Transit projects along Harbor Boulevard, Bristol Street, and Westminster Avenue/17th Street.
- Modification of mode split for automobile, bicycle, and pedestrians to reflect new bicycle/pedestrian trips.

Exhibit 2.3 presents the current MPSH, which is consistent with the No Project Alternative. Exhibit 2.4 shows current MPAH designations. Exhibit 2.5 shows the Santa Ana Fixed Guideway (OC Streetcar) alignment.

Exhibit 2.3 Master Plan of Streets and Highways (MPSH)

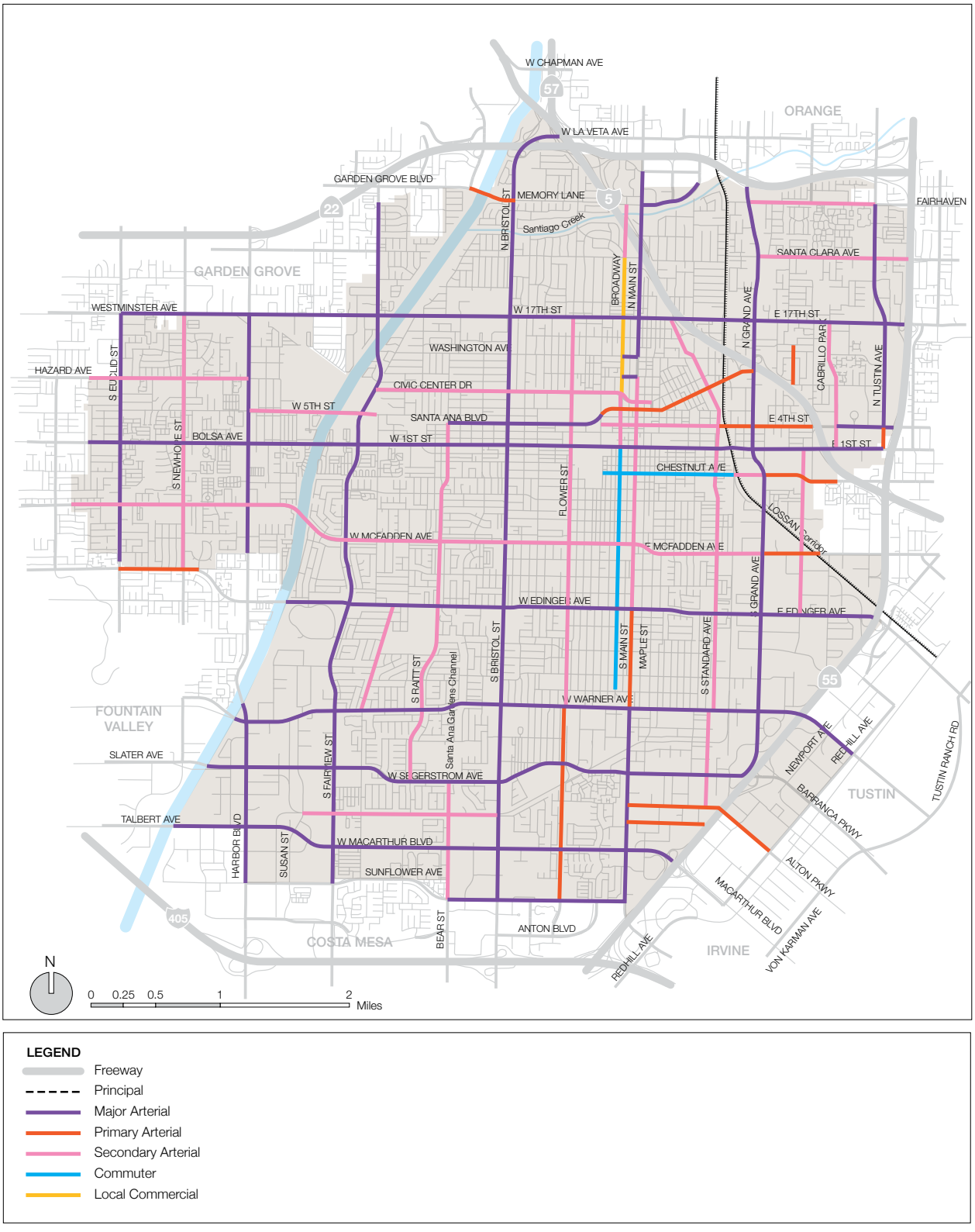


Exhibit 2.4 OCTA Master Plan of Arterial Highways (MPAH)

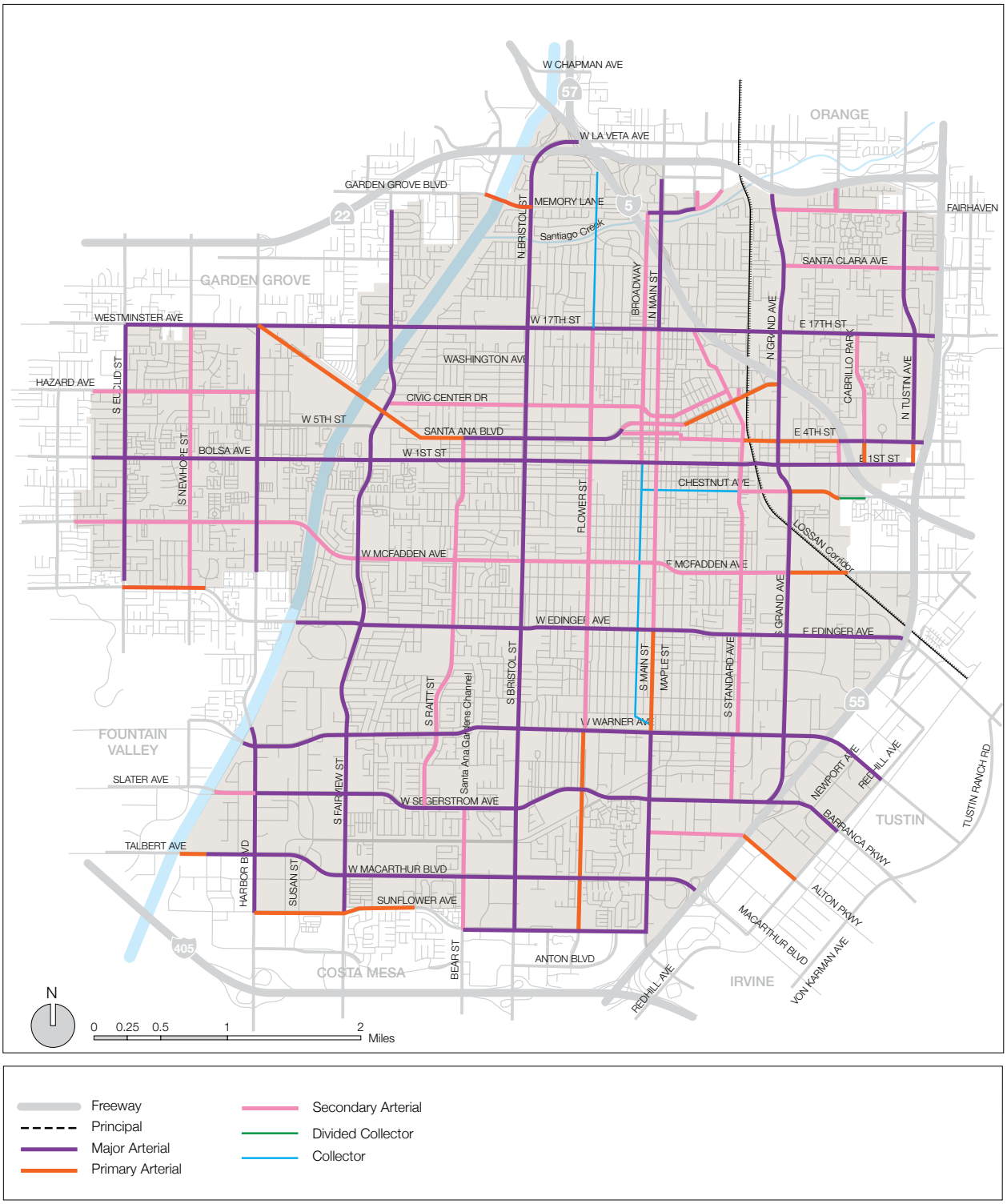


Exhibit 2.5 OC Streetcar Alignment



Source: Orange County Transportation Authority

2.7 FUTURE YEAR (2045) WITH PROJECT

This scenario is based on the Future Year (2045) No Project scenario, with modifications to both the transportation network and socio-economic data. Reclassifications to some roadways are proposed to facilitate the implementation of Complete Streets throughout the City as well as reconcile inconsistencies between the MPSH and MPAH. Additionally, this scenario considers the buildout of an updated Land Use Element for the General Plan.

The transportation network was developed through the evaluation of alternatives in the Circulation Element Update and in consideration of input provided by OCTA, Caltrans, and neighboring cities during outreach. This network is based on that of the No Project scenario, with the addition of the following modifications.

Classify the following streets as Divided Collector Arterials

- Santa Clara between Grand Avenue and SR-55 Freeway (currently Secondary)
- Flower Street between Warner Avenue and First Street (currently Secondary)
- Chestnut Avenue between Standard Avenue and East City Limit (currently Secondary/Primary)
- Raitt Street between Segerstrom and Santa Ana Boulevard (currently Secondary)
- Civic Center Drive between Fairview Street and Bristol Street (currently Secondary)
- Penn Way between I-5 on/off ramps and Washington Avenue (currently a secondary)
- Santiago Street between Washington Avenue and 6th Street (currently a secondary)
- Standard Avenue between 6th Street and Warner Avenue (currently a Secondary Arterial)
- Santa Ana Boulevard between French Street and Santiago Street (currently a Primary Arterial)
- Santa Ana Boulevard between Raitt and Flower (currently a Major)
- Cambridge Street between Fairhaven and SR-22 Freeway (currently Secondary)
- Hazard Avenue between Euclid and Harbor (currently Secondary)
- Halladay Avenue between Warner and Dyer (currently Secondary)
- McFadden Avenue between Harbor Blvd and Grand Ave (currently Secondary)
- Broadway between 1st and 17th (currently Secondary)
- 4th Street between French and Grand (currently Secondary/Primary)
- Fairhaven Avenue from Grand to Tustin (currently Secondary)

Classify the following streets as Primary Arterials

- Santa Ana Boulevard between Flower and Ross (currently a Major)
- 1st Street between Bristol Street and Tustin Avenue (currently Major)

Classify the following streets as Collector Arterials

- Civic Center Drive between French and Santiago (currently a Secondary Arterial)

Add the following streets to the MPAH as Divided Collector Arterials

- Greenville Street between Segerstrom Avenue and Warner Avenue

Add the following streets to the MPAH as Collector Streets

- Greenville Street between Edinger Avenue and Warner Avenue

Remove the following streets from the MPAH

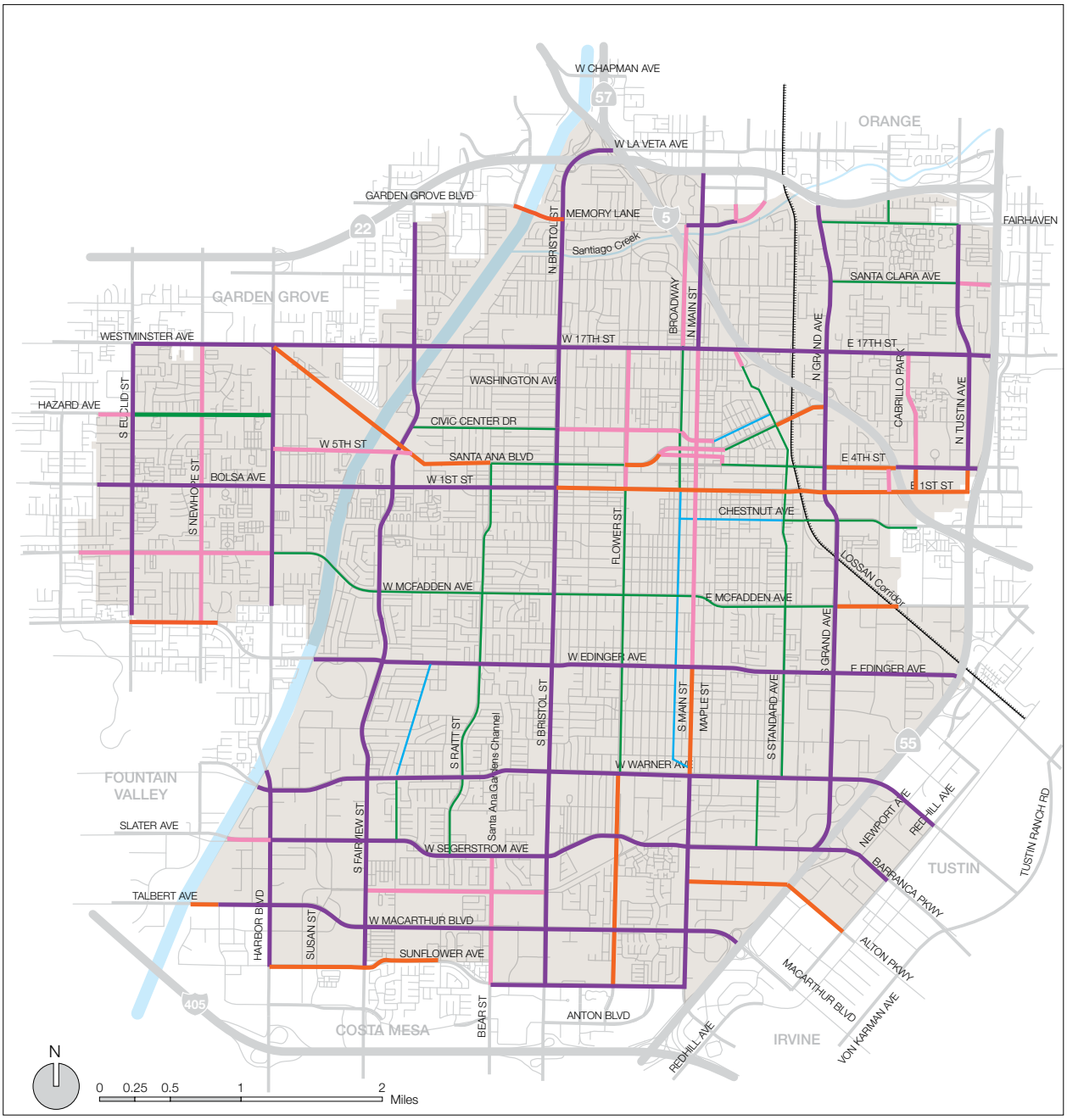
- Flower Street between 17th Street and its northern terminus
- Logan Street between Civic Center Drive and Santa Ana Boulevard

Exhibit 2.6 presents the proposed street reclassifications. Exhibit 2.7 presents the With Project transportation network.

Exhibit 2.6 Proposed Arterial Roadway Reclassifications



Exhibit 2.7 Proposed Santa Ana Circulation Element Transportation Network



LEGEND

- Freeway
- Principal
- Major Arterial
- Primary Arterial
- Secondary Arterial
- Divided Collector
- Collector

3.0 Analysis Methodology

The analysis methodology and impact criteria used in this analysis conform to the California Environmental Quality Act (CEQA) guidelines for a traffic impact analysis. CEQA identifies VMT as the preferred metric for identifying environmental impacts. Although VMT governs CEQA impacts, vehicle LOS is also assessed in order to address local operational performance and impacts. Additionally, roadway performance is assessed in order to monitor conformance with the Orange County Transportation Authority's (OCTA) Master Plan of Arterial Highways (MPAH) operational requirements.

Study intersections and roadway segments were assessed using the method specified by the City Santa Ana, Orange County Congestion Management Program (CMP) guidelines, and other governing agency guidelines. As such, intersections under the jurisdiction of the Cities of Santa Ana, Costa Mesa, Irvine, Tustin, Garden Grove, Fountain Valley, and Orange were assessed using the Intersection Capacity Utilization (ICU) method. Intersections under the jurisdiction of Caltrans were assessed using the Highway Capacity Manual 2000 method (HCM 2000).

3.1 VEHICLE MILES TRAVELED (VMT) ANALYSIS

The VMT analysis was prepared in conformance with the City of Santa Ana VMT Analysis Guidelines (VMT Guidelines; September 2019). The City's VMT Guidelines are based on the Office of Planning and Research's (OPR) Technical Advisory on Evaluating Transportation Impacts in CEQA (December 2018). A VMT analysis is to be conducted for land use and transportation projects that have the potential to increase the average VMT per service population (VMT/SP).

VMT is defined as the total miles traveled by vehicles (within a transportation network). Service population is described as the population generating the VMT of interest. A VMT analysis may be conducted for large-scale projects such as land use plans or individual transportation/development projects. For large-scale projects, it is appropriate to assess VMT impacts based on total VMT/SP. For individual projects, the VMT analysis may consider VMT and VMT/SP based on specific trip types (e.g. work-based trips) and the corresponding population which generates said VMT (e.g. employees).

VMT was generated with data from OCTAM 5.0 using the full-accounting, origin-destination (OD) methodology. The full-accounting, origin-destination methodology considers the VMT generated by all internal-internal (I-I), internal-external (I-X), and external-internal (X-I) trips. These trip types refer to trips that include an origin and/or destination within the study areas (City of Santa Ana and Orange County). For example, an internal-internal trip refers to a trip which begins in the City and also ends in the City; an internal-external trip refers to a trip which begins in the City but ends outside of the City; an external-internal trip refers to a trip which begins outside of the City but ends in the City. It should be noted that while the majority of trips are captured in their entirety, a small portion are cut off at the OCTAM 5.0 boundaries. Thus, while the number of trips to and from the City of Santa Ana which cross the model boundary are known at various locations along major access points (highways, freeways, etc.) the trip length past the OCTAM 5.0 boundary is not.

In order to account for the VMT generated by the portions of trips outside the model boundaries, trip length assumptions were developed. These assumptions are based on the destinations/attractions near the model boundary as well as likely attractions/destinations beyond the immediate developments near the boundary limits. For example, northbound and southbound trips along I-5 at the San Diego County Line were assigned a trip length of 20 miles past the model's southern boundary (the San Diego County Line). This assumption is intended to provide a general account for VMT which considers trips to various locations such as Camp Pendleton, the US-Mexico Border, and various cities within San Diego County. Trip lengths of zero (0) miles are assigned to those trips which are not considered to extend further than 5 miles past the OCTAM boundary; due to the approximate boundary of the model, it would not be appropriate to pursue such a level of accuracy. A summary of the trip length assumptions to/from major OCTAM 5.0 boundary access points is provided below:

- SR-101 near Ventura (20 miles)
- SR-150 near Ojai (0 miles)
- SR-33 near Ventura (0 miles)
- I-5 near Santa Clarita (50 miles)
- SR-14 near Santa Clarita (20 miles)
- I-15 north of I-215 (50 miles)
- I-10 near Banning (30 miles)
- SR-243 south of Banning (10 miles)
- SR-74 east of Hemet (20 miles)
- SR-79 near Temecula (20 miles)
- I-15 near Temecula (0 miles)
- I-5 at San Diego County Line (20 miles)

Due to the nature and scale of the proposed project, the analysis considers the total VMT and service population which consists of the aggregate of total employees and population within the study area. When aggregating employees and residents, an employee reduction factor was applied to account for overlaps in the two (employees who are also residents). Reduction factors were applied to both the City of Santa Ana and County of Orange employees then aggregated to the resident population. Reduction factors are based on employment data within the Southern California Association of Governments (SCAG) Local Profiles Reports (2019) for the City of Santa Ana and Orange County. The SCAG reports show that 20.8% of employees within the City are also residents of the City and that 65.3% of employees within the County are also residents of the County.

VMT and VMT/SP is provided for all scenarios. Existing year (2020) VMT, SP, and VMT/SP values were developed through linear interpolation of 2016 and 2045 baseline OCTAM scenarios. This method was chosen due to the availability of consistent VMT and socio-economic data (SED) used to derive VMT/SP. Although existing year (2020) SED is available, it does not have an accompanying VMT dataset since there is no baseline OCTAM roadway/highway network for existing year (2020) conditions. Therefore, the traffic study uses both interpolated VMT and SED based on OCTAM 2016 baseline and the 2045 forecasts to develop the existing year (2020) values.

Per the City of Santa Ana Traffic Impact Study Guidelines (2019), the project should be screened to determine the need for a full VMT analysis based on the following screening criteria. Firstly, two conditions should be considered:

- Does the project have the potential to reduce VMT/SP?
- Is the project consistent with the Regional Transportation Plan (RTP) / Sustainable Communities Strategy's (SCS)?

If both conditions are met, then a project may be precluded from a full VMT analysis if any of the following criteria are met:

- Projects which serve the local community and have the potential to reduce VMT, such as neighborhood K-12 schools and local-serving retail less than 50,000 square feet (Charter schools are excluded from this criteria.
- Projects that generate less than 110 net daily trips.
- Projects located within Transit Priority Areas (TPAs)
- Projects located in a low-VMT generating Traffic Analysis Zone (TAZ).
    - Proposed land use shall be consistent with the existing land use generating the low VMT/SP.
    - Proposed land use shall be consistent with the RTP/SCS assumptions or decreases VMT/SP compared to RTP/SCS.

The criteria above are presented in a compact manner. Detailed criteria descriptions and supporting material are provided in the City of Santa Ana Traffic Impact Study Guidelines (2019).

VMT impacts are assessed for the Future Year (2045) With Project scenario, based on changes from both the Existing Year (2020) and Future Year (2045) No Project scenarios. VMT impact thresholds for land use plans (such as General Plans and Specific Plans) are listed below:

- Project Level Impact
    - A project is considered to have a significant impact if the project VMT/SP (for the land use plan) exceeds 15% below the existing total daily VMT/SP for the County; or
- Cumulative Impact
    - A project is considered to have a significant impact if the project results in a negative effect on VMT/SP at the citywide level (City VMT/SP is higher with the project than without it.

Where an impact is identified, the project's VMT/SP should be mitigated to be at or less than 15% below the existing Countywide VMT/SP. Mitigation measures may include changes to the project, implementation of transportation demand management (TDM) measures, impact fees, a mitigation exchange, or any other measure approved by the City's Traffic Engineering Division and Planning Department.

3.2 ROADWAY LEVEL OF SERVICE ANALYSIS

Roadway performance was assessed based on the volume-to-capacity (V/C) ratio, as specified in the Santa Ana Circulation Element (1998). The V/C ratio is calculated by dividing the roadway volume by the capacity of the roadway. Roadway capacities are presented in Table 3-1 and resemble those presented in the Santa Ana Circulation Element. The Santa Ana Circulation Element, however, does not provide roadway capacities for the Divided Collector and five-lane divided arterial roadway types. Capacities for the Divided Collector roadway were taken from the Orange County Master Plan of Arterial and Highways (OC MPAH); capacities for the five-lane divided arterial roadway were developed by averaging the capacities of the Major and Primary Arterial roadway types. The capacities shown

correspond to the maximum volume that can be experienced in order to achieve the corresponding level of service (LOS). Therefore, a volume greater than that listed in the column labeled "LOS E" will result in LOS F. The minimum level of service for roadways in the City of Santa Ana is LOS D, per the Circulation Element (1998). The OC MPAH suggests a minimum LOS of C be maintained for roadways, with the intent of maintaining LOS D through intersections. Descriptions of operation and the range of volume-to-capacity ratios for each LOS grade are presented in Table 3-2.

Table 3-1 Roadway Capacity and LOS

CLASSIFICATION	LANES	LOS A	LOS B	LOS C	LOS D	LOS E
Principal Arterial	8D	45,000	52,500	60,000	67,500	75,000
Major Arterial	6D	33,900	39,400	45,000	50,600	56,300
Major / Primary Arterial	5D	28,200	32,850	37,500	42,200	46,900
Primary Arterial	4D	22,500	26,300	30,000	33,800	37,500
Secondary Arterial	4U	15,000	17,500	20,000	22,500	25,000
Divided Collector	2D	9,000	12,000	15,000	20,000	22,000
Collector	2U	7,500	8,800	10,000	11,300	12,500

Sources: City of Santa Ana, General Plan – Circulation Element (1998); Orange County Master Plan of Arterials and Highways (2017).

3.3 INTERSECTION LEVEL OF SERVICE ANALYSIS

Study intersections were assessed using the methodology described in the governing agency's traffic impact analysis guidelines. As such, intersections were assessed using the Intersection Capacity Utilization (ICU) methodology or the Highway Capacity Manual 2000 method (HCM 2000). A description of each as well as the level of service criteria is provided in the following sections.

3.3.1 ICU Methodology

Signalized intersections under the jurisdiction of the Cities of Santa Ana, Costa Mesa, Orange, Irvine, Tustin, and Fountain Valley were assessed using the ICU methodology. The ICU methodology is based on intersection V/C ratio. The V/C ratio value for each movement is the observed volume divided by the saturation flow volume. The intersection ICU value is the sum of the V/C values for the critical movement on each leg, where critical movements are the pairs of conflicting movements with the highest combined V/C values. ICU is expressed as a decimal value (e.g. 0.74), where 1.00 represents the saturated condition where the volume of traffic flow is equal to the capacity. LOS descriptions and their corresponding ranges of V/C ratios are provided in Table 3-2.

This study uses maximum saturation volumes of 1,600 vehicles per hour per lane (VPHPL) for turn lanes and 1,700 VPHPL for through lanes. A 5% loss time per cycle length was applied.

Table 3-2 ADT / ICU Level of Service Descriptions

LOS	V/C	Definition
A	0.00 – 0.60	Free Flow. Individual users are virtually unaffected by the presence of others in the traffic stream. Freedom to select desired speeds and to maneuver within the traffic stream is extremely high. The general level of comfort and convenience provided to the motorist, passenger, or pedestrian is excellent.
B	0.61 – 0.70	Stable flow. The presence of other users in the traffic stream begins to be noticeable. Freedom to select desired speeds is relatively unaffected, but there is a slight decline in the freedom to maneuver within the traffic stream with LOS A. The general level of comfort and convenience provided is somewhat less than that of LOS A, because the presence of others in the traffic stream begins to affect individual behavior.
C	0.71 – 0.80	Stable flow. This LOS marks the beginning of the range of flow in which the operation of individual users becomes significantly affected by interactions with others in the traffic stream. The selection of speed is affected by the presence of others, and maneuvering within the traffic stream requires substantial vigilance on the part of the user. The general level of comfort and convenience declines noticeably at this level.
D	0.81 – 0.90	High density, but stable flow. Speed and freedom to maneuver are severely restricted, and the drive or pedestrian experiences a generally poor level of comfort and convenience. Small increases in traffic flow will generally cause operational problems at this level.
E	0.91 – 1.00	Operating conditions at or near the capacity level. All speeds are reduced to a slow but relatively uniform value. Freedom to maneuver within the traffic stream is extremely difficult, and generally accomplished by forcing a vehicle or pedestrian to "give way" to accommodate such maneuvers. Comfort and convenience levels are extremely poor, and drivers or pedestrian frustration is generally high. Operations at this level are usually unstable because small increases in flow or minor variations within the stream will cause a breakdown.
F	> 1.00	Forced or breakdown flow. This condition exists wherever the amount of traffic approaching a point exceeds the amount which can traverse that point. Queues form up behind such locations as arrival flow exceeds discharge flow.

Source: City of Santa Ana, General Plan – Circulation Element, 1998

3.3.2 Highway Capacity Manual (HCM) Methodology

Study intersections that are Caltrans operated intersections were evaluated using the Highway Capacity Manual 2000 (HCM 2000) Operations methodology consistent with Caltrans guidelines. Chapter 16 of the Highway Capacity Manual (HCM) 2000 contains the operations methodology for signalized intersections, which evaluates LOS based on controlled delay per vehicle. Controlled delay is defined as the portion of the total delay attributed to the traffic signal operation including deceleration delay, queue move-up time, stopped delay, and final acceleration delay. Consistent with HCM 2000 methodology the maximum saturation flow rate for intersections analyzed using HCM 2000 is 1,900 VPHPL. The relationship between controlled delay per vehicle and LOS for signalized intersections is summarized in Table 3-3.

Table 3-3 HCM 2000 Level of Service Descriptions

LOS	Description of Traffic Conditions	Delay (sec/veh)
A	Insignificant delays: no approach phase is fully utilized and no vehicle waits longer than one red indication.	≤ 10
B	Minimal delays: an occasional approach phase is fully utilized. Drivers begin to feel restricted.	> 10 – 20
C	Acceptable delays: major approach phase may become fully utilized. Most drivers feel somewhat restricted.	> 20 – 35
D	Tolerable delays: drivers may wait through more than one red indication. Queues may develop but dissipate rapidly, without excessive delays.	> 35 – 55
E	Significant delays: volumes approaching capacity. Vehicles may wait through several cycles and long vehicle queues form upstream.	> 55 – 80
F	Excessive delays: represents conditions at capacity, with extremely long delays. Queues may block upstream intersections.	> 80

Source: Highway Capacity Manual, Transportation Research Board, 2000.

Unsignalized intersections were evaluated using the methodology described in Chapter 17 of the HCM. The criteria for unsignalized intersections have different threshold values than do those for signalized intersections because drivers expect signalized intersections to carry higher traffic volumes, so higher levels of control delay are acceptable. The relationship between controlled delay per vehicle and LOS for unsignalized intersections is summarized in Table 3-4.

Table 3-4 Level of Service for Unsignalized Intersections

LOS	Control Delay (sec/veh)
A	≤ 10
B	> 10 – 15
C	> 15 – 25
D	> 25 – 35
E	> 35 – 50
F	> 50

Source: Highway Capacity Manual, Transportation Research Board, 2000. Exhibit 17-22

The intersection Level of Service analysis for this report was performed using TRAFFIX software. TRAFFIX is a network-based interactive computer program that enables calculation of levels of service at signalized and unsignalized intersections for multiple locations and scenarios. TRAFFIX also calculates signal timing (green times and cycle lengths) and maximum queue lengths to assist in evaluating signalized intersections.

3.4 INTERSECTION LOS AND SIGNIFICANT IMPACT CRITERIA

3.4.1 Cities of Santa Ana, Costa Mesa, Fountain Valley, and Orange

The minimum LOS for intersections in the Cities of Santa Ana, Costa Mesa, and Fountain Valley is LOS D. Impacts are considered significant if:

- An intersection degrades from an acceptable LOS (LOS D or better) to an unacceptable LOS (LOS E or F) during the peak hours; or
- The project increases traffic demand at the study intersection by 1% of capacity (0.01) if the intersection already operates at an unacceptable level (LOS E or F).

3.4.2 City of Tustin

The minimum LOS for intersections in the City of Tustin is LOS D. Significant impacts are not explicitly outlined, but the preservation of the minimum LOS with and without the project is required. Therefore, a significant impact can be assumed to occur if:

- An intersection is expected to operate at an unacceptable LOS (LOS E or F) in the With Project scenario; an effort to return the intersection LOS to an acceptable level is to be made.

3.4.3 City of Irvine

The minimum LOS for intersections in the City of Irvine is LOS D. Impacts are considered significant if:

- An intersection degrades from an acceptable LOS (LOS D or better) to an unacceptable LOS (LOS E or F) during the peak hours; or
- An intersection is operating at an unacceptable LOS (LOS E or F) and experiences an increase in V/C of 0.02 or more.

3.4.4 Orange County Congestion Management Program

The minimum LOS for intersections part of the Orange County Congestion Management Program (OC CMP) establishes LOS E as the minimum level of operation for CMP roadways (freeways and Smart Streets). Impacts are considered significant if:

- An intersection degrades from an acceptable LOS (LOS E or better) to an unacceptable LOS (LOS F) during the peak hours; or
- The project increases traffic demand at the study intersection by 10% of capacity (0.10) if the intersection already operates at an unacceptable level (LOS F)

OC CMP intersections included in this analysis are:

- INT 6 – Harbor Boulevard and SR-22 WB Off-Ramp / Banner Drive
- INT 9 – Harbor Boulevard and 1st Street
- INT 12 – Harbor Boulevard and Warner Avenue
- INT 17 – Harbor Boulevard and I-405 NB Off-Ramp
- INT 18 – Harbor Boulevard and I-405 SB Off-Ramp
- INT 87 – Mabury Street and 1st Street
- INT 95 – SR-55 SB Ramps / Auto Mall Drive and Edinger Avenue

3.4.5 Caltrans

The minimum LOS for Caltrans intersections is LOS C. Impacts are considered significant if:

- An intersection degrades from an acceptable LOS (LOS C or better) to an unacceptable LOS (LOS D or worse) during the peak hours; or
- Any increase in delay is expected at an intersection already operating at an unacceptable LOS.

3.5 TRAFFIC VOLUMES

This study assesses both roadway and intersection performance for all existing and future year scenarios. Roadway and intersection counts were utilized to determine existing year (2020) average daily traffic (ADT) on study roadway segments and peak hour turning movement counts at study intersections. To determine future year (2045) volumes, an annual growth rate applied to existing traffic volumes was developed through the Orange County Transportation Analysis Model (OCTAM). The traffic volume development procedure is further detailed in the following sections.

3.5.1 Traffic Count Data

Roadway ADT was taken from the City of Santa Ana count database, with counts conducted in September and October 2019. A compound annual growth rate of 1% was applied to the ADT volumes to bring them to levels consistent with those expected in the year 2020.

Intersection turning movement volumes are provided by recent count data and supplemented with historical count data. Updated traffic counts for the year 2020 were in the process of being scheduled and conducted when widespread closures of schools, businesses, and other facilities were ordered due to the onset of the COVID-19 pandemic, disrupting normal traffic patterns. As a result, updated counts were only partially conducted and were supplemented with historic count data. Where they were able to be conducted, updated counts were conducted during the morning peak period (6:00 AM to 9:00 AM) and the afternoon peak period (4:00 PM to 7:00 PM). The AM and PM peak hour analyses are based on the hour of highest total intersection volume during the morning and afternoon peak periods. A compound annual growth rate (CAGR) of 1% was applied to the volumes for traffic counts conducted prior to 2020 to bring them to levels consistent with those expected in the year 2020. The CAGR 1% was selected based on comparisons of historical ADT data for various locations throughout the City. An inventory of intersection count data is provided in Table 3-5.

3.5.2 Travel Demand Forecasting

The horizon year 2045 volumes were derived based on existing peak hour count data and forecast link volumes obtained from the Orange County Transportation Analysis Model (OCTAM) version 5.0. OCTAM 5.0 is the accepted sub-regional model for forecasting travel demand in Orange County, and is based on socioeconomic data from Orange County Projections 2018. Growth factors for each intersection approach and departure were interpolated from OCTAM forecast for the 2016 and 2045 baseline scenarios. These growth factors were then applied to existing counts to forecast future turning movement volumes. OCTAM 5.0 was recently updated by OCTA and this traffic analysis update utilizes this new TransCad version of the model.

Table 3-5 Intersection Count Inventory

ID	INTERSECTION	COUNT YEAR
1	Euclid Street and Bolsa Avenue	2019
2	Euclid Street and McFadden Avenue	2016
3	Euclid Street and Edinger Avenue	2019
4	Newhope Street and Hazard Avenue	2016
5	Newhope Street and McFadden Avenue	2016
6	Harbor Boulevard and SR-22 WB Off-Ramp / Banner Drive	2016
7	Trask Avenue and SR-22 EB On-Ramp	2013
8	Harbor Boulevard and Westminster Avenue	2019
9	Harbor Boulevard and 1st Street	2019
10	Harbor Boulevard and McFadden Avenue	2016
11	Harbor Boulevard and Edinger Avenue	2019
12	Harbor Boulevard and Warner Avenue	2019
13	Harbor Boulevard and Segerstrom Avenue	2016
14	Hyland Avenue / OCTA Bus Base and MacArthur Boulevard	2016
15	Harbor Boulevard and MacArthur Boulevard	2019
16	Harbor Boulevard and Sunflower Avenue	2016
17	Harbor Boulevard and I-405 NB Off-Ramp	2019
18	Harbor Boulevard and I-405 SB Off-Ramp	2019
19	Fairview Street and Civic Center Drive	2016
20	Fairview Street and 1st Street	2018
21	Fairview Street and McFadden Avenue	2016
22	Fairview Street and Edinger Avenue	2019
23	Fairview Street and Warner Avenue	2016
24	Fairview Street and MacArthur Boulevard	2019
25	Fairview Road and Sunflower Avenue	2016
26	Greenville Street and Edinger Avenue	2017
27	Greenville Street and Segerstrom Avenue	2017
28	Raitt Street and McFadden Avenue	2016
29	Raitt Street and Edinger Avenue	2016
30	Bear Street and MacArthur Boulevard	2017
31	Bristol Street and 17th Street	2019
32	Bristol Street and Civic Center Drive	2016

ID	INTERSECTION	COUNT YEAR
33	Bristol Street and Santa Ana Boulevard	2016
34	Bristol Street and 1st Street	2019
35	Bristol Street and McFadden Avenue	2017
36	Bristol Street and Warner Avenue	2019
37	Bristol Street and Segerstrom Avenue	2016
38	Bristol Street and Alton Avenue	2017
39	Bristol Street and MacArthur Boulevard	2017
40	Bristol Street and Sunflower Avenue	2016
41	Bristol Street and I-405 NB Ramps	2018
42	Bristol Street and I-405 SB Ramps	2018
43	Flower Street and Santa Ana Boulevard	2016
44	Flower Street and 1st Street	2017
45	Flower Street and McFadden Avenue	2016
46	Flower Street and Segerstrom Avenue	2016
47	Flower Street and MacArthur Boulevard	2016
48	Main Street and La Veta Avenue	2018
49	Main Street and Mainplace Drive / Memory Lane	2019
50	Main Street and 17th Street	2019
51	Main Street and Civic Center Drive	2016
52	Main Street and Santa Ana Boulevard	2016
53	Main Street and 4th Street	2016
54	Main Street and 1st Street	2019
55	Main Street and McFadden Avenue	2016
56	Main Street and Edinger Avenue	2019
57	Main Street and MacArthur Boulevard	2019
58	Penn Way and 17th Street	2017
59	Santiago Street / I-5 NB Ramps and 17th Street	2016
60	Penn Way and I-5 SB Ramps	2017
61	Santiago Street and Civic Center Drive	2017
62	Santiago Street and Santa Ana Boulevard	2016
63	Standard Avenue and 4th Street	-
64	Standard Avenue and 1st Street	2017
65	Standard Avenue and McFadden Avenue	2018

ID	INTERSECTION	COUNT YEAR
66	Halladay Street and Warner Avenue	2016
67	Halladay Street and Dyer Road	2019
68	SR-55 SB Ramps and MacArthur Boulevard	2019
69	SR-55 NB Ramps and MacArthur Boulevard	2019
70	SR-55 SB Ramps and Dyer Road	2019
71	Glassell Street and La Veta Avenue	2016
72	Glassell Street and SR-22 WB Ramps	2016
73	Glassell Street / Grand Avenue and SR-22 EB Ramps	2016
74	Grand Avenue and Fairhaven Avenue	2016
75	Grand Avenue and Santa Clara Avenue	2017
76	Grand Avenue and 17th Street	2019
77	Grand Avenue and I-5 NB Ramps	2017
78	Grand Avenue and Santa Ana Boulevard	2017
79	Grand Avenue and 1st Street	2019
80	Grand Avenue and Chestnut Avenue	2016
81	Grand Avenue and McFadden Avenue	2016
82	Grand Avenue and Edinger Avenue	2019
83	Grand Avenue and Warner Avenue	2019
84	SR-55 NB Ramps and Dyer Road	2019
85	Cambridge Street and La Veta Avenue	2016
86	Cambridge Street and Fairhaven Avenue	2020
87	Mabury Street and 1st Street	2020
88	Tustin Street and La Veta Avenue	2020
89	Tustin Street and SR-22 WB On-Ramp	2020
90	Tustin Street and SR-22 EB Off-Ramp / Seba Avenue	2020
91	Tustin Street and Fairhaven Avenue	2020
92	Tustin Avenue and Santa Clara Avenue	2020
93	Tustin Avenue and 17th Street	2020
94	Tustin Avenue and 4th Street	2020
95	SR-55 SB Ramps / Auto Mall Drive and Edinger Avenue	2020
96	SR-55 NB Ramps / Del Amo Avenue and Newport Avenue	2020
97	Red Hill Avenue and Edinger Avenue	2020
98	Red Hill Avenue and Warner Avenue	2020

ID	INTERSECTION	COUNT YEAR
99	Red Hill Avenue and Barranca Parkway	2020
100	Red Hill Avenue and Alton Parkway	2020
101	Red Hill Avenue and MacArthur Boulevard	2020
102	Red Hill Avenue and Main Street	2020
103	I-5 SB Ramps and Santa Ana Boulevard	2017
104	Tustin Ranch Road and Warner Avenue	2020
105	Von Karman Avenue and Barranca Parkway	2020

4.0 Existing Year (2020) Conditions

The project study area includes arterial roadways and signalized intersections in the vicinity of the project corridor. Descriptions of geometrical features and intersection level of service analysis results for the year 2020 are included in this section.

4.1 EXISTING ROADWAY NETWORK

Selected arterials that are located in the vicinity of the project corridor are described in this section. Items of note include existing geometry, pedestrian and bicycle facilities, and adjacent land uses.

Euclid Street is classified as a Major Arterial that travels north and south through the study area. Within the study area, there are three travel lanes in each direction with a striped center median functioning as a two-way left-turn lane. Euclid Street is five lanes (2 northbound and 3 southbound) south of Davit Avenue. The posted speed limit is 45 miles per hour. On-street parking is not permitted and no bike facilities are provided. OCTA provides service along this corridor via Route 37 at approximately 30 minute headways.

Newhope Street is classified as a Secondary Arterial that travels north and south on the west end of Santa Ana. Within the study area, there are two travel lanes in each direction with a striped center median functioning as a two-way left-turn lane. The posted speed limit is 40 miles per hour. On-street parking is generally not permitted north of McFadden Avenue, except between 1st Street and 5th Street. Class II bike lanes were recently implemented between Westminster Avenue and McFadden Avenue. There are currently no bus routes that serve this corridor.

Hazard Avenue is classified as a Secondary Arterial that travels east and west through the study area. Within the study area, there are two travel lanes in each direction with a striped center median functioning as a two-way left-turn lane. Hazard Avenue transitions to a Divided Collector west of Harbor Boulevard, and Undivided Collector east of Harbor Boulevard. Limited on-street parking is permitted adjacent to Rosita Park. The posted speed limit is 35 miles per hour and 25 miles per hour near the school zone. No bus routes or bike facilities are provided along this corridor.

McFadden Avenue is classified as a Secondary Arterial (between Euclid Street and Grand Avenue) and a Primary Arterial (east of Grand Avenue) that travels east and west through the study area. Within the study area, there are two travel lanes in each direction with a striped center median functioning as a two-way left-turn lane and a raised, landscaped median on occasion. McFadden Avenue narrows to a two-lane undivided roadway between Bristol Street and Standard Avenue. The posted speed limit is 30-45 miles per hour. On-street parking is permitted along parts of the corridor. Bike lanes are provided west of Harbor Boulevard. OCTA provides service along this corridor via Route 66 and Route 145 at approximately 15-45 minute headways.

Harbor Boulevard is classified as a Major Arterial that travels north and south through the study area. Within the study area, there are three travel lanes in each direction with a raised, landscaped median. The posted speed limit is 40-45 miles per hour. On-street parking is not permitted and limited bike facilities are provided. OCTA provides service along this corridor via Route 43 and Route 543 at approximately 15-20 minute headways.

Fairview Avenue is classified as a Major Arterial that travels north and south through the study area. Within the study area, there are three lanes in each direction with a raised, landscaped median. Fairview Avenue transitions to a five-lane divided roadway then a Primary Arterial north of Civic Center Drive. The posted speed limit is 45 miles per hour. On-street parking is not permitted and bike facilities are not provided. OCTA provides service along this corridor via Route 47 at approximately 15 minute headways.

Civic Center Drive is classified as a Secondary Arterial that travels east and west through the study area. Within the study area, Civic Center Drive is a four-lane undivided roadway; Civic Center Drive becomes a four-lane divided with bike facilities east of Bristol Street until Broadway. It is a four-lane

divided road until Minter Street and then becomes a two-lane undivided. The posted speed limit is 35 miles per hour, unless otherwise noted to be 25 miles per hour in a school zone. On-street parking is allowed along some sections of Civic Center Drive. On-street parking is permitted along certain segments. There are some bike lanes or bike routes provided along this roadway. OCTA provides service along this corridor via Routes 83, 145, and 862 with headways of approximately 30 minutes for each route.

MacArthur Boulevard is classified as a Major Arterial that travels east and west through the study area. Within the study area, there are three travel lanes in each direction with a raised, landscaped median. MacArthur Boulevard is classified as a Primary Arterial west of Hyland Avenue. The posted speed limit is 40 miles per hour. On-street parking is not permitted throughout the corridor and there are currently no bike facilities provided. OCTA operates four bus routes along this corridor via Routes 53, 55, 76 and 173. Headways range between every 10 minutes for Route 53, 30 minutes for Route 55, 60 minutes for Route 76, and 45 minutes for Route 173.

Raitt Street is classified as a Secondary Arterial that travels north and south through the study area. Between MacArthur Boulevard and Edinger Avenue, Raitt Street is a four-lane divided roadway. Between Edinger Avenue and Myrtle Street, Raitt Street is a four-lane undivided roadway. Then from Myrtle Street to 1st Street it is a two-lane divided roadway. Between First Street and Washington Avenue, Raitt Street narrows down to a two-lane undivided roadway. The speed limit is limited to 25 miles per hour in school zones and 35-40 miles per hour outside of the school zone. On-street parking is permitted along the west side of the street between McFadden Avenue and Edinger Avenue and on both sides of the street north of McFadden Avenue to Richland Avenue. Currently bike facilities are not provided along this roadway. OCTA operates Route 145 approximately every 45 minutes during peak operation.

1st Street is classified as a Major Arterial that travels east and west through the study area. Within the study area, there are three lanes in each direction with a raised, landscaped median. The posted speed limit is 40 miles per hour. On-street parking is not permitted along this corridor. There are currently no bike lanes or bike routes provided. OCTA provides service along this corridor via Route 64 at approximately 15 minute headways.

Bristol Street is classified as a Major Arterial that travels north and south through the study area. Within the study area, there are three travel lanes in each direction with a raised, landscaped median. The posted speed limit is 40 miles per hour. Bikeways exist between 17th Street and Edinger Avenue. On-street parking is not permitted along this corridor. OCTA provides service along this corridor via Route 57 at approximately 30 minute headways.

17th Street/ Westminster Avenue is classified as a Major Arterial traveling east and west through the northern region in Santa Ana. There are three lanes in each direction divided by a center median. The posted speed limit is 40 miles per hour. On-street parking is not permitted and there are currently no bike facilities provided along the corridor. Several OCTA bus routes serve this corridor, including Routes 51 and 60 with approximately 15-20 minute headways.

Santa Ana Boulevard is classified as a Major Arterial (between Raitt Street and Ross Street) and a Primary Arterial (between Ross Street and Grand Avenue). The roadway fluctuates between a four-lane undivided roadway (Raitt Street to Flower Street), six-lane divided roadway (Flower Street to Ross Street and Santiago Street to Grand Avenue), and a three-lane one-way roadway (Ross Street to French Street). On-street parking is permitted along certain segments of the corridor. There are currently no bike lanes or bike routes provided. The posted speed limit is 35 miles per hour. Several OCTA bus routes serve this corridor, including Routes 83, 145, and 206 with approximately 30-45 minute headways.

Segerstrom Avenue is classified as a Major Arterial that travels east and west through the study area. Segerstrom Avenue is a six-lane divided roadway between Susan Street and Harbor Boulevard and a four-lane divided roadway along the rest of the corridor. The posted speed limit is 40 miles per hour.

On-street parking is not permitted and there are currently no bike facilities provided along this corridor. There are currently no transit services that operate along Segerstrom Avenue within the study area.

Memory Lane is classified as a Major Arterial that travels east and west through the study area. There are three travel lanes in each direction with a raised, landscaped median. The posted speed limit is 40 miles per hour. On-street parking is not permitted along this roadway. Class II bike lanes are provided between Bristol Street and Flower Street. OCTA provides service along this corridor via Route 453 at approximately 30 minute headways.

Tustin Avenue is classified as a Major Arterial that travels north and south parallel to the SR-55 Freeway. There are three travel lanes in each direction with a striped center median. The posted speed limit is 40 miles per hour. On-street parking is not permitted and bike lanes are not available along Tustin Avenue. Two OCTA bus routes operate along Tustin Avenue. Routes 60 and 71 operate at frequencies of 45 minutes and 1 hour respectively.

Warner Avenue is classified as a Major Arterial that travels east and west through the study area. Within the study area, Warner Avenue is a four-lane undivided roadway. The posted speed limit is 40-45 miles per hour. On-street parking is not permitted and there are currently limited bike lanes provided along the corridor. There are two OCTA bus routes that serve Warner Avenue in the study area. Routes 72 and 463 operate with headways at approximately 20-60 minutes.

Edinger Avenue is classified as a Major Arterial that travels east and west through the study area. The roadway is a four-lane divided roadway between Euclid Street and Main Street and six-lane divided roadway between Main Street and SR-55 freeway. The posted speed limit is 40 miles per hour. On-street parking is not permitted throughout most of the corridor, with the exception of the eastbound direction between Main Street and Maple Street; and between Cedar Street and Evergreen Street. There are bike facilities provided along parts of the corridor. OCTA provides service along Edinger Avenue via Route 70 at approximately 30 minute headways.

Flower Street is classified as a Collector Street (north of 17th Street), Secondary Arterial (between 17th Street and Warner Avenue), and Primary Arterial (south of Warner Avenue) that travels north and south through the study area. The roadway fluctuates between a two-lane divided roadway (Warner Avenue to First Street and 17th Street to Memory Lane) and a four-lane divided roadway (Sunflower Avenue to Warner Avenue and First Street to 17th Street). On-street parking is permitted along residential areas. There are currently no bike facilities provided along the corridor. OCTA provides service along this corridor via Route 51 at approximately 30 minute headways.

Main Street is classified as a Major Arterial (north of Washington Avenue and south of Warner Avenue), Secondary Arterial (between Washington Avenue to Edinger Avenue), and a Primary Arterial (between Edinger Avenue and Warner Avenue) that travels north and south through the study area. There are two travel lanes in each direction, with a striped center median operating as a two-way left-turn lane. The posted speed limit is 35-40 miles per hour. Parking is permitted on the east side of the street between Goetz Avenue and Dyer Road, and prohibited on Main Street through the rest of the study area. South of Dyer Road, Main Street widens to a six-lane facility with a center two-way left-turn lane. OCTA provides service along this corridor via Route 53 at approximately 20 minute headways.

Grand Avenue is classified as a Major Arterial that travels north and south through the study area. From the SR-55 freeway to the northern study area boundary, Grand Avenue is a six-lane major arterial with a center two-way left-turn lane. Approximately 900 feet north of Warner Avenue, Grand Avenue narrows to two lanes in the southbound direction, and the third southbound lane is restored about 300 feet north of Warner Avenue. South of the SR-55 freeway, Grand Avenue has two northbound and three southbound lanes divided by a landscaped median. Parking is not permitted and there are currently no bike lanes or bike routes along Grand Avenue. The posted speed limit is 45 miles per hour. OCTA provides service along this corridor via Route 59 at approximately 30 minute headways.

4.2 STUDY INTERSECTION GEOMETRY AND PEAK HOUR VOLUMES

Intersections were selected in consultation with the City of Santa Ana based on expected traffic impacts and vehicle volumes. The existing study intersection lane geometries are provided in the Appendix. Year 2020 AM peak hour turning movement volumes are shown in Table 4-1, and the PM peak hour volumes are shown in Table 4-2.

Table 4-1 Existing Year 2020 Volumes – AM Peak Hour

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
1	Euclid Street and Bolsa Avenue	124	1044	179	207	1844	104	218	811	153	184	586	188
2	Euclid Street and McFadden Avenue	145	1100	131	137	1906	158	227	463	287	166	368	140
3	Euclid Street and Edinger Avenue	134	845	72	127	1842	169	172	753	460	100	609	111
4	Newhope Street and Hazard Avenue	98	738	105	91	1341	123	78	199	147	51	170	68
5	Newhope Street and McFadden Avenue	110	491	173	119	1183	110	135	492	160	176	449	100
6	Harbor Boulevard and SR-22 WB Off-Ramp / Banner Drive	70	1274	0	0	1354	18	74	0	121	836	44	117
7	Trask Avenue and SR-22 EB On-Ramp	0	0	0	0	0	0	651	815	0	0	738	138
8	Harbor Boulevard and Westminster Avenue	178	1173	251	212	1663	95	176	927	33	212	574	156
9	Harbor Boulevard and 1st Street	117	935	202	227	1914	124	140	932	181	245	621	145
10	Harbor Boulevard and McFadden Avenue	104	1059	101	182	1919	64	167	435	130	190	430	123
11	Harbor Boulevard and Edinger Avenue	103	772	146	183	2105	61	107	644	323	221	491	204
12	Harbor Boulevard and Warner Avenue	115	701	178	457	2059	123	97	1385	364	151	792	134
13	Harbor Boulevard and Segerstrom Avenue	111	791	50	171	2299	86	88	737	297	117	362	82
14	Hyland Avenue / OCTA Bus Base and MacArthur Boulevard	69	3	19	8	2	19	7	1953	775	64	490	20
15	Harbor Boulevard and MacArthur Boulevard	128	898	92	316	1820	126	126	1141	344	133	443	132
16	Harbor Boulevard and Sunflower Avenue	217	1107	205	246	2324	57	9	109	42	130	166	94
17	Harbor Boulevard and I-405 NB Off-Ramp	0	1567	0	0	2327	0	0	0	0	508	0	894
18	Harbor Boulevard and I-405 SB Off-Ramp	0	1729	0	0	1504	0	385	0	429	0	0	0
19	Fairview Street and Civic Center Drive	6	1278	414	243	1402	6	6	23	23	300	9	155
20	Fairview Street and 1st Street	157	1206	227	241	1401	161	214	1233	191	163	732	245
21	Fairview Street and McFadden Avenue	121	1294	173	142	1727	125	221	522	120	194	500	40
22	Fairview Street and	188	831	116	215	1503	127	237	657	176	421	553	110

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
	Edinger Avenue												
23	Fairview Street and Warner Avenue	215	952	125	205	1628	119	69	1010	301	307	926	300
24	Fairview Street and MacArthur Boulevard	248	771	89	307	1546	173	125	1019	150	235	494	140
25	Fairview Road and Sunflower Avenue	185	1071	163	204	1738	111	49	322	65	306	256	132
26	Greenville Street and Edinger Avenue	186	0	251	0	0	0	0	960	234	172	915	0
27	Greenville Street and Segerstrom Avenue	47	193	78	154	472	160	33	739	118	164	662	76
28	Raitt Street and McFadden Avenue	62	570	90	100	762	104	116	704	90	172	542	97
29	Raitt Street and Edinger Avenue	188	501	181	152	716	85	165	1008	145	117	916	54
30	Bear Street and MacArthur Boulevard	81	280	119	199	756	300	97	1663	114	70	1180	100
31	Bristol Street and 17th Street	192	988	167	335	1376	181	324	963	156	359	824	273
32	Bristol Street and Civic Center Drive	123	1087	96	318	1284	80	149	755	93	121	401	83
33	Bristol Street and Santa Ana Boulevard	65	1419	197	137	1507	41	83	403	35	96	240	41
34	Bristol Street and 1st Street	183	1278	162	275	1133	160	206	1268	128	108	813	119
35	Bristol Street and McFadden Avenue	118	1054	53	161	1571	162	275	472	232	155	432	50
36	Bristol Street and Warner Avenue	192	862	159	386	1307	148	206	1149	286	188	860	146
37	Bristol Street and Segerstrom Avenue	65	725	136	283	1011	119	189	913	84	122	554	71
38	Bristol Street and Alton Avenue	9	628	47	227	1571	36	190	117	244	41	87	31
39	Bristol Street and MacArthur Boulevard	92	478	120	326	1419	148	192	1394	283	191	910	132
40	Bristol Street and Sunflower Avenue	94	461	136	245	1388	104	114	942	381	246	400	138
41	Bristol Street and I-405 NB Ramps	0	1489	191	0	1967	8	0	0	34	121	73	665
42	Bristol Street and I-405 SB Ramps	113	1108	0	0	985	847	573	0	524	0	0	0
43	Flower Street and Santa Ana Boulevard	87	811	118	168	568	76	114	710	81	101	323	135
44	Flower Street and 1st Street	139	602	159	175	554	58	165	1514	60	209	861	123
45	Flower Street and McFadden Avenue	60	438	38	66	367	53	51	581	44	64	432	58
46	Flower Street and Segerstrom Avenue	63	331	73	98	544	120	155	1065	297	74	546	60
47	Flower Street and MacArthur Boulevard	22	142	73	163	352	264	165	1879	80	54	909	58
48	Main Street and La Veta Avenue	84	579	300	198	1034	173	350	495	215	316	276	230
49	Main Street and Mainplace Drive / Memory Lane	28	764	250	32	1244	260	148	233	16	198	226	15

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
50	Main Street and 17th Street	128	760	163	195	1086	70	116	1225	61	325	1300	19
51	Main Street and Civic Center Drive	138	901	79	59	1042	159	90	545	99	43	566	34
52	Main Street and Santa Ana Boulevard	68	1083	0	0	1107	98	0	0	0	61	848	65
53	Main Street and 4th Street	0	977	23	0	1098	27	0	93	14	0	114	28
54	Main Street and 1st Street	116	596	60	88	882	65	119	1393	147	88	1097	61
55	Main Street and McFadden Avenue	89	838	42	175	1364	27	42	396	43	140	373	107
56	Main Street and Edinger Avenue	109	617	67	228	1052	55	75	971	131	122	709	87
57	Main Street and MacArthur Boulevard	54	311	264	645	848	192	240	1393	252	146	435	218
58	Penn Way and 17th Street	43	0	217	0	0	0	0	1238	631	214	1722	0
59	Santiago Street / I-5 NB Ramps and 17th Street	768	35	22	57	0	295	99	1040	376	0	1245	20
60	Penn Way and I-5 SB Ramps	0	158	140	799	165	0	0	0	0	179	0	100
61	Santiago Street and Civic Center Drive	206	143	20	5	303	74	121	46	246	72	66	13
62	Santiago Street and Santa Ana Boulevard	24	71	56	311	161	133	48	474	14	108	807	271
63	Standard Avenue and 4th Street	-	-	-	-	-	-	-	-	-	-	-	-
64	Standard Avenue and 1st Street	108	191	119	12	231	25	86	1317	214	64	1012	12
65	Standard Avenue and McFadden Avenue	74	284	128	220	531	37	17	589	71	138	366	82
66	Halladay Street and Warner Avenue	43	0	100	0	0	0	0	1283	145	160	607	0
67	Halladay Street and Dyer Road	11	18	49	183	37	39	41	1375	10	115	626	101
68	SR-55 SB Ramps and MacArthur Boulevard	0	0	0	961	0	870	0	1441	1027	0	1275	145
69	SR-55 NB Ramps and MacArthur Boulevard	908	0	976	0	0	0	0	1609	796	0	517	244
70	SR-55 SB Ramps and Dyer Road	221	26	505	50	242	60	35	1338	285	408	647	108
71	Glassell Street and La Veta Avenue	263	412	125	17	358	79	38	195	375	236	249	6
72	Glassell Street and SR-22 WB Ramps	500	561	0	0	743	346	0	0	0	285	5	426
73	Glassell Street / Grand Avenue and SR-22 EB Ramps	0	818	218	280	733	0	258	0	1012	0	0	0
74	Grand Avenue and Fairhaven Avenue	18	737	106	174	1444	32	99	60	59	254	28	229
75	Grand Avenue and Santa Clara Avenue	66	463	119	290	1418	319	143	233	48	120	231	211
76	Grand Avenue and 17th Street	163	476	327	286	1198	235	224	835	106	309	807	88
77	Grand Avenue and I-5 NB Ramps	0	831	480	58	1869	0	0	0	0	456	0	126
78	Grand Avenue and	71	1003	51	141	1166	1029	263	217	428	5	373	34

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
	Santa Ana Boulevard												
79	Grand Avenue and 1st Street	213	751	35	99	1432	174	332	793	225	247	867	99
80	Grand Avenue and Chestnut Avenue	53	737	163	149	1788	49	60	419	110	118	347	216
81	Grand Avenue and McFadden Avenue	171	722	108	237	1590	155	186	601	182	117	420	106
82	Grand Avenue and Edinger Avenue	47	472	92	224	1146	224	319	1332	128	151	579	126
83	Grand Avenue and Warner Avenue	169	510	218	199	574	194	256	914	359	88	388	131
84	SR-55 NB Ramps and Dyer Road	557	0	456	0	0	0	0	1533	613	0	719	433
85	Cambridge Street and La Veta Avenue	160	204	0	0	480	245	117	0	187	0	0	0
86	Cambridge Street and Fairhaven Avenue	0	0	0	515	0	166	80	216	0	0	247	192
87	Mabury Street and 1st Street	10	0	244	238	141	533	0	1062	11	46	701	0
88	Tustin Street and La Veta Avenue	8	333	96	74	1452	4	9	11	35	447	2	69
89	Tustin Street and SR-22 WB On-Ramp	656	610	0	0	1395	833	0	0	0	0	0	0
90	Tustin Street and SR-22 EB Off-Ramp / Seba Avenue	0	996	5	22	1349	0	244	24	1051	6	0	43
91	Tustin Street and Fairhaven Avenue	89	514	90	235	2026	40	68	258	444	306	338	442
92	Tustin Avenue and Santa Clara Avenue	73	397	74	127	2600	82	101	255	186	118	244	122
93	Tustin Avenue and 17th Street	111	198	236	602	1737	19	187	738	126	488	770	188
94	Tustin Avenue and 4th Street	48	245	219	664	845	262	114	676	28	103	579	376
95	SR-55 SB Ramps / Auto Mall Drive and Edinger Avenue	421	63	498	50	30	8	28	1104	408	399	940	123
96	SR-55 NB Ramps / Del Amo Avenue and Newport Avenue	227	60	38	14	344	504	253	153	146	8	223	4
97	Red Hill Avenue and Edinger Avenue	81	299	85	226	960	450	190	611	130	224	1085	152
98	Red Hill Avenue and Warner Avenue	56	339	46	19	891	179	202	343	253	98	349	62
99	Red Hill Avenue and Barranca Parkway	127	316	104	199	876	115	118	710	281	578	645	126
100	Red Hill Avenue and Alton Parkway	174	512	321	251	1141	183	9	28	28	223	189	106
101	Red Hill Avenue and MacArthur Boulevard	64	569	10	208	537	492	1248	645	106	23	223	571
102	Red Hill Avenue and Main Street	162	556	361	67	313	108	139	1309	254	154	309	73
103	I-5 SB Ramps and Santa Ana Boulevard	0	0	0	367	0	36	323	554	0	0	1130	330
104	Tustin Ranch Road and Warner Avenue	0	0	0	265	0	33	18	238	0	0	1717	395

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
105	Von Karman Avenue and Barranca Parkway	107	142	85	119	1164	509	130	488	163	769	1263	39

Table 4-2 Existing Year 2020 Volumes – PM Peak Hour

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
1	Euclid Street and Bolsa Avenue	159	1477	119	185	1053	199	212	678	106	217	909	217
2	Euclid Street and McFadden Avenue	175	1426	92	110	1354	164	183	419	185	127	532	246
3	Euclid Street and Edinger Avenue	526	1572	112	122	803	204	161	622	186	91	840	136
4	Newhope Street and Hazard Avenue	116	1281	50	49	1038	124	108	214	131	32	202	58
5	Newhope Street and McFadden Avenue	202	1084	190	164	733	142	81	496	72	107	589	168
6	Harbor Boulevard and SR-22 WB Off-Ramp / Banner Drive	88	1992	0	0	1519	49	93	0	78	593	80	170
7	Trask Avenue and SR-22 EB On-Ramp	0	0	0	0	0	0	838	628	0	0	606	57
8	Harbor Boulevard and Westminster Avenue	276	1374	325	207	1430	165	187	667	32	230	797	181
9	Harbor Boulevard and 1st Street	287	1480	164	278	1218	164	188	766	145	230	1073	175
10	Harbor Boulevard and McFadden Avenue	167	1716	125	298	1273	115	214	520	65	211	506	146
11	Harbor Boulevard and Edinger Avenue	282	1773	214	283	822	75	134	564	100	140	813	271
12	Harbor Boulevard and Warner Avenue	774	3713	279	274	1525	182	241	1707	390	260	2700	862
13	Harbor Boulevard and Segerstrom Avenue	299	1881	60	91	1027	91	111	552	181	99	1074	390
14	Hyland Avenue / OCTA Bus Base and MacArthur Boulevard	1201	7	70	12	4	14	36	891	160	25	2350	12
15	Harbor Boulevard and MacArthur Boulevard	588	1519	88	213	1073	176	140	489	181	87	1467	276
16	Harbor Boulevard and Sunflower Avenue	178	1771	263	112	1393	42	67	182	189	254	533	199
17	Harbor Boulevard and I-405 NB Off-Ramp	0	1529	0	0	2475	0	0	0	0	701	0	1132
18	Harbor Boulevard and I-405 SB Off-Ramp	0	1861	0	0	2147	0	157	0	722	0	0	0
19	Fairview Street and Civic Center Drive	5	1430	402	144	1338	0	2	5	4	438	0	162
20	Fairview Street and 1st Street	149	1094	99	203	1264	251	215	870	151	159	1204	186
21	Fairview Street and McFadden Avenue	159	1462	127	155	1257	196	244	659	163	158	545	134
22	Fairview Street and Edinger Avenue	188	1793	110	198	616	151	230	703	188	373	789	173
23	Fairview Street and Warner Avenue	217	1728	161	188	951	96	197	955	148	188	1108	240
24	Fairview Street and MacArthur Boulevard	243	1672	91	171	955	88	280	731	246	179	1365	239
25	Fairview Road and Sunflower Avenue	201	1806	393	144	1196	83	203	430	95	265	664	173

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
26	Greenville Street and Edinger Avenue	327	0	218	0	0	0	0	982	200	95	1128	0
27	Greenville Street and Segerstrom Avenue	57	476	36	61	200	61	108	823	47	27	1037	148
28	Raitt Street and McFadden Avenue	58	835	130	45	474	76	126	747	65	88	742	102
29	Raitt Street and Edinger Avenue	190	816	229	111	384	96	139	961	137	86	972	87
30	Bear Street and MacArthur Boulevard	200	962	251	112	271	82	97	910	94	93	1673	266
31	Bristol Street and 17th Street	236	1148	191	243	1293	220	336	750	136	284	1069	362
32	Bristol Street and Civic Center Drive	200	1399	73	136	1250	128	190	534	63	169	723	82
33	Bristol Street and Santa Ana Boulevard	75	1524	87	105	1402	53	59	163	38	233	371	100
34	Bristol Street and 1st Street	286	1235	182	206	1115	252	189	1062	136	217	1330	126
35	Bristol Street and McFadden Avenue	213	1566	85	84	1171	270	262	512	102	97	504	59
36	Bristol Street and Warner Avenue	309	1157	211	251	838	163	193	868	153	173	1021	299
37	Bristol Street and Segerstrom Avenue	179	1322	233	92	843	176	200	716	57	119	1074	54
38	Bristol Street and Alton Avenue	43	1681	158	119	896	56	75	105	195	91	150	29
39	Bristol Street and MacArthur Boulevard	335	1428	260	223	821	129	357	796	169	265	1536	290
40	Bristol Street and Sunflower Avenue	645	1363	220	201	750	215	293	573	222	263	1012	322
41	Bristol Street and I-405 NB Ramps	0	2094	183	0	2148	20	0	0	194	357	306	1184
42	Bristol Street and I-405 SB Ramps	127	1555	0	0	1506	880	740	0	331	0	0	0
43	Flower Street and Santa Ana Boulevard	124	754	62	104	530	48	134	386	63	146	510	151
44	Flower Street and 1st Street	132	625	149	170	466	135	139	1217	101	210	1415	147
45	Flower Street and McFadden Avenue	42	567	35	46	352	57	64	478	19	48	595	85
46	Flower Street and Segerstrom Avenue	120	843	80	66	369	106	158	786	83	75	1258	81
47	Flower Street and MacArthur Boulevard	133	634	74	85	205	197	209	994	63	62	1989	177
48	Main Street and La Veta Avenue	307	938	439	145	911	235	296	422	189	257	513	206
49	Main Street and Mainplace Drive / Memory Lane	90	945	179	51	1055	139	293	325	87	185	151	73
50	Main Street and 17th Street	197	1019	222	173	757	117	221	1237	73	178	1089	72
51	Main Street and Civic Center Drive	89	1126	94	61	995	63	159	881	105	38	384	39
52	Main Street and Santa Ana Boulevard	54	1229	0	0	1083	53	0	0	0	65	656	85
53	Main Street and 4th Street	0	1055	50	0	1037	47	0	98	55	0	181	46

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
54	Main Street and 1st Street	169	826	72	175	671	143	120	1214	90	114	1275	89
55	Main Street and McFadden Avenue	146	1147	71	134	908	66	78	345	42	85	480	216
56	Main Street and Edinger Avenue	99	1050	48	122	609	83	127	752	84	97	1296	134
57	Main Street and MacArthur Boulevard	488	1169	321	283	368	278	298	613	59	198	1570	456
58	Penn Way and 17th Street	848	0	414	0	0	0	0	1590	513	123	1440	0
59	Santiago Street / I-5 NB Ramps and 17th Street	345	46	18	51	0	133	120	1099	722	0	1525	41
60	Penn Way and I-5 SB Ramps	0	363	238	550	127	0	0	0	0	175	0	150
61	Santiago Street and Civic Center Drive	97	299	30	13	187	48	266	76	269	14	19	18
62	Santiago Street and Santa Ana Boulevard	36	133	84	297	120	61	71	649	20	73	492	187
63	Standard Avenue and 4th Street	-	-	-	-	-	-	-	-	-	-	-	-
64	Standard Avenue and 1st Street	185	367	119	19	189	23	177	1218	129	91	1148	21
65	Standard Avenue and McFadden Avenue	169	667	82	137	300	52	53	351	60	117	712	206
66	Halladay Street and Warner Avenue	169	0	352	0	0	0	2	926	93	141	1121	0
67	Halladay Street and Dyer Road	41	180	208	114	15	32	150	1298	9	45	1451	116
68	SR-55 SB Ramps and MacArthur Boulevard	0	0	0	288	0	697	0	1167	1040	0	1532	602
69	SR-55 NB Ramps and MacArthur Boulevard	641	0	426	0	0	0	0	711	731	0	1501	1081
70	SR-55 SB Ramps and Dyer Road	319	10	240	103	226	110	65	1523	207	556	991	64
71	Glassell Street and La Veta Avenue	472	490	160	15	499	37	82	268	461	155	280	11
72	Glassell Street and SR-22 WB Ramps	557	910	0	0	878	255	0	0	0	201	7	357
73	Glassell Street / Grand Avenue and SR-22 EB Ramps	0	1202	307	381	681	0	229	4	503	0	0	0
74	Grand Avenue and Fairhaven Avenue	46	1214	243	222	894	82	79	45	33	255	48	181
75	Grand Avenue and Santa Clara Avenue	88	1185	246	220	868	220	159	194	36	136	244	260
76	Grand Avenue and 17th Street	246	877	221	270	582	128	346	886	149	300	1107	201
77	Grand Avenue and I-5 NB Ramps	0	1297	1001	48	1164	0	0	0	0	143	0	172
78	Grand Avenue and Santa Ana Boulevard	167	1888	31	34	750	532	328	200	316	20	134	97
79	Grand Avenue and 1st Street	165	1261	38	120	706	282	277	918	91	151	941	173
80	Grand Avenue and Chestnut Avenue	140	1050	182	149	664	112	93	285	62	107	333	194
81	Grand Avenue and McFadden Avenue	202	1150	198	138	626	168	128	476	83	86	686	151

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
82	Grand Avenue and Edinger Avenue	242	1397	121	165	408	229	201	868	70	74	1333	173
83	Grand Avenue and Warner Avenue	242	873	193	146	471	217	342	656	142	151	964	316
84	SR-55 NB Ramps and Dyer Road	188	0	51	0	0	0	0	1339	681	0	1545	965
85	Cambridge Street and La Veta Avenue	191	266	0	0	196	181	219	0	144	0	0	0
86	Cambridge Street and Fairhaven Avenue	0	0	0	219	0	76	141	246	0	0	273	340
87	Mabury Street and 1st Street	25	0	256	207	88	390	0	1512	24	40	549	0
88	Tustin Street and La Veta Avenue	46	1033	245	90	801	15	13	5	20	183	5	112
89	Tustin Street and SR-22 WB On-Ramp	532	1529	0	0	711	365	0	0	0	0	0	0
90	Tustin Street and SR-22 EB Off-Ramp / Seba Avenue	0	1547	4	52	707	0	458	41	581	28	0	67
91	Tustin Street and Fairhaven Avenue	239	1275	113	327	862	99	80	180	147	94	208	196
92	Tustin Avenue and Santa Clara Avenue	163	789	121	183	1444	100	61	192	113	125	157	148
93	Tustin Avenue and 17th Street	216	926	476	346	371	37	444	872	118	229	753	419
94	Tustin Avenue and 4th Street	55	598	194	459	339	124	180	642	33	88	596	367
95	SR-55 SB Ramps / Auto Mall Drive and Edinger Avenue	377	30	398	110	50	37	35	1258	354	385	1003	120
96	SR-55 NB Ramps / Del Amo Avenue and Newport Avenue	583	174	34	8	97	373	104	36	21	23	348	16
97	Red Hill Avenue and Edinger Avenue	159	1146	182	145	289	234	327	838	102	62	803	392
98	Red Hill Avenue and Warner Avenue	382	1308	150	58	223	283	222	603	94	25	525	69
99	Red Hill Avenue and Barranca Parkway	356	1339	345	185	313	147	203	753	99	127	966	282
100	Red Hill Avenue and Alton Parkway	27	1499	217	124	528	4	132	324	166	433	74	267
101	Red Hill Avenue and MacArthur Boulevard	125	809	10	286	594	1042	597	275	42	39	818	574
102	Red Hill Avenue and Main Street	416	897	242	74	538	292	198	705	191	279	1589	89
103	I-5 SB Ramps and Santa Ana Boulevard	0	0	0	278	0	61	561	634	0	0	668	183
104	Tustin Ranch Road and Warner Avenue	0	0	0	653	0	45	54	1795	0	0	548	236
105	Von Karman Avenue and Barranca Parkway	224	1290	281	67	343	236	560	1023	143	204	646	220

4.3 EXISTING YEAR (2020) VEHICLE MILES TRAVELED (VMT)

Table 4-3 presents the VMT analysis results for the existing year (2020) scenario. VMT calculations are based on the scenario parameters and methodology as described in Sections 2.5 and 3.1, respectively.

Table 4-3 Existing Year (2020) VMT Summary

	I – I VMT	I – X VMT	X – I VMT	Total VMT	Service Population	VMT/SP
City	697,779	5,356,504	5,352,841	11,407,124	507,924	22.5
County	48,242,429	25,463,887	25,637,825	99,344,141	3,834,949	25.9

SP = Service Population = Total Employees + Total Population. Reduction factor applied to Total Employees as noted in Section 3.1 and shown in Appendix F. I-I = Internal-Internal; I-X = Internal-External; X-I = External-Internal.

4.4 EXISTING YEAR (2020) ROADWAY SEGMENT LEVEL OF SERVICE

Table 4-4 includes the 24-hour count locations, volumes, and corresponding roadway segment level of service. The following study roadway segments currently operate at a deficient LOS:

- Segment 4 – Harbor Boulevard from Westminster Avenue/17th Street to Hazard Avenue
- Segment 11 – Bristol Street from 17th Street to Santa Clara Avenue
- Segment 13 – Bristol Street from 17th Street to Washington Avenue
- Segment 14 – Fairview Street from Trask Avenue to 17th Street
- Segment 17 – Flower Street from 1st Street to Bishop Street
- Segment 23 – Main Street from 1st Street to Bishop Street
- Segment 25 – Grand Avenue from Santa Ana Boulevard to 4th Street
- Segment 39 – Bristol Street from Edinger Avenue to Warner Avenue
- Segment 44 – Edinger Avenue from Flower Street to Main Street
- Segment 45 – Main Street from McFadden Avenue to Edinger Avenue
- Segment 57 – Dyer Road from Grand Avenue to Pullman Street
- Segment 58 – McFadden Avenue from Bristol Street to Flower Street
- Segment 72 – McFadden Avenue from Main Street to Standard Avenue
- Segment 88 – Harbor Boulevard from Edinger Avenue to Warner Avenue

Table 4-4 Existing Year (2020) Roadway Segment LOS

ID	Street	Segment	Lanes	ADT	LOS
1	1st Street	Euclid Street to Ward Street	6D	25,233	A
2	Euclid Street	1st Street to McFadden Avenue	6D	40,731	C
3	Westminster Avenue	Harbor Boulevard to Fairview Street	6D	30,459	A
4	Harbor Boulevard	Westminster Avenue/17th Street to Hazard Avenue	6D	54,137	E
5	1st Street	Harbor Boulevard to Jackson	6D	32,736	A
6	Edinger Avenue	Harbor Boulevard to Fairview Street	4D	27,838	C
7	Warner Avenue	Harbor Boulevard to Fairview Street	6D	31,945	A
8	Harbor Boulevard	Segerstrom Avenue to MacArthur Boulevard	6D	15,622	A
9	Fairview Street	1st Street to Willits Street	6D	42,605	C
10	1st Street	Sullivan Street to Raitt Street	6D	36,377	B
11	Bristol Street	17th Street to Santa Clara Avenue	4D	45,676	F
12	17th Street	College Avenue to Bristol Street	6D	37,345	B
13	Bristol Street	17th Street to Washington Avenue	4D	42,005	F
14	Fairview Street	Trask Avenue to 17th Street	4D	40,432	F
15	Bristol Street	1st Street to Bishop Street	6D	42,663	C
16	Civic Center Drive	Bristol Street to Flower Street	4D	17,589	A
17	Flower Street	1st Street to Bishop Street	2U	15,622	F
18	Main Street	17th Street to 20th Street	4D	32,044	D
19	Main Street	Washington Street to Civic Center Drive	4D	33,489	D
20	Civic Center Drive	Flower Street to Ross Street	4D	17,427	A
21	Santa Ana Boulevard	Flower Street to Ross Street	6D	14,689	A
22	1st Street	Main Street to Standard Avenue	6D	42,699	C
23	Main Street	1st Street to Bishop Street	4U	30,125	F
24	Grand Avenue	Santa Clara Avenue to Fairhaven Street	4D	30,206	D
25	Grand Avenue	Santa Ana Boulevard to 4th Street	4D	36,678	E
26	Santa Clara Avenue	Grand Avenue to Tustin Avenue	2U	10,585	D
27	Tustin Avenue	Santa Clara Avenue to Fairhaven Street	6D	35,410	B
28	17th Street	Cabrillo Park Drive to Tustin Avenue	4D	32,080	D
29	Tustin Avenue	Fruit Street to 4th Street	6D	25,174	A
30	1st Street	Grand Avenue to Elk Lane	6D	28,638	A
31	1st Street	Cabrillo Park Drive to Tustin Avenue	6D	22,083	A
32	Fairview Street	Edinger Avenue to Harvard Street	6D	37,524	B
33	Fairview Street	Warner Avenue to Segerstrom Avenue	6D	39,878	C
34	MacArthur Boulevard	Harbor Boulevard to Fairview Street	6D	26,235	A
35	Edinger Avenue	Fairview Street to Greenville Street	4D	29,115	C
36	McFadden Avenue	Fairview Street to Raitt Street	4D	20,997	A
37	MacArthur Boulevard	Fairview Street to Raitt Street	6D	28,809	A
38	Segerstrom Avenue	Fairview Street to Raitt Street	4D	19,326	A
39	Bristol Street	Edinger Avenue to Warner Avenue	4D	37,238	E
40	Bristol Street	Warner Avenue to Segerstrom Avenue	6D	38,007	B

ID	Street	Segment	Lanes	ADT	LOS
41	Warner Avenue	Raitt Street to Bristol Street	5D	34,555	C
42	Bristol Street	MacArthur Boulevard to Sunflower Avenue	6D	34,731	B
43	Flower Street	Warner Avenue to Segerstrom Avenue	4D	15,378	A
44	Edinger Avenue	Flower Street to Main Street	4D	36,534	E
45	Main Street	McFadden Avenue to Edinger Avenue	4U	28,622	F
46	Main Street	Edinger Avenue to Warner Avenue	4D	27,972	C
47	Main Street	Warner Avenue to Dyer Road	5D	30,484	B
48	Segerstrom Avenue	Bristol Street to Flower Street	4D	22,959	B
49	MacArthur Boulevard	Flower Street to Main Street	6D	37,946	B
50	Main Street	MacArthur Boulevard to Sunflower Avenue	6D	23,692	A
51	Grand Avenue	Edinger Avenue to Warner Avenue	6D	17,735	A
52	Edinger Avenue	Richie Street to Newport Avenue	6D	40,435	C
53	Warner Avenue	Grand Avenue to Red Hill Avenue	6D	22,435	A
54	Warner Avenue	Main Street to Standard Avenue	6D	27,391	A
55	McFadden Avenue	Newhope Street to Harbor Boulevard	4D	18,495	A
56	McFadden Avenue	Standard Avenue to Grand Avenue	4D	20,188	A
57	Dyer Road	Grand Avenue to Pullman Street	6D	51,084	E
58	McFadden Avenue	Bristol Street to Flower Street	2U	14,951	F
59	Main Street	La Veta Avenue to Memory Lane	6D	31,004	A
60	1st Street	Bristol Street to Flower Street	6D	39,006	B
61	Fairhaven Avenue	Grand Avenue to Tustin Avenue	4D	10,218	A
62	4th Street	French Street to Standard Avenue	2D	14,855	C
63	Broadway	17th Street to Civic Center Drive	4D	20,630	A
64	Broadway	Civic Center Drive to 1st Street	4U	16,005	B
65	Cambridge Street	SR-22 to Fairhaven Avenue	2U	- *	-
66	Chestnut Avenue	Standard Avenue to Lyon Street	4U	2,587	A
67	Civic Center Drive	French Street to Santiago Street	2U	10,266	D
68	Civic Center Drive	Fairview Road to Bristol Street	4U	13,772	A
69	Flower Street	Edinger Avenue to Warner Avenue	2U	10,964	D
70	Halladay Avenue	Warner Avenue to Dyer Road	2D	- *	-
71	Hazard Avenue	Euclid Street to Harbor Boulevard	4U	9,546	A
72	McFadden Avenue	Main Street to Standard Avenue	2U	16,064	F
73	Penn Way/Santiago Street	I-5 SB Ramps to Santa Ana Boulevard	4U	- *	-
74	Raitt Street	Santa Ana Boulevard to 1st Street	2U	9,978	C
75	Raitt Street	Warner Avenue to Segerstrom Avenue	4D	14,746	A
76	Santa Ana Boulevard	French Street to Santiago Street	2D	6,616	A
77	Santa Ana Boulevard	Raitt Street to Bristol Street	4U	4,236	A
78	Standard Avenue	Chestnut Avenue to McFadden Avenue	4U	12,871	A
79	Standard Avenue	Edinger Avenue to Warner Avenue	4U	12,852	A
80	Greenville Street	Edinger Avenue to Warner Avenue	2U	7,090	A
81	Greenville Street	Warner Avenue to Segerstrom Avenue	4D	8,578	A
82	Flower Street	17th Street to Northern Terminus	2D	11,155	B

ID	Street	Segment	Lanes	ADT	LOS
83	Logan Street	Civic Center Drive to Santa Ana Boulevard	2U	- *	-
84	Dyer Road	Pullman Street to Red Hill Avenue	6D	31,248	A
85	Fairhaven Avenue	Tustin Street to Yorba Street	4U	16,381	B
86	Santa Clara Avenue	Tustin Street to Yorba Street	2U	8,568	B
87	Harbor Boulevard	McFadden Avenue to Edinger Avenue	6D	49,281	D
88	Harbor Boulevard	Edinger Avenue to Warner Avenue	6D	52,363	E

Notes:
* Not available.
(1) ADT – Average Daily Traffic Volumes
(2) LOS – Level of Service
(3) #D – total number of lanes in both directions divided by raised or striped median; #U – total number of undivided lanes in both directions
(4) Roadway capacities shown in Table 3-1.
(5) Yellow and red shading indicates an unacceptable level of service.

4.5 EXISTING YEAR (2020) INTERSECTION LEVEL OF SERVICE

A summary of the AM and PM peak hour level of service analysis results for the existing year (2020) existing condition are included in Table 4-5. Intersections are general considered unacceptable if the level of service (LOS) is “E” or “F”. It should be noted that Caltrans and CMP intersection have a minimum acceptable LOS of C and E, respectively. The following intersections are forecast to have unacceptable level of service in the existing year (2020) condition:

- INT 2 – Euclid street and McFadden Avenue (City of Santa Ana)
- INT 12 – Harbor Boulevard and Warner Avenue (City of Santa Ana – CMP)
- INT 13 – Harbor Boulevard and Segerstrom Avenue (City of Santa Ana)
- INT 14 – Hyland Avenue and MacArthur Boulevard (City of Costa Mesa / City of Santa Ana)
- INT 37 – Bristol Street and Segerstrom Avenue (City of Santa Ana)
- INT 45 – Flower Street and McFadden Avenue (City of Santa Ana)
- INT 47 – Flower Street and MacArthur Boulevard (City of Santa Ana)
- INT 82 – Grand Avenue and Edinger Avenue (City of Santa Ana)
- INT 91 – Tustin Street and Fairhaven Avenue (City of Orange)

Table 4-5: Existing Year (2020) LOS Summary

ID	INTERSECTION	AM		PM	
		V/C or Delay	LOS	V/C or Delay	LOS
1	Euclid Street and Bolsa Avenue	0.85	D	0.87	D
2	Euclid Street and McFadden Avenue	0.91	E	0.83	D
3	Euclid Street and Edinger Avenue	0.86	D	0.79	C
4	Newhope Street and Hazard Avenue	0.71	C	0.64	B
5	Newhope Street and McFadden Avenue	0.83	D	0.84	D
6	Harbor Boulevard and SR-22 WB Off-Ramp / Banner Drive	27.4	C	23.6	C
7	Trask Avenue and SR-22 EB On-Ramp	11.5	B	12.4	B
8	Harbor Boulevard and Westminster Avenue	0.82	D	0.79	C
9	Harbor Boulevard and 1st Street	0.81	D	0.83	D
10	Harbor Boulevard and McFadden Avenue	0.79	C	0.81	D
11	Harbor Boulevard and Edinger Avenue	0.77	C	0.70	B
12	Harbor Boulevard and Warner Avenue	0.81	D	1.58	F
13	Harbor Boulevard and Segerstrom Avenue	0.98	E	0.90	D
14	Hyland Avenue / OCTA Bus Base and MacArthur Boulevard	0.61	B	0.92	E
15	Harbor Boulevard and MacArthur Boulevard	0.75	C	0.82	D
16	Harbor Boulevard and Sunflower Avenue	0.70	B	0.78	C
17	Harbor Boulevard and I-405 NB Off-Ramp	19.7	B	23.7	C
18	Harbor Boulevard and I-405 SB Off-Ramp	13.2	B	19.4	B
19	Fairview Street and Civic Center Drive	0.67	B	0.66	B
20	Fairview Street and 1st Street	0.87	D	0.88	D
21	Fairview Street and McFadden Avenue	0.79	C	0.80	C
22	Fairview Street and Edinger Avenue	0.75	C	0.81	D
23	Fairview Street and Warner Avenue	0.85	D	0.85	D
24	Fairview Street and MacArthur Boulevard	0.70	B	0.83	D
25	Fairview Road and Sunflower Avenue	0.71	C	0.71	C
26	Greenville Street and Edinger Avenue	0.56	A	0.59	A
27	Greenville Street and Segerstrom Avenue	0.73	C	0.69	B
28	Raitt Street and McFadden Avenue	0.72	C	0.72	C
29	Raitt Street and Edinger Avenue	0.85	D	0.86	D
30	Bear Street and MacArthur Boulevard	0.74	C	0.87	D
31	Bristol Street and 17th Street	0.73	C	0.69	B
32	Bristol Street and Civic Center Drive	0.83	D	0.83	D
33	Bristol Street and Santa Ana Boulevard	0.67	B	0.66	B
34	Bristol Street and 1st Street	0.75	C	0.78	C
35	Bristol Street and McFadden Avenue	0.75	C	0.75	C
36	Bristol Street and Warner Avenue	0.77	C	0.79	C
37	Bristol Street and Segerstrom Avenue	0.79	C	0.91	E
38	Bristol Street and Alton Avenue	0.56	A	0.69	B
39	Bristol Street and MacArthur Boulevard	0.74	C	0.81	D

ID	INTERSECTION	AM		PM	
		V/C or Delay	LOS	V/C or Delay	LOS
40	Bristol Street and Sunflower Avenue	0.64	B	0.69	B
41	Bristol Street and I-405 NB Ramps	19.1	B	28.2	C
42	Bristol Street and I-405 SB Ramps	22.3	C	18.4	B
43	Flower Street and Santa Ana Boulevard	0.60	A	0.57	A
44	Flower Street and 1st Street	0.80	C	0.75	C
45	Flower Street and McFadden Avenue	0.82	D	0.92	E
46	Flower Street and Segerstrom Avenue	0.77	C	0.90	D
47	Flower Street and MacArthur Boulevard	0.70	B	0.91	E
48	Main Street and La Veta Avenue	0.57	A	0.63	B
49	Main Street and Mainplace Drive / Memory Lane	0.49	A	0.47	A
50	Main Street and 17th Street	0.82	D	0.73	C
51	Main Street and Civic Center Drive	0.76	C	0.80	C
52	Main Street and Santa Ana Boulevard	0.67	B	0.61	B
53	Main Street and 4th Street	0.49	A	0.54	A
54	Main Street and 1st Street	0.76	C	0.80	C
55	Main Street and McFadden Avenue	0.90	D	0.85	D
56	Main Street and Edinger Avenue	0.83	D	0.85	D
57	Main Street and MacArthur Boulevard	0.74	C	0.78	C
58	Penn Way and 17th Street	11.2	B	20.7	C
59	Santiago Street / I-5 NB Ramps and 17th Street	31.2	C	19.5	B
60	Penn Way and I-5 SB Ramps	19.0	B	22.0	C
61	Santiago Street and Civic Center Drive	16.9	C	17.1	C
62	Santiago Street and Santa Ana Boulevard	0.55	A	0.57	A
63*	Standard Avenue and 4th Street	-	-	-	-
64	Standard Avenue and 1st Street	0.80	C	0.84	D
65	Standard Avenue and McFadden Avenue	0.61	B	0.68	B
66	Halladay Street and Warner Avenue	0.66	B	0.68	B
67	Halladay Street and Dyer Road	0.57	A	0.78	C
68	SR-55 SB Ramps and MacArthur Boulevard	19.8	B	17.3	B
69	SR-55 NB Ramps and MacArthur Boulevard	19.2	B	14.8	B
70	SR-55 SB Ramps and Dyer Road	23.7	C	25.7	C
71	Glassell Street and La Veta Avenue	0.60	A	0.62	B
72	Glassell Street and SR-22 WB Ramps	28.5	C	24.8	C
73	Glassell Street / Grand Avenue and SR-22 EB Ramps	27.0	C	26.5	C
74	Grand Avenue and Fairhaven Avenue	0.68	B	0.68	B
75	Grand Avenue and Santa Clara Avenue	0.89	D	0.75	C
76	Grand Avenue and 17th Street	0.80	C	0.83	D
77	Grand Avenue and I-5 NB Ramps	12.9	B	10.0	A
78	Grand Avenue and Santa Ana Boulevard	23.8	C	21.1	C
79	Grand Avenue and 1st Street	0.80	C	0.79	C

ID	INTERSECTION	AM		PM	
		V/C or Delay	LOS	V/C or Delay	LOS
80	Grand Avenue and Chestnut Avenue	0.71	C	0.62	B
81	Grand Avenue and McFadden Avenue	0.83	D	0.76	C
82	Grand Avenue and Edinger Avenue	0.76	C	0.91	E
83	Grand Avenue and Warner Avenue	0.53	A	0.70	B
84	SR-55 NB Ramps and Dyer Road	16.3	B	5.7	A
85	Cambridge Street and La Veta Avenue	17.3	C	12.5	B
86	Cambridge Street and Fairhaven Avenue	0.56	A	0.58	A
87	Mabury Street and 1st Street	28.3	C	26.1	C
88	Tustin Street and La Veta Avenue	0.52	A	0.44	A
89	Tustin Street and SR-22 WB On-Ramp	12.9	B	12.1	B
90	Tustin Street and SR-22 EB Off-Ramp / Seba Avenue	24.2	C	23.7	C
91	Tustin Street and Fairhaven Avenue	0.98	E	0.61	B
92	Tustin Avenue and Santa Clara Avenue	0.87	D	0.65	B
93	Tustin Avenue and 17th Street	0.75	C	0.72	C
94	Tustin Avenue and 4th Street	0.70	B	0.69	B
95	SR-55 SB Ramps / Auto Mall Drive and Edinger Avenue	24.4	C	25.1	C
96	SR-55 NB Ramps / Del Amo Avenue and Newport Avenue	24.0	C	27.4	C
97	Red Hill Avenue and Edinger Avenue	0.57	A	0.67	B
98	Red Hill Avenue and Warner Avenue	0.49	A	0.59	A
99	Red Hill Avenue and Barranca Parkway	0.58	A	0.56	A
100	Red Hill Avenue and Alton Parkway	0.52	A	0.67	B
101	Red Hill Avenue and MacArthur Boulevard	0.66	B	0.66	B
102	Red Hill Avenue and Main Street	0.67	B	0.77	C
103	I-5 SB Ramps and Santa Ana Boulevard	17.4	B	18.9	B
104	Tustin Ranch Road and Warner Avenue	0.48	A	0.61	B
105	Von Karman Avenue and Barranca Parkway	0.76	C	0.87	D

Notes: (1) LOS – Level of Service; (2) V/C – Volume to Capacity
*Intersection does not currently exist; assessed in future scenarios only.

5.0 Future Year 2045 Forecast Modeling

5.1 ORANGE COUNTY TRANSPORTATION AUTHORITY MODEL (OCTAM)

The Orange County Transportation Authority (OCTA) is responsible for regional transportation planning in Orange County. OCTA's role as the regional planning agency is to plan for and evaluate multi-modal transportation alternatives to support regional planning activities. The Orange County Transportation Authority Model (OCTAM) is OCTA's regional model that is based on the traditional four-step sequential modeling methodology with "feedback loops" procedures to insure internal modeling consistency. The model incorporates multi-modal analytical capabilities to analyze the following modes of travel: local and express bus transit, urban rail, commuter rail, toll roads, carpools, truck traffic, as well as non-motorized transportation which includes pedestrian and bicycle trips. The model responds to changes in land use types, household characteristics, transportation infrastructure, and travel costs such as transit fares, parking costs, tolls, and auto operating costs.

5.2 PEAK HOUR FORECASTS

The future year 2040 volumes were derived based on existing peak hour count data and forecast link volumes obtained from the Orange County Transportation Analysis Model (OCTAM 5.0). OCTAM 5.0 (TransCad) is the accepted sub-regional model for forecasting travel demand for Orange County. Growth factors for each intersection approach and departure were interpolated from OCTAM 5.0 link plots for the baseline 2016 and 2045 scenarios. These growth factors were then applied to existing counts to forecast future turning movement volumes at each of the study intersections.

5.2.1 Turning Movement Development

It should be noted that Intersection #63 (Standard Avenue and 4th Street) does not currently exist, but is assumed to be constructed in the future year (2045) scenarios. Turning movement volumes for this intersection were developed through use of a post-processor. Because this intersection may affect local travel patterns, turning movement volumes for two adjacent intersections were also developed using this method. These intersections are #62 (Santiago Street and Santa Ana Boulevard) and #64 (Standard Avenue and 1st Street).

The process of converting link level traffic volumes to turning movement volumes is called "post processing." The fundamental theory behind the post-processor is the belief that with proper calibration, travel demand models provide good forecasts at the link level. Turning level forecasts are obtainable from the model, but the reliability of the forecast breaks down at this level of detail, and the abstraction of reality into numerical methods becomes apparent. The frictionless nature of left and right turns in the travel demand model allow them to occur as easily as through movements, which does not represent a realistic situation.

IBI Group has developed a spreadsheet-based postprocessor that uses existing turning movement volumes or split percentages along with forecast link-level data to determine future turn movement volumes at selected intersections. For intersections where existing data is not available (#63), assumed split percentages of 50% for the through movements, 25% for the right turn movements, and 25% for the left turn movements are used.

The postprocessor generates another solution to the possible combination of turning movements at a given intersection. There are numerous approaches to post processing. The IBI Group postprocessor utilizes a procedure documented in Chapter 8 of the National Cooperative Highway Research Program Report 255. It uses an iterative process to balance the intersection throughput volumes, which provides the best combination of reasonable turning movement volumes and traceability of results. Most importantly, the model link volumes (especially the approach intersection volumes) are maintained

when using this methodology. The "post processing" method can be summarized into the following steps:

1. If existing peak hour turning movement volumes are not available, calculate a base set of turning volumes from the known/assumed splits and the peak hour approach link volumes.
2. Calculate the inbound and outbound volumes for each leg of the intersection from the initial set of turning volumes.
3. Multiply the inbound volumes by the ratio of the forecast inbound volume to the calculated inbound volume for each leg. The calculated inbound volumes should now match the forecast inbound volumes.
4. Multiply the new volumes by the ratio of the forecast outbound volume to the calculated outbound volume for each leg. The calculated outbound volumes should now match the forecast outbound volumes for each leg, but the calculated inbound volumes may no longer be equal to the forecast inbound volumes.
5. Compare the calculated approach and departure volumes with the forecast approach and departure volumes.
6. If the deviation is within the tolerance level, the solution is obtained.
7. If the deviation is not within the tolerance level, repeat steps 3 and 4.
8. Repeat this process until the tolerance is met.

5.2.2 Roadway Segment Forecasts

Forecast roadway segment volumes were also derived based on existing average daily traffic (ADT) volumes and forecast ADT link volumes obtained from OCTAM 5.0. Growth factors for each study roadway segment were interpolated from OCTAM 5.0 link plots for the 2016 and 2045 baseline scenarios. The growth factors were then applied to existing ADT counts to forecast future roadway segment ADT.

5.2.3 Santa Ana Fixed Guideway (OC Streetcar)

As described in Section 2.0 of this report, the OC Streetcar is a proposed streetcar service that would travel primarily along Santa Ana Boulevard and the Pacific Electric Right-of-Way (PEROW) between the Santa Ana Regional Transportation Center (SARTC) and Garden Grove (4 miles long). The streetcar is proposed to travel along an exclusive guideway or path on rails that are embedded in the pavement. The streetcar would adhere to the same signal operations as on-street automobiles for most locations where Transit Signal Priority is not implemented. Santa Ana Boulevard is reclassified between Raitt Street and Shelton Street from a four lane undivided roadway to a two lane roadway (divided with a center left turn lane or raised median with left turn pockets. The project also proposes to reconfigure portions of Santa Ana Boulevard between Raitt Street and SARTC to accommodate a bike lane on the north side of the street throughout the alignment as well as 14-inch high platforms. The Fixed Guideway service is proposed to run every 10 to 15 minutes from approximately 6:00 AM to 11:00 PM. Within OCTAM, the Fixed Guideway is modeled as ‘bus transit,’ which would translate to six extra trips assigned during the peak hour in the direction of operations.

In 2016, the OCTA Board conditionally approved the following additional MPAH amendments:

- Fourth Street from French Street to Grand Avenue – Secondary to Divided Collector
- Santa Ana Boulevard from Flower Street to Ross – Major to Primary
- Santa Ana Boulevard from French Street to Santiago Street – Primary to Divided Collector
- Santa Ana Boulevard from Raitt Street to Flower Streer – Major to Divided Collector

6.0 Future Year (2045) No Project

This scenario is used as the point of comparison for the network alternatives and consists of the key elements presented in Section 2.6.

This scenario will serve as a base for comparison and establish impacts of the proposed With Project network. The Future Year 2045 No Project intersection geometries are the same as existing. The future year 2045 No Project volumes for the AM peak hour are shown in Table 6-1 and the PM peak hour volumes are shown in Table 6-2.

Table 6-1 Future Year (2045) No Project Volumes – AM Peak Hour

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
1	Euclid Street and Bolsa Avenue	103	870	149	200	1785	101	252	937	177	190	606	194
2	Euclid Street and McFadden Avenue	162	1227	146	148	2062	171	255	521	323	151	335	127
3	Euclid Street and Edinger Avenue	127	801	68	168	2442	224	160	700	428	125	761	139
4	Newhope Street and Hazard Avenue	78	590	84	88	1302	119	108	276	204	119	397	159
5	Newhope Street and McFadden Avenue	77	342	120	122	1213	113	135	492	160	190	484	108
6	Harbor Boulevard and SR-22 WB Off-Ramp / Banner Drive	84	1534	0	0	1465	19	81	0	133	836	44	117
7	Trask Avenue and SR-22 EB On-Ramp	0	0	0	0	0	0	713	893	0	0	852	159
8	Harbor Boulevard and Westminster Avenue	178	1173	251	258	2021	115	192	1010	36	415	1124	306
9	Harbor Boulevard and 1st Street	95	756	163	236	1989	129	138	919	178	196	497	116
10	Harbor Boulevard and McFadden Avenue	104	1059	101	190	2004	67	177	462	138	138	311	89
11	Harbor Boulevard and Edinger Avenue	103	772	146	204	2349	68	123	743	373	191	424	176
12	Harbor Boulevard and Warner Avenue	109	665	169	429	1933	116	101	1440	379	138	723	122
13	Harbor Boulevard and Segerstrom Avenue	107	759	48	169	2274	85	82	689	278	140	434	98
14	Hyland Avenue / OCTA Bus Base and MacArthur Boulevard	86	4	24	8	2	19	7	1934	767	68	517	21
15	Harbor Boulevard and MacArthur Boulevard	131	920	94	295	1701	118	131	1183	357	128	427	127
16	Harbor Boulevard and Sunflower Avenue	221	1128	209	233	2204	54	13	153	59	139	178	101
17	Harbor Boulevard and I-405 NB Off-Ramp	0	1915	0	0	1978	0	0	0	0	399	0	702
18	Harbor Boulevard and I-405 SB Off-Ramp	0	1963	0	0	1253	0	475	0	529	0	0	0
19	Fairview Street and Civic Center Drive	7	1569	508	332	1915	8	7	25	25	100	3	52
20	Fairview Street and 1st Street	200	1538	289	245	1426	164	200	1153	179	120	539	181

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
21	Fairview Street and McFadden Avenue	141	1510	202	154	1878	136	221	522	120	147	380	30
22	Fairview Street and Edinger Avenue	200	883	123	223	1558	132	361	1002	268	421	553	110
23	Fairview Street and Warner Avenue	222	981	129	194	1543	113	68	991	295	267	807	261
24	Fairview Street and MacArthur Boulevard	261	811	94	290	1460	163	121	984	145	235	494	140
25	Fairview Road and Sunflower Avenue	185	1071	163	191	1627	104	49	322	65	357	299	154
26	Greenville Street and Edinger Avenue	186	0	251	0	0	0	0	1821	444	153	813	0
27	Greenville Street and Segerstrom Avenue	47	193	78	154	472	160	46	1035	165	190	768	88
28	Raitt Street and McFadden Avenue	67	618	98	123	938	128	122	743	95	146	461	82
29	Raitt Street and Edinger Avenue	197	524	189	163	770	91	324	1980	285	122	954	56
30	Bear Street and MacArthur Boulevard	71	245	104	199	756	300	92	1581	108	62	1041	88
31	Bristol Street and 17th Street	251	1292	218	487	2001	263	324	963	156	368	845	280
32	Bristol Street and Civic Center Drive	133	1178	104	403	1628	101	165	836	103	127	421	87
33	Bristol Street and Santa Ana Boulevard	69	1502	209	153	1684	46	171	831	72	185	462	79
34	Bristol Street and 1st Street	205	1430	181	315	1298	183	200	1231	124	90	678	99
35	Bristol Street and McFadden Avenue	125	1117	56	204	1986	205	309	531	261	262	729	84
36	Bristol Street and Warner Avenue	221	993	183	600	2033	230	165	919	229	231	1058	180
37	Bristol Street and Segerstrom Avenue	70	778	146	283	1011	119	280	1354	125	134	607	78
38	Bristol Street and Alton Avenue	11	739	55	230	1591	36	164	101	211	41	87	31
39	Bristol Street and MacArthur Boulevard	102	531	133	329	1434	150	166	1203	244	159	758	110
40	Bristol Street and Sunflower Avenue	111	546	161	242	1370	103	112	926	375	295	480	166
41	Bristol Street and I-405 NB Ramps	0	1638	210	0	1918	8	0	0	36	145	87	796
42	Bristol Street and I-405 SB Ramps	125	1225	0	0	1030	886	696	0	636	0	0	0
43	Flower Street and Santa Ana Boulevard	100	936	136	214	723	97	211	1315	150	143	458	191
44	Flower Street and 1st Street	139	602	159	334	1058	111	148	1355	54	209	861	123
45	Flower Street and McFadden Avenue	70	511	44	118	655	95	112	1278	97	113	764	103
46	Flower Street and Segerstrom Avenue	86	451	100	98	544	120	223	1531	427	104	764	84
47	Flower Street and MacArthur Boulevard	26	170	88	167	361	270	151	1716	73	62	1049	67
48	Main Street and	105	724	375	198	1034	173	361	510	222	316	276	230

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
	La Veta Avenue												
49	Main Street and Mainplace Drive / Memory Lane	34	920	301	36	1386	290	185	291	20	132	151	10
50	Main Street and 17th Street	133	788	169	237	1322	85	127	1336	67	356	1425	21
51	Main Street and Civic Center Drive	142	925	81	55	970	148	120	727	132	43	566	34
52	Main Street and Santa Ana Boulevard	66	1050	0	0	1135	100	0	0	0	90	1250	96
53	Main Street and 4th Street	0	939	22	0	961	24	0	70	11	0	140	34
54	Main Street and 1st Street	103	532	54	77	772	57	106	1241	131	86	1072	60
55	Main Street and McFadden Avenue	93	874	44	162	1259	25	86	815	89	164	438	126
56	Main Street and Edinger Avenue	99	561	61	211	975	51	96	1248	168	108	630	77
57	Main Street and MacArthur Boulevard	51	295	250	539	709	160	230	1336	242	138	411	206
58	Penn Way and 17th Street	52	0	260	0	0	0	0	1341	684	214	1722	0
59	Santiago Street / I-5 NB Ramps and 17th Street	720	33	21	54	0	279	89	932	337	0	1245	20
60	Penn Way and I-5 SB Ramps	0	260	230	951	196	0	0	0	0	196	0	110
61	Santiago Street and Civic Center Drive	487	338	47	10	590	144	182	69	369	108	99	20
62	Santiago Street and Santa Ana Boulevard	188	611	303	409	972	253	118	732	99	297	699	258
63	Standard Avenue and 4th Street	191	473	248	431	604	332	298	624	219	250	550	340
64	Standard Avenue and 1st Street	45	631	312	123	901	41	161	1,959	121	194	1,282	120
65	Standard Avenue and McFadden Avenue	94	360	162	226	547	38	34	1178	142	150	397	89
66	Halladay Street and Warner Avenue	95	0	220	0	0	0	0	1833	207	260	986	0
67	Halladay Street and Dyer Road	15	24	65	450	91	96	46	1537	11	104	566	91
68	SR-55 SB Ramps and MacArthur Boulevard	0	0	0	998	0	903	0	1349	961	0	1187	135
69	SR-55 NB Ramps and MacArthur Boulevard	946	0	1017	0	0	0	0	1468	726	0	517	244
70	SR-55 SB Ramps and Dyer Road	286	34	654	50	242	60	37	1422	303	348	552	92
71	Glassell Street and La Veta Avenue	263	412	125	14	295	65	57	293	563	367	387	9
72	Glassell Street and SR-22 WB Ramps	511	574	0	0	1017	473	0	0	0	323	6	483
73	Glassell Street / Grand Avenue and SR-22 EB Ramps	0	918	245	373	977	0	270	0	1058	0	0	0
74	Grand Avenue and Fairhaven Avenue	21	840	121	207	1,718	38	99	60	59	213	24	192
75	Grand Avenue and Santa Clara Avenue	75	528	136	345	1,687	380	143	233	48	200	385	352
76	Grand Avenue and 17th Street	186	543	373	340	1,426	280	198	739	94	342	894	98

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
77	Grand Avenue and I-5 NB Ramps	0	1056	610	90	2893	0	0	0	0	507	0	140
78	Grand Avenue and Santa Ana Boulevard	49	694	35	202	1670	1473	268	221	436	6	475	43
79	Grand Avenue and 1st Street	208	732	34	99	1432	174	436	1041	295	216	759	87
80	Grand Avenue and Chestnut Avenue	53	737	163	161	1937	53	34	239	63	130	384	239
81	Grand Avenue and McFadden Avenue	181	764	114	249	1672	163	205	663	201	123	440	111
82	Grand Avenue and Edinger Avenue	47	472	92	243	1244	243	347	1447	139	189	724	158
83	Grand Avenue and Warner Avenue	142	429	184	183	528	178	333	1188	467	92	406	137
84	SR-55 NB Ramps and Dyer Road	610	0	499	0	0	0	0	1618	647	0	599	361
85	Cambridge Street and La Veta Avenue	185	235	0	0	480	245	273	0	436	0	0	0
86	Cambridge Street and Fairhaven Avenue	0	0	0	567	0	183	80	216	0	0	195	152
87	Mabury Street and 1st Street	6	0	157	248	147	554	0	1230	13	38	579	0
88	Tustin Street and La Veta Avenue	7	301	87	67	1307	4	9	11	35	307	1	47
89	Tustin Street and SR-22 WB On-Ramp	614	571	0	0	1171	699	0	0	0	0	0	0
90	Tustin Street and SR-22 EB Off-Ramp / Seba Avenue	0	953	5	17	1049	0	275	27	1182	6	0	43
91	Tustin Street and Fairhaven Avenue	101	586	103	280	2,411	48	84	317	546	416	460	601
92	Tustin Avenue and Santa Clara Avenue	83	453	84	151	3,094	98	335	847	618	160	332	166
93	Tustin Avenue and 17th Street	127	226	269	716	2,067	23	621	2,450	418	664	1,047	256
94	Tustin Avenue and 4th Street	80	408	365	601	765	237	126	748	31	125	704	457
95	SR-55 SB Ramps / Auto Mall Drive and Edinger Avenue	456	68	540	50	30	8	36	1426	527	510	1202	157
96	SR-55 NB Ramps / Del Amo Avenue and Newport Avenue	392	104	66	21	512	750	269	163	155	8	223	4
97	Red Hill Avenue and Edinger Avenue	101	374	106	195	828	388	397	1276	271	255	1234	173
98	Red Hill Avenue and Warner Avenue	68	414	56	20	935	188	273	464	342	120	427	76
99	Red Hill Avenue and Barranca Parkway	152	379	125	163	719	94	124	748	296	511	571	111
100	Red Hill Avenue and Alton Parkway	225	663	415	212	962	154	158	490	490	342	290	163
101	Red Hill Avenue and MacArthur Boulevard	83	742	13	186	481	441	1153	596	98	32	308	789
102	Red Hill Avenue and Main Street	201	691	449	56	261	90	139	1309	254	154	309	73
103	I-5 SB Ramps and Santa Ana Boulevard	0	0	0	382	0	38	332	569	0	0	1267	370

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
104	Tustin Ranch Road and Warner Avenue	0	0	0	141	0	18	27	357	0	0	2130	490
105	Von Karman Avenue and Barranca Parkway	401	533	319	104	1021	446	219	823	275	902	1482	46

Table 6-2 Future Year (2045) No Project Volumes – PM Peak Hour

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
1	Euclid Street and Bolsa Avenue	169	1569	126	152	866	164	212	678	106	209	877	209
2	Euclid Street and McFadden Avenue	242	1973	127	96	1179	143	207	475	210	114	479	221
3	Euclid Street and Edinger Avenue	539	1610	115	136	897	228	141	544	163	128	1181	191
4	Newhope Street and Hazard Avenue	105	1159	45	46	982	117	75	148	91	88	556	160
5	Newhope Street and McFadden Avenue	188	1009	177	140	625	121	61	372	54	104	574	164
6	Harbor Boulevard and SR-22 WB Off-Ramp / Banner Drive	109	2457	0	0	1801	58	102	0	86	576	78	165
7	Trask Avenue and SR-22 EB On-Ramp	0	0	0	0	0	0	927	695	0	0	786	74
8	Harbor Boulevard and Westminster Avenue	237	1178	279	254	1752	202	215	768	37	357	1238	281
9	Harbor Boulevard and 1st Street	290	1497	166	245	1073	145	179	730	138	199	929	151
10	Harbor Boulevard and McFadden Avenue	169	1737	127	256	1094	99	241	585	73	198	474	137
11	Harbor Boulevard and Edinger Avenue	254	1597	193	290	843	77	109	460	81	196	1138	379
12	Harbor Boulevard and Warner Avenue	762	3656	275	262	1457	174	217	1539	352	249	2589	827
13	Harbor Boulevard and Segerstrom Avenue	273	1717	55	87	986	87	118	585	192	117	1269	461
14	Hyland Avenue / OCTA Bus Base and MacArthur Boulevard	1201	7	70	12	4	14	36	891	160	25	2350	12
15	Harbor Boulevard and MacArthur Boulevard	558	1441	83	204	1028	169	140	489	181	81	1373	258
16	Harbor Boulevard and Sunflower Avenue	154	1532	228	114	1417	43	81	220	229	301	632	236
17	Harbor Boulevard and I-405 NB Off-Ramp	0	1529	0	0	2570	0	0	0	0	496	0	801
18	Harbor Boulevard and I-405 SB Off-Ramp	0	1881	0	0	2281	0	177	0	815	0	0	0
19	Fairview Street and Civic Center Drive	6	1690	475	192	1784	0	2	6	4	424	0	157
20	Fairview Street and 1st Street	169	1241	112	236	1468	291	193	781	136	140	1060	164
21	Fairview Street and McFadden Avenue	164	1507	131	191	1553	242	158	427	106	158	545	134
22	Fairview Street and Edinger Avenue	184	1759	108	204	635	156	205	628	168	650	1375	302
23	Fairview Street and Warner Avenue	213	1695	158	182	921	93	168	815	126	168	989	214
24	Fairview Street and MacArthur Boulevard	225	1550	84	171	955	88	240	627	211	167	1272	223
25	Fairview Road and Sunflower Avenue	192	1725	375	140	1162	81	262	555	123	227	569	148

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
26	Greenville Street and Edinger Avenue	293	0	195	0	0	0	0	1061	216	153	1811	0
27	Greenville Street and Segerstrom Avenue	50	414	31	52	171	52	127	967	55	37	1414	202
28	Raitt Street and McFadden Avenue	61	872	136	51	533	86	108	640	56	98	827	114
29	Raitt Street and Edinger Avenue	171	736	206	114	396	99	125	862	123	192	2171	194
30	Bear Street and MacArthur Boulevard	273	1314	343	115	278	84	80	753	78	67	1211	193
31	Bristol Street and 17th Street	375	1823	303	337	1794	305	238	530	96	311	1170	396
32	Bristol Street and Civic Center Drive	222	1550	81	159	1458	149	195	549	65	184	789	89
33	Bristol Street and Santa Ana Boulevard	84	1702	97	117	1558	59	155	428	100	392	624	168
34	Bristol Street and 1st Street	392	1693	250	223	1209	273	160	900	115	196	1204	114
35	Bristol Street and McFadden Avenue	224	1649	90	111	1554	358	245	478	95	209	1088	127
36	Bristol Street and Warner Avenue	316	1182	216	338	1128	219	176	791	140	251	1481	434
37	Bristol Street and Segerstrom Avenue	182	1347	237	101	927	194	224	803	64	145	1313	66
38	Bristol Street and Alton Avenue	43	1681	158	116	870	54	25	35	65	91	150	29
39	Bristol Street and MacArthur Boulevard	321	1369	249	233	858	135	335	746	158	236	1370	259
40	Bristol Street and Sunflower Avenue	651	1376	222	215	804	230	301	588	228	247	952	303
41	Bristol Street and I-405 NB Ramps	0	2094	183	0	2375	22	0	0	204	350	300	1162
42	Bristol Street and I-405 SB Ramps	128	1570	0	0	1611	942	740	0	331	0	0	0
43	Flower Street and Santa Ana Boulevard	198	1206	99	87	442	40	357	1029	168	186	649	192
44	Flower Street and 1st Street	224	1063	253	200	549	159	102	895	74	210	1415	147
45	Flower Street and McFadden Avenue	91	1234	76	46	352	57	124	928	37	93	1159	166
46	Flower Street and Segerstrom Avenue	122	859	82	84	469	135	203	1011	107	92	1545	99
47	Flower Street and MacArthur Boulevard	130	621	72	93	224	215	199	947	60	62	1989	177
48	Main Street and La Veta Avenue	347	1059	496	164	1031	266	327	466	209	282	563	226
49	Main Street and Mainplace Drive / Memory Lane	113	1186	225	52	1083	143	391	433	116	278	227	110
50	Main Street and 17th Street	197	1019	222	222	969	150	224	1256	74	199	1220	81
51	Main Street and Civic Center Drive	92	1169	98	52	853	54	239	1322	158	38	384	39
52	Main Street and Santa Ana Boulevard	51	1152	0	0	1083	53	0	0	0	92	924	120
53	Main Street and 4th Street	0	950	45	0	864	39	0	70	39	0	229	58

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
54	Main Street and 1st Street	152	745	65	146	559	119	116	1172	87	111	1244	87
55	Main Street and McFadden Avenue	132	1037	64	123	834	61	182	805	98	133	752	338
56	Main Street and Edinger Avenue	101	1069	49	129	646	88	118	698	78	102	1364	141
57	Main Street and MacArthur Boulevard	446	1067	293	265	345	260	313	643	62	193	1529	444
58	Penn Way and 17th Street	1113	0	543	0	0	0	0	1672	539	123	1440	0
59	Santiago Street / I-5 NB Ramps and 17th Street	330	44	17	50	0	130	114	1047	688	0	1564	42
60	Penn Way and I-5 SB Ramps	0	631	414	592	137	0	0	0	0	214	0	183
61	Santiago Street and Civic Center Drive	194	598	60	32	456	117	266	76	269	14	19	18
62	Santiago Street and Santa Ana Boulevard	135	833	236	378	639	103	164	673	87	255	546	347
63	Standard Avenue and 4th Street	251	543	214	254	428	297	302	477	201	233	648	351
64	Standard Avenue and 1st Street	109	724	259	142	679	47	168	1,279	65	264	1,888	116
65	Standard Avenue and McFadden Avenue	194	765	94	237	520	90	95	632	108	123	749	217
66	Halladay Street and Warner Avenue	338	0	704	0	0	0	2	868	87	272	2159	0
67	Halladay Street and Dyer Road	61	266	307	296	39	83	147	1271	9	51	1655	132
68	SR-55 SB Ramps and MacArthur Boulevard	0	0	0	299	0	724	0	1187	1058	0	1474	579
69	SR-55 NB Ramps and MacArthur Boulevard	620	0	412	0	0	0	0	711	731	0	1467	1056
70	SR-55 SB Ramps and Dyer Road	386	12	291	103	226	110	59	1379	187	566	1008	65
71	Glassell Street and La Veta Avenue	573	595	194	14	471	35	106	346	594	211	382	15
72	Glassell Street and SR-22 WB Ramps	660	1079	0	0	1007	293	0	0	0	251	9	446
73	Glassell Street / Grand Avenue and SR-22 EB Ramps	0	1450	370	461	824	0	229	4	503	0	0	0
74	Grand Avenue and Fairhaven Avenue	54	1,420	284	242	974	89	79	45	33	221	42	157
75	Grand Avenue and Santa Clara Avenue	103	1,386	288	240	946	240	114	139	26	157	282	300
76	Grand Avenue and 17th Street	288	1,026	259	294	634	140	321	823	138	279	1,031	187
77	Grand Avenue and I-5 NB Ramps	0	1855	1432	62	1501	0	0	0	0	151	0	182
78	Grand Avenue and Santa Ana Boulevard	185	2091	34	38	839	595	377	230	363	34	228	165
79	Grand Avenue and 1st Street	188	1435	43	113	667	266	326	1080	107	154	958	176
80	Grand Avenue and Chestnut Avenue	152	1142	198	161	716	121	70	214	47	139	433	252
81	Grand Avenue and McFadden Avenue	222	1264	218	153	696	187	122	454	79	96	762	168

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
82	Grand Avenue and Edinger Avenue	244	1411	122	203	501	281	182	787	63	93	1673	217
83	Grand Avenue and Warner Avenue	183	660	146	140	452	208	306	587	127	241	1539	504
84	SR-55 NB Ramps and Dyer Road	181	0	49	0	0	0	0	1183	602	0	1488	929
85	Cambridge Street and La Veta Avenue	235	327	0	0	206	191	375	0	247	0	0	0
86	Cambridge Street and Fairhaven Avenue	0	0	0	219	0	76	132	230	0	0	237	295
87	Mabury Street and 1st Street	23	0	233	195	83	367	0	1334	21	40	549	0
88	Tustin Street and La Veta Avenue	38	863	205	79	701	13	13	5	20	146	4	90
89	Tustin Street and SR-22 WB On-Ramp	466	1340	0	0	573	294	0	0	0	0	0	0
90	Tustin Street and SR-22 EB Off-Ramp / Seba Avenue	0	1416	4	40	550	0	440	39	558	28	0	67
91	Tustin Street and Fairhaven Avenue	280	1,492	132	356	940	108	89	200	163	100	220	208
92	Tustin Avenue and Santa Clara Avenue	191	923	142	199	1,574	109	72	227	133	133	166	157
93	Tustin Avenue and 17th Street	253	1,083	557	377	404	40	524	1,029	139	243	798	444
94	Tustin Avenue and 4th Street	28	299	97	459	339	124	208	743	38	118	798	491
95	SR-55 SB Ramps / Auto Mall Drive and Edinger Avenue	401	32	423	110	50	37	47	1677	472	468	1220	146
96	SR-55 NB Ramps / Del Amo Avenue and Newport Avenue	879	262	51	17	209	803	156	54	32	23	348	16
97	Red Hill Avenue and Edinger Avenue	204	1469	233	105	209	169	410	1051	128	90	1161	567
98	Red Hill Avenue and Warner Avenue	374	1281	147	79	304	385	246	668	104	76	1598	210
99	Red Hill Avenue and Barranca Parkway	338	1270	327	241	408	192	172	637	84	110	839	245
100	Red Hill Avenue and Alton Parkway	26	1458	211	122	519	4	434	1065	545	1299	222	801
101	Red Hill Avenue and MacArthur Boulevard	175	1133	14	315	654	1148	499	230	35	39	810	568
102	Red Hill Avenue and Main Street	436	941	254	88	640	348	200	714	193	277	1575	88
103	I-5 SB Ramps and Santa Ana Boulevard	0	0	0	304	0	67	765	865	0	0	621	170
104	Tustin Ranch Road and Warner Avenue	0	0	0	553	0	38	65	2154	0	0	856	369
105	Von Karman Avenue and Barranca Parkway	346	1992	434	80	407	280	765	1397	195	232	734	250

6.1 FUTURE YEAR (2045) NO PROJECT VEHICLE MILES TRAVELED (VMT)

Table 7-6 presents the VMT analysis results for the Future Year (2045) No Project scenario. VMT calculations are based on the scenario parameters and methodology as described in Sections 2.6 and 3.1, respectively.

Table 6-3 Future Year (2045) VMT Summary

	I – I VMT	I – X VMT	X – I VMT	Total VMT	Service Population	VMT/SP
City	692,704	5,737,798	5,733,292	12,163,794	534,238	22.8
County	52,994,916	29,554,879	29,660,308	112,210,103	4,221,830	26.6

SP = Service Population = Total Employees + Total Population. Reduction factor applied to Total Employees as noted in Section 3.1 and shown in Appendix F. I-I = Internal-Internal; I-X = Internal-External; X-I = External-Internal.

6.2 ROADWAY SEGMENT LEVEL OF SERVICE

A summary of the forecast roadway segment volumes and corresponding level of service results for the future year 2045 No Project condition is shown in Table 6-4. The following roadway segments are forecast to operate at a deficient LOS:

- Segment 8 – Harbor Boulevard from Segerstrom Avenue to MacArthur Boulevard
- Segment 23 – Main Street from 1st Street to Bishop Street
- Segment 39 – Bristol Street from Edinger Avenue to Warner Avenue
- Segment 43 – Flower Street from Warner Avenue to Segerstrom Avenue
- Segment 45 – Main Street from McFadden Avenue to Edinger Avenue
- Segment 52 – Edinger Avenue from Richie Street to Newport Avenue
- Segment 57 – Dyer Road from Grand Avenue to Pullman Street
- Segment 58 – McFadden Avenue from Bristol Street to Flower Street
- Segment 62 – Fourth Street from French Street to Standard Avenue
- Segment 63 – Broadway from 17th Street to Civic Center Drive
- Segment 72 – McFadden Avenue from Main Street to Standard Avenue
- Segment 73 – Penn Way/Santiago Street from I-5 SB Ramps to Santa Ana Boulevard
- Segment 74 – Raitt Street from Santa Ana Boulevard to 1st Street
- Segment 84 – Dyer Road from Pullman Street to Red Hill Avenue

Table 6-4 Future Year (2045) No Project Roadway Segment LOS

ID	Street	Segment	Lanes	ADT	LOS
1	1st Street	Euclid Street to Ward Street	6D	21,700	A
2	Euclid Street	1st Street to McFadden Avenue	6D	35,900	B
3	Westminster Avenue	Harbor Boulevard to Fairview Street	6D	25,400	A
4	Harbor Boulevard	Westminster Avenue/17th Street to Hazard Avenue	6D	41,100	C
5	1st Street	Harbor Boulevard to Jackson	6D	23,700	A
6	Edinger Avenue	Harbor Boulevard to Fairview Street	6D	23,300	A
7	Warner Avenue	Harbor Boulevard to Fairview Street	6D	28,500	A
8	Harbor Boulevard	Segerstrom Avenue to MacArthur Boulevard	6D	59,600	F
9	Fairview Street	1st Street to Willits Street	6D	38,800	B
10	1st Street	Sullivan Street to Raitt Street	6D	24,400	A
11	Bristol Street	17th Street to Santa Clara Avenue	6D	43,200	C
12	17th Street	College Avenue to Bristol Street	6D	30,800	A
13	Bristol Street	17th Street to Washington Avenue	6D	47,300	D
14	Fairview Street	Trask Avenue to 17th Street	6D	47,400	D
15	Bristol Street	1st Street to Bishop Street	6D	45,500	D
16	Civic Center Drive	Bristol Street to Flower Street	4U	18,400	C
17	Flower Street	1st Street to Bishop Street	4U	9,400	A
18	Main Street	17th Street to 20th Street	6D	40,800	C
19	Main Street	Washington Street to Civic Center Drive	4U	16,600	B
20	Civic Center Drive	Flower Street to Ross Street	4U	10,200	A
21	Santa Ana Boulevard	Flower Street to Ross Street	6D	22,200	A
22	1st Street	Main Street to Standard Avenue	6D	38,200	B
23	Main Street	1st Street to Bishop Street	4U	29,400	F
24	Grand Avenue	Santa Clara Avenue to Fairhaven Street	6D	31,200	A
25	Grand Avenue	Santa Ana Boulevard to 4th Street	6D	24,600	A
26	Santa Clara Avenue	Grand Avenue to Tustin Avenue	4U	10,800	A
27	Tustin Avenue	Santa Clara Avenue to Fairhaven Street	6D	23,400	A
28	17th Street	Cabrillo Park Drive to Tustin Avenue	6D	32,700	A
29	Tustin Avenue	Fruit Street to 4th Street	6D	22,100	A
30	1st Street	Grand Avenue to Elk Lane	6D	29,300	A
31	1st Street	Cabrillo Park Drive to Tustin Avenue	6D	15,600	A
32	Fairview Street	Edinger Avenue to Harvard Street	6D	46,200	D
33	Fairview Street	Warner Avenue to Segerstrom Avenue	6D	43,200	C
34	MacArthur Boulevard	Harbor Boulevard to Fairview Street	6D	33,500	A
35	Edinger Avenue	Fairview Street to Greenville Street	6D	20,400	A
36	McFadden Avenue	Fairview Street to Raitt Street	4U	20,000	D
37	MacArthur Boulevard	Fairview Street to Raitt Street	6D	29,700	A
38	Segerstrom Avenue	Fairview Street to Raitt Street	6D	26,300	A
39	Bristol Street	Edinger Avenue to Warner Avenue	6D	53,500	E
40	Bristol Street	Warner Avenue to Segerstrom Avenue	6D	45,100	D

ID	Street	Segment	Lanes	ADT	LOS
41	Warner Avenue	Raitt Street to Bristol Street	6D	22,600	A
42	Bristol Street	MacArthur Boulevard to Sunflower Avenue	6D	49,100	D
43	Flower Street	Warner Avenue to Segerstrom Avenue	4D	36,200	E
44	Edinger Avenue	Flower Street to Main Street	6D	23,200	A
45	Main Street	McFadden Avenue to Edinger Avenue	4U	25,100	F
46	Main Street	Edinger Avenue to Warner Avenue	5D	34,400	C
47	Main Street	Warner Avenue to Dyer Road	6D	36,700	B
48	Segerstrom Avenue	Bristol Street to Flower Street	6D	24,400	A
49	MacArthur Boulevard	Flower Street to Main Street	6D	35,400	B
50	Main Street	MacArthur Boulevard to Sunflower Avenue	6D	26,300	A
51	Grand Avenue	Edinger Avenue to Warner Avenue	6D	34,100	B
52	Edinger Avenue	Richie Street to Newport Avenue	6D	52,800	E
53	Warner Avenue	Grand Avenue to Red Hill Avenue	6D	31,800	A
54	Warner Avenue	Main Street to Standard Avenue	6D	22,700	A
55	McFadden Avenue	Newhope Street to Harbor Boulevard	4U	13,800	A
56	McFadden Avenue	Standard Avenue to Grand Avenue	4U	16,100	B
57	Dyer Road	Grand Avenue to Pullman Street	6D	73,900	F
58	McFadden Avenue	Bristol Street to Flower Street	4U	23,300	E
59	Main Street	La Veta Avenue to Memory Lane	8D	48,800	B
60	1st Street	Bristol Street to Flower Street	6D	26,600	A
61	Fairhaven Avenue	Grand Avenue to Tustin Avenue	4D	13,500	A
62	4th Street	French Street to Standard Avenue	4U	22,800	E
63	Broadway	17th Street to Civic Center Drive	4U	29,600	F
64	Broadway	Civic Center Drive to 1st Street	4U	12,200	A
65	Cambridge Street	SR-22 to Fairhaven Avenue	4U	9,200	A
66	Chestnut Avenue	Standard Avenue to Lyon Street	4U	11,900	A
67	Civic Center Drive	French Street to Santiago Street	4U	2,100	A
68	Civic Center Drive	Fairview Road to Bristol Street	4U	20,300	D
69	Flower Street	Edinger Avenue to Warner Avenue	4U	13,300	A
70	Halladay Avenue	Warner Avenue to Dyer Road	4U	22,100	D
71	Hazard Avenue	Euclid Street to Harbor Boulevard	4U	15,400	B
72	McFadden Avenue	Main Street to Standard Avenue	4U	24,700	E
73	Penn Way/Santiago Street	I-5 SB Ramps to Santa Ana Boulevard	4U	26,300	F
74	Raitt Street	Santa Ana Boulevard to 1st Street	4U	23,100	E
75	Raitt Street	Warner Avenue to Segerstrom Avenue	4U	20,000	D
76	Santa Ana Boulevard	French Street to Santiago Street	4D	16,500	A
77	Santa Ana Boulevard	Raitt Street to Bristol Street	6D	27,000	A
78	Standard Avenue	Chestnut Avenue to McFadden Avenue	4U	19,600	C
79	Standard Avenue	Edinger Avenue to Warner Avenue	4U	18,200	C
80	Greenville Street	Edinger Avenue to Warner Avenue	2U	6,600	A
81	Greenville Street	Warner Avenue to Segerstrom Avenue	4D	8,400	A
82	Flower Street	17th Street to Northern Terminus	2D	5,900	A

ID	Street	Segment	Lanes	ADT	LOS
83	Logan Street	Civic Center Drive to Santa Ana Boulevard	2U	- *	-
84	Dyer Road	Pullman Street to Red Hill Avenue	6D	58,000	F
85	Fairhaven Avenue	Tustin Street to Yorba Street	4U	17,214	B
86	Santa Clara Avenue	Tustin Street to Yorba Street	4U	9,004	A
87	Harbor Boulevard	McFadden Avenue to Edinger Avenue	6D	36,400	B
88	Harbor Boulevard	Edinger Avenue to Warner Avenue	6D	44,900	C

Notes:
* Not available.
(1) ADT – Average Daily Traffic Volumes
(2) LOS – Level of Service
(3) #D – total number of lanes in both directions divided by raised or striped median; #U – total number of undivided lanes in both directions.
(4) Roadway capacities shown in Table 3-1.

6.3 INTERSECTION LEVEL OF SERVICE

A summary of the AM and PM peak hour level of service analysis results for the Future Year (2045) No Project condition is provided in Table 6-5. Intersections are considered unacceptable if the level of service (LOS) is "E" or "F". It should be noted that Caltrans and CMP intersection have a minimum LOS of C and E, respectively. The following intersections are forecast to have unacceptable level of service in the Future Year (2045) No Project condition:

- INT 2 – Euclid Street and McFadden Avenue (City of Santa Ana)
- INT 3 – Euclid Street and Edinger Avenue (City of Fountain Valley)
- INT 8 – Harbor Boulevard and Westminster Avenue (City of Garden Grove / City of Santa Ana)
- INT 12 – Harbor Boulevard and Warner Avenue (City of Santa Ana – CMP)
- INT 13 – Harbor Boulevard and Segerstrom Avenue (City of Santa Ana)
- INT 14 – Hyland Avenue and MacArthur Boulevard (City of Costa Mesa)
- INT 22 – Fairview Street and Edinger Avenue (City of Santa Ana)
- INT 29 – Raitt Street and Edinger Avenue (City of Santa Ana)
- INT 32 – Bristol Street and Civic Center Drive (City of Santa Ana)
- INT 35 – Bristol Street and McFadden Avenue (City of Santa Ana)
- INT 36 – Bristol Street and Warner Avenue (City of Santa Ana)
- INT 44 – Flower Street and 1st Street (City of Santa Ana)
- INT 45 – Flower Street and McFadden Avenue (City of Santa Ana)
- INT 51 – Main Street and Civic Center Drive (City of Santa Ana)
- INT 52 – Main Street and Santa Ana Boulevard (City of Santa Ana)
- INT 55 – Main Street and McFadden Avenue (City of Santa Ana)
- INT 61 – Santiago Street and Civic Center Drive (City of Santa Ana)
- INT 62 – Santiago Street and Santa Ana Boulevard (City of Santa Ana)

- INT 63 – Standard Avenue and 4th Street (City of Santa Ana)
- INT 64 – Standard Avenue and 1st Street (City of Santa Ana)
- INT 66 – Halladay Street and Warner Avenue (City of Santa Ana)
- INT 67 – Halladay Street and Dyer Road (City of Santa Ana)
- INT 82 – Grand Avenue and Edinger Avenue (City of Santa Ana)
- INT 91 – Tustin Street and Fairhaven Avenue (City of Orange)
- INT 92 – Tustin Avenue and Santa Clara Avenue (City of Santa Ana)
- INT 93 – Tustin Avenue and 17th Street (City of Santa Ana)
- INT 96 – SR-55 NB Ramps / Del Amo Avenue and Newport Avenue (Caltrans)
- INT 100 – Red Hill Avenue and Alton Parkway (City of Irvine)
- INT 105 – Von Karman Avenue and Barranca Parkway (City of Irvine)

Table 6-5: Future Year (2045) No Project Intersection LOS Summary

ID	INTERSECTION	AM		PM	
		V/C or Delay	LOS	V/C or Delay	LOS
1	Euclid Street and Bolsa Avenue	0.86	D	0.86	D
2	Euclid Street and McFadden Avenue	0.98	E	0.90	D
3	Euclid Street and Edinger Avenue	0.99	E	0.89	D
4	Newhope Street and Hazard Avenue	0.78	C	0.72	C
5	Newhope Street and McFadden Avenue	0.83	D	0.78	C
6	Harbor Boulevard and SR-22 WB Off-Ramp / Banner Drive	28.4	C	24.2	C
7	Trask Avenue and SR-22 EB On-Ramp	12.0	B	13.6	B
8	Harbor Boulevard and Westminster Avenue	0.99	E	0.88	D
9	Harbor Boulevard and 1st Street	0.79	C	0.78	C
10	Harbor Boulevard and McFadden Avenue	0.79	C	0.81	D
11	Harbor Boulevard and Edinger Avenue	0.84	D	0.73	C
12	Harbor Boulevard and Warner Avenue	0.79	C	1.54	F
13	Harbor Boulevard and Segerstrom Avenue	0.97	E	0.92	E
14	Hyland Avenue / OCTA Bus Base and MacArthur Boulevard	0.61	B	0.92	E
15	Harbor Boulevard and MacArthur Boulevard	0.74	C	0.78	C
16	Harbor Boulevard and Sunflower Avenue	0.70	B	0.72	C
17	Harbor Boulevard and I-405 NB Off-Ramp	18.0	B	18.3	B
18	Harbor Boulevard and I-405 SB Off-Ramp	15.0	B	17.9	B
19	Fairview Street and Civic Center Drive	0.74	C	0.76	C
20	Fairview Street and 1st Street	0.86	D	0.90	D
21	Fairview Street and McFadden Avenue	0.80	C	0.78	C
22	Fairview Street and Edinger Avenue	0.86	D	0.92	E
23	Fairview Street and Warner Avenue	0.82	D	0.80	C
24	Fairview Street and MacArthur Boulevard	0.68	B	0.77	C
25	Fairview Road and Sunflower Avenue	0.70	B	0.72	C
26	Greenville Street and Edinger Avenue	0.77	C	0.60	A
27	Greenville Street and Segerstrom Avenue	0.84	D	0.78	C
28	Raitt Street and McFadden Avenue	0.78	C	0.76	C
29	Raitt Street and Edinger Avenue	0.99	E	0.99	E
30	Bear Street and MacArthur Boulevard	0.71	C	0.85	D
31	Bristol Street and 17th Street	0.87	D	0.82	D
32	Bristol Street and Civic Center Drive	0.94	E	0.91	E
33	Bristol Street and Santa Ana Boulevard	0.81	D	0.85	D
34	Bristol Street and 1st Street	0.77	C	0.83	D
35	Bristol Street and McFadden Avenue	0.96	E	1.03	F
36	Bristol Street and Warner Avenue	0.92	E	0.85	D
37	Bristol Street and Segerstrom Avenue	0.81	D	0.87	D
38	Bristol Street and Alton Avenue	0.55	A	0.63	B
39	Bristol Street and MacArthur Boulevard	0.70	B	0.77	C

ID	INTERSECTION	AM		PM	
		V/C or Delay	LOS	V/C or Delay	LOS
40	Bristol Street and Sunflower Avenue	0.65	B	0.69	B
41	Bristol Street and I-405 NB Ramps	20.8	C	28.5	C
42	Bristol Street and I-405 SB Ramps	23.9	C	18.3	B
43	Flower Street and Santa Ana Boulevard	0.81	D	0.86	D
44	Flower Street and 1st Street	0.93	E	0.88	D
45	Flower Street and McFadden Avenue	0.90	D	1.04	F
46	Flower Street and Segerstrom Avenue	0.78	C	0.87	D
47	Flower Street and MacArthur Boulevard	0.68	B	0.90	D
48	Main Street and La Veta Avenue	0.59	A	0.70	B
49	Main Street and Mainplace Drive / Memory Lane	0.51	A	0.56	A
50	Main Street and 17th Street	0.88	D	0.76	C
51	Main Street and Civic Center Drive	0.78	C	0.97	E
52	Main Street and Santa Ana Boulevard	0.93	E	0.79	C
53	Main Street and 4th Street	0.41	A	0.45	A
54	Main Street and 1st Street	0.68	B	0.74	C
55	Main Street and McFadden Avenue	0.89	D	0.93	E
56	Main Street and Edinger Avenue	0.80	C	0.87	D
57	Main Street and MacArthur Boulevard	0.68	B	0.75	C
58	Penn Way and 17th Street	12.2	B	25.5	C
59	Santiago Street / I-5 NB Ramps and 17th Street	30.0	C	19.0	B
60	Penn Way and I-5 SB Ramps	21.2	C	23.0	C
61	Santiago Street and Civic Center Drive	159.2	F	110.0	F
62	Santiago Street and Santa Ana Boulevard	1.04	F	1.02	F
63*	Standard Avenue and 4th Street	0.94	E	0.89	D
64	Standard Avenue and 1st Street	0.98	E	0.97	E
65	Standard Avenue and McFadden Avenue	0.85	D	0.83	D
66	Halladay Street and Warner Avenue	0.78	C	0.97	E
67	Halladay Street and Dyer Road	0.77	C	1.06	F
68	SR-55 SB Ramps and MacArthur Boulevard	19.9	B	17.8	B
69	SR-55 NB Ramps and MacArthur Boulevard	19.3	B	14.6	B
70	SR-55 SB Ramps and Dyer Road	25.7	C	26.5	C
71	Glassell Street and La Veta Avenue	0.80	C	0.74	C
72	Glassell Street and SR-22 WB Ramps	32.6	C	30.5	C
73	Glassell Street / Grand Avenue and SR-22 EB Ramps	30.6	C	31.6	C
74	Grand Avenue and Fairhaven Avenue	0.61	B	0.72	C
75	Grand Avenue and Santa Clara Avenue	0.84	D	0.81	D
76	Grand Avenue and 17th Street	0.79	C	0.86	D
77	Grand Avenue and I-5 NB Ramps	13.2	B	10.1	B
78	Grand Avenue and Santa Ana Boulevard	26.0	C	25.3	C
79	Grand Avenue and 1st Street	0.74	C	0.73	C

ID	INTERSECTION	AM		PM	
		V/C or Delay	LOS	V/C or Delay	LOS
80	Grand Avenue and Chestnut Avenue	0.70	B	0.69	B
81	Grand Avenue and McFadden Avenue	0.89	D	0.82	D
82	Grand Avenue and Edinger Avenue	0.84	D	1.00	E
83	Grand Avenue and Warner Avenue	0.61	B	0.83	D
84	SR-55 NB Ramps and Dyer Road	17.1	B	5.7	A
85	Cambridge Street and La Veta Avenue	31.6	D	19.7	C
86	Cambridge Street and Fairhaven Avenue	0.56	A	0.45	A
87	Mabury Street and 1st Street	27.4	C	27.1	C
88	Tustin Street and La Veta Avenue	0.45	A	0.38	A
89	Tustin Street and SR-22 WB On-Ramp	12.2	B	11.1	B
90	Tustin Street and SR-22 EB Off-Ramp / Seba Avenue	25.0	C	23.1	C
91	Tustin Street and Fairhaven Avenue	1.20	F	0.69	B
92	Tustin Avenue and Santa Clara Avenue	1.04	F	0.71	C
93	Tustin Avenue and 17th Street	0.91	E	0.80	C
94	Tustin Avenue and 4th Street	0.83	D	0.71	C
95	SR-55 SB Ramps / Auto Mall Drive and Edinger Avenue	25.2	C	25.5	C
96	SR-55 NB Ramps / Del Amo Avenue and Newport Avenue	25.4	C	39.1	D
97	Red Hill Avenue and Edinger Avenue	0.61	B	0.85	D
98	Red Hill Avenue and Warner Avenue	0.56	A	0.88	D
99	Red Hill Avenue and Barranca Parkway	0.55	A	0.54	A
100	Red Hill Avenue and Alton Parkway	0.86	D	1.18	F
101	Red Hill Avenue and MacArthur Boulevard	0.69	B	0.70	B
102	Red Hill Avenue and Main Street	0.72	C	0.81	D
103	I-5 SB Ramps and Santa Ana Boulevard	17.4	B	18.4	B
104	Tustin Ranch Road and Warner Avenue	0.52	A	0.65	B
105	Von Karman Avenue and Barranca Parkway	0.93	E	1.23	F

Notes: (1) LOS – Level of Service; (2) V/C – Volume to Capacity

7.0 Future Year (2045) With Project

This network alternative includes all the improvements listed in the No Project alternative, in addition to the improvements presented in Section 2.7.

Separate OCTAM 5.0 forecasts were produced for the With Project condition in order to understand traffic changes resulting from the proposed roadway improvements listed above. The Future Year (2045) With Project – AM and PM peak hour intersection turning movement volumes are shown in Tables 7-1 and 7-2.

Table 7-1 Future Year (2045) With Project Volumes – AM Peak Hour

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
1	Euclid Street and Bolsa Avenue	100	845	145	194	1725	97	194	721	136	190	606	194
2	Euclid Street and McFadden Avenue	156	1185	141	146	2031	168	227	463	287	106	234	89
3	Euclid Street and Edinger Avenue	127	801	68	180	2613	240	136	595	364	133	812	148
4	Newhope Street and Hazard Avenue	94	708	101	86	1262	116	17	44	33	17	57	23
5	Newhope Street and McFadden Avenue	77	342	120	116	1153	107	106	386	125	122	311	69
6	Harbor Boulevard and SR-22 WB Off-Ramp / Banner Drive	83	1508	0	0	1354	18	81	0	133	836	44	117
7	Trask Avenue and SR-22 EB On-Ramp	0	0	0	0	0	0	558	699	0	0	624	117
8	Harbor Boulevard and Westminster Avenue	159	1045	224	251	1970	113	173	910	32	406	1100	299
9	Harbor Boulevard and 1st Street	102	816	176	223	1876	122	118	788	153	229	580	135
10	Harbor Boulevard and McFadden Avenue	101	1026	98	179	1891	63	136	353	106	85	193	55
11	Harbor Boulevard and Edinger Avenue	95	713	135	212	2441	71	123	743	373	211	469	195
12	Harbor Boulevard and Warner Avenue	106	647	164	451	2034	122	100	1422	374	125	654	111
13	Harbor Boulevard and Segerstrom Avenue	107	759	48	173	2324	87	90	753	303	146	453	103
14	Hyland Avenue / OCTA Bus Base and MacArthur Boulevard	86	4	24	8	2	19	7	1915	760	82	626	26
15	Harbor Boulevard and MacArthur Boulevard	128	898	92	289	1662	115	133	1204	363	152	506	151
16	Harbor Boulevard and Sunflower Avenue	221	1128	209	242	2284	56	9	109	42	149	190	107
17	Harbor Boulevard and I-405 NB Off-Ramp	0	1915	0	0	2007	0	0	0	0	399	0	702
18	Harbor Boulevard and I-405 SB Off-Ramp	0	1993	0	0	1278	0	475	0	529	0	0	0
19	Fairview Street and Civic Center Drive	7	1435	465	314	1812	8	7	25	25	50	2	26
20	Fairview Street and 1st Street	196	1508	284	272	1579	181	170	977	151	180	809	271

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
21	Fairview Street and McFadden Avenue	138	1479	198	184	2242	162	92	216	50	93	240	19
22	Fairview Street and Edinger Avenue	212	935	131	225	1576	133	361	1002	268	463	608	121
23	Fairview Street and Warner Avenue	235	1039	136	205	1628	119	69	1010	301	277	836	271
24	Fairview Street and MacArthur Boulevard	280	870	100	269	1357	152	121	984	145	298	627	178
25	Fairview Road and Sunflower Avenue	182	1051	160	191	1627	104	44	286	58	357	299	154
26	Greenville Street and Edinger Avenue	140	0	188	0	0	0	0	1788	436	178	949	0
27	Greenville Street and Segerstrom Avenue	47	193	78	154	472	160	50	1127	180	210	847	97
28	Raitt Street and McFadden Avenue	34	309	49	69	528	72	52	313	40	60	190	34
29	Raitt Street and Edinger Avenue	85	228	82	80	376	45	330	2016	290	127	992	59
30	Bear Street and MacArthur Boulevard	61	210	89	192	728	289	99	1690	116	103	1735	147
31	Bristol Street and 17th Street	241	1241	210	449	1845	243	341	1015	164	386	886	293
32	Bristol Street and Civic Center Drive	129	1141	101	396	1597	100	80	404	50	145	481	100
33	Bristol Street and Santa Ana Boulevard	68	1482	206	172	1891	51	54	264	23	52	129	22
34	Bristol Street and 1st Street	208	1451	184	315	1298	183	206	1268	128	126	949	139
35	Bristol Street and McFadden Avenue	130	1159	58	228	2223	229	155	266	131	145	405	47
36	Bristol Street and Warner Avenue	221	993	183	643	2178	247	172	958	238	231	1058	180
37	Bristol Street and Segerstrom Avenue	70	778	146	283	1011	119	306	1480	136	157	712	91
38	Bristol Street and Alton Avenue	10	702	53	224	1551	36	190	117	244	41	87	31
39	Bristol Street and MacArthur Boulevard	95	491	123	319	1389	145	211	1531	311	191	910	132
40	Bristol Street and Sunflower Avenue	119	583	172	232	1316	99	135	1115	451	295	480	166
41	Bristol Street and I-405 NB Ramps	0	1638	210	0	2088	8	0	0	36	147	89	807
42	Bristol Street and I-405 SB Ramps	122	1195	0	0	1097	943	675	0	618	0	0	0
43	Flower Street and Santa Ana Boulevard	64	593	86	275	929	124	63	394	45	118	377	158
44	Flower Street and 1st Street	89	387	102	366	1158	121	107	983	39	161	664	95
45	Flower Street and McFadden Avenue	30	219	19	66	367	53	58	658	50	54	366	49
46	Flower Street and Segerstrom Avenue	86	451	100	98	544	120	237	1631	455	115	846	93
47	Flower Street and MacArthur Boulevard	26	170	88	163	352	264	163	1852	79	79	1329	85
48	Main Street and	123	844	438	161	843	141	350	495	215	334	291	243

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
	La Veta Avenue												
49	Main Street and Mainplace Drive / Memory Lane	35	955	313	27	1066	223	123	194	13	264	301	20
50	Main Street and 17th Street	156	929	199	225	1251	81	114	1203	60	369	1475	22
51	Main Street and Civic Center Drive	138	901	79	69	1222	186	75	454	83	52	679	41
52	Main Street and Santa Ana Boulevard	66	1050	0	0	1162	103	0	0	0	61	848	65
53	Main Street and 4th Street	0	1052	25	0	1098	27	0	93	14	0	105	26
54	Main Street and 1st Street	103	532	54	94	937	69	69	805	85	78	972	54
55	Main Street and McFadden Avenue	93	874	44	184	1434	28	44	419	46	97	259	74
56	Main Street and Edinger Avenue	104	589	64	239	1103	58	99	1288	174	149	867	106
57	Main Street and MacArthur Boulevard	60	344	292	539	709	160	230	1336	242	187	556	279
58	Penn Way and 17th Street	49	0	250	0	0	0	0	1279	652	236	1901	0
59	Santiago Street / I-5 NB Ramps and 17th Street	720	33	21	60	0	312	101	1062	384	0	1508	24
60	Penn Way and I-5 SB Ramps	0	192	170	837	173	0	0	0	0	170	0	95
61	Santiago Street and Civic Center Drive	318	221	31	6	335	82	212	81	431	126	116	23
62	Santiago Street and Santa Ana Boulevard	188	611	303	409	972	253	118	732	99	297	699	258
63	Standard Avenue and 4th Street	191	473	248	431	604	332	298	624	219	250	550	340
64	Standard Avenue and 1st Street	45	631	312	123	901	41	161	1959	121	194	1282	120
65	Standard Avenue and McFadden Avenue	44	170	77	123	297	21	21	736	89	81	214	48
66	Halladay Street and Warner Avenue	69	0	160	0	0	0	0	1772	200	210	797	0
67	Halladay Street and Dyer Road	16	26	71	155	31	33	54	1820	13	126	686	111
68	SR-55 SB Ramps and MacArthur Boulevard	0	0	0	998	0	903	0	1423	1014	0	1363	155
69	SR-55 NB Ramps and MacArthur Boulevard	965	0	1037	0	0	0	0	1503	744	0	591	279
70	SR-55 SB Ramps and Dyer Road	312	37	713	50	242	60	34	1282	273	418	663	111
71	Glassell Street and La Veta Avenue	292	458	139	13	274	60	57	293	563	380	401	10
72	Glassell Street and SR-22 WB Ramps	568	638	0	0	1017	473	0	0	0	247	4	369
73	Glassell Street / Grand Avenue and SR-22 EB Ramps	0	968	258	345	904	0	258	0	1012	0	0	0
74	Grand Avenue and Fairhaven Avenue	25	1,032	148	184	1,531	34	99	60	59	152	17	137
75	Grand Avenue and Santa Clara Avenue	92	648	167	307	1,503	338	179	291	60	140	270	246
76	Grand Avenue and 17th Street	228	666	458	303	1,270	249	246	915	116	401	1,047	114

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
77	Grand Avenue and I-5 NB Ramps	0	1145	662	104	3338	0	0	0	0	557	0	154
78	Grand Avenue and Santa Ana Boulevard	79	1119	57	231	1908	1684	242	200	394	5	407	37
79	Grand Avenue and 1st Street	186	655	31	121	1753	213	318	760	216	268	939	107
80	Grand Avenue and Chestnut Avenue	53	737	163	174	2086	57	64	449	118	81	237	148
81	Grand Avenue and McFadden Avenue	262	1104	165	294	1974	192	109	352	107	95	340	86
82	Grand Avenue and Edinger Avenue	65	649	127	253	1293	253	396	1654	159	170	651	142
83	Grand Avenue and Warner Avenue	187	564	241	204	589	199	314	1120	440	140	617	208
84	SR-55 NB Ramps and Dyer Road	637	0	521	0	0	0	0	1550	620	0	859	517
85	Cambridge Street and La Veta Avenue	185	235	0	0	480	245	195	0	312	0	0	0
86	Cambridge Street and Fairhaven Avenue	0	0	0	618	0	199	69	185	0	0	130	101
87	Mabury Street and 1st Street	5	0	122	276	164	618	0	922	10	46	701	0
88	Tustin Street and La Veta Avenue	9	365	105	48	944	3	9	11	35	279	1	43
89	Tustin Street and SR-22 WB On-Ramp	762	708	0	0	897	536	0	0	0	0	0	0
90	Tustin Street and SR-22 EB Off-Ramp / Seba Avenue	0	1213	6	13	779	0	252	25	1084	6	0	43
91	Tustin Street and Fairhaven Avenue	125	720	126	249	2,148	42	78	297	511	306	338	442
92	Tustin Avenue and Santa Clara Avenue	102	556	104	135	2,756	87	267	673	491	118	244	122
93	Tustin Avenue and 17th Street	155	277	330	638	1,841	20	494	1,948	333	488	770	188
94	Tustin Avenue and 4th Street	40	204	183	743	946	293	159	942	39	129	727	472
95	SR-55 SB Ramps / Auto Mall Drive and Edinger Avenue	456	68	540	50	30	8	34	1334	493	510	1202	157
96	SR-55 NB Ramps / Del Amo Avenue and Newport Avenue	413	109	69	20	497	728	269	163	155	8	223	4
97	Red Hill Avenue and Edinger Avenue	122	449	128	203	861	403	397	1276	271	250	1213	170
98	Red Hill Avenue and Warner Avenue	118	716	97	20	949	191	239	407	300	152	543	96
99	Red Hill Avenue and Barranca Parkway	184	458	151	184	809	106	126	758	300	522	583	114
100	Red Hill Avenue and Alton Parkway	235	693	434	233	1059	170	185	574	574	335	284	159
101	Red Hill Avenue and MacArthur Boulevard	83	742	13	205	529	485	1194	617	101	34	329	843
102	Red Hill Avenue and Main Street	201	691	449	78	365	126	139	1309	254	167	335	79
103	I-5 SB Ramps and Santa Ana Boulevard	0	0	0	382	0	38	287	492	0	0	1027	300

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
104	Tustin Ranch Road and Warner Avenue	0	0	0	174	0	22	27	357	0	0	2130	490
105	Von Karman Avenue and Barranca Parkway	375	497	298	98	960	420	247	928	310	917	1506	47

Table 7-2 Future Year (2045) With Project Volumes – PM Peak Hour

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
1	Euclid Street and Bolsa Avenue	165	1532	123	137	781	148	201	641	100	168	702	168
2	Euclid Street and McFadden Avenue	248	2020	130	87	1070	130	134	307	136	86	359	166
3	Euclid Street and Edinger Avenue	551	1648	117	136	897	228	121	467	140	119	1103	179
4	Newhope Street and Hazard Avenue	102	1129	44	45	954	114	33	66	40	8	51	15
5	Newhope Street and McFadden Avenue	188	1009	177	121	539	104	45	273	40	72	398	113
6	Harbor Boulevard and SR-22 WB Off-Ramp / Banner Drive	103	2324	0	0	1823	59	102	0	86	558	75	160
7	Trask Avenue and SR-22 EB On-Ramp	0	0	0	0	0	0	785	588	0	0	449	42
8	Harbor Boulevard and Westminster Avenue	222	1106	262	254	1752	202	227	808	39	294	1018	231
9	Harbor Boulevard and 1st Street	280	1446	160	240	1053	142	192	784	148	185	865	141
10	Harbor Boulevard and McFadden Avenue	182	1866	136	261	1114	101	147	358	45	63	151	43
11	Harbor Boulevard and Edinger Avenue	262	1645	199	283	822	75	109	460	81	188	1089	363
12	Harbor Boulevard and Warner Avenue	768	3685	277	250	1389	166	198	1399	320	253	2626	838
13	Harbor Boulevard and Segerstrom Avenue	275	1732	55	86	973	86	118	585	192	121	1308	475
14	Hyland Avenue / OCTA Bus Base and MacArthur Boulevard	1139	7	66	12	4	14	35	869	156	25	2320	12
15	Harbor Boulevard and MacArthur Boulevard	527	1362	79	206	1039	170	143	501	185	85	1429	269
16	Harbor Boulevard and Sunflower Avenue	156	1552	230	118	1464	44	85	230	239	282	592	221
17	Harbor Boulevard and I-405 NB Off-Ramp	0	1488	0	0	2570	0	0	0	0	513	0	828
18	Harbor Boulevard and I-405 SB Off-Ramp	0	1881	0	0	2254	0	181	0	833	0	0	0
19	Fairview Street and Civic Center Drive	6	1777	499	205	1903	0	2	6	4	164	0	61
20	Fairview Street and 1st Street	182	1339	121	236	1468	291	215	870	151	126	952	147
21	Fairview Street and McFadden Avenue	174	1596	139	195	1577	246	79	214	53	51	177	43
22	Fairview Street and Edinger Avenue	190	1810	111	198	616	151	205	628	168	639	1353	297
23	Fairview Street and Warner Avenue	211	1679	156	179	906	92	163	792	123	178	1049	227
24	Fairview Street and MacArthur Boulevard	228	1568	85	162	902	83	253	661	223	159	1210	212
25	Fairview Road and Sunflower Avenue	190	1705	371	138	1145	79	245	520	115	208	522	136

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
26	Greenville Street and Edinger Avenue	327	0	218	0	0	0	0	1178	240	155	1840	0
27	Greenville Street and Segerstrom Avenue	50	414	31	44	143	44	138	1049	60	39	1490	213
28	Raitt Street and McFadden Avenue	34	482	75	24	252	40	59	347	30	38	318	44
29	Raitt Street and Edinger Avenue	84	361	101	61	209	52	177	1226	175	195	2203	197
30	Bear Street and MacArthur Boulevard	262	1258	328	115	278	84	90	847	88	82	1475	235
31	Bristol Street and 17th Street	370	1801	300	347	1847	314	243	543	98	280	1055	357
32	Bristol Street and Civic Center Drive	214	1494	78	173	1589	163	130	366	43	149	635	72
33	Bristol Street and Santa Ana Boulevard	85	1722	98	120	1597	60	70	194	45	125	199	54
34	Bristol Street and 1st Street	434	1872	276	216	1169	264	259	1454	186	155	953	90
35	Bristol Street and McFadden Avenue	249	1833	99	116	1618	373	114	222	44	82	424	50
36	Bristol Street and Warner Avenue	312	1170	213	343	1144	223	159	715	126	282	1662	487
37	Bristol Street and Segerstrom Avenue	177	1309	231	98	894	187	279	998	79	156	1408	71
38	Bristol Street and Alton Avenue	43	1667	157	114	858	54	50	70	130	0	0	0
39	Bristol Street and MacArthur Boulevard	335	1428	260	237	871	137	357	796	169	247	1432	270
40	Bristol Street and Sunflower Avenue	687	1452	234	226	844	242	391	764	296	254	976	311
41	Bristol Street and I-405 NB Ramps	0	2235	195	0	2458	23	0	0	204	357	306	1184
42	Bristol Street and I-405 SB Ramps	135	1647	0	0	1657	968	740	0	331	0	0	0
43	Flower Street and Santa Ana Boulevard	215	1307	107	95	486	44	104	300	49	106	371	110
44	Flower Street and 1st Street	211	1000	238	140	383	111	135	1181	98	144	969	101
45	Flower Street and McFadden Avenue	54	734	45	26	201	33	56	422	17	43	532	76
46	Flower Street and Segerstrom Avenue	120	843	80	80	449	129	241	1198	126	101	1699	109
47	Flower Street and MacArthur Boulevard	116	553	65	97	233	224	264	1254	80	61	1967	175
48	Main Street and La Veta Avenue	310	948	444	191	1199	309	347	495	222	332	662	266
49	Main Street and Mainplace Drive / Memory Lane	102	1075	204	48	999	132	195	217	58	370	302	146
50	Main Street and 17th Street	239	1237	270	243	1062	164	174	975	58	187	1147	76
51	Main Street and Civic Center Drive	98	1234	103	63	1031	65	168	930	111	38	384	39
52	Main Street and Santa Ana Boulevard	56	1280	0	0	1108	54	0	0	0	65	656	85
53	Main Street and 4th Street	0	1108	53	0	951	43	0	112	63	0	157	40

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
54	Main Street and 1st Street	179	875	76	160	615	131	91	921	68	75	834	58
55	Main Street and McFadden Avenue	149	1169	72	138	933	68	78	345	42	71	400	180
56	Main Street and Edinger Avenue	111	1181	54	129	646	88	159	940	105	117	1569	162
57	Main Street and MacArthur Boulevard	488	1169	321	271	352	266	385	792	76	188	1488	432
58	Penn Way and 17th Street	795	0	388	0	0	0	0	1712	552	126	1480	0
59	Santiago Street / I-5 NB Ramps and 17th Street	375	50	20	55	0	142	128	1168	767	0	1622	44
60	Penn Way and I-5 SB Ramps	0	363	238	508	117	0	0	0	0	214	0	183
61	Santiago Street and Civic Center Drive	133	411	41	20	281	72	266	76	269	14	19	18
62	Santiago Street and Santa Ana Boulevard	135	833	236	378	639	103	164	673	87	255	546	347
63	Standard Avenue and 4th Street	251	543	214	254	428	297	302	477	201	233	648	351
64	Standard Avenue and 1st Street	109	724	259	142	679	47	168	1279	65	264	1888	116
65	Standard Avenue and McFadden Avenue	103	407	50	110	240	42	53	351	60	68	412	119
66	Halladay Street and Warner Avenue	200	0	417	0	0	0	2	955	96	287	2284	0
67	Halladay Street and Dyer Road	57	249	287	125	17	35	166	1433	10	52	1680	134
68	SR-55 SB Ramps and MacArthur Boulevard	0	0	0	321	0	777	0	1305	1163	0	1547	608
69	SR-55 NB Ramps and MacArthur Boulevard	726	0	483	0	0	0	0	764	785	0	1444	1040
70	SR-55 SB Ramps and Dyer Road	386	12	291	103	226	110	66	1544	210	507	904	58
71	Glassell Street and La Veta Avenue	573	595	194	15	499	37	112	367	631	225	407	16
72	Glassell Street and SR-22 WB Ramps	640	1045	0	0	1110	323	0	0	0	268	9	476
73	Glassell Street / Grand Avenue and SR-22 EB Ramps	0	1316	336	505	903	0	238	4	522	0	0	0
74	Grand Avenue and Fairhaven Avenue	50	1,311	262	242	974	89	79	45	33	153	29	109
75	Grand Avenue and Santa Clara Avenue	95	1,280	266	240	946	240	114	139	26	115	206	220
76	Grand Avenue and 17th Street	266	947	239	294	634	140	371	949	160	284	1,046	190
77	Grand Avenue and I-5 NB Ramps	0	2036	1571	67	1623	0	0	0	0	168	0	202
78	Grand Avenue and Santa Ana Boulevard	213	2411	40	44	964	684	370	226	356	44	295	213
79	Grand Avenue and 1st Street	195	1493	45	133	784	313	272	900	89	121	753	138
80	Grand Avenue and Chestnut Avenue	180	1352	234	149	664	112	47	143	31	91	283	165
81	Grand Avenue and McFadden Avenue	232	1320	227	176	800	215	64	238	42	96	762	168

ID	Intersection	Northbound			Southbound			Eastbound			Westbound		
		L	T	R	L	T	R	L	T	R	L	T	R
82	Grand Avenue and Edinger Avenue	259	1494	129	236	583	327	207	895	72	98	1758	228
83	Grand Avenue and Warner Avenue	218	788	174	178	575	265	342	656	142	244	1556	510
84	SR-55 NB Ramps and Dyer Road	188	0	51	0	0	0	0	1355	689	0	1411	882
85	Cambridge Street and La Veta Avenue	176	246	0	0	206	191	469	0	309	0	0	0
86	Cambridge Street and Fairhaven Avenue	0	0	0	195	0	68	85	148	0	0	218	272
87	Mabury Street and 1st Street	25	0	256	164	70	310	0	1275	20	31	423	0
88	Tustin Street and La Veta Avenue	31	693	164	96	851	16	13	5	20	134	4	82
89	Tustin Street and SR-22 WB On-Ramp	383	1100	0	0	642	330	0	0	0	0	0	0
90	Tustin Street and SR-22 EB Off-Ramp / Seba Avenue	0	1155	3	43	589	0	440	39	558	28	0	67
91	Tustin Street and Fairhaven Avenue	258	1,377	122	356	940	108	82	185	151	86	191	180
92	Tustin Avenue and Santa Clara Avenue	176	852	131	199	1,574	109	48	150	88	115	144	136
93	Tustin Avenue and 17th Street	233	1,000	514	377	404	40	346	680	92	211	693	385
94	Tustin Avenue and 4th Street	35	381	123	536	396	145	226	806	41	123	833	513
95	SR-55 SB Ramps / Auto Mall Drive and Edinger Avenue	401	32	423	110	50	37	46	1649	464	437	1139	136
96	SR-55 NB Ramps / Del Amo Avenue and Newport Avenue	879	262	51	19	231	889	165	57	33	23	348	16
97	Red Hill Avenue and Edinger Avenue	208	1501	238	108	214	173	410	1051	128	91	1176	574
98	Red Hill Avenue and Warner Avenue	343	1175	135	89	341	432	324	880	137	78	1643	216
99	Red Hill Avenue and Barranca Parkway	344	1293	333	314	531	249	178	662	87	129	979	286
100	Red Hill Avenue and Alton Parkway	27	1513	219	148	628	5	453	1111	569	2003	342	1235
101	Red Hill Avenue and MacArthur Boulevard	179	1160	14	340	707	1240	515	237	36	39	818	574
102	Red Hill Avenue and Main Street	467	1006	272	88	640	348	212	757	205	284	1617	91
103	I-5 SB Ramps and Santa Ana Boulevard	0	0	0	357	0	78	595	672	0	0	590	162
104	Tustin Ranch Road and Warner Avenue	0	0	0	653	0	45	64	2124	0	0	925	398
105	Von Karman Avenue and Barranca Parkway	362	2082	454	80	407	280	785	1435	201	254	805	274

7.1 FUTURE YEAR (2045) WITH PROJECT VEHICLE MILES TRAVELED (VMT)

Table 7-3 presents the VMT analysis results for the Future Year (2045) With Project scenario. VMT calculations are based on the scenario parameters and methodology as described in Sections 2.7 and 3.1, respectively.

Table 7-3 Future Year (2045) VMT Summary

	I – I VMT	I – X VMT	X – I VMT	Total VMT	Service Population	VMT/SP
City	637,655	5,432,337	5,448,967	11,518,959	566,616	20.3
County	52,931,787	29,642,498	29,748,229	112,322,514	4,262,124	26.4

SP = Service Population = Total Employees + Total Population. Reduction factor applied to Total Employees as noted in Section 3.1 and shown in Appendix F. I-I = Internal-Internal; I-X = Internal-External; X-I = External-Internal.

7.2 ROADWAY SEGMENT LEVEL OF SERVICE

A summary of the forecast roadway segment volumes and corresponding level of service results for the Future Year (2045) With Project condition is shown in Table 7-4. The following roadway segments are forecast to operate at a deficient LOS:

- Segment 8 – Harbor Boulevard from Segerstrom Avenue to MacArthur Boulevard
- Segment 23 – Main Street from 1st Street to Bishop Street
- Segment 39 – Bristol Street from Edinger Avenue to Warner Avenue
- Segment 42 – Bristol Street from MacArthur Boulevard to Sunflower Avenue
- Segment 45 – Main Street from McFadden Avenue to Edinger Avenue
- Segment 46 – Main Street from Edinger Avenue to Warner Avenue
- Segment 57 – Dyer Road from Grand Avenue to Pullman Street
- Segment 63 – Broadway from 17th Street to Civic Center Drive
- Segment 84 – Dyer Road from Pullman Street to Red Hill Avenue

Table 7-4 Future Year (2045) With Project Roadway Segment LOS

ID	Street	Segment	Lanes	ADT	LOS
1	1st Street	Euclid Street to Ward Street	6D	18,700	A
2	Euclid Street	1st Street to McFadden Avenue	6D	34,000	B
3	Westminster Avenue	Harbor Boulevard to Fairview Street	6D	17,400	A
4	Harbor Boulevard	Westminster Avenue/17th Street to Hazard Avenue	6D	36,200	B
5	1st Street	Harbor Boulevard to Jackson	6D	23,100	A
6	Edinger Avenue	Harbor Boulevard to Fairview Street	6D	23,300	A
7	Warner Avenue	Harbor Boulevard to Fairview Street	6D	26,300	A
8	Harbor Boulevard	Segerstrom Avenue to MacArthur Boulevard	6D	56,900	F
9	Fairview Street	1st Street to Willits Street	6D	38,600	B
10	1st Street	Sullivan Street to Raitt Street	6D	26,600	A
11	Bristol Street	17th Street to Santa Clara Avenue	6D	41,500	C
12	17th Street	College Avenue to Bristol Street	6D	29,500	A
13	Bristol Street	17th Street to Washington Avenue	6D	45,100	D
14	Fairview Street	Trask Avenue to 17th Street	6D	48,100	D
15	Bristol Street	1st Street to Bishop Street	6D	49,000	D
16	Civic Center Drive	Bristol Street to Flower Street	4U	18,600	C
17	Flower Street	1st Street to Bishop Street	2D	6,900	A
18	Main Street	17th Street to 20th Street	6D	43,000	C
19	Main Street	Washington Street to Civic Center Drive	4U	19,000	C
20	Civic Center Drive	Flower Street to Ross Street	4U	10,200	A
21	Santa Ana Boulevard	Flower Street to Ross Street	4D	15,800	A
22	1st Street	Main Street to Standard Avenue	4D	32,900	D
23	Main Street	1st Street to Bishop Street	4U	30,500	F
24	Grand Avenue	Santa Clara Avenue to Fairhaven Street	6D	31,100	A
25	Grand Avenue	Santa Ana Boulevard to 4th Street	6D	35,000	B
26	Santa Clara Avenue	Grand Avenue to Tustin Avenue	2D	8,700	A
27	Tustin Avenue	Santa Clara Avenue to Fairhaven Street	6D	20,400	A
28	17th Street	Cabrillo Park Drive to Tustin Avenue	6D	34,600	B
29	Tustin Avenue	Fruit Street to 4th Street	6D	28,100	A
30	1st Street	Grand Avenue to Elk Lane	4D	30,800	D
31	1st Street	Cabrillo Park Drive to Tustin Avenue	4D	14,600	A
32	Fairview Street	Edinger Avenue to Harvard Street	6D	45,100	D
33	Fairview Street	Warner Avenue to Segerstrom Avenue	6D	41,800	C
34	MacArthur Boulevard	Harbor Boulevard to Fairview Street	6D	32,600	A
35	Edinger Avenue	Fairview Street to Greenville Street	6D	22,200	A
36	McFadden Avenue	Fairview Street to Raitt Street	2D	8,200	A
37	MacArthur Boulevard	Fairview Street to Raitt Street	6D	28,900	A
38	Segerstrom Avenue	Fairview Street to Raitt Street	6D	29,600	A
39	Bristol Street	Edinger Avenue to Warner Avenue	6D	54,500	E
40	Bristol Street	Warner Avenue to Segerstrom Avenue	6D	44,800	C

ID	Street	Segment	Lanes	ADT	LOS
41	Warner Avenue	Raitt Street to Bristol Street	6D	22,300	A
42	Bristol Street	MacArthur Boulevard to Sunflower Avenue	6D	50,800	E
43	Flower Street	Warner Avenue to Segerstrom Avenue	4D	33,300	D
44	Edinger Avenue	Flower Street to Main Street	6D	25,200	A
45	Main Street	McFadden Avenue to Edinger Avenue	4U	27,500	F
46	Main Street	Edinger Avenue to Warner Avenue	4D	38,200	F
47	Main Street	Warner Avenue to Dyer Road	6D	38,600	B
48	Segerstrom Avenue	Bristol Street to Flower Street	6D	25,900	A
49	MacArthur Boulevard	Flower Street to Main Street	6D	39,800	C
50	Main Street	MacArthur Boulevard to Sunflower Avenue	6D	29,000	A
51	Grand Avenue	Edinger Avenue to Warner Avenue	6D	37,300	B
52	Edinger Avenue	Richie Street to Newport Avenue	6D	49,700	D
53	Warner Avenue	Grand Avenue to Red Hill Avenue	6D	34,600	B
54	Warner Avenue	Main Street to Standard Avenue	6D	23,900	A
55	McFadden Avenue	Newhope Street to Harbor Boulevard	4U	8,700	A
56	McFadden Avenue	Standard Avenue to Grand Avenue	2D	8,600	A
57	Dyer Road	Grand Avenue to Pullman Street	6D	80,700	F
58	McFadden Avenue	Bristol Street to Flower Street	2D	11,800	B
59	Main Street	La Veta Avenue to Memory Lane	8D	50,200	B
60	1st Street	Bristol Street to Flower Street	4D	25,700	B
61	Fairhaven Avenue	Grand Avenue to Tustin Avenue	2D	11,100	B
62	4th Street	French Street to Standard Avenue	2D	16,500	D
63	Broadway	17th Street to Civic Center Drive	2D	22,700	F
64	Broadway	Civic Center Drive to 1st Street	2D	13,800	C
65	Cambridge Street	SR-22 to Fairhaven Avenue	2D	8,800	A
66	Chestnut Avenue	Standard Avenue to Lyon Street	2D	7,700	A
67	Civic Center Drive	French Street to Santiago Street	2U	1,900	A
68	Civic Center Drive	Fairview Road to Bristol Street	2D	12,200	C
69	Flower Street	Edinger Avenue to Warner Avenue	2D	6,300	A
70	Halladay Avenue	Warner Avenue to Dyer Road	2D	16,100	D
71	Hazard Avenue	Euclid Street to Harbor Boulevard	2D	7,900	A
72	McFadden Avenue	Main Street to Standard Avenue	2D	14,300	C
73	Penn Way/Santiago Street	I-5 SB Ramps to Santa Ana Boulevard	2D	19,400	D
74	Raitt Street	Santa Ana Boulevard to 1st Street	2D	15,500	D
75	Raitt Street	Warner Avenue to Segerstrom Avenue	2D	11,600	B
76	Santa Ana Boulevard	French Street to Santiago Street	2D	13,500	C
77	Santa Ana Boulevard	Raitt Street to Bristol Street	2D	13,000	C
78	Standard Avenue	Chestnut Avenue to McFadden Avenue	2D	11,000	B
79	Standard Avenue	Edinger Avenue to Warner Avenue	2D	10,000	B
80	Greenville Street	Edinger Avenue to Warner Avenue	2U	8,000	B
81	Greenville Street	Warner Avenue to Segerstrom Avenue	2D	8,000	A
82	Flower Street	17th Street to Northern Terminus	2D	4,700	A

ID	Street	Segment	Lanes	ADT	LOS
83	Logan Street	Civic Center Drive to Santa Ana Boulevard	2U	- *	-
84	Dyer Road	Pullman Street to Red Hill Avenue	6D	57,500	F
85	Fairhaven Avenue	Tustin Street to Yorba Street	4U	14,947	A
86	Santa Clara Avenue	Tustin Street to Yorba Street	4U	7,818	A
87	Harbor Boulevard	McFadden Avenue to Edinger Avenue	6D	35,100	B
88	Harbor Boulevard	Edinger Avenue to Warner Avenue	6D	43,100	C

Notes:
* Not available.
(1) ADT – Average Daily Traffic Volumes
(2) LOS – Level of Service
(3) #D – total number of lanes in both directions divided by raised or striped median; #U – total number of undivided lanes in both directions.
(4) Roadway capacities shown in Table 3-1.
(5) Yellow and red shading indicates an unacceptable level of service

7.3 INTERSECTION LEVEL OF SERVICE

A summary of the AM and PM peak hour intersection level of service analysis results for the Future Year (2045) With Project condition is included in Table 7-5. The following intersections are expected to operate at an unacceptable level of service:

- INT 2 – Euclid street and McFadden Avenue (City of Santa Ana)
- INT 3 – Euclid Street and Edinger Avenue (City of Fountain Valley)
- INT 8 – Harbor Boulevard and Westminster Avenue (City of Santa Ana)
- INT 12 – Harbor Boulevard and Warner Avenue (City of Santa Ana – CMP)
- INT 13 – Harbor Boulevard and Segerstrom Avenue (City of Santa Ana)
- INT 22 – Fairview Street and Edinger Avenue (City of Santa Ana)
- INT 29 – Raitt Street and Edinger Avenue (City of Santa Ana)
- INT 32 – Bristol Street and Civic Center Drive (City of Santa Ana)
- INT 34 – Bristol Street and 1st Street (City of Santa Ana)
- INT 35 – Bristol Street and McFadden Avenue (City of Santa Ana)
- INT 36 – Bristol Street and Warner Avenue (City of Santa Ana)
- INT 37 – Bristol Street and Segerstrom Avenue (City of Santa Ana)
- INT 44 – Flower Street and 1st Street (City of Santa Ana)
- INT 45 – Flower Street and McFadden Avenue (City of Santa Ana)
- INT 46 – Flower Street and Segerstrom Avenue (City of Santa Ana)
- INT 47 – Flower Street and MacArthur Boulevard (City of Santa Ana)
- INT 55 – Main Street and McFadden Avenue (City of Santa Ana)
- INT 56 – Main Street and Edinger Avenue (City of Santa Ana)
- INT 61 – Santiago Street and Civic Center Drive (City of Santa Ana)

- INT 62 – Santiago Street and Santa Ana Boulevard (City of Santa Ana)
- INT 63 – Standard Avenue and 4th Street (City of Santa Ana)
- INT 64 – Standard Avenue and 1st Street (City of Santa Ana)
- INT 67 – Halladay Street and Dyer Road (City of Santa Ana)
- INT 80 – Grand Avenue and Chestnut Avenue (City of Santa Ana)
- INT 81 – Grand Avenue and McFadden Avenue (City of Santa Ana)
- INT 82 – Grand Avenue and Edinger Avenue (City of Santa Ana)
- INT 91 – Tustin Street and Fairhaven Avenue (City of Santa Ana)
- INT 92 – Tustin Avenue and Santa Clara Avenue (City of Santa Ana)
- INT 96 – SR-55 NB Ramps / Del Amo Avenue and Newport Avenue (Caltrans)
- INT 98 – Red Hill Avenue and Warner Avenue (City of Tustin)
- INT 100 – Red Hill Avenue and Alton Parkway (City of Irvine)
- INT 105 – Von Karman Avenue and Barranca Parkway (City of Irvine)

Table 7-5: Future Year (2045) With Project Intersection LOS Summary

ID	INTERSECTION	AM		PM	
		V/C or Delay	LOS	V/C or Delay	LOS
1	Euclid Street and Bolsa Avenue	0.79	C	0.79	C
2	Euclid Street and McFadden Avenue	0.91	E	0.79	C
3	Euclid Street and Edinger Avenue	1.01	F	0.86	D
4	Newhope Street and Hazard Avenue	0.60	A	0.52	A
5	Newhope Street and McFadden Avenue	0.73	C	0.68	B
6	Harbor Boulevard and SR-22 WB Off-Ramp / Banner Drive	28.5	C	23.8	C
7	Trask Avenue and SR-22 EB On-Ramp	11.0	B	11.0	B
8	Harbor Boulevard and Westminster Avenue	0.94	E	0.84	D
9	Harbor Boulevard and 1st Street	0.75	C	0.75	C
10	Harbor Boulevard and McFadden Avenue	0.73	C	0.71	C
11	Harbor Boulevard and Edinger Avenue	0.86	D	0.72	C
12	Harbor Boulevard and Warner Avenue	0.80	C	1.54	F
13	Harbor Boulevard and Segerstrom Avenue	1.01	F	0.93	E
14	Hyland Avenue / OCTA Bus Base and MacArthur Boulevard	0.62	B	0.89	D
15	Harbor Boulevard and MacArthur Boulevard	0.75	C	0.79	C
16	Harbor Boulevard and Sunflower Avenue	0.71	C	0.71	C
17	Harbor Boulevard and I-405 NB Off-Ramp	17.9	B	18.8	B
18	Harbor Boulevard and I-405 SB Off-Ramp	15.0	B	18.1	B
19	Fairview Street and Civic Center Drive	0.68	B	0.71	C
20	Fairview Street and 1st Street	0.89	D	0.89	D
21	Fairview Street and McFadden Avenue	0.80	C	0.68	B
22	Fairview Street and Edinger Avenue	0.88	D	0.92	E
23	Fairview Street and Warner Avenue	0.85	D	0.81	D
24	Fairview Street and MacArthur Boulevard	0.69	B	0.76	C
25	Fairview Road and Sunflower Avenue	0.69	B	0.69	B
26	Greenville Street and Edinger Avenue	0.74	C	0.65	B
27	Greenville Street and Segerstrom Avenue	0.88	D	0.80	C
28	Raitt Street and McFadden Avenue	0.70	B	0.68	B
29	Raitt Street and Edinger Avenue	0.93	E	0.99	E
30	Bear Street and MacArthur Boulevard	0.76	C	0.90	D
31	Bristol Street and 17th Street	0.85	D	0.81	D
32	Bristol Street and Civic Center Drive	0.93	E	0.90	D
33	Bristol Street and Santa Ana Boulevard	0.72	C	0.73	C
34	Bristol Street and 1st Street	0.90	D	0.97	E
35	Bristol Street and McFadden Avenue	0.95	E	0.89	D
36	Bristol Street and Warner Avenue	0.96	E	0.87	D
37	Bristol Street and Segerstrom Avenue	0.85	D	0.92	E
38	Bristol Street and Alton Avenue	0.55	A	0.58	A
39	Bristol Street and MacArthur Boulevard	0.76	C	0.80	C

ID	INTERSECTION	AM		PM	
		V/C or Delay	LOS	V/C or Delay	LOS
40	Bristol Street and Sunflower Avenue	0.68	B	0.74	C
41	Bristol Street and I-405 NB Ramps	20.9	C	29.0	C
42	Bristol Street and I-405 SB Ramps	23.6	C	18.2	B
43	Flower Street and Santa Ana Boulevard	0.59	A	0.67	B
44	Flower Street and 1st Street	0.93	E	1.22	F
45	Flower Street and McFadden Avenue	0.81	D	0.97	E
46	Flower Street and Segerstrom Avenue	0.82	D	0.92	E
47	Flower Street and MacArthur Boulevard	0.71	C	0.91	E
48	Main Street and La Veta Avenue	0.54	A	0.76	C
49	Main Street and Mainplace Drive / Memory Lane	0.46	A	0.50	A
50	Main Street and 17th Street	0.85	D	0.80	C
51	Main Street and Civic Center Drive	0.85	D	0.86	D
52	Main Street and Santa Ana Boulevard	0.79	C	0.70	B
53	Main Street and 4th Street	0.44	A	0.47	A
54	Main Street and 1st Street	0.75	C	0.80	C
55	Main Street and McFadden Avenue	0.92	E	0.94	E
56	Main Street and Edinger Avenue	0.88	D	0.98	E
57	Main Street and MacArthur Boulevard	0.72	C	0.78	C
58	Penn Way and 17th Street	12.1	B	20.3	C
59	Santiago Street / I-5 NB Ramps and 17th Street	31.4	C	20.3	C
60	Penn Way and I-5 SB Ramps	19.5	B	22.8	C
61	Santiago Street and Civic Center Drive	62.6	F	27.6	D
62	Santiago Street and Santa Ana Boulevard	1.44	F	1.41	F
63*	Standard Avenue and 4th Street	1.340	F	1.340	F
64	Standard Avenue and 1st Street	1.52	F	1.48	F
65	Standard Avenue and McFadden Avenue	0.84	D	0.77	C
66	Halladay Street and Warner Avenue	0.69	B	0.82	D
67	Halladay Street and Dyer Road	0.67	B	0.95	E
68	SR-55 SB Ramps and MacArthur Boulevard	20.2	C	18.4	B
69	SR-55 NB Ramps and MacArthur Boulevard	19.5	B	16.2	B
70	SR-55 SB Ramps and Dyer Road	25.9	C	26.5	C
71	Glassell Street and La Veta Avenue	0.83	D	0.77	C
72	Glassell Street and SR-22 WB Ramps	30.1	C	33.4	C
73	Glassell Street / Grand Avenue and SR-22 EB Ramps	29.6	C	31.8	C
74	Grand Avenue and Fairhaven Avenue	0.57	A	0.67	B
75	Grand Avenue and Santa Clara Avenue	0.80	C	0.73	C
76	Grand Avenue and 17th Street	0.86	D	0.85	D
77	Grand Avenue and I-5 NB Ramps	15.3	B	11.3	B
78	Grand Avenue and Santa Ana Boulevard	26.3	C	28.5	C
79	Grand Avenue and 1st Street	0.83	D	0.72	C

ID	INTERSECTION	AM		PM	
		V/C or Delay	LOS	V/C or Delay	LOS
80	Grand Avenue and Chestnut Avenue	0.94	E	0.78	C
81	Grand Avenue and McFadden Avenue	1.01	F	0.81	D
82	Grand Avenue and Edinger Avenue	0.90	D	1.08	F
83	Grand Avenue and Warner Avenue	0.66	B	0.90	D
84	SR-55 NB Ramps and Dyer Road	17.3	B	6.0	A
85	Cambridge Street and La Veta Avenue	23.4	C	26.1	D
86	Cambridge Street and Fairhaven Avenue	0.62	B	0.53	A
87	Mabury Street and 1st Street	27.4	C	27.2	C
88	Tustin Street and La Veta Avenue	0.36	A	0.34	A
89	Tustin Street and SR-22 WB On-Ramp	13.2	B	7.7	A
90	Tustin Street and SR-22 EB Off-Ramp / Seba Avenue	23.8	C	23.3	C
91	Tustin Street and Fairhaven Avenue	1.24	F	0.76	C
92	Tustin Avenue and Santa Clara Avenue	1.10	F	0.78	C
93	Tustin Avenue and 17th Street	0.81	D	0.74	C
94	Tustin Avenue and 4th Street	0.79	C	0.78	C
95	SR-55 SB Ramps / Auto Mall Drive and Edinger Avenue	25.1	C	25.4	C
96	SR-55 NB Ramps / Del Amo Avenue and Newport Avenue	25.6	C	46.2	D
97	Red Hill Avenue and Edinger Avenue	0.62	B	0.87	D
98	Red Hill Avenue and Warner Avenue	0.58	A	0.95	E
99	Red Hill Avenue and Barranca Parkway	0.58	A	0.59	A
100	Red Hill Avenue and Alton Parkway	0.93	E	1.49	F
101	Red Hill Avenue and MacArthur Boulevard	0.71	C	0.72	C
102	Red Hill Avenue and Main Street	0.73	C	0.84	D
103	I-5 SB Ramps and Santa Ana Boulevard	17.8	B	19.9	B
104	Tustin Ranch Road and Warner Avenue	0.53	A	0.67	B
105	Von Karman Avenue and Barranca Parkway	0.93	E	1.28	F

Notes: (1) LOS – Level of Service; (2) V/C – Volume to Capacity

8.0 VMT and LOS Analysis

This section presents the significant impact analysis for the Future Year (2045) With Project scenario. Significant impacts are identified based on the criteria established in Section 3.1.

8.1 VMT IMPACT ANALYSIS

Per the City's VMT Guidelines, VMT impacts are to be identified for the Future Year (2045) With Project scenario. VMT impacts were assessed using the methodology described in Section 3.1. The following presents the screening and impact analysis process. First, the two general conditions are considered:

- **Does the project have the potential to reduce VMT/SP?**
    - It should be noted that the project consists of land use and the transportation network changes. Due to the scale of land use changes not all land uses are high-density and/or transit-oriented, thus the land use changes cannot be assumed to have the same VMT-reducing effect as, for example, a TOD Specific Plan. The proposed transportation network, however, has the potential to reduce VMT/SP since it proposes to reclassify streets in order to implement transit, Complete Streets, and active transportation improvements.
- **Is the project consistent with the Regional Transportation Plan (RTP) / Sustainable Communities Strategy's (SCS)?**
    - While the proposed General Plan does forecast growth beyond the RTP/SCS, it is occurring in a manner consistent with the goals of the RTP/SCS in that growth occurs in high density in/near high-quality transit corridors (HQTC). Additionally, the proposed transportation network supports active transportation and the reduction of VMT.

Since it is not clear that the land use changes will reduce VMT/SP, the project is not eligible to be screened from a full VMT analysis. Project and cumulative impacts are assessed below.

Project Level Impact

A project is considered to have a significant impact if the project VMT/SP (for the land use plan) exceeds 15% below the existing total daily VMT/SP for the County. Table 8-1 provides a summary of the VMT impact analysis for project level impacts. A significant impact at the project level is not expected due to the implementation of the project.

Table 8-1: Future Year (2045) VMT Impact Analysis – Project Impact

Metric	2045 – With Project City VMT/SP	2020 – No Project County VMT/SP	VMT Threshold 15% below 2020 – No Project County VMT/SP	Impact?
VMT/SP	20.3	25.9	22.0	NO

Cumulative Impact

A project is considered to have a significant cumulative impact if the project results in a negative effect on VMT/SP at the citywide level (City VMT/SP is higher with the project than without it). Table 8-2 provides a summary of the VMT impact analysis for project level impacts. A significant cumulative impact is not expected due to the implementation of the project.

Table 8-2: Future Year (2045) VMT Impact Analysis – Cumulative Impact

Metric	2045 – No Project City VMT/SP	2045 – With Project City VMT/SP	Impact?
VMT/SP	22.8	20.3	NO

The VMT impact analysis results show that the project is not expected to cause a significant project level or cumulative impact.

8.2 LEVEL OF SERVICE ASSESSMENT

If an intersection is expected to be significantly impacted due to the implementation of the proposed project, mitigation measures are to be identified such that the project impact is mitigated. This is generally accomplished by returning the V/C or Delay to pre-project levels and/or the resulting LOS to an acceptable LOS. This analysis for intersection LOS is provided here only for analysis with regard to MPAH reclassifications and consistency with OCTA's MPAH guidelines. Intersection LOS analysis is no longer required under CEQA, and the deficient LOS values identified here are not considered to be significant environmental impacts.

The cities of Santa Ana, Costa Mesa, Irvine, Fountain Valley, Tustin, and Orange have established LOS D as their minimum acceptable LOS. Caltrans and the OC CMP established LOS C and E as their minimum acceptable LOS, respectively.

Strategies and improvements to provide acceptable intersection LOS are developed in a manner which first considers lower intensity modifications, such as striping modifications, before considering more complex modifications such as signal controls or physical improvements. All strategies and improvements are subject to the approval of the presiding public authority, including funding and implementation. The City of Santa Ana will pursue the development of a Transportation System Improvement Assessment (TSIA) in order to provide funding for the implementation of the full transportation network buildout, including the strategies and improvements identified in this report.

The Future Year (2045) significant impact summary is provided in Table 8-3. Improvements are identified in Table 8-4. Table 8-5 provides a summary of the intersection performance with the implementation of the mitigation measures.

Unacceptable LOS and traffic impacts are expected at the intersections listed below (peak hour / jurisdiction). The intersection impacts were investigated to determine a probable cause for the impact (land use, transportation network, or both). In order to do this, link volumes and intersection LOS were compared for two scenarios: 1) a year 2045 scenario which includes the proposed General Plan land use updates and utilizes the 2045 baseline (No Project) network described in Section 2.6; and 2) the 2045 With Project scenario which includes all proposed General Plan land use updates and transportation network modifications described in Section 2.7. OCTAM volume comparison plots and a LOS comparison table are provided in Appendix G.

- **INT 3 – Euclid Street and Edinger Avenue (AM peak hour / Fountain Valley)**
    - The impact at this intersection is expected to occur as a result of the proposed reclassifications. Specific improvements to this intersection to address this identified impact are presented in Table 8.4 later in this report. As this impact is in part caused by the proposed MPAH reclassifications, OCTA and the City of Santa Ana will enter into a memorandum of understanding (MOU) that describes the timing and triggers for implementation of the improvements identified in Table 8.4. Additionally, the City of Santa will contribute to improvements based on the calculated fair-share percentage presented in Table 8-4.
- **INT 13 – Harbor Boulevard and Segerstrom Avenue (AM peak hour / Santa Ana)**
    - The impact at this intersection is expected to occur as a result of the proposed land use changes. Specific improvements to this intersection to address this identified impact are presented in Table 8.4 later in this report.
- **INT 34 – Bristol Street and 1st Street (AM and PM peak hour / City of Santa Ana)**
    - The impact at this intersection is expected to occur as a result of the proposed reclassifications. Specific improvements to this intersection to address this identified impact are presented in Table 8.4 later in this report. As this impact is in part caused by the proposed MPAH reclassifications, OCTA and the City of Santa Ana will enter into a memorandum of understanding (MOU) that describes the timing and triggers for implementation of the improvements identified in Table 8.4.
- **INT 36 – Bristol Street and Warner Avenue (AM peak hour / City of Santa Ana)**
    - The impact at this intersection is expected to occur as a result of the proposed reclassifications. Specific improvements to this intersection to address this identified impact are presented in Table 8.4 later in this report. As this impact is in part caused by the proposed MPAH reclassifications, OCTA and the City of Santa Ana will enter into a memorandum of understanding (MOU) that describes the timing and triggers for implementation of the improvements identified in Table 8.4.
- **INT 37 – Bristol Street and Segerstrom Avenue (PM peak hour / City of Santa Ana)**
    - The impact at this intersection is expected to occur as a result of both the proposed land use changes and reclassifications. Specific improvements to this intersection to address this identified impact are presented in Table 8.4 later in this report. As this impact is in part caused by the proposed MPAH reclassifications, OCTA and the City

of Santa Ana will enter into a memorandum of understanding (MOU) that describes the timing and triggers for implementation of the improvements identified in Table 8.4.

- **INT 44 – Flower Street 1st Street (PM peak hour / City of Santa Ana)**
    - The impact at this intersection is expected to occur as a result of both the proposed land use changes and reclassifications. Specific improvements to this intersection to address this identified impact are presented in Table 8.4 later in this report. As this impact is in part caused by the proposed MPAH reclassifications, OCTA and the City of Santa Ana will enter into a memorandum of understanding (MOU) that describes the timing and triggers for implementation of the improvements identified in Table 8.4.
- **INT 46 – Flower Street and Segerstrom Avenue (PM peak hour / City of Santa Ana)**
    - The impact at this intersection is expected to occur as a result of both the proposed land use changes and reclassifications. Specific improvements to this intersection to address this identified impact are presented in Table 8.4 later in this report. As this impact is in part caused by the proposed MPAH reclassifications, OCTA and the City of Santa Ana will enter into a memorandum of understanding (MOU) that describes the timing and triggers for implementation of the improvements identified in Table 8.4.
- **INT 47 – Flower Street and MacArthur Boulevard (PM peak hour / City of Santa Ana)**
    - The impact at this intersection is expected to occur as a result of the proposed land use changes. Specific improvements to this intersection to address this identified impact are presented in Table 8.4 later in this report.
- **INT 55 – Main Street and McFadden Avenue (AM peak hour / City of Santa Ana)**
    - The impact at this intersection is expected to occur as a result of both the proposed land use changes and reclassifications. Specific improvements to this intersection to address this identified impact are presented in Table 8.4 later in this report. As this impact is in part caused by the proposed MPAH reclassifications, OCTA and the City of Santa Ana will enter into a memorandum of understanding (MOU) that describes the timing and triggers for implementation of the improvements identified in Table 8.4.
- **INT 56 – Main Street and Edinger Avenue (PM peak hour / City of Santa Ana)**
    - The impact at this intersection is expected to occur as a result of the proposed reclassifications. Specific improvements to this intersection to address this identified impact are presented in Table 8.4 later in this report. As this impact is in part caused by the proposed MPAH reclassifications, OCTA and the City of Santa Ana will enter into a memorandum of understanding (MOU) that describes the timing and triggers for implementation of the improvements identified in Table 8.4.

- **INT 62 – Santiago Street & Santa Ana Boulevard (AM & PM peak hour / City of Santa Ana)**
    - The impact at this intersection is expected to occur as a result of the proposed reclassifications. Specific improvements to this intersection to address this identified impact are presented in Table 8.4 later in this report. As this impact is in part caused by the proposed MPAH reclassifications, OCTA and the City of Santa Ana will enter into a memorandum of understanding (MOU) that describes the timing and triggers for implementation of the improvements identified in Table 8.4.
- **INT 63 – Standard Avenue and 4th Street (AM and PM peak hour / City of Santa Ana)**
    - The impact at this intersection is expected to occur as a result of the proposed reclassifications. Specific improvements to this intersection to address this identified impact are presented in Table 8.4 later in this report. As this impact is in part caused by the proposed MPAH reclassifications, OCTA and the City of Santa Ana will enter into a memorandum of understanding (MOU) that describes the timing and triggers for implementation of the improvements identified in Table 8.4.
- **INT 64 – Standard Avenue and 1st Street (AM and PM peak hour / City of Santa Ana)**
    - The impact at this intersection is expected to occur as a result of the proposed reclassifications. Specific improvements to this intersection to address this identified impact are presented in Table 8.4 later in this report. As this impact is in part caused by the proposed MPAH reclassifications, OCTA and the City of Santa Ana will enter into a memorandum of understanding (MOU) that describes the timing and triggers for implementation of the improvements identified in Table 8.4.
- **INT 80 – Grand Avenue and Chestnut Avenue (AM peak hour / City of Santa Ana)**
    - The impact at this intersection is expected to occur as a result of the proposed reclassifications. Specific improvements to this intersection to address this identified impact are presented in Table 8.4 later in this report. As this impact is in part caused by the proposed MPAH reclassifications, OCTA and the City of Santa Ana will enter into a memorandum of understanding (MOU) that describes the timing and triggers for implementation of the improvements identified in Table 8.4.
- **INT 81 – Grand Avenue and McFadden Avenue (AM peak hour / City of Santa Ana)**
    - The impact at this intersection is expected to occur as a result of the proposed reclassifications. Specific improvements to this intersection to address this identified impact are presented in Table 8.4 later in this report. As this impact is in part caused by the proposed MPAH reclassifications, OCTA and the City of Santa Ana will enter

into a memorandum of understanding (MOU) that describes the timing and triggers for implementation of the improvements identified in Table 8.4.

- **INT 82 – Grand Avenue and Edinger Avenue (PM peak hour / City of Santa Ana)**
    - The impact at this intersection is expected to occur as a result of the proposed reclassifications. Specific improvements to this intersection to address this identified impact are presented in Table 8.4 later in this report. As this impact is in part caused by the proposed MPAH reclassifications, OCTA and the City of Santa Ana will enter into a memorandum of understanding (MOU) that describes the timing and triggers for implementation of the improvements identified in Table 8.4.
- **INT 91 – Tustin Street and Fairhaven Avenue (AM peak hour / City of Santa Ana)**
    - The impact at this intersection is expected to occur as a result of the proposed reclassifications. Specific improvements to this intersection to address this identified impact are presented in Table 8.4 later in this report. As this impact is in part caused by the proposed MPAH reclassifications, OCTA and the City of Santa Ana will enter into a memorandum of understanding (MOU) that describes the timing and triggers for implementation of the improvements identified in Table 8.4.
- **INT 92 – Tustin Avenue and Santa Clara Avenue (AM peak hour / City of Santa Ana)**
    - The impact at this intersection is expected to occur as a result of the proposed reclassifications. Specific improvements to this intersection to address this identified impact are presented in Table 8.4 later in this report. As this impact is in part caused by the proposed MPAH reclassifications, OCTA and the City of Santa Ana will enter into a memorandum of understanding (MOU) that describes the timing and triggers for implementation of the improvements identified in Table 8.4.
- **INT 96 – SR-55 NB Ramps/Del Amo Avenue & Newport Avenue (PM peak hour / Caltrans)**
    - The impact at this intersection is expected to occur as a result of the proposed land use changes. Specific improvements to this intersection to address this identified impact are presented in Table 8.4 later in this report. Additionally, the City of Santa will contribute to improvements based on the calculated fair-share percentage presented in Table 8-4. Additionally, the City of Santa will contribute to improvements based on the calculated fair-share percentage presented in Table 8-4.
- **INT 98 – Red Hill Avenue and Warner Avenue (PM peak hour / City of Tustin)**
    - The impact at this intersection is expected to occur as a result of the proposed land use changes. Specific improvements to this intersection to address this identified impact are presented in Table 8.4 later in this report. Additionally, the City of Santa will

contribute to improvements based on the calculated fair-share percentage presented in Table 8-4.

- **INT 100 – Red Hill Avenue and Alton Parkway (AM and PM peak hour / City of Irvine)**
    - The impact at this intersection is expected to occur as a result of the proposed land use changes. Specific improvements to this intersection to address this identified impact are presented in Table 8.4 later in this report. Additionally, the City of Santa will contribute to improvements based on the calculated fair-share percentage presented in Table 8-4.
- **INT 105 – Von Karman Avenue and Barranca Parkway (PM peak hour / City of Irvine)**
    - The impact at this intersection is expected to occur as a result of the proposed land use changes. Specific improvements to this intersection to address this identified impact are presented in Table 8.4 later in this report. Additionally, the City of Santa will contribute to improvements based on the calculated fair-share percentage presented in Table 8-4.

In addition to assessing intersection impacts, a performance assessment of study roadway segments was conducted in order to determine the potential adverse effects of the project. Although not considered an environmental impact, changes in roadway performance monitored for congestion management and fair-share contribution purposes. Highlighted below is a roadway segment expected to operate beyond capacity in the future 2045 scenarios; improvements are thus required to accomodate forecasted volumes.

- **Segment #84 – Dyer Road from Pullman Street to Red Hill Avenue (City of Santa Ana)**
    - This segment is expected to operate at LOS F in both the 2045 No Project and 2045 With Project scenarios. Planned improvements include a widening of this section of Dyer Road in order to accommodate forecasted traffic volumes. As of the time this report is written, the City of Irvine is expected to be fully responsible for the improvements. However, considering the project's contribution to traffic volumes the City of Santa Ana will provide a fair-share contribution of 23% for future improvements.

Fair-Share Calculation

As noted, the City of Santa Ana will contribute to intersection improvements based on the fair-share contribution to intersection volumes due to the proposed MPAH reclassifications. The fair-share for a given intersection is calculated using the formula shown below and is presented in Table 8-4 for each intersection which is not, or only partially, under the jurisdiction of the City of Santa Ana. Fair-share is the portion of new traffic generated by the project as a percentage of the total change in traffic from the existing year analysis.

$$Fair - Share\ (\%) = \left(\frac{Future\ With\ Project\ Volume - Future\ No\ Project\ Volume}{Future\ With\ Project\ Volume - Existing\ No\ Project\ Volume}\right) \times 100\%$$

Table 8-3: Future Year (2045) LOS Impact Summary

ID	INTERSECTION	PEAK HOUR	2045 No Project		2045 With Project		DELTA	LOS IMPACT?
			V/C or Delay	LOS	V/C or Delay	LOS		
1	Euclid Street and Bolsa Avenue	AM	0.86	D	0.79	C	-0.07	NO
		PM	0.86	D	0.79	C	-0.07	NO
2	Euclid Street and McFadden Avenue	AM	0.98	E	0.91	E	-0.07	NO
		PM	0.90	D	0.79	C	-0.11	NO
3	Euclid Street and Edinger Avenue	AM	0.99	E	1.01	F	0.02	YES
		PM	0.89	D	0.86	D	-0.03	NO
4	Newhope Street and Hazard Avenue	AM	0.78	C	0.60	A	-0.18	NO
		PM	0.72	C	0.52	A	-0.20	NO
5	Newhope Street and McFadden Avenue	AM	0.83	D	0.73	C	-0.10	NO
		PM	0.78	C	0.68	B	-0.10	NO
6	Harbor Boulevard and SR-22 WB Off-Ramp / Banner Drive	AM	28.4	C	28.5	C	0.1	NO / NO *
		PM	24.2	C	23.8	C	-0.4	NO / NO *
7	Trask Avenue and SR-22 EB On-Ramp	AM	12.0	B	11.0	B	-1.0	NO
		PM	13.6	B	11.0	B	-2.6	NO
8	Harbor Boulevard and Westminster Avenue	AM	0.99	E	0.94	E	-0.05	NO
		PM	0.88	D	0.84	D	-0.04	NO
9	Harbor Boulevard and 1st Street	AM	0.79	C	0.75	C	-0.04	NO
		PM	0.78	C	0.75	C	-0.03	NO
10	Harbor Boulevard and McFadden Avenue	AM	0.79	C	0.73	C	-0.06	NO
		PM	0.81	D	0.71	C	-0.10	NO
11	Harbor Boulevard and Edinger Avenue	AM	0.84	D	0.86	D	0.02	NO
		PM	0.73	C	0.72	C	-0.01	NO
12	Harbor Boulevard and Warner Avenue	AM	0.79	C	0.80	C	0.01	NO
		PM	1.54	F	1.54	F	0.00	NO
13	Harbor Boulevard and Segerstrom Avenue	AM	0.97	E	1.01	F	0.04	YES

ID	INTERSECTION	PEAK HOUR	2045 No Project		2045 With Project		DELTA	LOS IMPACT?
			V/C or Delay	LOS	V/C or Delay	LOS		
		PM	0.92	E	0.93	E	0.01	NO
14	Hyland Avenue / OCTA Bus Base and MacArthur Boulevard	AM	0.61	B	0.62	B	0.01	NO
		PM	0.92	E	0.89	D	-0.03	NO
15	Harbor Boulevard and MacArthur Boulevard	AM	0.74	C	0.75	C	0.01	NO
		PM	0.78	C	0.79	C	0.01	NO
16	Harbor Boulevard and Sunflower Avenue	AM	0.70	B	0.71	C	0.01	NO
		PM	0.72	C	0.71	C	-0.01	NO
17	Harbor Boulevard and I-405 NB Off-Ramp	AM	18.0	B	17.9	B	-0.1	NO / NO *
		PM	18.3	B	18.8	B	0.5	NO / NO *
18	Harbor Boulevard and I-405 SB Off-Ramp	AM	15.0	B	15.0	B	0.0	NO / NO *
		PM	17.9	B	18.1	B	0.2	NO / NO *
19	Fairview Street and Civic Center Drive	AM	0.74	C	0.68	B	-0.06	NO
		PM	0.76	C	0.71	C	-0.05	NO
20	Fairview Street and 1st Street	AM	0.86	D	0.89	D	0.03	NO
		PM	0.90	D	0.89	D	-0.01	NO
21	Fairview Street and McFadden Avenue	AM	0.80	C	0.80	C	0.00	NO
		PM	0.78	C	0.68	B	-0.10	NO
22	Fairview Street and Edinger Avenue	AM	0.86	D	0.88	D	0.02	NO
		PM	0.92	E	0.92	E	0.00	NO
23	Fairview Street and Warner Avenue	AM	0.82	D	0.85	D	0.03	NO
		PM	0.80	C	0.81	D	0.01	NO
24	Fairview Street and MacArthur Boulevard	AM	0.68	B	0.69	B	0.01	NO
		PM	0.77	C	0.76	C	-0.01	NO
25	Fairview Road and Sunflower Avenue	AM	0.70	B	0.69	B	-0.01	NO
		PM	0.72	C	0.69	B	-0.03	NO
26	Greenville Street and Edinger Avenue	AM	0.77	C	0.74	C	-0.03	NO
		PM	0.60	A	0.65	B	0.05	NO

ID	INTERSECTION	PEAK HOUR	2045 No Project		2045 With Project		DELTA	LOS IMPACT?
			V/C or Delay	LOS	V/C or Delay	LOS		
27	Greenville Street and Segerstrom Avenue	AM	0.84	D	0.88	D	0.04	NO
		PM	0.78	C	0.80	C	0.02	NO
28	Raitt Street and McFadden Avenue	AM	0.78	C	0.70	B	-0.08	NO
		PM	0.76	C	0.68	B	-0.08	NO
29	Raitt Street and Edinger Avenue	AM	0.99	E	0.93	E	-0.06	NO
		PM	0.99	E	0.99	E	0.00	NO
30	Bear Street and MacArthur Boulevard	AM	0.71	C	0.76	C	0.05	NO
		PM	0.85	D	0.90	D	0.05	NO
31	Bristol Street and 17th Street	AM	0.87	D	0.85	D	-0.02	NO
		PM	0.82	D	0.81	D	-0.01	NO
32	Bristol Street and Civic Center Drive	AM	0.94	E	0.93	E	-0.01	NO
		PM	0.91	E	0.90	D	-0.01	NO
33	Bristol Street and Santa Ana Boulevard	AM	0.81	D	0.72	C	-0.09	NO
		PM	0.85	D	0.73	C	-0.12	NO
34	Bristol Street and 1st Street	AM	0.77	C	0.90	D	0.13	NO
		PM	0.83	D	0.97	E	0.14	YES
35	Bristol Street and McFadden Avenue	AM	0.96	E	0.95	E	-0.01	NO
		PM	1.03	F	0.89	D	-0.14	NO
36	Bristol Street and Warner Avenue	AM	0.92	E	0.96	E	0.04	YES
		PM	0.85	D	0.87	D	0.02	NO
37	Bristol Street and Segerstrom Avenue	AM	0.81	D	0.85	D	0.04	NO
		PM	0.87	D	0.92	E	0.05	YES
38	Bristol Street and Alton Avenue	AM	0.55	A	0.55	A	0.00	NO
		PM	0.63	B	0.58	A	-0.05	NO
39	Bristol Street and MacArthur Boulevard	AM	0.70	B	0.76	C	0.06	NO
		PM	0.77	C	0.80	C	0.03	NO
40	Bristol Street and Sunflower Avenue	AM	0.65	B	0.68	B	0.03	NO

ID	INTERSECTION	PEAK HOUR	2045 No Project		2045 With Project		DELTA	LOS IMPACT?
			V/C or Delay	LOS	V/C or Delay	LOS		
		PM	0.69	B	0.74	C	0.05	NO
41	Bristol Street and I-405 NB Ramps	AM	20.8	C	20.9	C	0.1	NO
		PM	28.5	C	29.0	C	0.5	NO
42	Bristol Street and I-405 SB Ramps	AM	23.9	C	23.6	C	-0.3	NO
		PM	18.3	B	18.2	B	-0.1	NO
43	Flower Street and Santa Ana Boulevard	AM	0.81	D	0.59	A	-0.22	NO
		PM	0.86	D	0.67	B	-0.19	NO
44	Flower Street and 1st Street	AM	0.93	E	0.93	E	0.00	NO
		PM	0.88	D	1.22	F	0.34	YES
45	Flower Street and McFadden Avenue	AM	0.90	D	0.81	D	-0.09	NO
		PM	1.04	F	0.97	E	-0.07	NO
46	Flower Street and Segerstrom Avenue	AM	0.78	C	0.82	D	0.04	NO
		PM	0.87	D	0.92	E	0.05	YES
47	Flower Street and MacArthur Boulevard	AM	0.68	B	0.71	C	0.03	NO
		PM	0.90	D	0.91	E	0.01	YES
48	Main Street and La Veta Avenue	AM	0.59	A	0.54	A	-0.05	NO
		PM	0.70	B	0.76	C	0.06	NO
49	Main Street and Mainplace Drive / Memory Lane	AM	0.51	A	0.46	A	-0.05	NO
		PM	0.56	A	0.50	A	-0.06	NO
50	Main Street and 17th Street	AM	0.88	D	0.85	D	-0.03	NO
		PM	0.76	C	0.80	C	0.04	NO
51	Main Street and Civic Center Drive	AM	0.78	C	0.85	D	0.07	NO
		PM	0.97	E	0.86	D	-0.11	NO
52	Main Street and Santa Ana Boulevard	AM	0.93	E	0.79	C	-0.14	NO
		PM	0.79	C	0.70	B	-0.09	NO
53	Main Street and 4th Street	AM	0.41	A	0.44	A	0.03	NO
		PM	0.45	A	0.47	A	0.02	NO

ID	INTERSECTION	PEAK HOUR	2045 No Project		2045 With Project		DELTA	LOS IMPACT?
			V/C or Delay	LOS	V/C or Delay	LOS		
54	Main Street and 1st Street	AM	0.68	B	0.75	C	0.07	NO
		PM	0.74	C	0.80	C	0.06	NO
55	Main Street and McFadden Avenue	AM	0.89	D	0.92	E	0.03	YES
		PM	0.93	E	0.94	E	0.01	NO
56	Main Street and Edinger Avenue	AM	0.80	C	0.88	D	0.08	NO
		PM	0.87	D	0.98	E	0.11	YES
57	Main Street and MacArthur Boulevard	AM	0.68	B	0.72	C	0.04	NO
		PM	0.75	C	0.78	C	0.03	NO
58	Penn Way and 17th Street	AM	12.2	B	12.1	B	-0.1	NO
		PM	25.5	C	20.3	C	-5.2	NO
59	Santiago Street / I-5 NB Ramps and 17th Street	AM	30.0	C	31.4	C	1.4	NO
		PM	19.0	B	20.3	C	1.3	NO
60	Penn Way and I-5 SB Ramps	AM	21.2	C	19.5	B	-1.7	NO
		PM	23.0	C	22.8	C	-0.2	NO
61	Santiago Street and Civic Center Drive	AM	159.2	F	62.6	F	-96.6	NO
		PM	110.0	F	27.6	D	-82.4	NO
62	Santiago Street and Santa Ana Boulevard	AM	1.04	F	1.44	F	0.40	YES
		PM	1.02	F	1.41	F	0.39	YES
63	Standard Avenue and 4th Street	AM	0.94	E	1.34	F	0.40	YES
		PM	0.89	D	1.34	F	0.45	YES
64	Standard Avenue and 1st Street	AM	0.98	E	1.52	F	0.54	YES
		PM	0.97	E	1.48	F	0.51	YES
65	Standard Avenue and McFadden Avenue	AM	0.85	D	0.84	D	-0.01	NO
		PM	0.83	D	0.77	C	-0.06	NO
66	Halladay Street and Warner Avenue	AM	0.78	C	0.69	B	-0.09	NO
		PM	0.97	E	0.82	D	-0.16	NO
67	Halladay Street and Dyer Road	AM	0.77	C	0.67	B	-0.10	NO

ID	INTERSECTION	PEAK HOUR	2045 No Project		2045 With Project		DELTA	LOS IMPACT?
			V/C or Delay	LOS	V/C or Delay	LOS		
		PM	1.06	F	0.95	E	-0.11	NO
68	SR-55 SB Ramps and MacArthur Boulevard	AM	19.9	B	20.2	C	0.3	NO
		PM	17.8	B	18.4	B	0.6	NO
69	SR-55 NB Ramps and MacArthur Boulevard	AM	19.3	B	19.5	B	0.2	NO
		PM	14.6	B	16.2	B	1.6	NO
70	SR-55 SB Ramps and Dyer Road	AM	25.7	C	25.9	C	0.2	NO
		PM	26.5	C	26.5	C	0.0	NO
71	Glassell Street and La Veta Avenue	AM	0.80	C	0.83	D	0.03	NO
		PM	0.74	C	0.77	C	0.03	NO
72	Glassell Street and SR-22 WB Ramps	AM	32.6	C	30.1	C	-2.5	NO
		PM	30.5	C	33.4	C	2.9	NO
73	Glassell Street / Grand Avenue and SR-22 EB Ramps	AM	30.6	C	29.6	C	-1.0	NO
		PM	31.6	C	31.8	C	0.2	NO
74	Grand Avenue and Fairhaven Avenue	AM	0.61	B	0.57	A	-0.04	NO
		PM	0.72	C	0.67	B	-0.05	NO
75	Grand Avenue and Santa Clara Avenue	AM	0.84	D	0.80	C	-0.04	NO
		PM	0.81	D	0.73	C	-0.08	NO
76	Grand Avenue and 17th Street	AM	0.79	C	0.86	D	0.07	NO
		PM	0.86	D	0.85	D	-0.01	NO
77	Grand Avenue and I-5 NB Ramps	AM	13.2	B	15.3	B	2.1	NO
		PM	10.1	B	11.3	B	1.2	NO
78	Grand Avenue and Santa Ana Boulevard	AM	26.0	C	26.3	C	0.3	NO
		PM	25.3	C	28.5	C	3.2	NO
79	Grand Avenue and 1st Street	AM	0.74	C	0.83	D	0.09	NO
		PM	0.73	C	0.72	C	-0.01	NO
80	Grand Avenue and Chestnut Avenue	AM	0.70	B	0.94	E	0.24	YES
		PM	0.69	B	0.78	C	0.09	NO

ID	INTERSECTION	PEAK HOUR	2045 No Project		2045 With Project		DELTA	LOS IMPACT?
			V/C or Delay	LOS	V/C or Delay	LOS		
81	Grand Avenue and McFadden Avenue	AM	0.89	D	1.01	F	0.12	YES
		PM	0.82	D	0.81	D	-0.01	NO
82	Grand Avenue and Edinger Avenue	AM	0.84	D	0.90	D	0.06	NO
		PM	1.00	E	1.08	F	0.08	YES
83	Grand Avenue and Warner Avenue	AM	0.61	B	0.66	B	0.05	NO
		PM	0.83	D	0.90	D	0.07	NO
84	SR-55 NB Ramps and Dyer Road	AM	17.1	B	17.3	B	0.2	NO
		PM	5.7	A	6.0	A	0.3	NO
85	Cambridge Street and La Veta Avenue	AM	31.6	D	23.4	C	-8.2	NO
		PM	19.7	C	26.1	D	6.4	NO
86	Cambridge Street and Fairhaven Avenue	AM	0.56	A	0.62	B	0.06	NO
		PM	0.45	A	0.53	A	0.08	NO
87	Mabury Street and 1st Street	AM	27.4	C	27.4	C	0.0	NO / NO *
		PM	27.1	C	27.2	C	0.1	NO / NO *
88	Tustin Street and La Veta Avenue	AM	0.45	A	0.36	A	-0.09	NO
		PM	0.38	A	0.34	A	-0.04	NO
89	Tustin Street and SR-22 WB On-Ramp	AM	12.2	B	13.2	B	1.0	NO
		PM	11.1	B	7.7	A	-3.4	NO
90	Tustin Street and SR-22 EB Off-Ramp / Seba Avenue	AM	25.0	C	23.8	C	-1.2	NO
		PM	23.1	C	23.3	C	0.2	NO
91	Tustin Street and Fairhaven Avenue	AM	1.20	F	1.24	F	0.04	YES
		PM	0.69	B	0.76	C	0.07	NO
92	Tustin Avenue and Santa Clara Avenue	AM	1.04	F	1.10	F	0.06	YES
		PM	0.71	C	0.78	C	0.07	NO
93	Tustin Avenue and 17th Street	AM	0.91	E	0.81	D	-0.10	NO
		PM	0.80	C	0.74	C	-0.06	NO
94	Tustin Avenue and 4th Street	AM	0.83	D	0.79	C	-0.04	NO

ID	INTERSECTION	PEAK HOUR	2045 No Project		2045 With Project		DELTA	LOS IMPACT?
			V/C or Delay	LOS	V/C or Delay	LOS		
		PM	0.71	C	0.78	C	0.07	NO
95	SR-55 SB Ramps / Auto Mall Drive and Edinger Avenue	AM	25.2	C	25.1	C	-0.1	NO / NO *
		PM	25.5	C	25.4	C	-0.1	NO / NO *
96	SR-55 NB Ramps / Del Amo Avenue and Newport Avenue	AM	25.4	C	25.6	C	0.2	NO
		PM	39.1	D	46.2	D	7.1	YES
97	Red Hill Avenue and Edinger Avenue	AM	0.61	B	0.62	B	0.01	NO
		PM	0.85	D	0.87	D	0.02	NO
98	Red Hill Avenue and Warner Avenue	AM	0.56	A	0.58	A	0.02	NO
		PM	0.88	D	0.95	E	0.07	YES
99	Red Hill Avenue and Barranca Parkway	AM	0.55	A	0.58	A	0.03	NO
		PM	0.54	A	0.59	A	0.05	NO
100	Red Hill Avenue and Alton Parkway	AM	0.86	D	0.93	E	0.07	YES
		PM	1.18	F	1.49	F	0.31	YES
101	Red Hill Avenue and MacArthur Boulevard	AM	0.69	B	0.71	C	0.02	NO
		PM	0.70	B	0.72	C	0.02	NO
102	Red Hill Avenue and Main Street	AM	0.72	C	0.73	C	0.01	NO
		PM	0.81	D	0.84	D	0.03	NO
103	I-5 SB Ramps and Santa Ana Boulevard	AM	17.4	B	17.8	B	0.4	NO
		PM	18.4	B	19.9	B	1.5	NO
104	Tustin Ranch Road and Warner Avenue	AM	0.52	A	0.53	A	0.01	NO
		PM	0.65	B	0.67	B	0.02	NO
105	Von Karman Avenue and Barranca Parkway	AM	0.93	E	0.93	E	0.00	NO
		PM	1.23	F	1.28	F	0.05	YES

**Intersection is a Caltrans intersection and also part of OC CMP; XX/YY denotes LOS impacts per Caltrans/CMP impact criteria, respectively.*

Table 8-4: LOS Improvement Measures

ID	INTERSECTION	IMPROVEMENT
3	Euclid Street and Edinger Avenue	• Alternative 1 o Convert the WB right-turn lane into a shared thru-right. • Alternative 2 o Add EB-LT lane. • Alternative 3 o Add LT lane for EB and WB directions. • Fair-Share: 9.0% (AM)
13	Harbor Boulevard and Segerstrom Avenue	• Add a second WB left-turn lane. • Change WB and EB left-turn control to protected.
34	Bristol Street and 1st Street	• Maintain 2045 NP configuration for WB approach. o WB approach to be: 1L, 2T, 1TR • Add EB right-turn pocket.
36	Bristol Street and Warner Avenue	• Feasible improvements could not be identified.
37	Bristol Street and Segerstrom Avenue	• Maintain 2045 NP configurations for EB/WB approaches. o EB/WB approaches to be: 1LT, 2T, 1TR • Add NB right-turn pocket.
44	Flower Street and 1st Street	• Maintain 2045 NP configurations for NB approach. o NB approach to be: 1L, 2T, 1R
46	Flower Street and Segerstrom Avenue	• Feasible improvements could not be identified.
47	Flower Street and MacArthur Boulevard	• Feasible improvements could not be identified.
55	Main Street and McFadden Avenue	• Feasible improvements could not be identified.
56	Main Street and Edinger Avenue	• Feasible improvements could not be identified.
62	Santiago Street and Santa Ana Boulevard	• Feasible improvements could not be identified.

ID	INTERSECTION	IMPROVEMENT
63	Standard Avenue and 4th Street	• Maintain 2045 NP conditions. o NB / SB to be: 1L, 1T, 1R o EB / WB to be: 1L, 1T, 1TR
64	Standard Avenue and 1st Street	• Feasible improvements could not be identified.
80	Grand Avenue and Chestnut Avenue	• Feasible improvements could not be identified.
81	Grand Avenue and McFadden Avenue	• Maintain 2045 NP configuration for EB approach o EB approach to be: 1L, 1T, 1TR
82	Grand Avenue and Edinger Avenue	• Feasible improvements could not be identified.
91	Tustin Street and Fairhaven Avenue	• Maintain 2045 NP lane configurations for EB and WB approaches o EB approach to be: 1L, 1T, 1TR o WB approach to be: 1L, 1T, 1R
92	Tustin Avenue and Santa Clara Avenue	• Maintain 2045 NP lane configurations for EB and WB approaches. o EB and WB approaches to be: 1L, 1T, 1TR
96	SR-55 NB Ramps / Del Amo Avenue and Newport Avenue	• Add a second SB right-turn lane. • Fair-Share: 10.6% (PM)
98	Red Hill Avenue and Warner Avenue	• Add a left-turn lane to the EB approach. • Fair-Share: 17.3% (PM)
100	Red Hill Avenue and Alton Parkway	• Add a NB thru lane. • Add a left-turn lane and a shared thru-right to the EB approach. • Convert the WB right-turn lane into a shared thru-right. • Fair-Share: 21.0% (AM) / 34.7% (PM)
105	Von Karman Avenue and Barranca Parkway	• Convert one (1) WB thru lane into a shared thru-right. • Fair-Share: 14.1% (PM)

Table 8-5: Future Year (2045) With Project – Mitigated Impact Summary

ID	INTERSECTION	PEAK HOUR	2045 No Project		2045 With Project		DELTA	IMPACT?	2045 With Project – Mit.		DELTA	IMPACT?
			V/C or Delay	LOS	V/C or Delay	LOS			V/C or Delay	LOS		
3	Euclid Street and Edinger Avenue (with Alternative 1 measure)	AM	0.99	E	1.01	F	0.02	YES	1.00	E	0.01	NO
		PM	0.89	D	0.86	D	-0.03	NO	0.80	D	-0.09	NO
3	Euclid Street and Edinger Avenue (with Alternative 2 measure)	AM	0.99	E	1.01	F	0.02	YES	1.00	E	0.01	NO
		PM	0.89	D	0.86	D	-0.03	NO	0.82	D	-0.07	NO
3	Euclid Street and Edinger Avenue (with Alternative 3 measure)	AM	0.99	E	1.01	F	0.02	YES	0.97	E	-0.02	NO
		PM	0.89	D	0.86	D	-0.03	NO	0.82	D	-0.07	NO
13	Harbor Boulevard and Segerstrom Avenue	AM	0.97	E	1.01	F	0.04	YES	0.96	E	-0.01	NO
		PM	0.92	E	0.93	E	0.01	NO	0.93	E	0.01	NO
34	Bristol Street and 1st Street	AM	0.77	C	0.90	D	0.13	NO	0.79	C	0.02	NO
		PM	0.83	D	0.97	E	0.14	YES	0.87	D	0.04	NO
36	Bristol Street and Warner Avenue	AM	0.92	E	0.96	E	0.04	YES	0.96	E	0.04	YES
		PM	0.85	D	0.87	D	0.02	NO	0.87	D	0.02	NO
37	Bristol Street and Segerstrom Avenue	AM	0.81	D	0.85	D	0.04	NO	0.81	D	0.00	NO
		PM	0.87	D	0.92	E	0.05	YES	0.87	D	0.00	NO
44	Flower Street and 1st Street	AM	0.93	E	0.93	E	0.00	NO	0.93	E	0.00	NO
		PM	0.88	D	1.22	F	0.34	YES	0.92	E	0.04	YES
46	Flower Street and Segerstrom Avenue	AM	0.78	C	0.82	D	0.04	NO	0.82	D	0.04	NO
		PM	0.87	D	0.92	E	0.05	YES	0.92	E	0.05	YES
47	Flower Street and Macarthur Boulevard	AM	0.68	B	0.71	C	0.03	NO	0.71	C	0.03	NO
		PM	0.90	D	0.91	E	0.01	YES	0.91	E	0.01	YES
55	Main Street and McFadden Avenue	AM	0.89	D	0.92	E	0.03	YES	0.92	E	0.03	YES
		PM	0.93	E	0.94	E	0.01	NO	0.94	E	0.01	NO
56	Main Street and Edinger Avenue	AM	0.80	C	0.88	D	0.08	NO	0.88	D	0.08	NO
		PM	0.87	D	0.98	E	0.11	YES	0.98	E	0.11	YES

ID	INTERSECTION	PEAK HOUR	2045 No Project		2045 With Project		DELTA	IMPACT?	2045 With Project – Mit.		DELTA	IMPACT?
			V/C or Delay	LOS	V/C or Delay	LOS			V/C or Delay	LOS		
62	Santiago Street and Santa Ana Boulevard	AM	1.04	F	1.44	F	0.40	YES	1.44	F	0.40	YES
		PM	1.02	F	1.41	F	0.39	YES	1.41	F	0.39	YES
63	Standard Avenue and 4th Street	AM	0.94	E	1.34	F	0.40	YES	1.06	F	0.12	YES
		PM	0.89	D	1.34	F	0.45	YES	1.03	F	0.14	YES
64	Standard Avenue and 1st Street	AM	0.98	E	1.52	F	0.54	YES	1.52	F	0.54	YES
		PM	0.97	E	1.48	F	0.51	YES	1.48	F	0.51	YES
80	Grand Avenue and Chestnut Avenue	AM	0.70	B	0.94	E	0.24	YES	0.94	E	0.24	YES
		PM	0.69	B	0.78	C	0.09	NO	0.78	C	0.09	NO
81	Grand Avenue and McFadden Avenue	AM	0.89	D	1.01	F	0.12	YES	0.86	D	-0.03	NO
		PM	0.82	D	0.81	D	-0.01	NO	0.81	D	-0.01	NO
82	Grand Avenue and Edinger Avenue	AM	0.84	D	0.90	D	0.06	NO	0.90	D	0.06	NO
		PM	1.00	E	1.08	F	0.08	YES	1.08	F	0.08	YES
91	Tustin Street and Fairhaven Avenue	AM	1.20	F	1.24	F	0.04	YES	1.06	F	-0.14	NO
		PM	0.69	B	0.76	C	0.07	NO	0.64	B	-0.05	NO
92	Tustin Avenue and Santa Clara Avenue	AM	1.04	F	1.10	F	0.06	YES	0.94	E	-0.10	NO
		PM	0.71	C	0.78	C	0.07	NO	0.68	B	-0.03	NO
96	SR-55 NB Ramps / Del Amo Avenue and Newport Avenue	AM	25.4	C	25.6	C	0.2	NO	24.4	C	-1.0	NO
		PM	39.1	D	46.2	D	7.1	YES	28.5	C	-10.6	NO
98	Red Hill Avenue and Warner Avenue	AM	0.56	A	0.58	A	0.02	NO	0.56	A	0.00	NO
		PM	0.88	D	0.95	E	0.07	YES	0.80	D	-0.08	NO
100	Red Hill Avenue and Alton Parkway	AM	0.86	D	0.93	E	0.07	YES	0.68	B	-0.18	NO
		PM	1.18	F	1.49	F	0.31	YES	1.28	F	0.10	YES
105	Von Karman Avenue and Barranca Parkway	AM	0.93	E	0.93	E	0.00	NO	0.93	E	0.00	NO
		PM	1.23	F	1.28	F	0.05	YES	1.24	F	0.01	NO

As shown in Table 8-5, impacts are expected to remain at the following intersections:

- **INT 36 – Bristol Street and Warner Avenue (City of Santa Ana)**
    - Due to limited right-of-way, feasible improvements could not be developed. The LOS cannot be returned to an acceptable LOS; the impact is to remain in the AM peak hour.
- **INT 44 – Flower Street and 1st Street (City of Santa Ana)**
    - A mitigation measure was developed but only resulted in partial mitigation of the impact. The LOS cannot be returned to an acceptable LOS; the impact is to remain in the PM peak hour.
- **INT 46 – Flower Street and Segerstrom Avenue (City of Santa Ana)**
    - Due to limited right-of-way, feasible improvements could not be developed. The LOS cannot be returned to an acceptable LOS; the impact is to remain in the PM peak hour.
- **INT 47 – Flower Street and Segerstrom Avenue (City of Santa Ana)**
    - Due to limited right-of-way, feasible improvements could not be developed. The LOS cannot be returned to an acceptable LOS; the impact is to remain in the PM peak hour.
- **INT 55 – Main Street and McFadden Avenue (City of Santa Ana)**
    - Due to limited right-of-way, feasible improvements could not be developed. The LOS cannot be returned to an acceptable LOS; the impact is to remain in the AM peak hour.
- **INT 56 – Main Street and Edinger Avenue (City of Santa Ana)**
    - Due to limited right-of-way, feasible improvements could not be developed. The LOS cannot be returned to an acceptable LOS; the impact is to remain in the PM peak hour.
- **INT 62 – Santiago Street and Santa Ana Boulevard (City of Santa Ana)**
    - Due to limited right-of-way and the implementation of the OC Streetcar, a mitigation measure was developed but only resulted in partial mitigation of the impact. The LOS cannot be returned to an acceptable LOS; the impact is to remain in the AM and PM peak hours.
- **INT 63 – Standard Avenue and 4th Street (City of Santa Ana)**
    - A mitigation measure was developed but only resulted in partial mitigation of the impact. The LOS cannot be returned to an acceptable LOS; the impact is to remain in the AM and PM peak hours.

- **INT 64 – Standard Avenue and 1st Street (City of Santa Ana)**
    - Due to limited right-of-way and the planned implementation of the bike lanes along both Standard Avenue and First Street, no feasible improvements were identified.The LOS cannot be returned to an acceptable LOS; the impact is to remain in the AM and PM peak hours.
- **INT 80 – Grand Avenue and Chestnut Avenue (City of Santa Ana)**
    - Due to limited right-of-way and the planned implementation of the bike lanes along Chestnut Avenue, no feasible improvements were identified. The LOS cannot be returned to an acceptable LOS; the impact is to remain in the AM peak hour.
- **INT 82 – Grand Avenue and Edinger Avenue (City of Santa Ana)**
    - Due to limited right-of-way, feasible improvements could not be developed. The LOS cannot be returned to an acceptable LOS; the impact is to remain in the PM peak hour.
- **INT 100 – Red Hill Avenue and Alton Parkway (City of Irvine)**
    - Due to limited right-of-way, a mitigation measure was developed and resulted in mitigating the impacts in the AM peak hour only. The LOS cannot be returned to an acceptable LOS in the PM peak hour; additionally, the impact is to remain in the PM peak hour.

9.0 Conclusions

This traffic analysis report has been prepared in support of the General Plan Update (GPU) Draft Program Environmental Impact Report (PEIR) for the City of Santa Ana and to assess requested changes to the Orange County Transportation Authority's (OCTA) Master Plan of Arterial Highways (MPAH) by the City of Santa Ana. The purpose of this study is to identify traffic impacts due to the implementation of the proposed project (General Plan update). This study was prepared in conformance with CEQA guidelines and in consultation with the OPR Technical Advisory as well as the traffic impact analysis guidelines for the City of Santa Ana and local jurisdictions. CEQA identifies VMT as the preferred metric for identifying environmental impacts. However, intersection impacts and roadway LOS are also evaluated in order to address local network performance. A summary of the analysis is provided below.

9.1 VMT IMPACT ANALYSIS

A VMT analysis was conducted to determine impacts due to the implementation of the project. The project was evaluated to determine the potential project level and cumulative impacts.

A project is considered to have a significant impact at the project level if the project VMT/SP (for the land use plan) exceeds 15% below the existing total daily VMT/SP for the County. The VMT impact threshold (15% below the 2020 – No Project County VMT/SP) is 22.0 VMT/SP. The project resulted in a 2045 – With Project City VMT/SP of 20.3 VMT/SP. Thus, the project does not result in a project level VMT impact.

A project is considered to have a significant cumulative impact if the project results in a negative effect on VMT/SP at the citywide level (City VMT/SP is higher with the project than without it). The VMT impact threshold (2045 – No Project City VMT/SP) is 22.8 VMT/SP. The project resulted in a 2045 – With Project City VMT/SP of 20.3 VMT/SP. The project does not result in a negative impact on citywide VMT, therefore does not result in a cumulative impact.

9.2 INTERSECTION LOS ANALYSIS

An intersection impact analysis was conducted in order to determine projects impacts related to the OCTA MPAH and City-level LOS guidelines at study intersections. It should be noted that intersection impacts may be identified but are not indicative of an environmental impact, per CEQA. Significant impacts are expected at twenty-four (24) study intersections for the Future year (2045) With Project scenario. Mitigation of the impacts is not required for CEQA conformance, but measures were developed to address local network performance. All mitigation measures are subject to the approval of the presiding public authority, including funding and implementation. The City of Santa Ana will update its Transportation System Improvement Assessment (TSIA) in order to provide funding for the implementation of the City of Santa Ana Circulation Element Transportation Network buildout. For mitigation measures in adjoining jurisdictions, the City of Santa Ana will contribute to the fairshare of the corresponding mitigation measures.

All identified impacts are expected to be mitigated with the exception of:

- INT 36 – Bristol Street and Warner Avenue (City of Santa Ana)
- INT 44 – Flower Street and 1st Street (City of Santa Ana)
- INT 46 – Flower Street and Segerstrom Avenue (City of Santa Ana)
- INT 47 – Flower Street and Segerstrom Avenue (City of Santa Ana)
- INT 55 – Main Street and McFadden Avenue (City of Santa Ana)

- INT 56 – Main Street and Edinger Avenued (City of Santa Ana)
- INT 62 – Santiago Street and Santa Ana Boulevard (City of Santa Ana)
- INT 63 – Standard Avenue and 4th Street (City of Santa Ana)
- INT 64 – Standard Avenue and 1st Street (City of Santa Ana)
- INT 80 – Grand Avenue and Chestnut Avenue (City of Santa Ana)
- INT 82 – Grand Avenued and Alton Parkway (City of Santa Ana)
- INT 100 – Red Hill Avenue and Alton Parkway (City of Irvine)

9.3 ROADWAY LOS

Roadway LOS was assessed to determine conformance with the City of Santa Ana MPSH minimum level of service requirements. Eleven study roadways currently operate at an unacceptable LOS. Thirteen study roadways are expected to operate at an unacceptable LOS in the Future Year (2045) No Project scenario. Eight roadways are expected to operate at an unacceptable LOS in the Future Year (2045) With Project scenario. Although the LOS exceeds the standard established in the Circulation Element, physical improvements are not feasible due to right-of-way constraints.

10.0 Appendices

- APPENDIX A – LANE GEOMETRIES
- APPENDIX B – INTERSECTION VOLUMES
- APPENDIX C – OCTAM VOLUME PLOTS
- APPENDIX D – ANALYSIS WORKSHEETS
- APPENDIX E – MITIGATION WORKSHEETS
- APPENDIX F – VMT CALCULATIONS
- APPENDIX G – OCTAM 2045 VOLUME COMPARISON PLOTS
- APPENDIX H - COUNTS

APPENDIX A
LANE GEOMETRIES

APPENDIX A.1

LANE GEOMETRIES – 2020 NP

Santa Ana Circulation Element

Scenario Report

Scenario:	2020 NP AM Peak Hour
Command:	Default Command
Volume:	2020 NP AM
Geometry:	NP
Impact Fee:	Default Impact Fee
Trip Generation:	Default Trip Generation
Trip Distribution:	Default Trip Distribution
Paths:	Default Path
Routes:	Default Route
Configuration:	Default Configuration

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Santa Ana Circulation Element

Lane Geometry Report

Number of approach lanes: (L) (LT) (T) (RT) (R) (LTR)

Node	Intersection	NB	SB	EB	WB
1	Euclid St and 1st St	102100	102100	102100	102100
2	Euclid St and McFadden Ave	102100	102100	101100	101100
3	Euclid St and Edinger Ave	202100	202100	102010	102010
4	Newhope St and Hazard Ave	101100	101100	101100	101100
5	Newhope St and McFadden Ave	101100	101100	101100	101100
6	Harbor Blvd and SR-22 WB Off-Ramp /	103000	002100	000001	110010
7	Trask Avenue and SR-22 EB On-Ramp	000000	000000	201000	001100
8	Harbor Blvd and Westminster Ave	103010	103010	102100	103010
9	Harbor Blvd and 1st St	103010	103010	202100	202100
10	Harbor Blvd and McFadden Ave	203010	202100	101100	101100
11	Harbor Blvd and Edinger Ave	203010	203010	202100	203010
12	Harbor Blvd and Warner Ave	202100	203010	203010	203010
13	Harbor Blvd and Segerstrom Ave	202100	102100	101100	102010
14	MacArthur Blvd and Hyland Ave	200100	101010	103010	103010
15	MacArthur Blvd and Harbor Blvd	203010	203010	103010	103010
16	Harbor Blvd and Sunflower Ave	203010	203010	101100	110100
17	Harbor Blvd and I-405 WB Off-Ramp	004000	004000	000000	100011
18	Harbor Blvd and I-405 EB Off-Ramp	003010	004000	100011	000000
19	Fairview St and Civic Center Dr	102100	102100	010100	110010
20	Fairview St and 1st St	103010	102100	102100	102100
21	Fairview St and McFadden Ave	103010	103010	101100	101100
22	Fairview St and Edinger Ave	202100	202100	202100	202100
23	Fairview St and Warner Ave	202100	202100	202100	202100
24	Fairview St and MacArthur Blvd	202100	203010	203010	203010
25	Fairview Rd and Sunflower Ave	203010	202100	201100	202010
26	Greenville St and Edinger Ave	100010	000000	002100	102000
27	Greenville St and Segerstrom Ave	101100	101010	101100	101100
28	Raitt St and McFadden Ave	101100	101100	101100	101100
29	Raitt St and Edinger Ave	101100	101100	101100	101100
30	Bear St and MacArthur Blvd	102010	102010	102100	102100
31	Bristol St and 17th St	203010	203010	202100	203010
32	Bristol St and Civic Center Dr	102100	102100	101100	101100
33	Bristol St and Santa Ana Blvd	102100	102100	101100	101100
34	Bristol St and 1st St	203010	203010	102100	102100
35	Bristol St and McFadden Ave	103010	103010	101100	101100
36	Bristol St and Warner Ave	202010	202100	103010	103010
37	Bristol St and Segerstrom Ave	102100	102100	101100	101100
38	Bristol St and Alton Ave	102100	103010	101100	100100
39	Bristol St and MacArthur Blvd	203010	202100	203010	203010
40	Bristol St and Sunflower Ave	202110	203010	202110	203010
41	Bristol St and I-405 WB Ramps	004010	004100	000020	111020
42	Bristol St and I-405 EB Ramps	104000	002110	300010	000000
43	Flower St and Santa Ana Blvd	102010	102010	103010	102010
44	Flower St and 1st St	102010	101100	102100	102100
45	Flower St and McFadden Ave	100100	100100	100100	100100
46	Flower St and Segerstrom Ave	101100	101100	101100	101100
47	Flower St and MacArthur Blvd	101100	101100	102100	102100

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Santa Ana Circulation Element

Number of approach lanes: (L) (LT) (T) (RT) (R) (LTR)

Node	Intersection	NB	SB	EB	WB
48	Main St and La Veta Ave	203010	202100	202100	202010
49	Main St and Mainplace Dr / Memory L	202100	202100	202100	202010
50	Main St and 17th St	202010	201100	202100	202100
51	Main St and Civic Center Dr	101100	101100	101100	101100
52	Main St and Santa Ana Blvd	102000	001100	000000	011100
53	Main St and 4th St	001100	001100	000100	000100
54	Main St and 1st St	101100	102010	102100	102100
55	Main St and McFadden Ave	101100	101100	100100	101010
56	Main St and Edinger Ave	101100	101100	102010	102100
57	Main St and MacArthur Blvd	203010	203010	203010	203010
58	Penn Wy and 17th St	200020	000000	002100	103000
59	I-5 NB Off Ramps/17th Street	110010	100010	103010	002100
60	Penn Wy and I-5 SB Ramps	002010	202000	000000	100020
61	Santiago St and Civic Center Dr	100100	100100	010010	000001
62	Santiago St and Santa Ana Blvd	101010	101010	101100	102010
64	Standard Ave and 1st St	100100	000001	101100	101100
65	Standard Ave and Mcfadden Ave	101100	101100	101100	101100
66	Halladay St and Warner Ave	100010	000000	001100	011000
67	Halladay St and Dyer Rd	100100	100100	102100	102100
68	SR-55 SB Ramps and MacArthur Blvd	000000	200020	002110	003010
69	SR-55 NB Ramps and MacArthur Blvd	200010	000000	002020	003010
70	SR-55 SB Ramps and Dyer Rd	110020	011010	103010	203010
71	Glassell St and La Veta Ave	201010	101100	101010	101100
72	Glassell St and SR-22 WB Ramps	102000	002010	000000	100011
73	Grand Ave / Glassell St and SR-22 E	002010	102000	100011	000000
74	Grand Ave and Fairhaven Ave	102100	102010	101100	101010
75	Grand Ave and Santa Clara Ave	102100	101100	100100	101010
76	Grand Ave and 17th St	102100	102010	202100	202100
77	Grand Ave and I-5 NB Ramps	002010	103000	000000	200010
78	Grand Ave and Santa Ana Blvd	102100	102020	201020	010100
79	Grand Ave and 1st St	202100	203010	202100	201100
80	Grand Ave and Chestnut Ave	102100	102100	101100	101100
81	Grand Ave and McFadden Ave	102100	102100	101100	101100
82	Grand Ave and Edinger Ave	102100	102100	102100	102100
83	Grand Ave and Warner Ave	102100	102100	203010	202010
84	SR-55 NB Ramps and Dyer Rd	100011	000000	003010	002100
85	Cambridge St and La Veta Ave	101000	001010	100010	000000
86	Cambridge St and Fairhaven Ave	000000	100010	102000	001100
87	Mabury St and 1st Street	100010	100011	002100	102100
88	Tustin St and La Veta Ave	102100	102100	101100	110010
89	Tustin St and SR-22 WB On-Ramp	202000	002100	000000	000000
90	Tustin St and SR-22 EB Off-Ramp / S	002100	103000	100011	000001
91	Tustin Ave and Fairhaven Ave	202100	202100	101100	101010
92	Tustin Ave and Santa Clara Ave	102100	202100	101100	101100
93	Tustin Ave and 17th St	202010	202100	203010	203010
94	Tustin Ave and 4th St	102100	202010	102100	102100
95	SR-55 SB Ramps / Auto Mall Dr and E	110020	101010	103010	202100
96	SR-55 NB Ramps / Del Amo Ave and Ne	202100	103010	201100	101100
97	Red Hill Ave and Edinger Ave	203010	203010	203010	203010
98	Red Hill Ave and Warner Ave	203100	202100	102100	203010

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Santa Ana Circulation Element

Number of approach lanes: (L) (LT) (T) (RT) (R) (LTR)

Node	Intersection	NB	SB	EB	WB
99	Red Hill Ave and Dyer Rd / barranca	204010	204010	203100	204010
100	Red Hill Ave and Alton Pkwy	103010	103010	102010	201010
101	Red Hill Ave and MacArthur Blvd	202100	203010	203010	103010
102	Red Hill Ave and Main St	202010	202010	202100	202100
103	I-5 SB Ramps and Santa Ana Blvd	000000	200010	203000	002100
104	Tustin Ranch Rd and Warner Ave	000000	200010	203000	003010
105	Von Karman Ave and Barranca Pkwy	201100	202020	203010	204010
106	Red Hill Avenue and El Camino Real	203010	102100	101010	100100
107	Red Hill Avenue and I-5 NB Ramps	103000	003010	000000	200010
108	Red Hill Avenue and I-5 SB Ramps	004010	103000	010010	000000
109	Red Hill Avenue and Nisson Road	103100	102100	100100	100100
110	Red Hill Avenue and Walnut Avenue	102100	102100	101100	201100
111	Red Hill Avenue and Valencia Avenue	103100	102100	101010	201010
112	Tustin Ranch Road and Warner Avenue	003010	203000	000000	300020
113	Tustin Ranch Road and Walnut Avenue	203010	203010	201100	202010
114	SR-55 SB Ramps and Irvine Boulevard	000000	100011	002100	103000
115	SR-55 NB Ramps and Irvine Boulevard	100011	000000	103000	002100

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APPENDIX A.2

LANE GEOMETRIES – 2045 NP

Santa Ana Circulation Element

Scenario Report

Scenario:	2045 NP AM Peak Hour
Command:	Default Command
Volume:	2045 NP AM
Geometry:	MPAH
Impact Fee:	Default Impact Fee
Trip Generation:	Default Trip Generation
Trip Distribution:	Default Trip Distribution
Paths:	Default Path
Routes:	Default Route
Configuration:	Default Configuration

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Santa Ana Circulation Element

Lane Geometry Report

Number of approach lanes: (L) (LT) (T) (RT) (R) (LTR)

Node	Intersection	NB	SB	EB	WB
1	Euclid St and 1st St	102100	102100	102100	102100
2	Euclid St and McFadden Ave	102100	102100	101100	101100
3	Euclid St and Edinger Ave	202100	202100	102010	102010
4	Newhope St and Hazard Ave	101100	101100	101100	101100
5	Newhope St and McFadden Ave	101100	101100	101100	101100
6	Harbor Blvd and SR-22 WB Off-Ramp /	103000	002100	000001	110010
7	Trask Avenue and SR-22 EB On-Ramp	000000	000000	201000	001100
8	Harbor Blvd and Westminster Ave	103010	103010	102100	103010
9	Harbor Blvd and 1st St	103010	103010	202100	202100
10	Harbor Blvd and McFadden Ave	203010	202100	101100	101100
11	Harbor Blvd and Edinger Ave	203010	203010	202100	203010
12	Harbor Blvd and Warner Ave	202100	203010	203010	203010
13	Harbor Blvd and Segerstrom Ave	202100	102100	101100	102010
14	MacArthur Blvd and Hyland Ave	200100	101010	103010	103010
15	MacArthur Blvd and Harbor Blvd	203010	203010	103010	103010
16	Harbor Blvd and Sunflower Ave	203010	203010	101100	110100
17	Harbor Blvd and I-405 NB Off-Ramp	004000	004000	000000	100011
18	Harbor Blvd and I-405 SB Off-Ramp	003010	004000	100011	000000
19	Fairview St and Civic Center Dr	102100	102100	010100	110010
20	Fairview St and 1st St	103010	102100	102100	102100
21	Fairview St and McFadden Ave	103010	103010	101100	101100
22	Fairview St and Edinger Ave	202100	202100	202100	202100
23	Fairview St and Warner Ave	202100	202100	202100	202100
24	Fairview St and MacArthur Blvd	202100	203010	203010	203010
25	Fairview Rd and Sunflower Ave	203010	202100	201100	202010
26	Greenville St and Edinger Ave	100010	000000	002100	103000
27	Greenville St and Segerstrom Ave	100100	100100	102100	102100
28	Raitt St and McFadden Ave	101100	101100	101100	101100
29	Raitt St and Edinger Ave	101100	101100	102100	102100
30	Bear St and MacArthur Blvd	102010	102010	102100	102100
31	Bristol St and 17th St	203010	203010	202100	203010
32	Bristol St and Civic Center Dr	102100	102100	101100	101100
33	Bristol St and Santa Ana Blvd	102100	102100	102100	102100
34	Bristol St and 1st St	203010	203010	102100	102100
35	Bristol St and McFadden Ave	103010	103010	101100	101100
36	Bristol St and Warner Ave	202100	202100	103010	103010
37	Bristol St and Segerstrom Ave	102100	102100	102100	102100
38	Bristol St and Alton Ave	102100	103010	101100	100100
39	Bristol St and MacArthur Blvd	203010	202100	203010	203010
40	Bristol St and Sunflower Ave	202110	203010	202110	203010
41	Bristol St and I-405 NB Ramps	004010	004100	000020	111020
42	Bristol St and I-405 SB Ramps	104000	002110	300010	000000
43	Flower St and Santa Ana Blvd	102010	102010	103010	102100
44	Flower St and 1st St	102010	101100	102100	102100
45	Flower St and McFadden Ave	010100	010100	010100	010100
46	Flower St and Segerstrom Ave	101100	101100	102100	102100
47	Flower St and MacArthur Blvd	101100	101100	102100	102100

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Santa Ana Circulation Element

Number of approach lanes: (L) (LT) (T) (RT) (R) (LTR)

Node	Intersection	NB	SB	EB	WB
48	Main St and La Veta Ave	203010	202100	202100	202010
49	Main St and Mainplace Dr / Memory L	202100	202100	202100	202010
50	Main St and 17th St	202010	202010	202100	202100
51	Main St and Civic Center Dr	101100	101100	101100	101100
52	Main St and Santa Ana Blvd	102000	001100	000000	010100
53	Main St and 4th St	001100	001100	001100	001100
54	Main St and 1st St	101100	102010	102100	102100
55	Main St and McFadden Ave	101100	101100	101100	101100
56	Main St and Edinger Ave	101100	101100	102100	102100
57	Main St and MacArthur Blvd	203010	203010	203010	203010
58	Penn Wy and 17th St	200020	000000	002100	103000
59	I-5 NB Off Ramps/17th Street	110010	100010	103010	002100
60	Penn Wy and I-5 SB Ramps	002010	202000	000000	100020
61	Santiago St and Civic Center Dr	100100	100100	010010	000001
62	Santiago St and Santa Ana Blvd	101100	101100	101100	102010
63	Standard Ave and 4th St	102010	102010	101100	101100
64	Standard Ave and 1st St	101100	101100	102100	102100
65	Standard Ave and Mcfadden Ave	101100	101100	101100	101100
66	Halladay St and Warner Ave	100010	000000	002100	012000
67	Halladay St and Dyer Rd	100100	100100	102100	102100
68	SR-55 SB Ramps and MacArthur Blvd	000000	200020	002110	003010
69	SR-55 NB Ramps and MacArthur Blvd	200010	000000	002020	003010
70	SR-55 SB Ramps and Dyer Rd	110020	011010	103010	203010
71	Glassell St and La Veta Ave	201010	101100	101010	101100
72	Glassell St and SR-22 WB Ramps	102000	002010	000000	100011
73	Grand Ave / Glassell St and SR-22 E	002010	102000	100011	000000
74	Grand Ave and Fairhaven Ave	102100	102100	101100	101010
75	Grand Ave and Santa Clara Ave	102100	102100	100100	101010
76	Grand Ave and 17th St	102100	102100	202100	202100
77	Grand Ave and I-5 NB Ramps	002010	103000	000000	200010
78	Grand Ave and Santa Ana Blvd	102100	102020	201020	010100
79	Grand Ave and 1st St	202100	203010	202100	202100
80	Grand Ave and Chestnut Ave	102100	102100	101100	101100
81	Grand Ave and McFadden Ave	102100	102100	101100	101100
82	Grand Ave and Edinger Ave	102100	102100	102100	102100
83	Grand Ave and Warner Ave	102100	102100	203010	202100
84	SR-55 NB Ramps and Dyer Rd	100011	000000	003010	002100
85	Cambridge St and La Veta Ave	101000	001010	100010	000000
86	Cambridge St and Fairhaven Ave	000000	100010	102000	001100
87	Mabury St and 1st Street	100010	100011	002100	102100
88	Tustin St and La Veta Ave	102100	102100	101100	110010
89	Tustin St and SR-22 WB On-Ramp	202000	002100	000000	000000
90	Tustin St and SR-22 EB Off-Ramp / S	002100	103000	100011	000001
91	Tustin Ave and Fairhaven Ave	202100	202100	101100	101010
92	Tustin Ave and Santa Clara Ave	102100	202100	101100	101100
93	Tustin Ave and 17th St	202010	202100	203010	203010
94	Tustin Ave and 4th St	102100	202010	102110	102100
95	SR-55 SB Ramps / Auto Mall Dr and E	110020	101010	103010	202100
96	SR-55 NB Ramps / Del Amo Ave and Ne	202100	103010	201100	101100
97	Red Hill Ave and Edinger Ave	203010	203010	203010	203010

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Santa Ana Circulation Element

Number of approach lanes: (L) (LT) (T) (RT) (R) (LTR)

Node	Intersection	NB	SB	EB	WB
98	Red Hill Ave and Warner Ave	203100	202100	102100	203010
99	Red Hill Ave and Dyer Rd / barranca	204010	204010	203100	204010
100	Red Hill Ave and Alton Pkwy	103010	103010	102010	201010
101	Red Hill Ave and MacArthur Blvd	202100	203010	203010	103010
102	Red Hill Ave and Main St	202010	202010	202100	202100
103	I-5 SB Ramps and Santa Ana Blvd	000000	200010	203000	002100
104	Tustin Ranch Rd and Warner Ave	000000	200010	203000	003010
105	Von Karman Ave and Barranca Pkwy	201100	202020	203010	204010
106	Red Hill Avenue and El Camino Real	203010	102100	101010	100100
107	Red Hill Avenue and I-5 NB Ramps	103000	003010	000000	200010
108	Red Hill Avenue and I-5 SB Ramps	004010	103000	010010	000000
109	Red Hill Avenue and Nisson Road	103100	102100	100100	100100
110	Red Hill Avenue and Walnut Avenue	102100	102100	101100	201100
111	Red Hill Avenue and Valencia Avenue	103100	102100	101010	201010
112	Tustin Ranch Road and Warner Avenue	003010	203000	000000	300020
113	Tustin Ranch Road and Walnut Avenue	203010	203010	201100	202010
114	SR-55 SB Ramps and Irvine Boulevard	000000	100011	002100	103000
115	SR-55 NB Ramps and Irvine Boulevard	100011	000000	103000	002100

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APPENDIX A.3

LANE GEOMETRIES – 2045 WP

Santa Ana Circulation Element

Scenario Report

Scenario:	2045 WP AM Peak Hour
Command:	Default Command
Volume:	2045 AM
Geometry:	SACE
Impact Fee:	Default Impact Fee
Trip Generation:	Default Trip Generation
Trip Distribution:	Default Trip Distribution
Paths:	Default Path
Routes:	Default Route
Configuration:	Default Configuration

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Santa Ana Circulation Element

Lane Geometry Report

Number of approach lanes: (L) (LT) (T) (RT) (R) (LTR)

Node	Intersection	NB	SB	EB	WB
1	Euclid St and 1st St	102100	102100	102100	102100
2	Euclid St and McFadden Ave	102100	102100	101100	101100
3	Euclid St and Edinger Ave	202100	202100	102010	102010
4	Newhope St and Hazard Ave	101100	101100	100100	100100
5	Newhope St and McFadden Ave	101100	101100	101100	101100
6	Harbor Blvd and SR-22 WB Off-Ramp /	103000	002100	000001	110010
7	Trask Avenue and SR-22 EB On-Ramp	000000	000000	201000	001100
8	Harbor Blvd and Westminster Ave	103010	103010	102100	103010
9	Harbor Blvd and 1st St	103010	103010	202100	202100
10	Harbor Blvd and McFadden Ave	203010	202100	101100	100100
11	Harbor Blvd and Edinger Ave	203010	203010	202100	203010
12	Harbor Blvd and Warner Ave	202100	203010	203010	203010
13	Harbor Blvd and Segerstrom Ave	202100	102100	101100	102010
14	MacArthur Blvd and Hyland Ave	200100	101010	103010	103010
15	MacArthur Blvd and Harbor Blvd	203010	203010	103010	103010
16	Harbor Blvd and Sunflower Ave	203010	203010	101100	110100
17	Harbor Blvd and I-405 NB Off-Ramp	004000	004000	000000	100011
18	Harbor Blvd and I-405 SB Off-Ramp	003010	004000	100011	000000
19	Fairview St and Civic Center Dr	102100	102100	010100	110010
20	Fairview St and 1st St	103010	102100	102100	102100
21	Fairview St and McFadden Ave	103010	103010	100100	100100
22	Fairview St and Edinger Ave	202100	202100	202100	202100
23	Fairview St and Warner Ave	202100	202100	202100	202100
24	Fairview St and MacArthur Blvd	202100	203010	203010	203010
25	Fairview Rd and Sunflower Ave	203010	202100	201100	202010
26	Greenville St and Edinger Ave	100010	000000	002100	103000
27	Greenville St and Segerstrom Ave	100100	100100	102100	102100
28	Raitt St and McFadden Ave	100100	100100	100100	100100
29	Raitt St and Edinger Ave	100100	100100	102100	102100
30	Bear St and MacArthur Blvd	102010	102010	102100	102100
31	Bristol St and 17th St	203010	203010	202100	203010
32	Bristol St and Civic Center Dr	102100	102100	100100	101100
33	Bristol St and Santa Ana Blvd	102100	102100	100100	100100
34	Bristol St and 1st St	203010	203010	102100	101100
35	Bristol St and McFadden Ave	103010	103010	100100	100100
36	Bristol St and Warner Ave	202100	202100	103010	103010
37	Bristol St and Segerstrom Ave	102100	102100	102100	102100
38	Bristol St and Alton Ave	102100	103010	101100	100100
39	Bristol St and MacArthur Blvd	203010	202100	203010	203010
40	Bristol St and Sunflower Ave	202110	203010	202110	203010
41	Bristol St and I-405 NB Ramps	004010	004100	000020	111020
42	Bristol St and I-405 SB Ramps	104000	002110	300010	000000
43	Flower St and Santa Ana Blvd	102010	102010	102010	102010
44	Flower St and 1st St	101010	101100	101100	101100
45	Flower St and McFadden Ave	100100	100100	100100	100100
46	Flower St and Segerstrom Ave	101100	101100	102100	102100
47	Flower St and MacArthur Blvd	101100	101100	102100	102100

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Santa Ana Circulation Element

Number of approach lanes: (L) (LT) (T) (RT) (R) (LTR)

Node	Intersection	NB	SB	EB	WB
48	Main St and La Veta Ave	203010	202100	202100	202010
49	Main St and Mainplace Dr / Memory L	202100	202100	202100	202010
50	Main St and 17th St	202010	202010	202100	202100
51	Main St and Civic Center Dr	101100	101100	101100	101100
52	Main St and Santa Ana Blvd	102000	001100	000000	010100
53	Main St and 4th St	001100	001100	001100	001100
54	Main St and 1st St	101100	102010	101100	101100
55	Main St and McFadden Ave	101100	101100	100100	100100
56	Main St and Edinger Ave	101100	101100	102100	102100
57	Main St and MacArthur Blvd	203010	203010	203010	203010
58	Penn Wy and 17th St	200020	000000	002100	103000
59	I-5 NB Off Ramps/17th Street	110010	100010	103010	002100
60	Penn Wy and I-5 SB Ramps	002010	202000	000000	100020
61	Santiago St and Civic Center Dr	100100	100100	100100	000001
62	Santiago St and Santa Ana Blvd	101010	101010	100100	102010
63	Standard Ave and 4th St	101010	101010	100100	100100
64	Standard Ave and 1st St	100100	000001	101100	101100
65	Standard Ave and Mcfadden Ave	100100	100100	100100	100100
66	Halladay St and Warner Ave	100010	000000	002100	012000
67	Halladay St and Dyer Rd	100100	100100	102100	102100
68	SR-55 SB Ramps and MacArthur Blvd	000000	200020	002110	003010
69	SR-55 NB Ramps and MacArthur Blvd	200010	000000	002020	003010
70	SR-55 SB Ramps and Dyer Rd	110020	011010	103010	203010
71	Glassell St and La Veta Ave	201010	101100	101010	101100
72	Glassell St and SR-22 WB Ramps	102000	002010	000000	100011
73	Grand Ave / Glassell St and SR-22 E	002010	102000	100011	000000
74	Grand Ave and Fairhaven Ave	102100	102100	101100	100100
75	Grand Ave and Santa Clara Ave	102100	102100	100100	101010
76	Grand Ave and 17th St	102100	102100	202100	202100
77	Grand Ave and I-5 NB Ramps	002010	103000	000000	200010
78	Grand Ave and Santa Ana Blvd	102100	102020	201020	010100
79	Grand Ave and 1st St	202100	203010	202010	202010
80	Grand Ave and Chestnut Ave	102100	102100	100100	100100
81	Grand Ave and McFadden Ave	102100	102100	100100	101100
82	Grand Ave and Edinger Ave	102100	102100	102100	102100
83	Grand Ave and Warner Ave	102100	102100	203010	202100
84	SR-55 NB Ramps and Dyer Rd	100011	000000	003010	002100
85	Cambridge St and La Veta Ave	101000	001010	100010	000000
86	Cambridge St and Fairhaven Ave	000000	100010	101000	000100
87	Mabury St and 1st Street	100010	100011	001100	101100
88	Tustin St and La Veta Ave	102100	102100	101100	110010
89	Tustin St and SR-22 WB On-Ramp	202000	002100	000000	000000
90	Tustin St and SR-22 EB Off-Ramp / S	002100	103000	100011	000001
91	Tustin Ave and Fairhaven Ave	202100	202100	100100	100100
92	Tustin Ave and Santa Clara Ave	102100	202100	100100	100100
93	Tustin Ave and 17th St	202010	202100	203010	203010
94	Tustin Ave and 4th St	102100	202010	102110	102100
95	SR-55 SB Ramps / Auto Mall Dr and E	110020	101010	103010	202100
96	SR-55 NB Ramps / Del Amo Ave and Ne	202100	103010	201100	101100
97	Red Hill Ave and Edinger Ave	203010	203010	203010	203010

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Santa Ana Circulation Element

Number of approach lanes: (L) (LT) (T) (RT) (R) (LTR)

Node	Intersection	NB	SB	EB	WB
98	Red Hill Ave and Warner Ave	203100	202100	102100	203010
99	Red Hill Ave and Dyer Rd / barranca	204010	204010	203100	204010
100	Red Hill Ave and Alton Pkwy	103010	103010	102010	201010
101	Red Hill Ave and MacArthur Blvd	202100	203010	203010	103010
102	Red Hill Ave and Main St	202010	202010	202100	202100
103	I-5 SB Ramps and Santa Ana Blvd	000000	200010	203000	002100
104	Tustin Ranch Rd and Warner Ave	000000	200010	203000	003010
105	Von Karman Ave and Barranca Pkwy	201100	202020	203010	204010
106	Red Hill Avenue and El Camino Real	203010	102100	101010	100100
107	Red Hill Avenue and I-5 NB Ramps	103000	003010	000000	200010
108	Red Hill Avenue and I-5 SB Ramps	004010	103000	010010	000000
109	Red Hill Avenue and Nisson Road	103100	102100	100100	100100
110	Red Hill Avenue and Walnut Avenue	102100	102100	101100	201100
111	Red Hill Avenue and Valencia Avenue	103100	102100	101010	201010
112	Tustin Ranch Road and Warner Avenue	003010	203000	000000	300020
113	Tustin Ranch Road and Walnut Avenue	203010	203010	201100	202010
114	SR-55 SB Ramps and Irvine Boulevard	000000	100011	002100	103000
115	SR-55 NB Ramps and Irvine Boulevard	100011	000000	103000	002100

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APPENDIX B
INTERSECTION VOLUMES

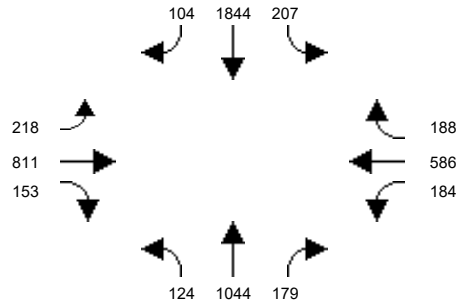
APPENDIX B.1

INTERSECTION VOLUMES –
2020 NP (AM PEAK HOUR)

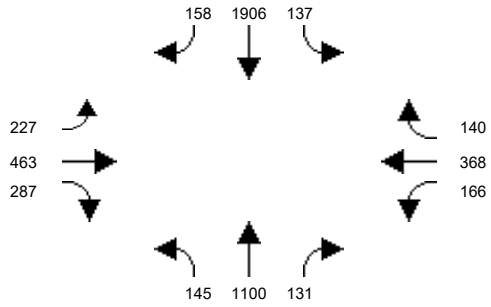
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2020 NP AM Peak Hour

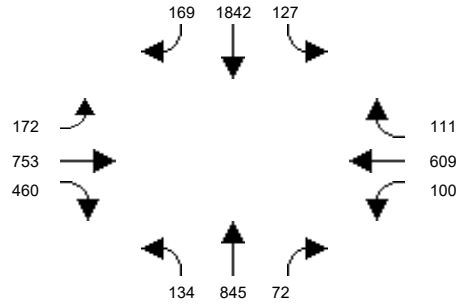
Intersection #1: Euclid St and 1st St



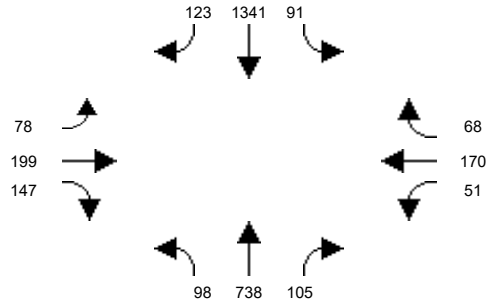
Intersection #2: Euclid St and McFadden Ave



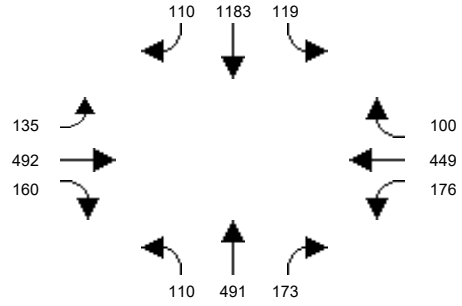
Intersection #3: Euclid St and Edinger Ave



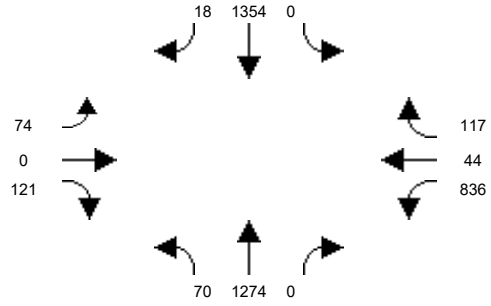
Intersection #4: Newhope St and Hazard Ave



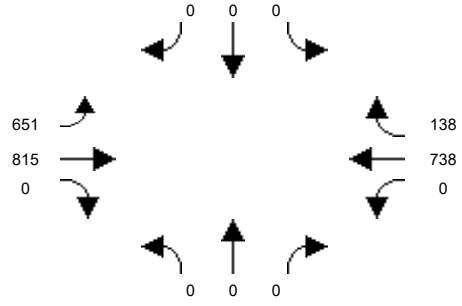
Intersection #5: Newhope St and McFadden Ave



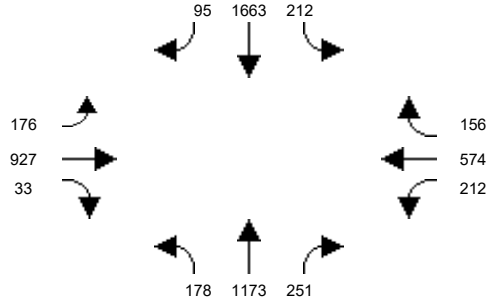
Intersection #6: Harbor Blvd and SR-22 WB Off-Ramp / Banner Dr



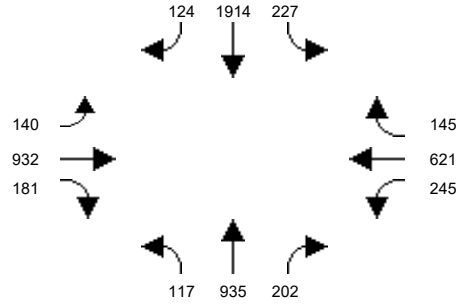
Intersection #7: Trask Avenue and SR-22 EB On-Ramp



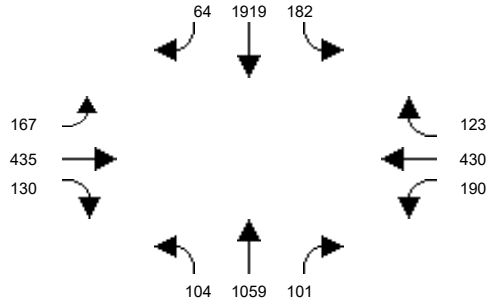
Intersection #8: Harbor Blvd and Westminster Ave



Intersection #9: Harbor Blvd and 1st St



Intersection #10: Harbor Blvd and McFadden Ave

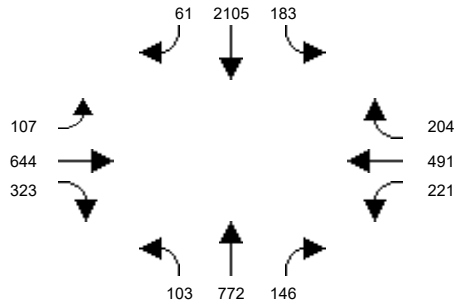


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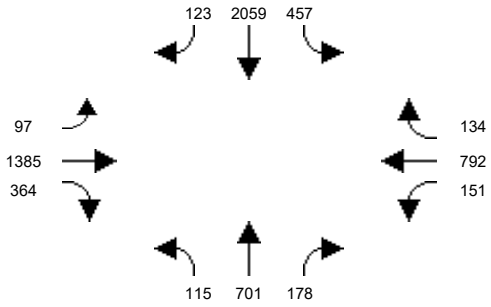
Santa Ana Circulation Element

Intersection Graphic Report
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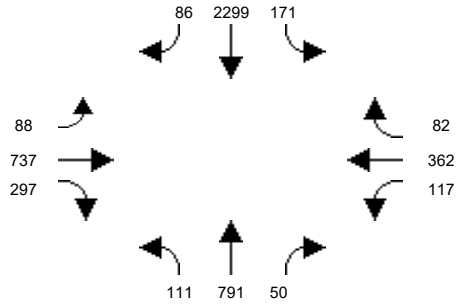
Intersection #11: Harbor Blvd and Edinger Ave



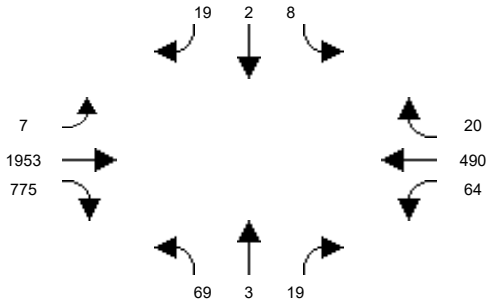
Intersection #12: Harbor Blvd and Warner Ave



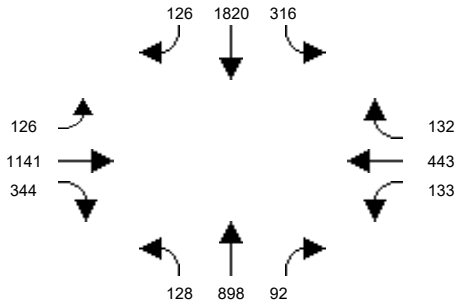
Intersection #13: Harbor Blvd and Segerstrom Ave



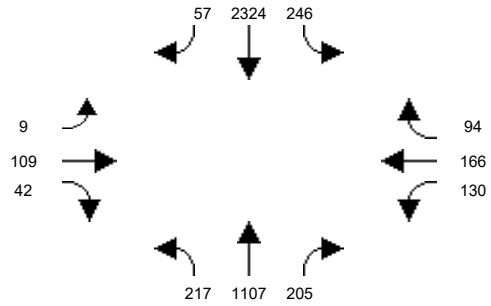
Intersection #14: MacArthur Blvd and Hyland Ave



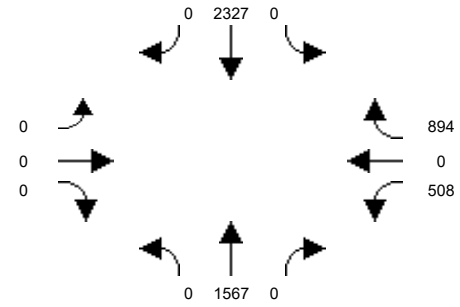
Intersection #15: MacArthur Blvd and Harbor Blvd



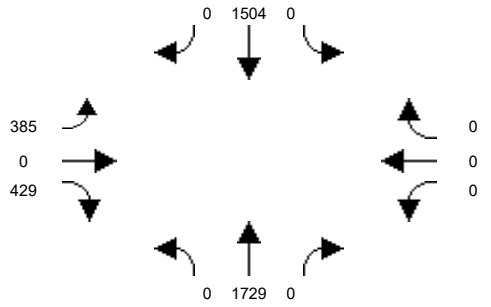
Intersection #16: Harbor Blvd and Sunflower Ave



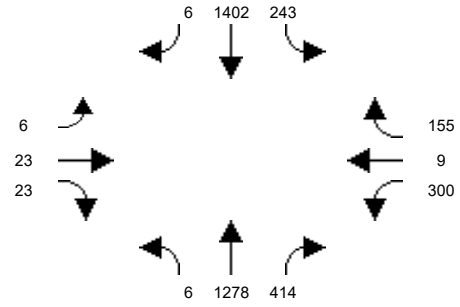
Intersection #17: Harbor Blvd and I-405 WB Off-Ramp



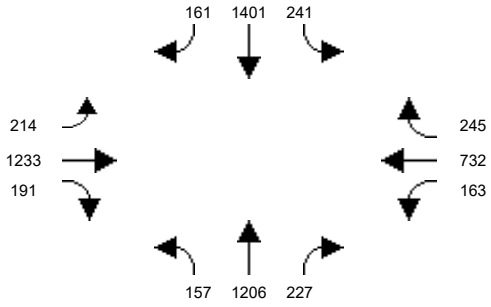
Intersection #18: Harbor Blvd and I-405 EB Off-Ramp



Intersection #19: Fairview St and Civic Center Dr



Intersection #20: Fairview St and 1st St

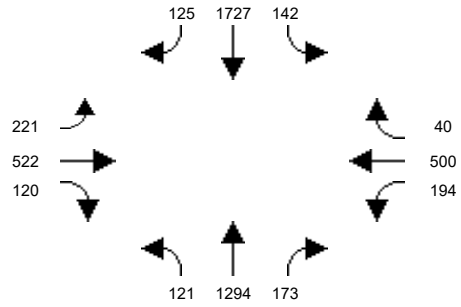


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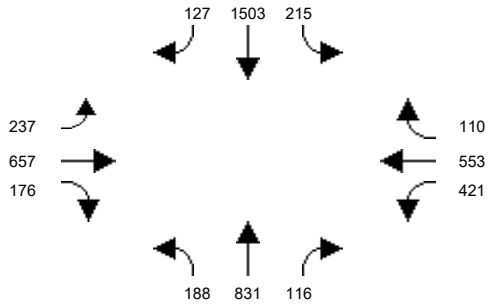
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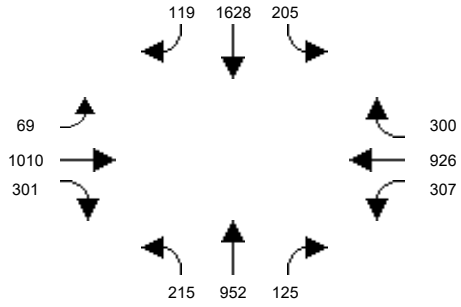
Intersection #21: Fairview St and McFadden Ave



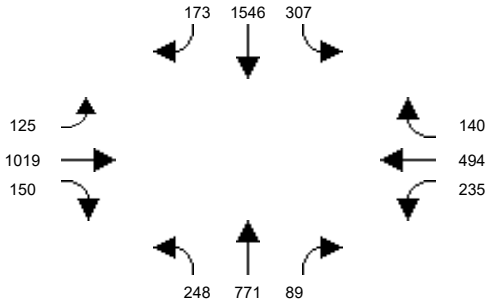
Intersection #22: Fairview St and Edinger Ave



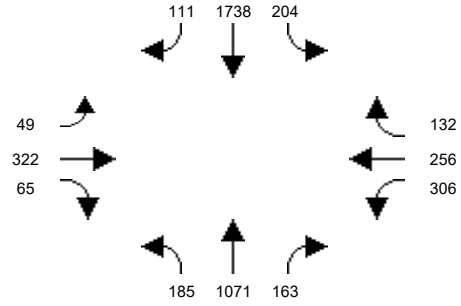
Intersection #23: Fairview St and Warner Ave



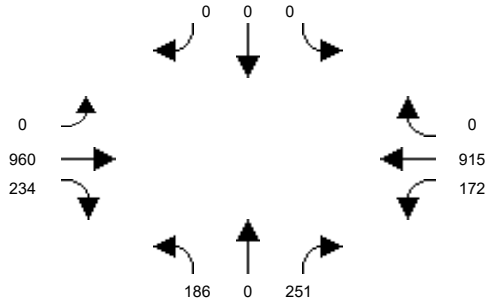
Intersection #24: Fairview St and MacArthur Blvd



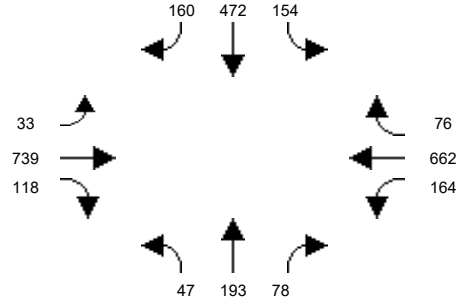
Intersection #25: Fairview Rd and Sunflower Ave



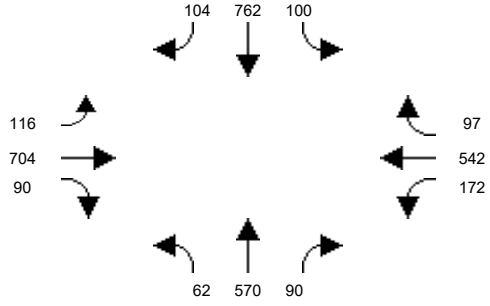
Intersection #26: Greenville St and Edinger Ave



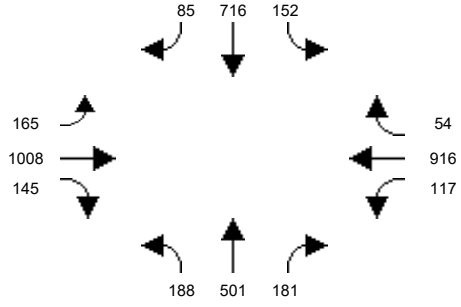
Intersection #27: Greenville St and Segerstrom Ave



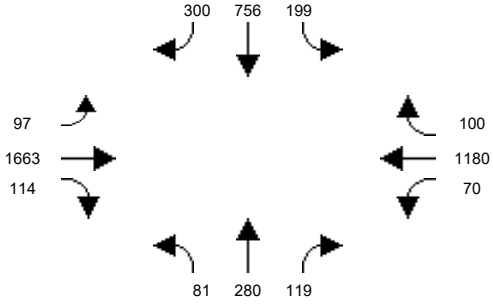
Intersection #28: Raitt St and McFadden Ave



Intersection #29: Raitt St and Edinger Ave



Intersection #30: Bear St and MacArthur Blvd

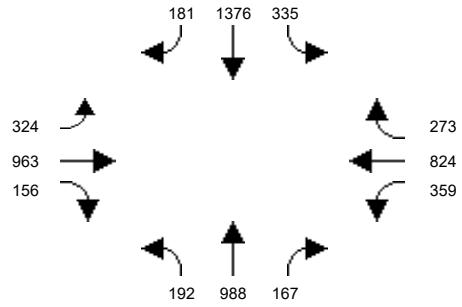


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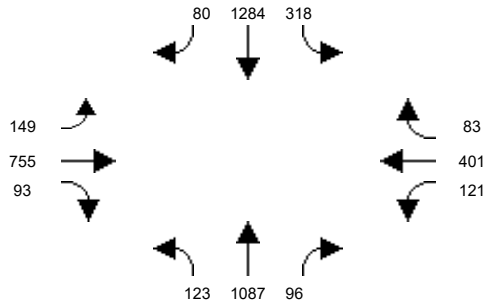
Santa Ana Circulation Element

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2020 NP AM Peak Hour

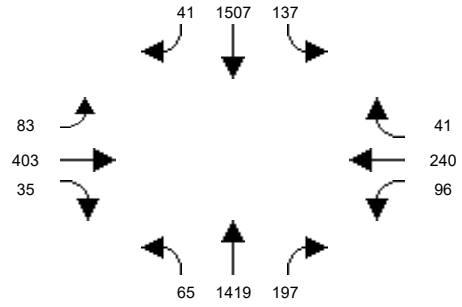
Intersection #31: Bristol St and 17th St



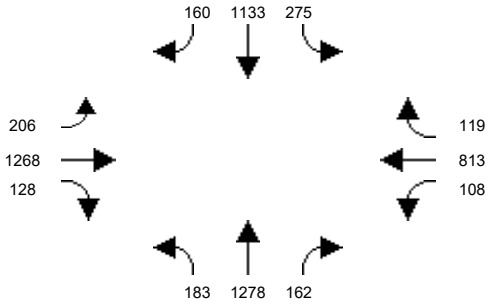
Intersection #32: Bristol St and Civic Center Dr



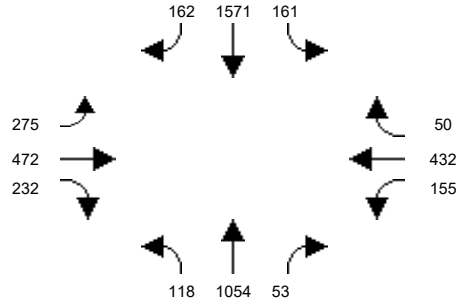
Intersection #33: Bristol St and Santa Ana Blvd



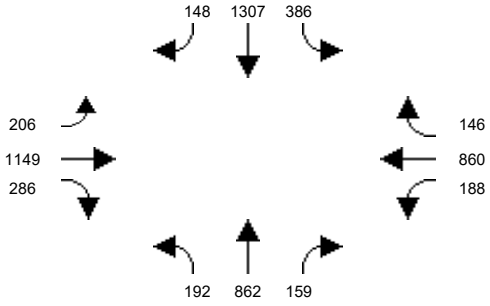
Intersection #34: Bristol St and 1st St



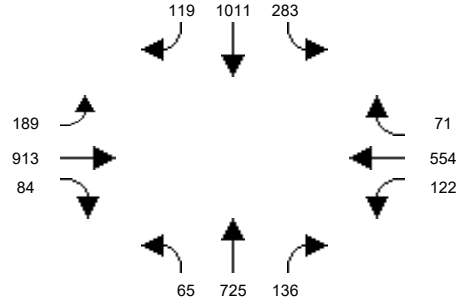
Intersection #35: Bristol St and McFadden Ave



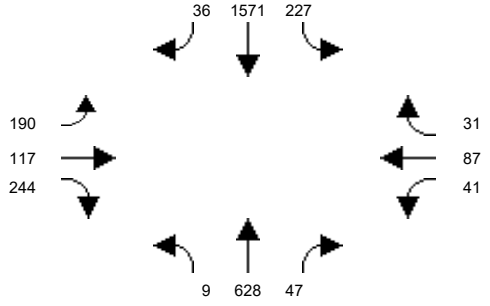
Intersection #36: Bristol St and Warner Ave



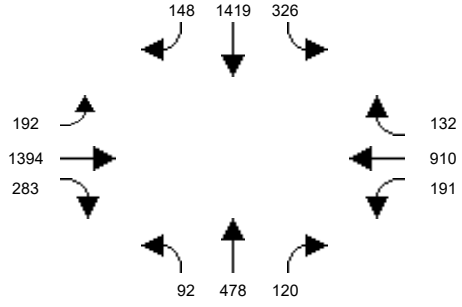
Intersection #37: Bristol St and Segerstrom Ave



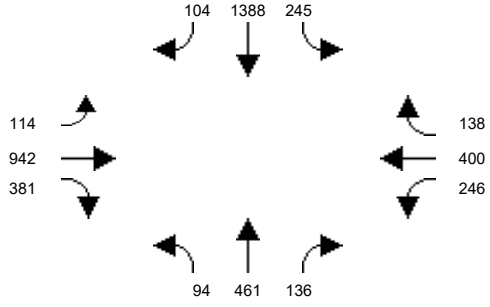
Intersection #38: Bristol St and Alton Ave



Intersection #39: Bristol St and MacArthur Blvd



Intersection #40: Bristol St and Sunflower Ave

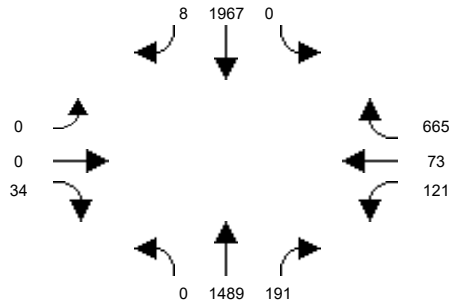


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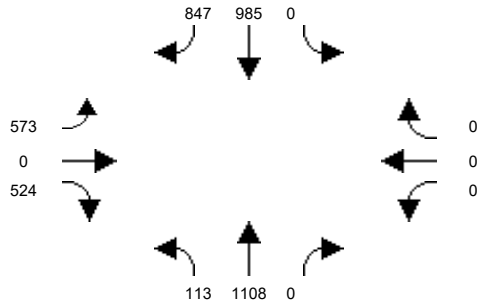
Santa Ana Circulation Element

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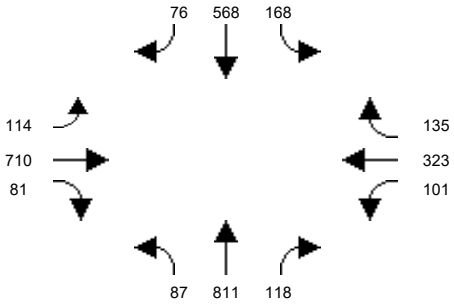
Intersection #41: Bristol St and I-405 WB Ramps



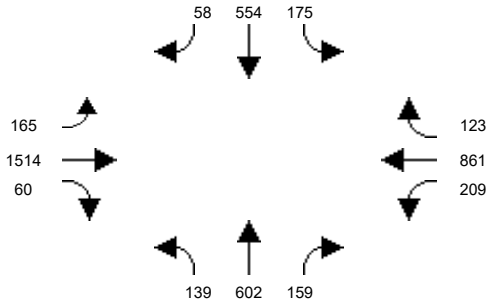
Intersection #42: Bristol St and I-405 EB Ramps



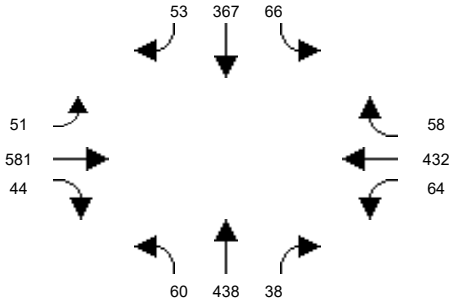
Intersection #43: Flower St and Santa Ana Blvd



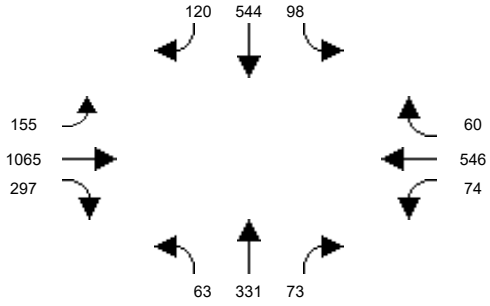
Intersection #44: Flower St and 1st St



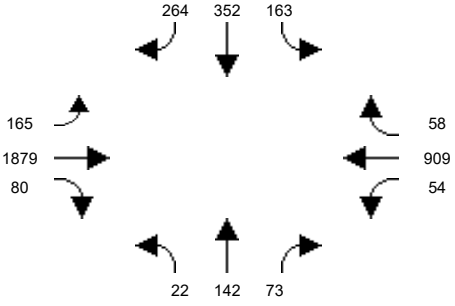
Intersection #45: Flower St and McFadden Ave



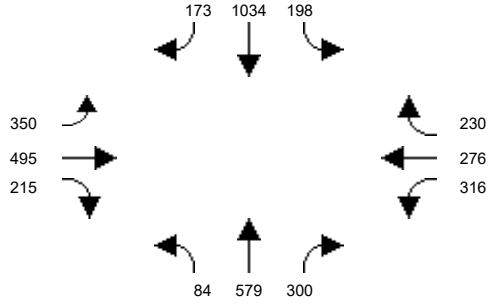
Intersection #46: Flower St and Segerstrom Ave



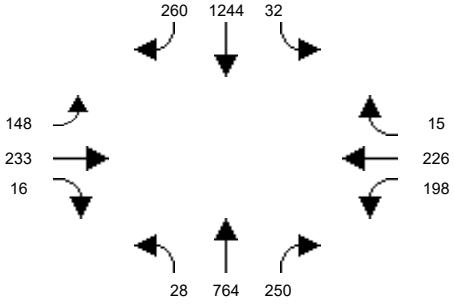
Intersection #47: Flower St and MacArthur Blvd



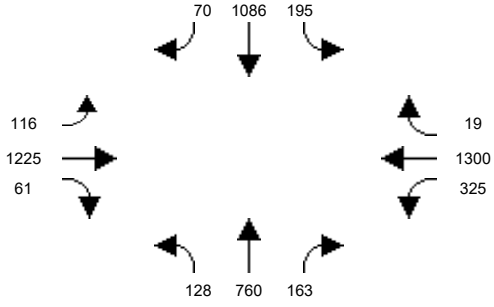
Intersection #48: Main St and La Veta Ave



Intersection #49: Main St and Mainplace Dr / Memory Ln



Intersection #50: Main St and 17th St

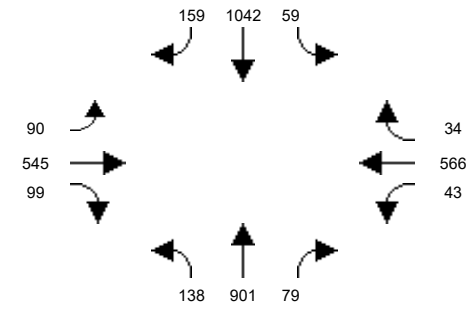


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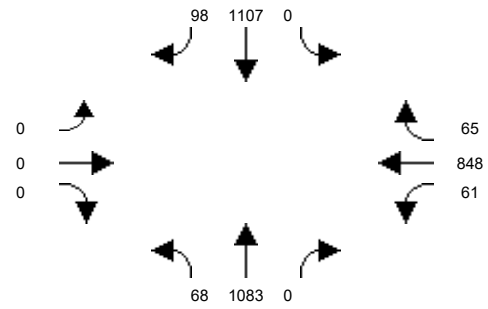
Santa Ana Circulation Element

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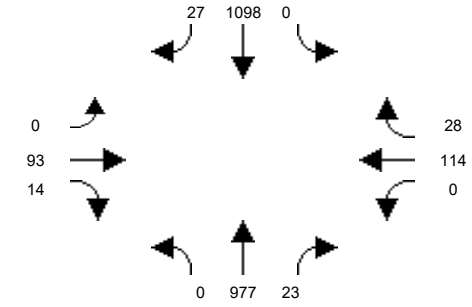
Intersection #51: Main St and Civic Center Dr



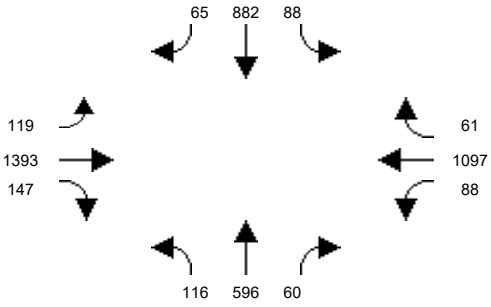
Intersection #52: Main St and Santa Ana Blvd



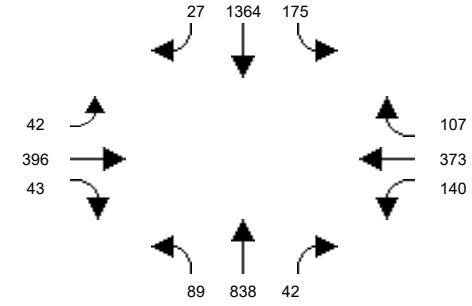
Intersection #53: Main St and 4th St



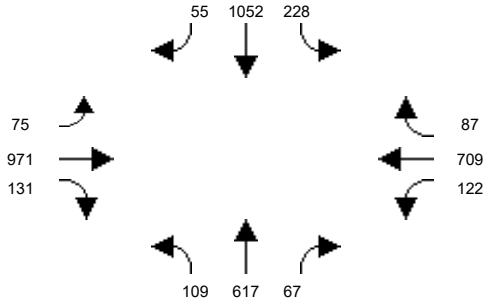
Intersection #54: Main St and 1st St



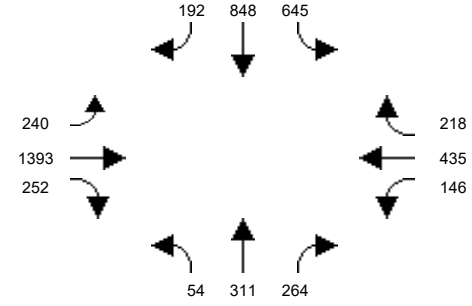
Intersection #55: Main St and McFadden Ave



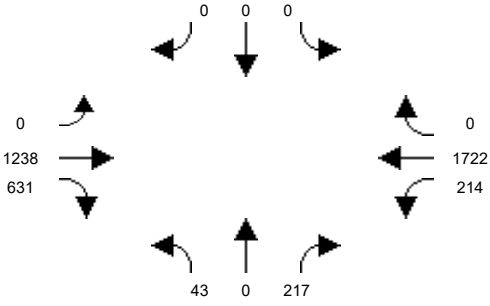
Intersection #56: Main St and Edinger Ave



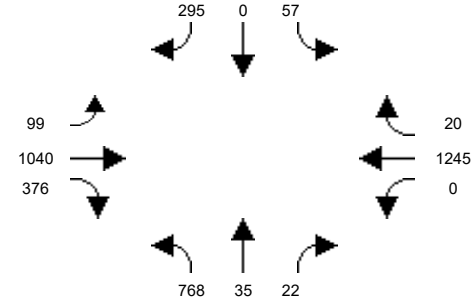
Intersection #57: Main St and MacArthur Blvd



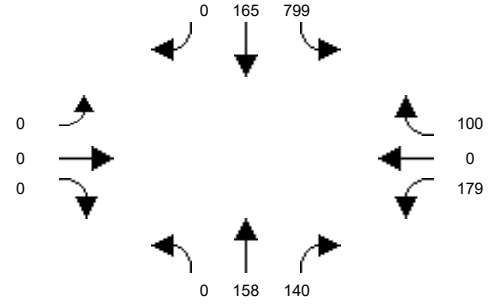
Intersection #58: Penn Wy and 17th St



Intersection #59: I-5 NB Off Ramps/17th Street



Intersection #60: Penn Wy and I-5 SB Ramps

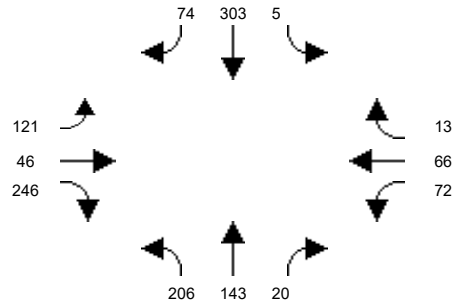


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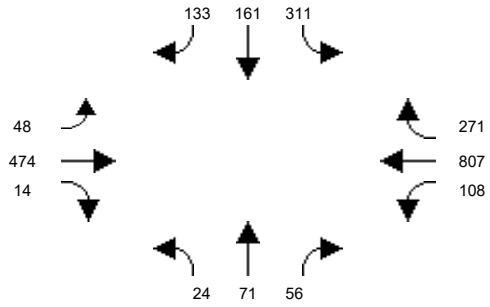
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Entered Volume (Base Alternative)
2020 NP AM Peak Hour

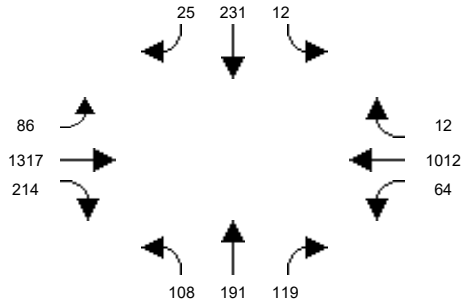
Intersection #61: Santiago St and Civic Center Dr



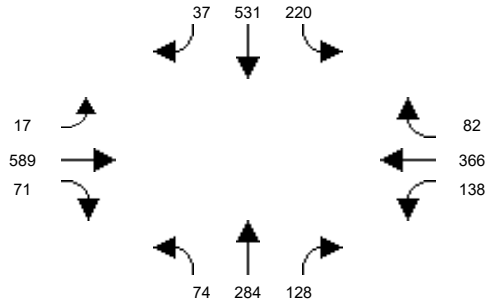
Intersection #62: Santiago St and Santa Ana Blvd



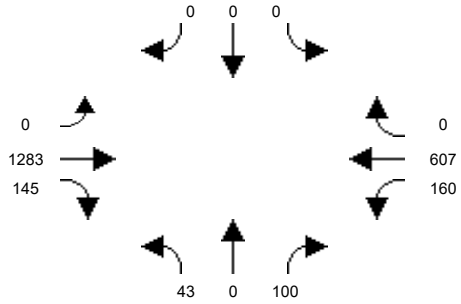
Intersection #64: Standard Ave and 1st St



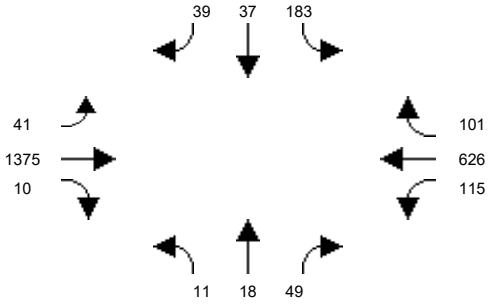
Intersection #65: Standard Ave and Mcfadden Ave



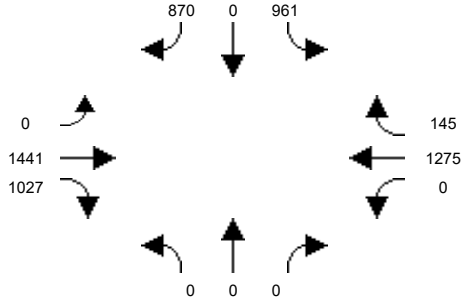
Intersection #66: Halladay St and Warner Ave



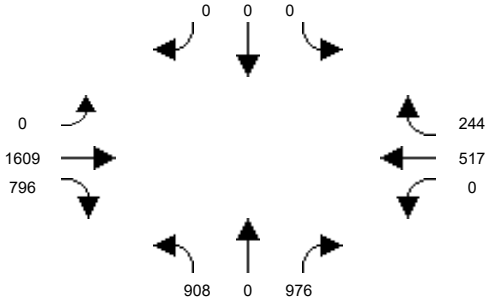
Intersection #67: Halladay St and Dyer Rd



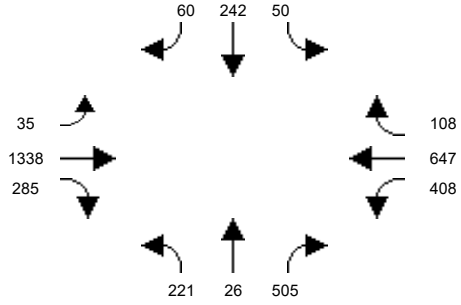
Intersection #68: SR-55 SB Ramps and MacArthur Blvd



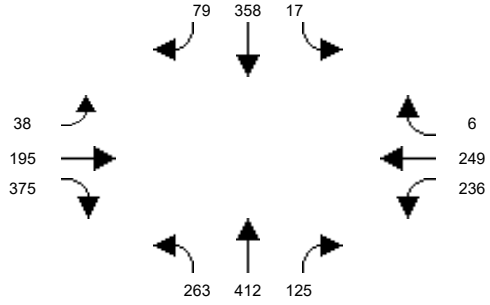
Intersection #69: SR-55 NB Ramps and MacArthur Blvd



Intersection #70: SR-55 SB Ramps and Dyer Rd



Intersection #71: Glassell St and La Veta Ave

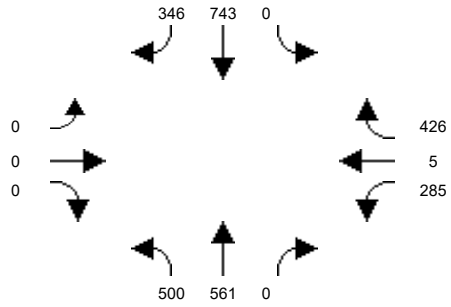


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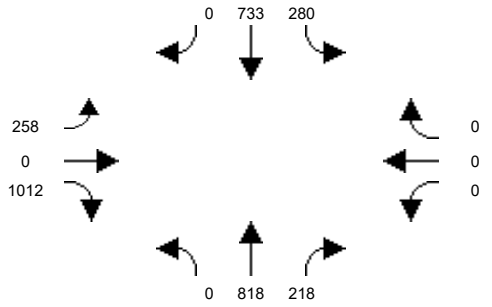
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2020 NP AM Peak Hour

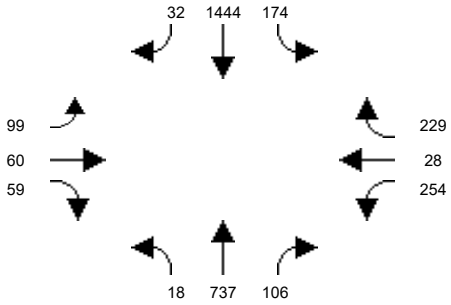
Intersection #72: Glassell St and SR-22 WB Ramps



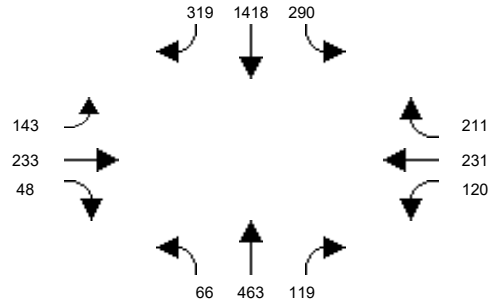
Intersection #73: Grand Ave / Glassell St and SR-22 EB Ramps



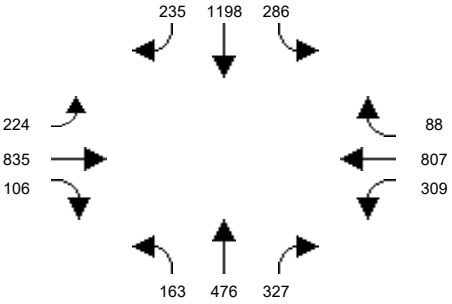
Intersection #74: Grand Ave and Fairhaven Ave



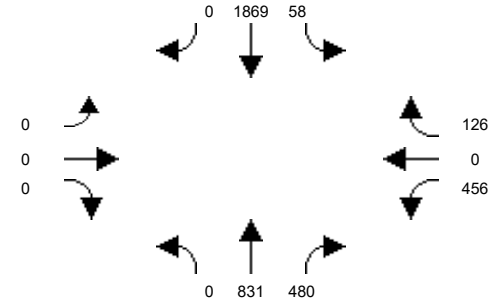
Intersection #75: Grand Ave and Santa Clara Ave



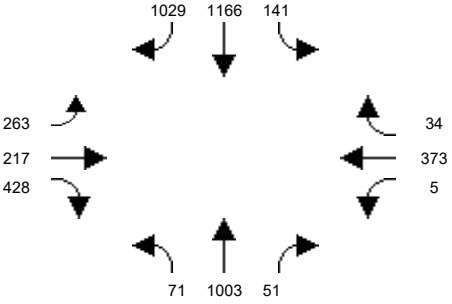
Intersection #76: Grand Ave and 17th St



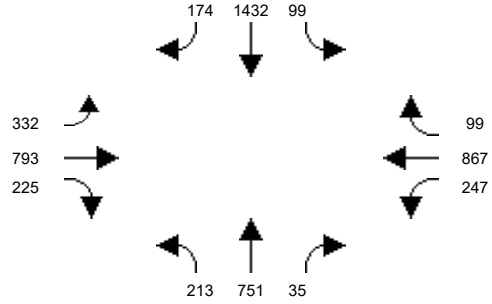
Intersection #77: Grand Ave and I-5 NB Ramps



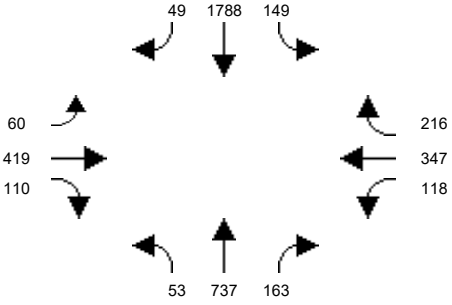
Intersection #78: Grand Ave and Santa Ana Blvd



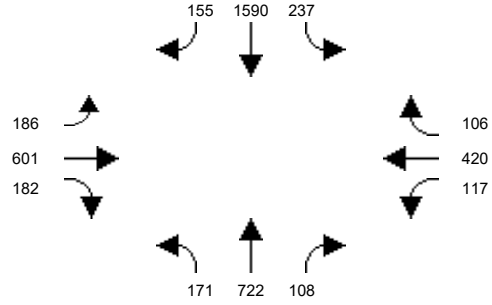
Intersection #79: Grand Ave and 1st St



Intersection #80: Grand Ave and Chestnut Ave



Intersection #81: Grand Ave and McFadden Ave

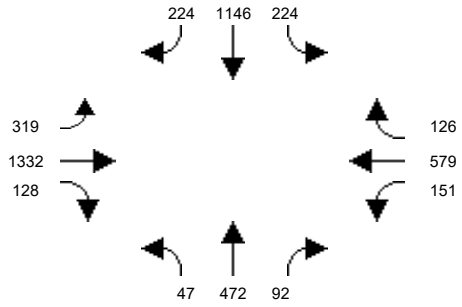


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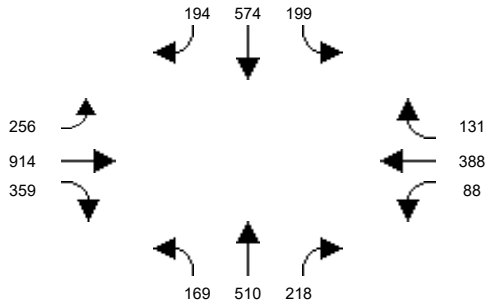
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2020 NP AM Peak Hour

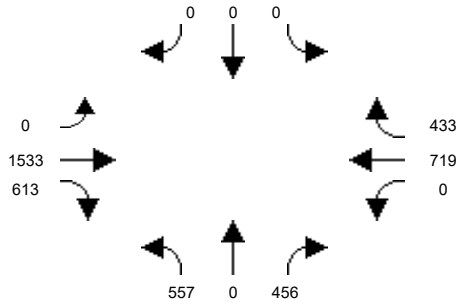
Intersection #82: Grand Ave and Edinger Ave



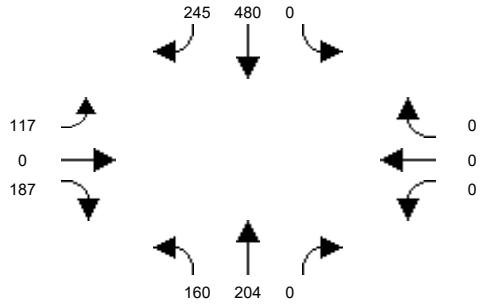
Intersection #83: Grand Ave and Warner Ave



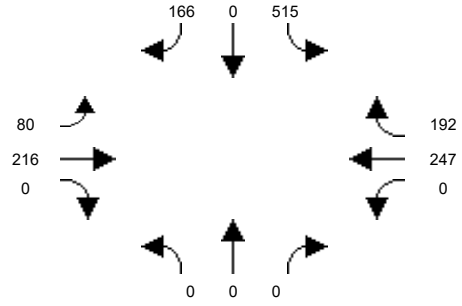
Intersection #84: SR-55 NB Ramps and Dyer Rd



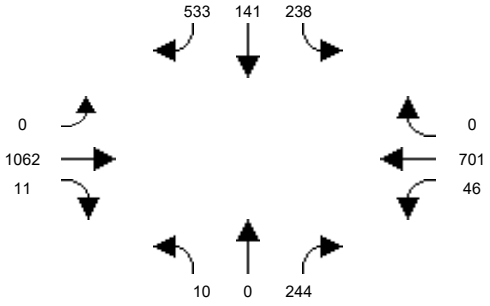
Intersection #85: Cambridge St and La Veta Ave



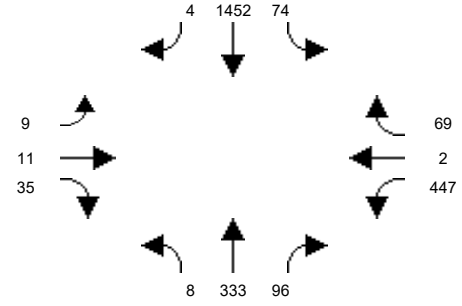
Intersection #86: Cambridge St and Fairhaven Ave



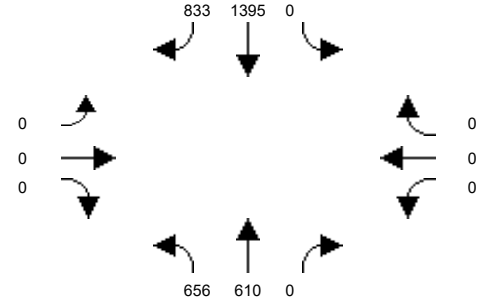
Intersection #87: Mabury St and 1st Street



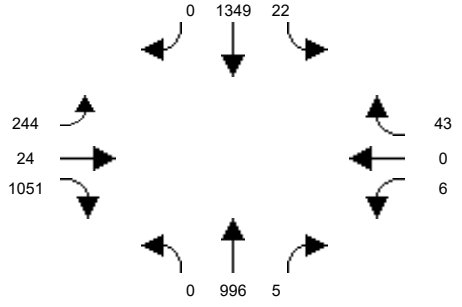
Intersection #88: Tustin St and La Veta Ave



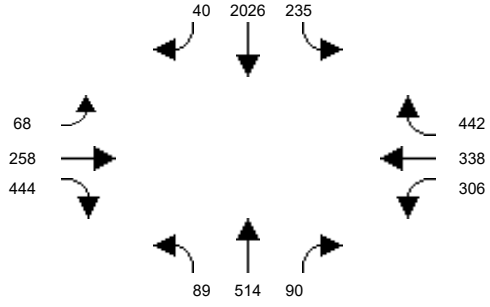
Intersection #89: Tustin St and SR-22 WB On-Ramp



Intersection #90: Tustin St and SR-22 EB Off-Ramp / Seba Ave



Intersection #91: Tustin Ave and Fairhaven Ave

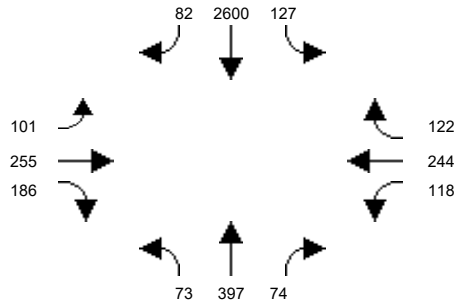


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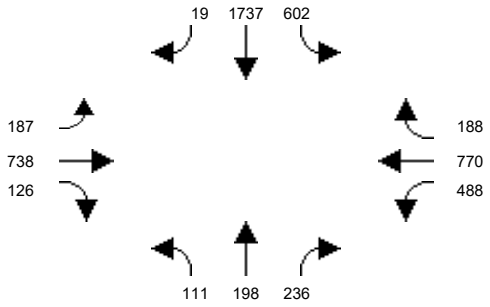
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2020 NP AM Peak Hour

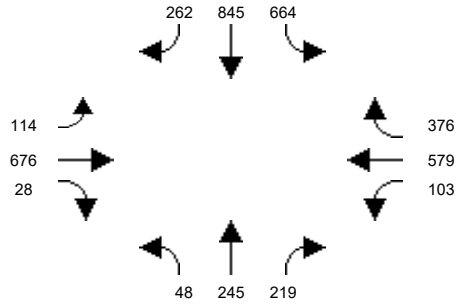
Intersection #92: Tustin Ave and Santa Clara Ave



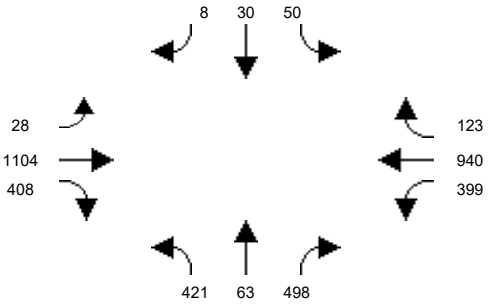
Intersection #93: Tustin Ave and 17th St



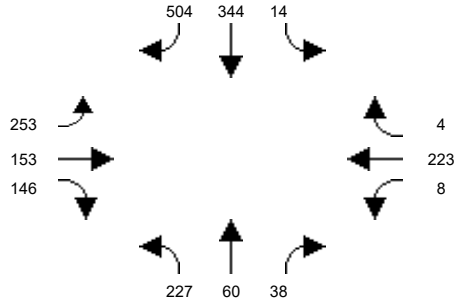
Intersection #94: Tustin Ave and 4th St



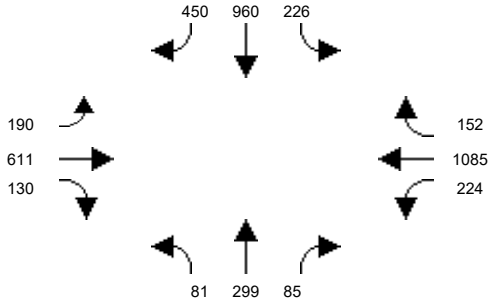
Intersection #95: SR-55 SB Ramps / Auto Mall Dr and Edinger Ave



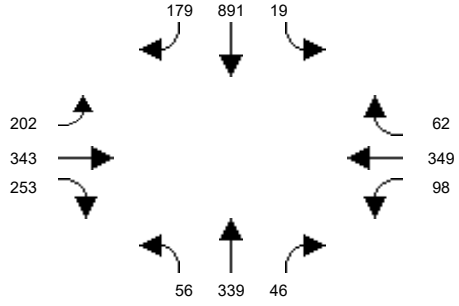
Intersection #96: SR-55 NB Ramps / Del Amo Ave and Newport



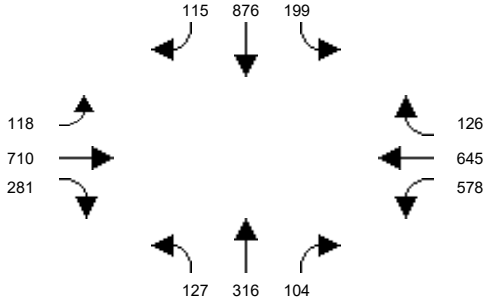
Intersection #97: Red Hill Ave and Edinger Ave



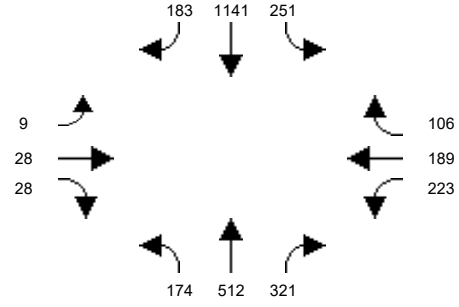
Intersection #98: Red Hill Ave and Warner Ave



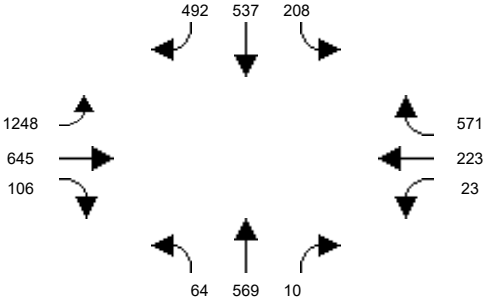
Intersection #99: Red Hill Ave and Dyer Rd / barranca Pkwy



Intersection #100: Red Hill Ave and Alton Pkwy



Intersection #101: Red Hill Ave and MacArthur Blvd

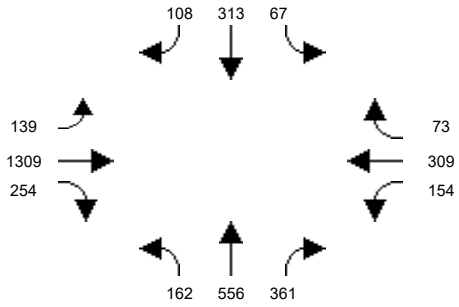


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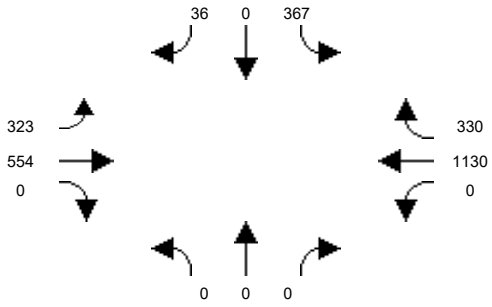
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2020 NP AM Peak Hour

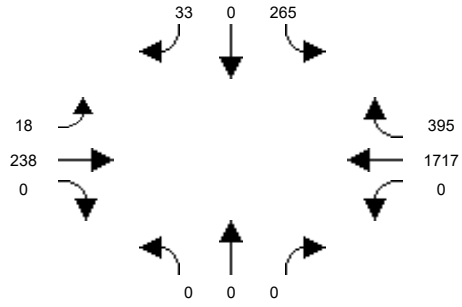
Intersection #102: Red Hill Ave and Main St



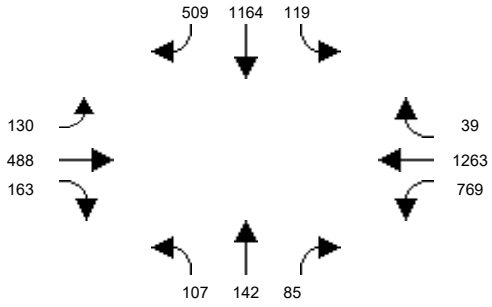
Intersection #103: I-5 SB Ramps and Santa Ana Blvd



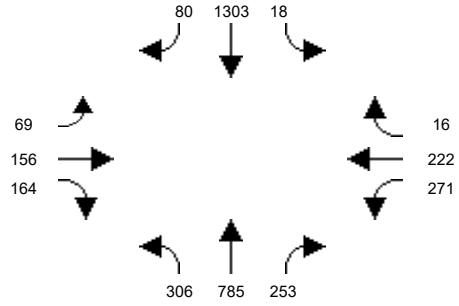
Intersection #104: Tustin Ranch Rd and Warner Ave



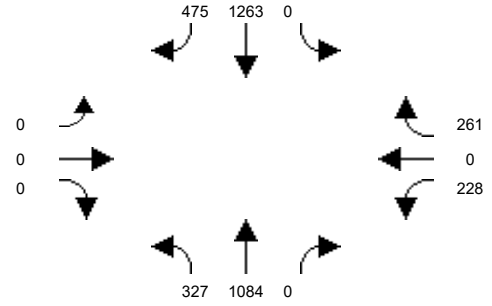
Intersection #105: Von Karman Ave and Barranca Pkwy



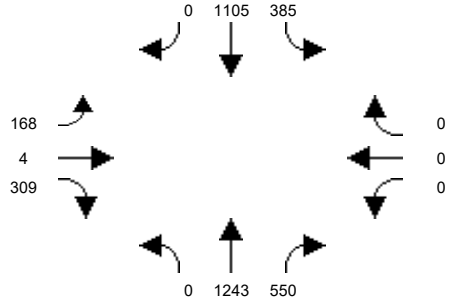
Intersection #106: Red Hill Avenue and El Camino Real



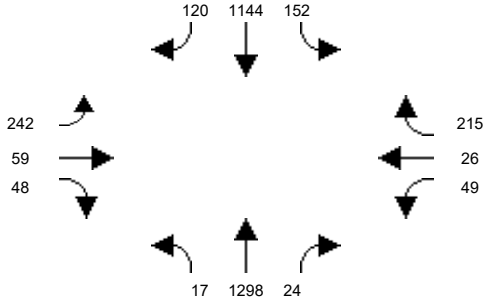
Intersection #107: Red Hill Avenue and I-5 NB Ramps



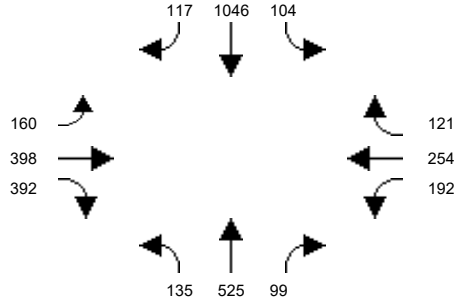
Intersection #108: Red Hill Avenue and I-5 SB Ramps



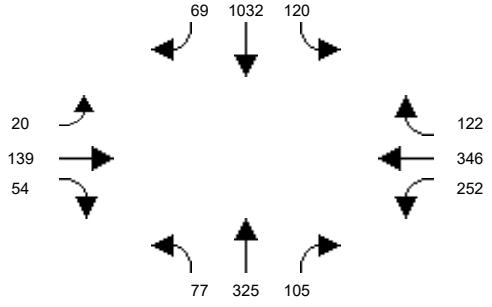
Intersection #109: Red Hill Avenue and Nisson Road



Intersection #110: Red Hill Avenue and Walnut Avenue



Intersection #111: Red Hill Avenue and Valencia Avenue

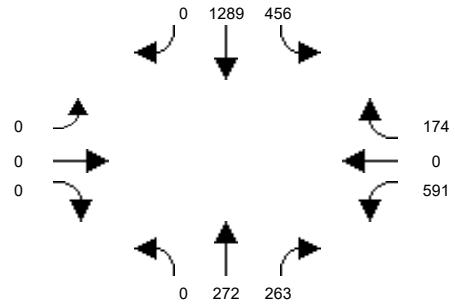


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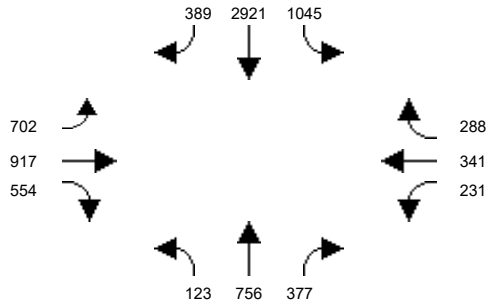
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2020 NP AM Peak Hour

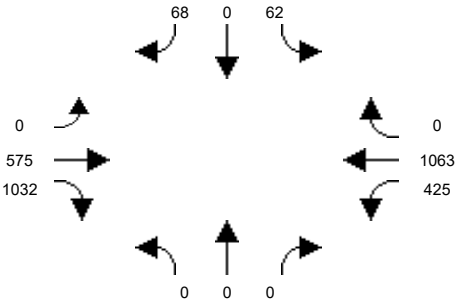
Intersection #112: Tustin Ranch Road and Warner Avenue North



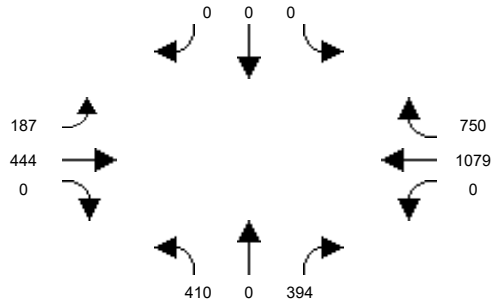
Intersection #113: Tustin Ranch Road and Walnut Avenue



Intersection #114: SR-55 SB Ramps and Irvine Boulevard



Intersection #115: SR-55 NB Ramps and Irvine Boulevard



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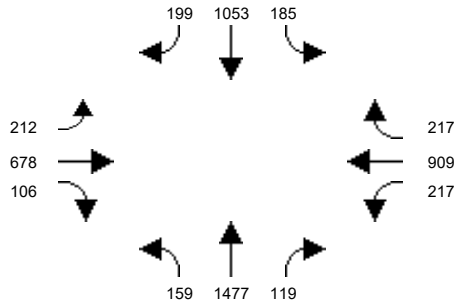
APPENDIX B.2

INTERSECTION VOLUMES –
2020 NP (PM PEAK HOUR)

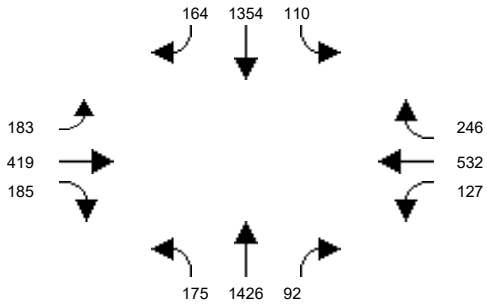
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2020 NP PM Peak Hour

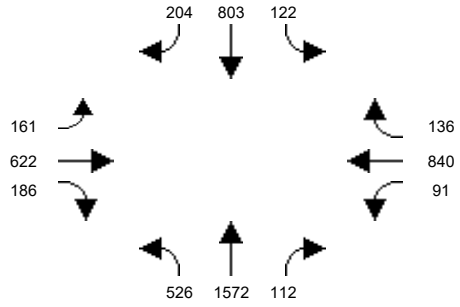
Intersection #1: Euclid St and 1st St



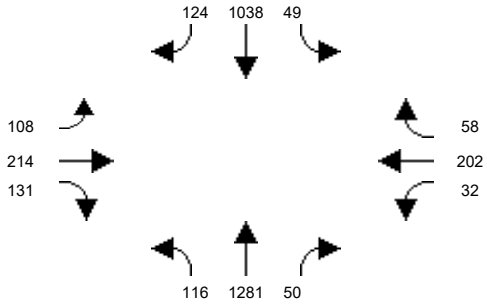
Intersection #2: Euclid St and McFadden Ave



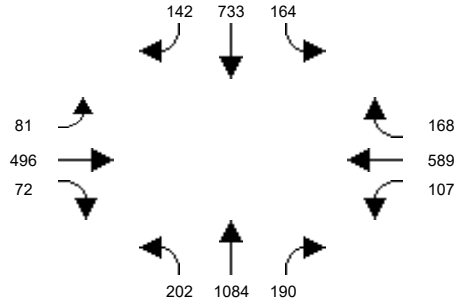
Intersection #3: Euclid St and Edinger Ave



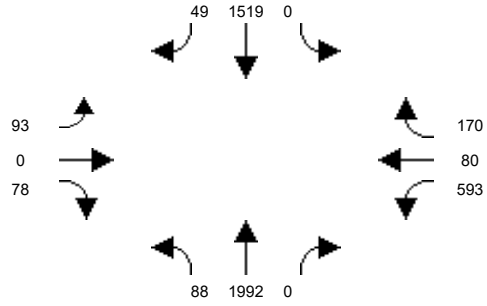
Intersection #4: Newhope St and Hazard Ave



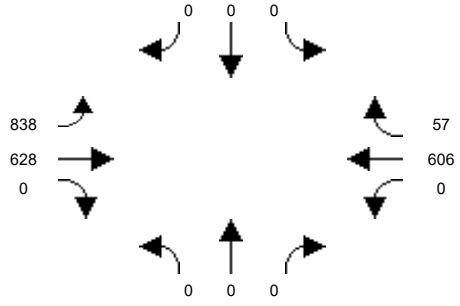
Intersection #5: Newhope St and McFadden Ave



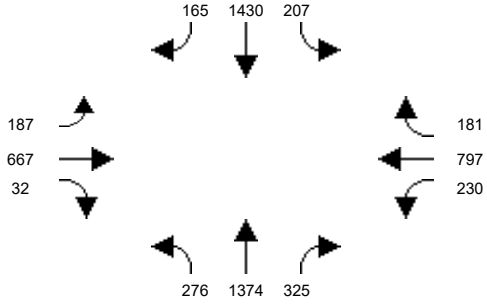
Intersection #6: Harbor Blvd and SR-22 WB Off-Ramp / Banner Dr



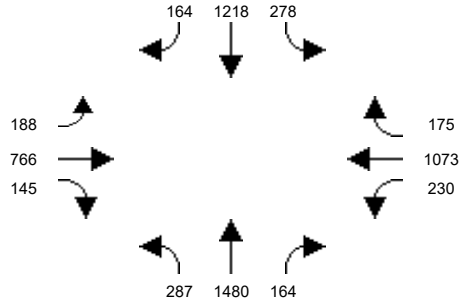
Intersection #7: Trask Avenue and SR-22 EB On-Ramp



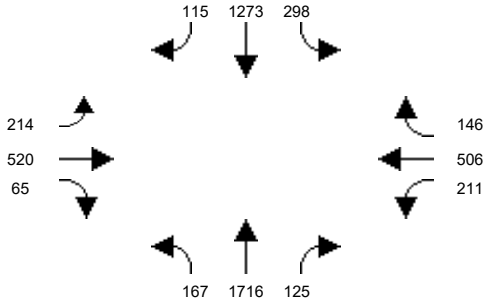
Intersection #8: Harbor Blvd and Westminster Ave



Intersection #9: Harbor Blvd and 1st St



Intersection #10: Harbor Blvd and McFadden Ave

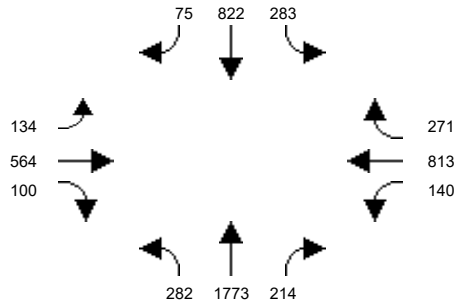


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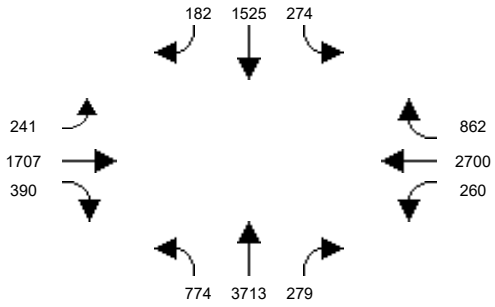
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2020 NP PM Peak Hour

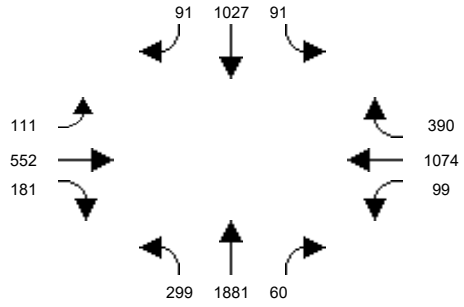
Intersection #11: Harbor Blvd and Edinger Ave



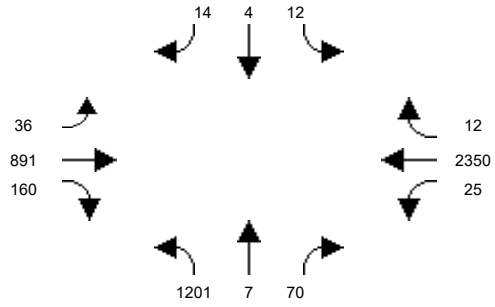
Intersection #12: Harbor Blvd and Warner Ave



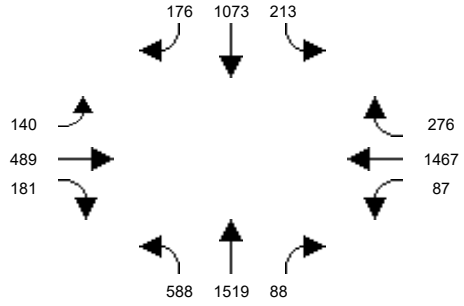
Intersection #13: Harbor Blvd and Segerstrom Ave



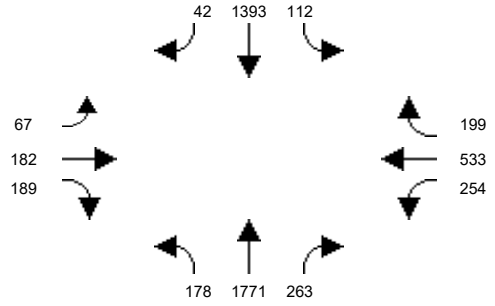
Intersection #14: MacArthur Blvd and Hyland Ave



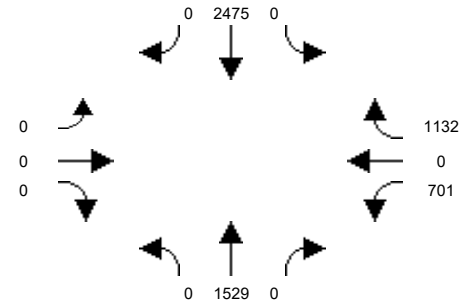
Intersection #15: MacArthur Blvd and Harbor Blvd



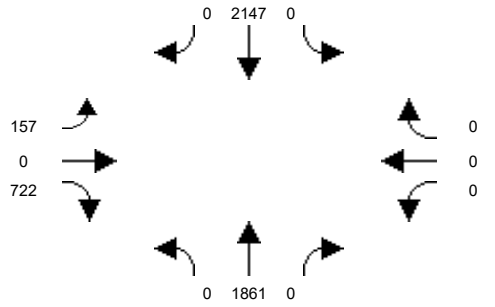
Intersection #16: Harbor Blvd and Sunflower Ave



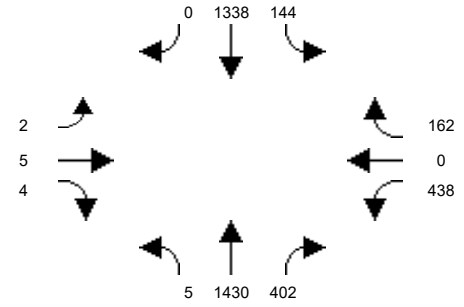
Intersection #17: Harbor Blvd and I-405 WB Off-Ramp



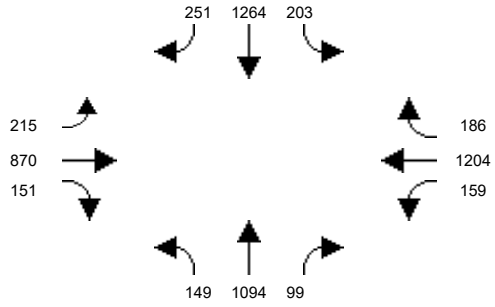
Intersection #18: Harbor Blvd and I-405 EB Off-Ramp



Intersection #19: Fairview St and Civic Center Dr



Intersection #20: Fairview St and 1st St

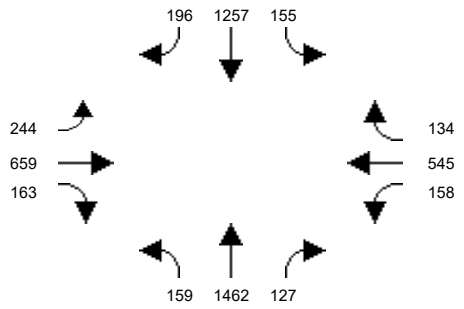


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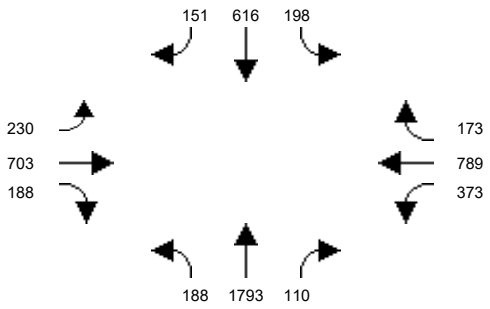
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2020 NP PM Peak Hour

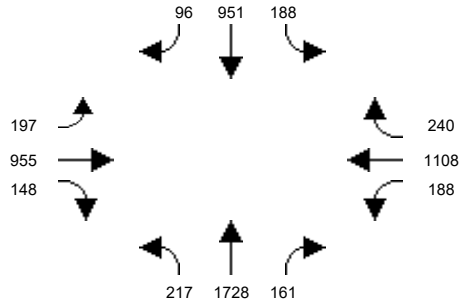
Intersection #21: Fairview St and McFadden Ave



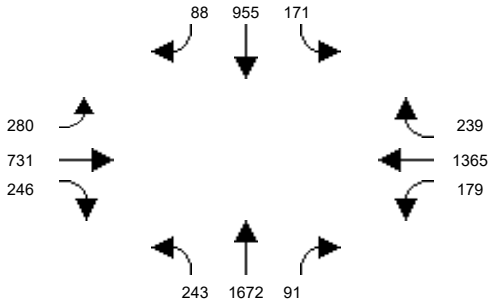
Intersection #22: Fairview St and Edinger Ave



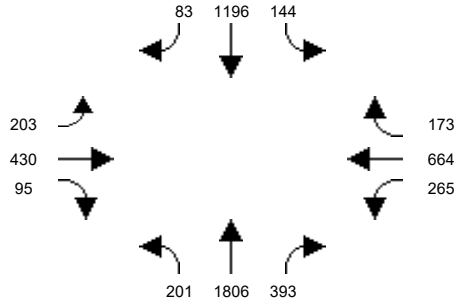
Intersection #23: Fairview St and Warner Ave



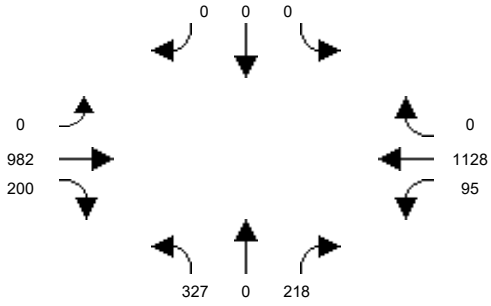
Intersection #24: Fairview St and MacArthur Blvd



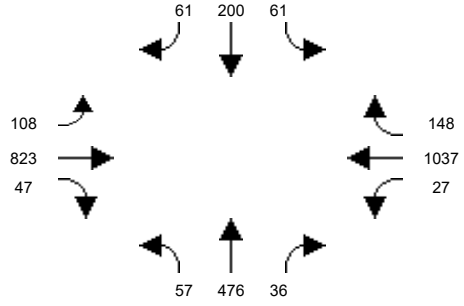
Intersection #25: Fairview Rd and Sunflower Ave



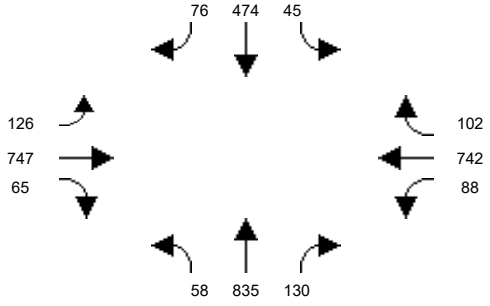
Intersection #26: Greenville St and Edinger Ave



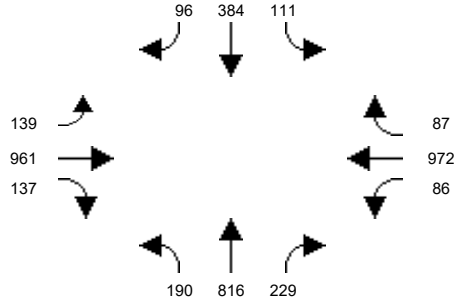
Intersection #27: Greenville St and Segerstrom Ave



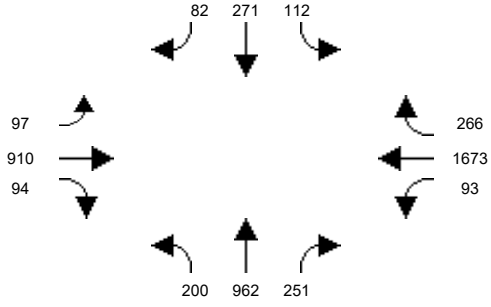
Intersection #28: Raitt St and McFadden Ave



Intersection #29: Raitt St and Edinger Ave



Intersection #30: Bear St and MacArthur Blvd

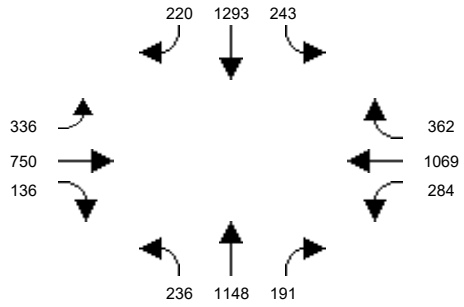


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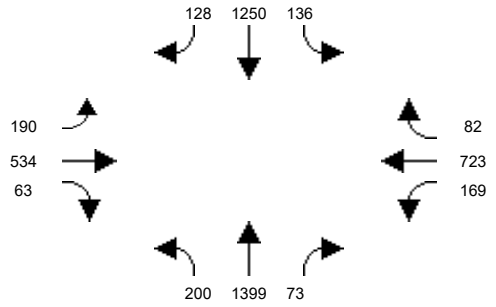
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2020 NP PM Peak Hour

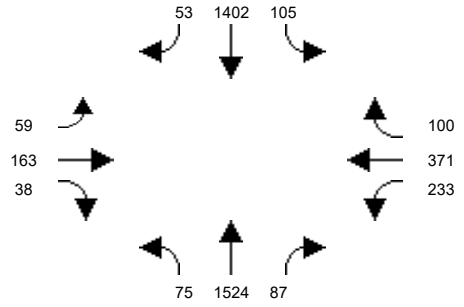
Intersection #31: Bristol St and 17th St



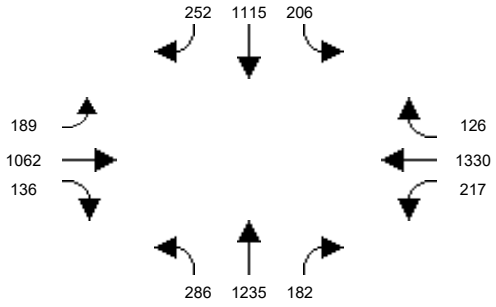
Intersection #32: Bristol St and Civic Center Dr



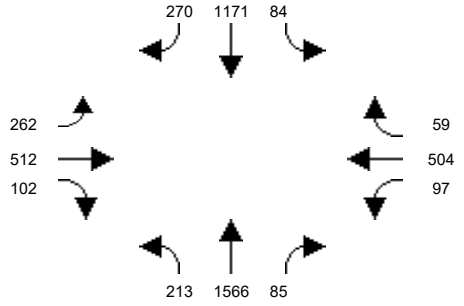
Intersection #33: Bristol St and Santa Ana Blvd



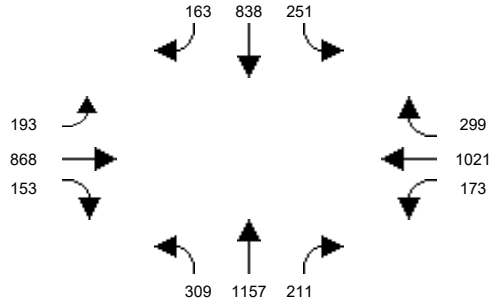
Intersection #34: Bristol St and 1st St



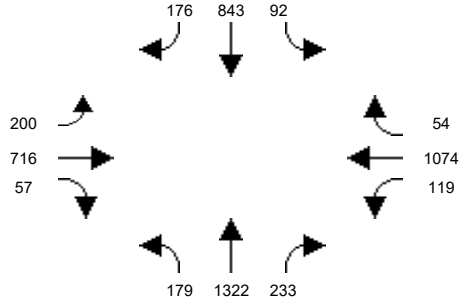
Intersection #35: Bristol St and McFadden Ave



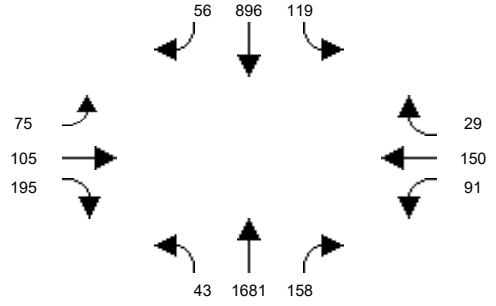
Intersection #36: Bristol St and Warner Ave



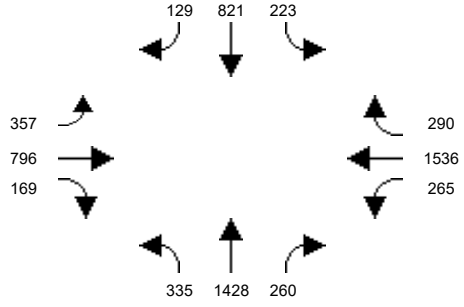
Intersection #37: Bristol St and Segerstrom Ave



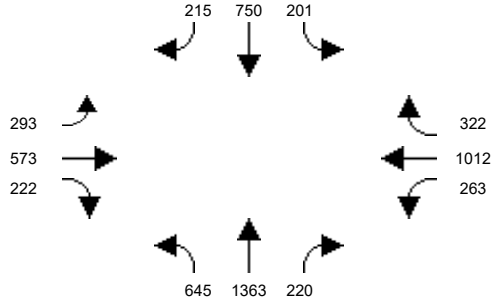
Intersection #38: Bristol St and Alton Ave



Intersection #39: Bristol St and MacArthur Blvd



Intersection #40: Bristol St and Sunflower Ave

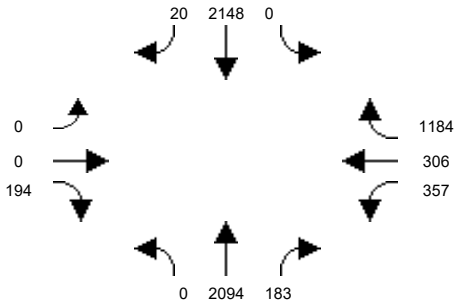


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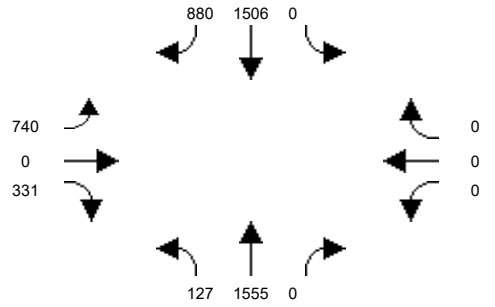
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2020 NP PM Peak Hour

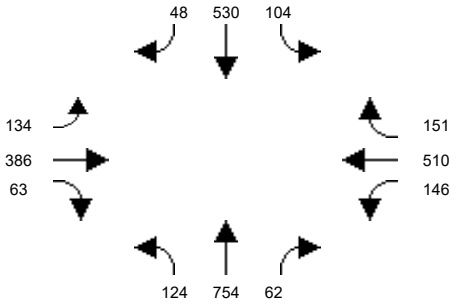
Intersection #41: Bristol St and I-405 WB Ramps



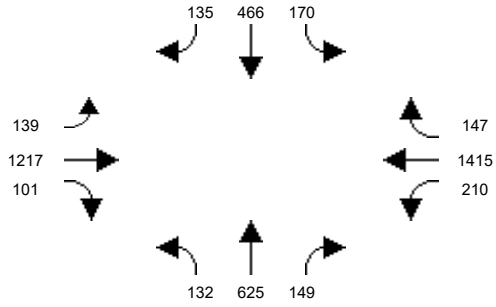
Intersection #42: Bristol St and I-405 EB Ramps



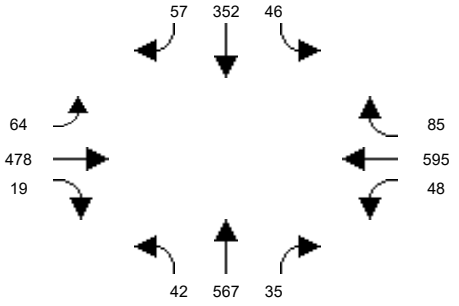
Intersection #43: Flower St and Santa Ana Blvd



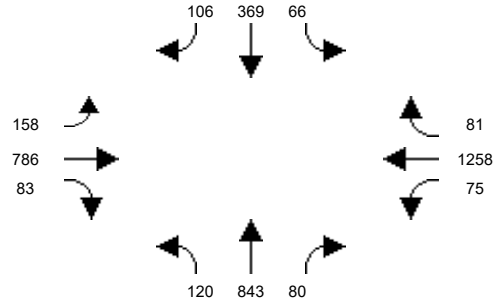
Intersection #44: Flower St and 1st St



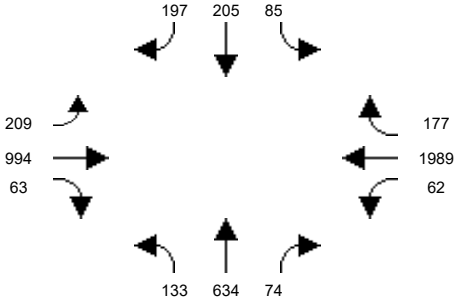
Intersection #45: Flower St and McFadden Ave



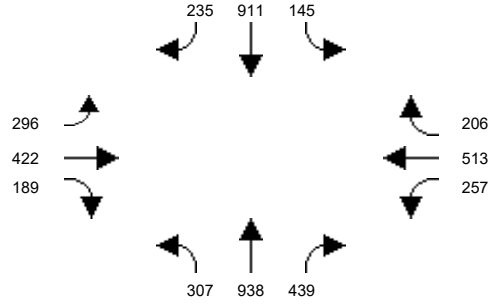
Intersection #46: Flower St and Segerstrom Ave



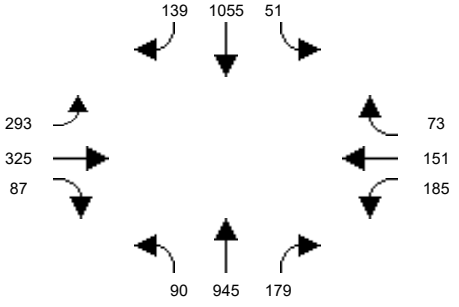
Intersection #47: Flower St and MacArthur Blvd



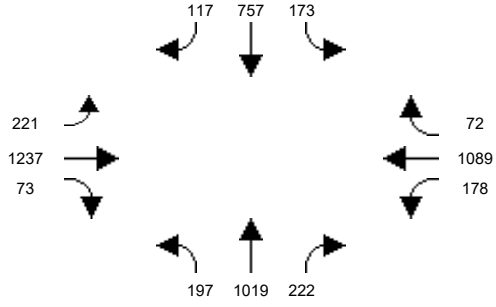
Intersection #48: Main St and La Veta Ave



Intersection #49: Main St and Mainplace Dr / Memory Ln



Intersection #50: Main St and 17th St

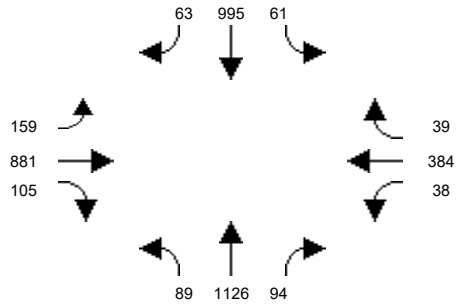


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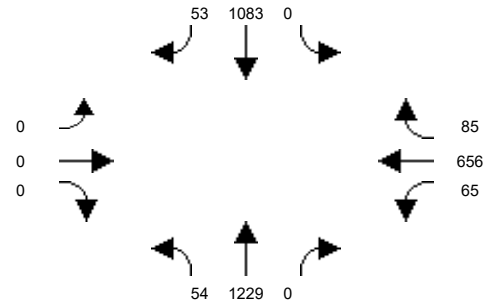
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2020 NP PM Peak Hour

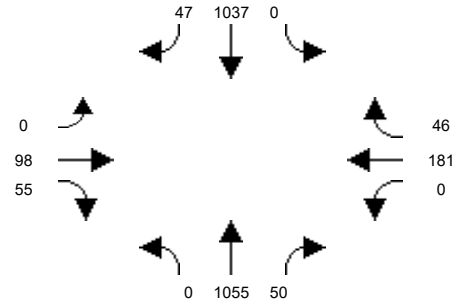
Intersection #51: Main St and Civic Center Dr



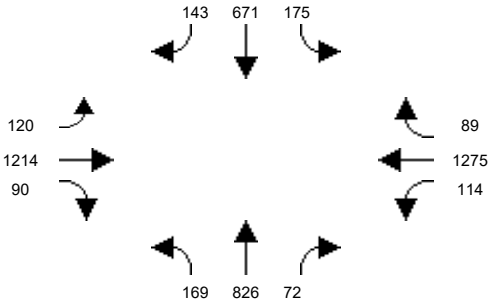
Intersection #52: Main St and Santa Ana Blvd



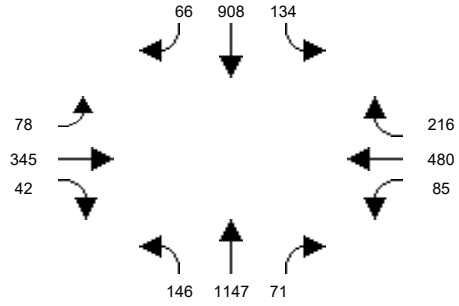
Intersection #53: Main St and 4th St



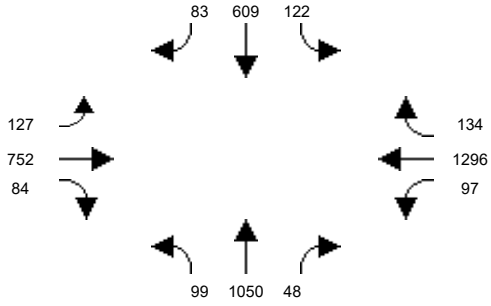
Intersection #54: Main St and 1st St



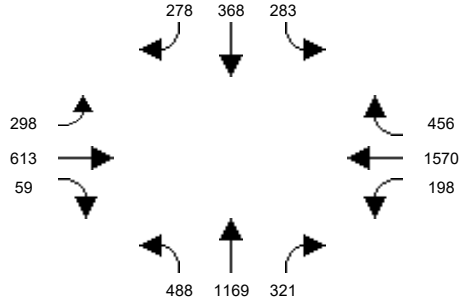
Intersection #55: Main St and McFadden Ave



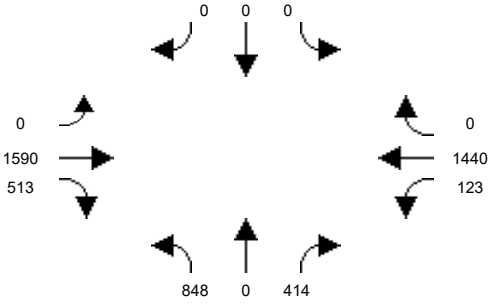
Intersection #56: Main St and Edinger Ave



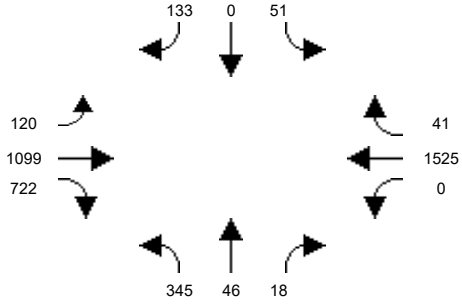
Intersection #57: Main St and MacArthur Blvd



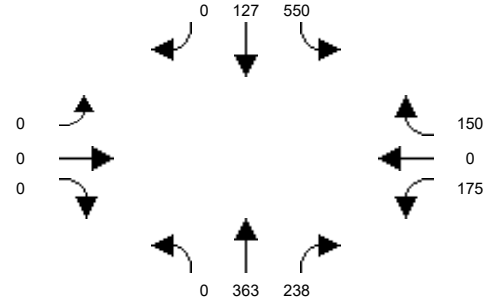
Intersection #58: Penn Wy and 17th St



Intersection #59: I-5 NB Off Ramps/17th Street



Intersection #60: Penn Wy and I-5 SB Ramps

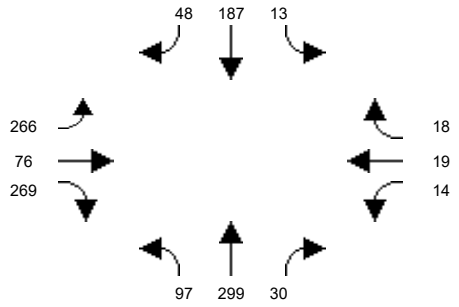


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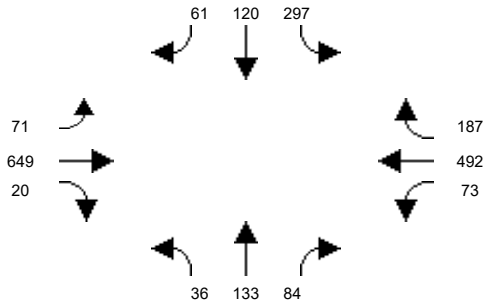
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2020 NP PM Peak Hour

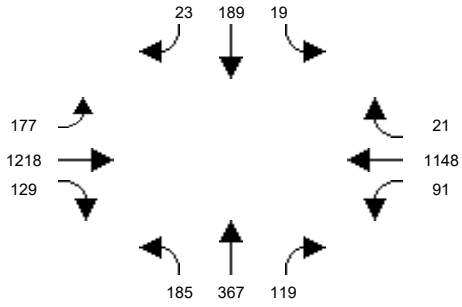
Intersection #61: Santiago St and Civic Center Dr



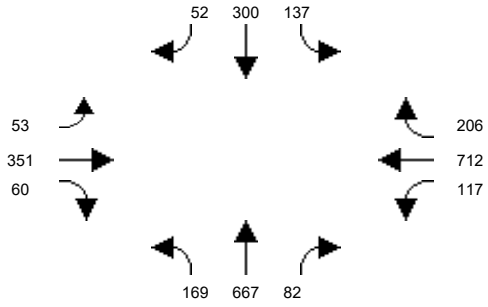
Intersection #62: Santiago St and Santa Ana Blvd



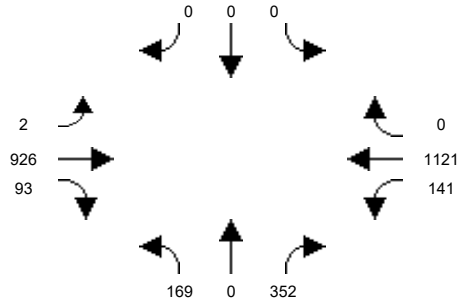
Intersection #64: Standard Ave and 1st St



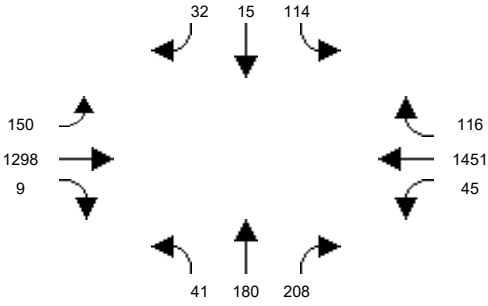
Intersection #65: Standard Ave and Mcfadden Ave



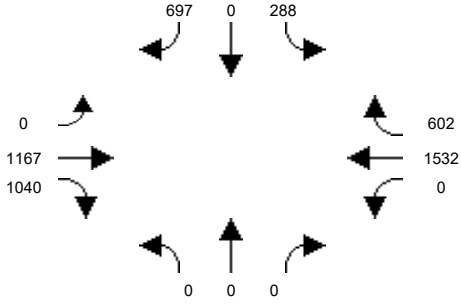
Intersection #66: Halladay St and Warner Ave



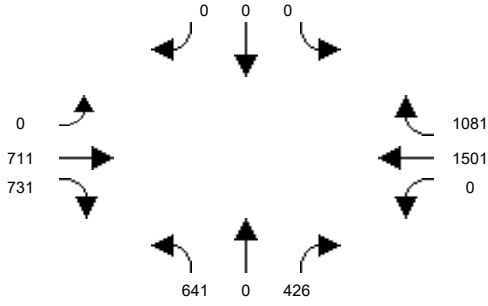
Intersection #67: Halladay St and Dyer Rd



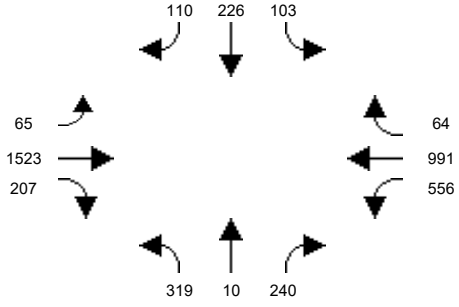
Intersection #68: SR-55 SB Ramps and MacArthur Blvd



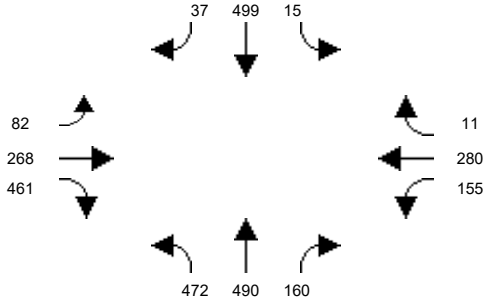
Intersection #69: SR-55 NB Ramps and MacArthur Blvd



Intersection #70: SR-55 SB Ramps and Dyer Rd



Intersection #71: Glassell St and La Veta Ave

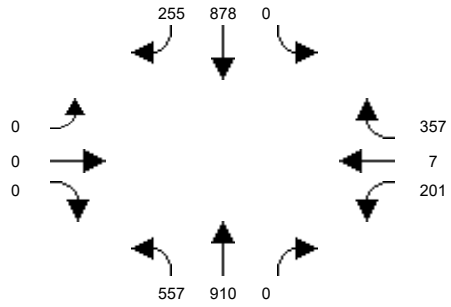


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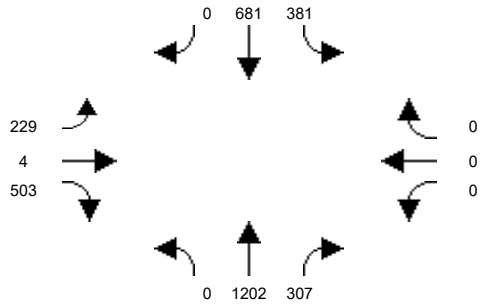
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2020 NP PM Peak Hour

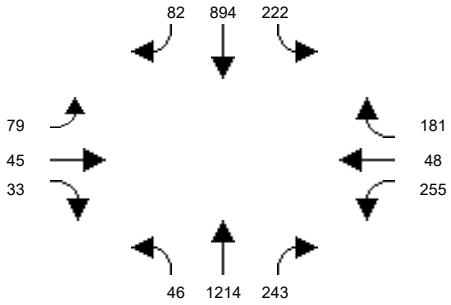
Intersection #72: Glassell St and SR-22 WB Ramps



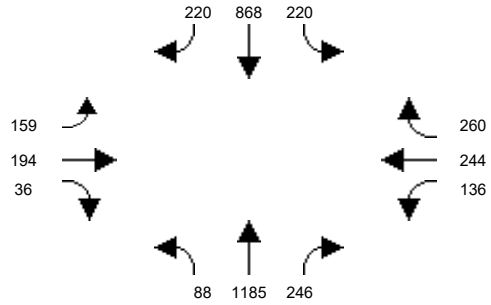
Intersection #73: Grand Ave / Glassell St and SR-22 EB Ramps



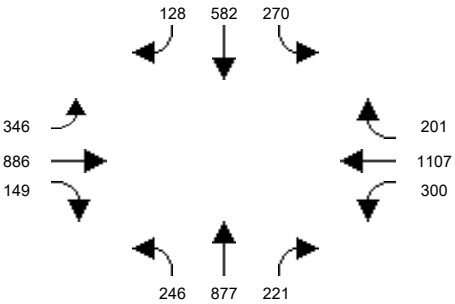
Intersection #74: Grand Ave and Fairhaven Ave



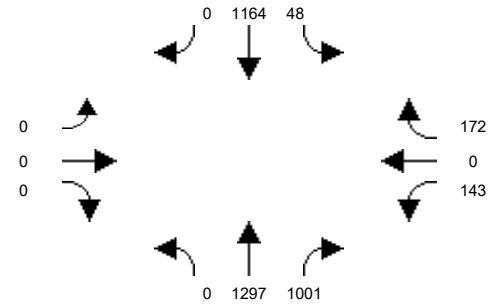
Intersection #75: Grand Ave and Santa Clara Ave



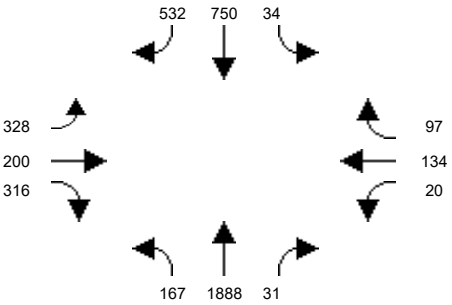
Intersection #76: Grand Ave and 17th St



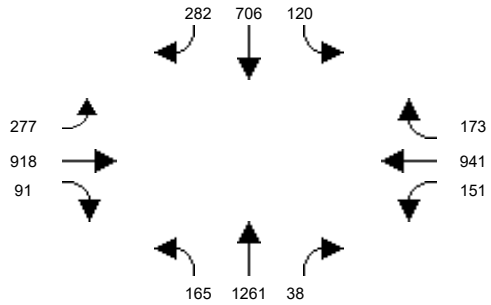
Intersection #77: Grand Ave and I-5 NB Ramps



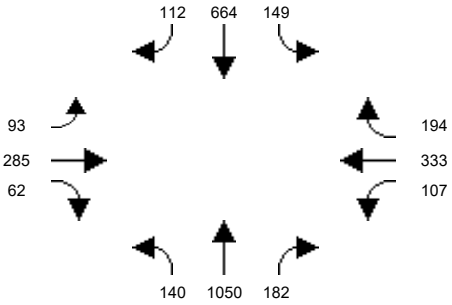
Intersection #78: Grand Ave and Santa Ana Blvd



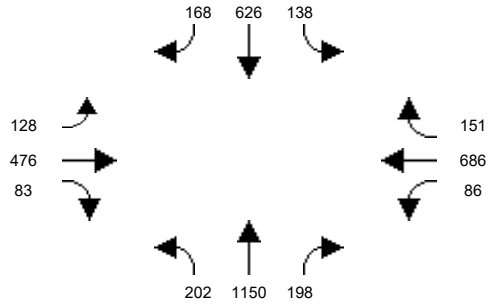
Intersection #79: Grand Ave and 1st St



Intersection #80: Grand Ave and Chestnut Ave



Intersection #81: Grand Ave and McFadden Ave

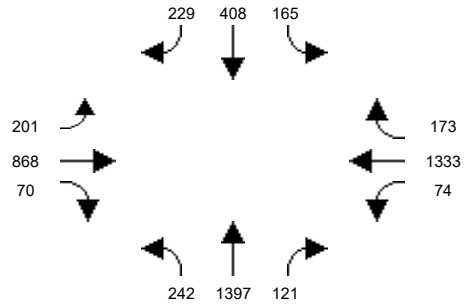


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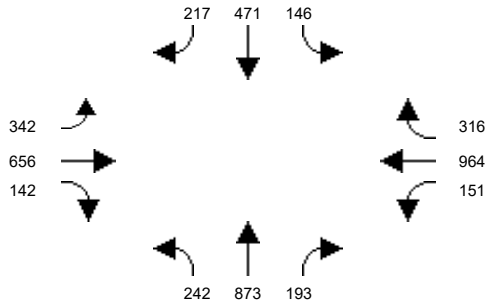
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2020 NP PM Peak Hour

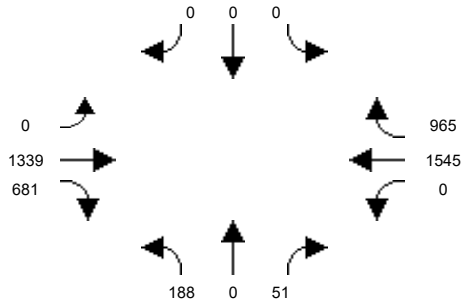
Intersection #82: Grand Ave and Edinger Ave



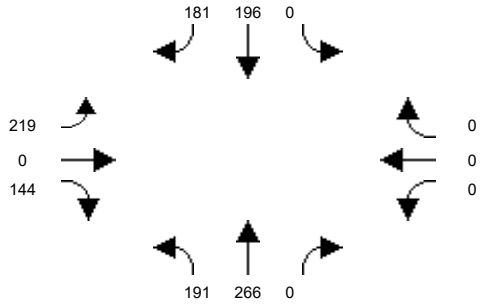
Intersection #83: Grand Ave and Warner Ave



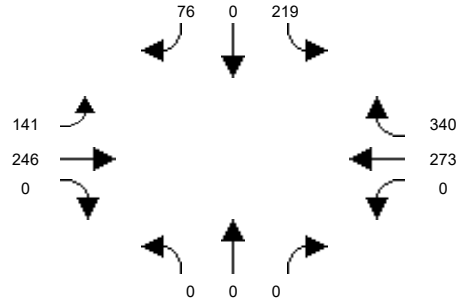
Intersection #84: SR-55 NB Ramps and Dyer Rd



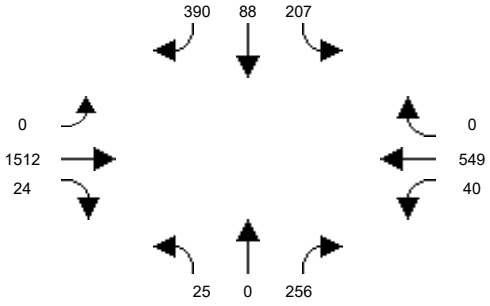
Intersection #85: Cambridge St and La Veta Ave



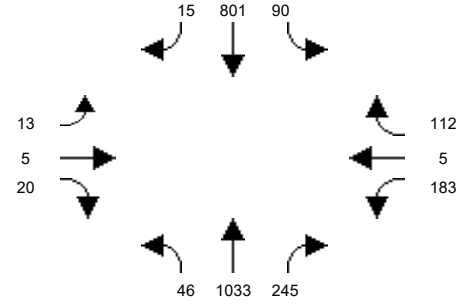
Intersection #86: Cambridge St and Fairhaven Ave



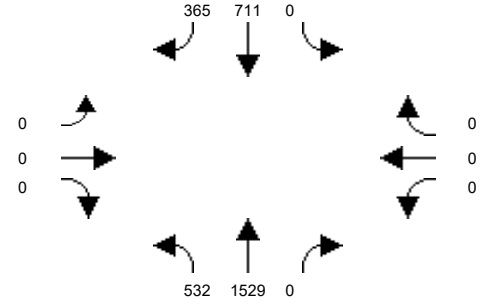
Intersection #87: Mabury St and 1st Street



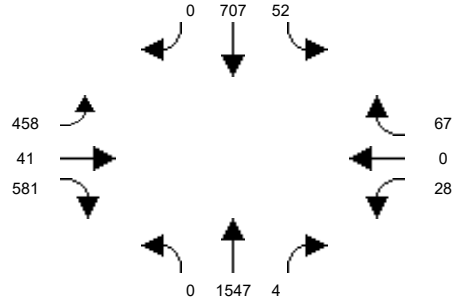
Intersection #88: Tustin St and La Veta Ave



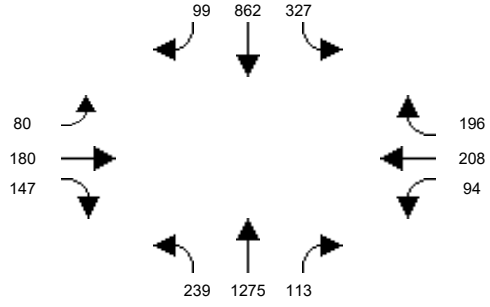
Intersection #89: Tustin St and SR-22 WB On-Ramp



Intersection #90: Tustin St and SR-22 EB Off-Ramp / Seba Ave



Intersection #91: Tustin Ave and Fairhaven Ave

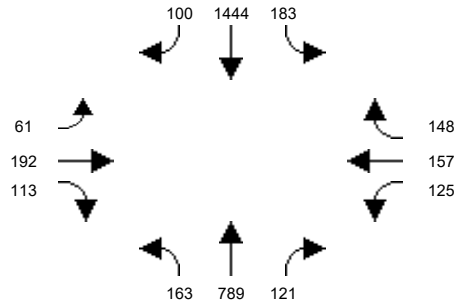


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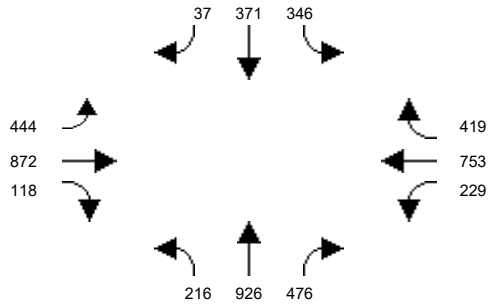
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2020 NP PM Peak Hour

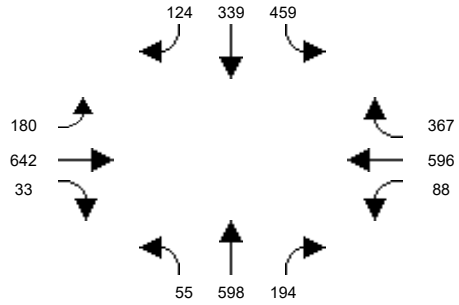
Intersection #92: Tustin Ave and Santa Clara Ave



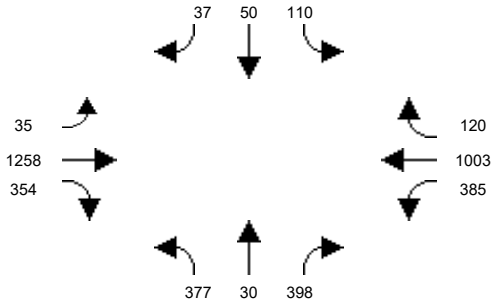
Intersection #93: Tustin Ave and 17th St



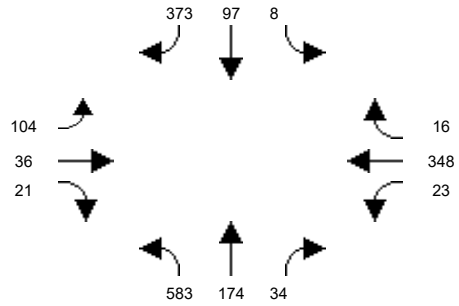
Intersection #94: Tustin Ave and 4th St



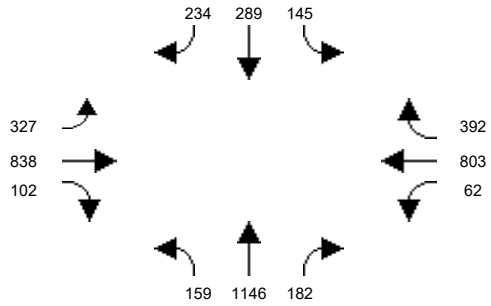
Intersection #95: SR-55 SB Ramps / Auto Mall Dr and Edinger Ave



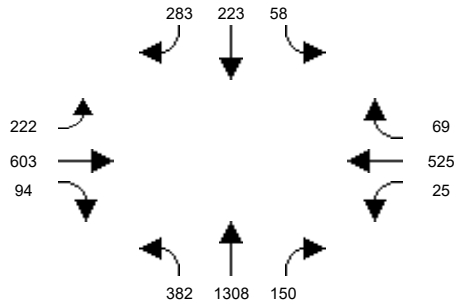
Intersection #96: SR-55 NB Ramps / Del Amo Ave and Newport



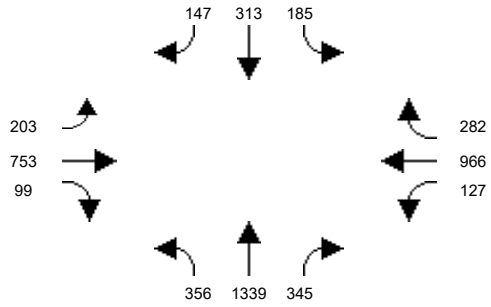
Intersection #97: Red Hill Ave and Edinger Ave



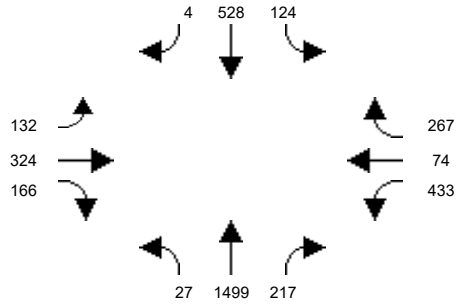
Intersection #98: Red Hill Ave and Warner Ave



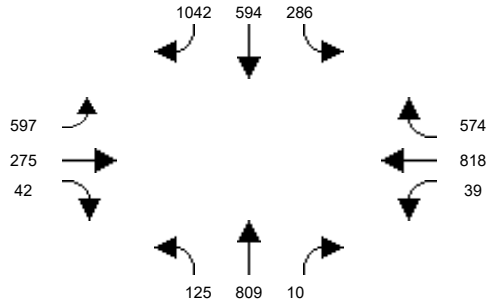
Intersection #99: Red Hill Ave and Dyer Rd / barranca Pkwy



Intersection #100: Red Hill Ave and Alton Pkwy



Intersection #101: Red Hill Ave and MacArthur Blvd

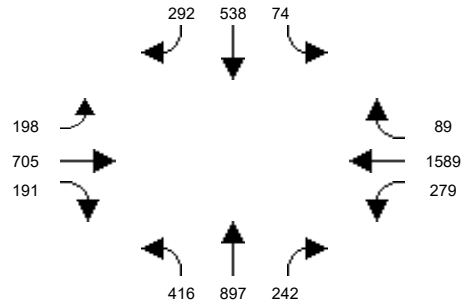


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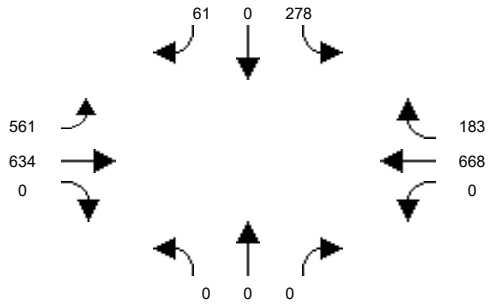
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2020 NP PM Peak Hour

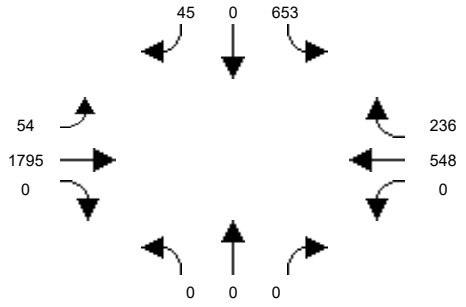
Intersection #102: Red Hill Ave and Main St



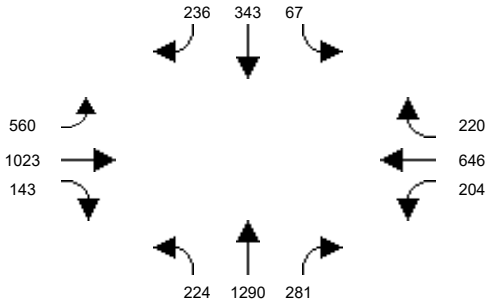
Intersection #103: I-5 SB Ramps and Santa Ana Blvd



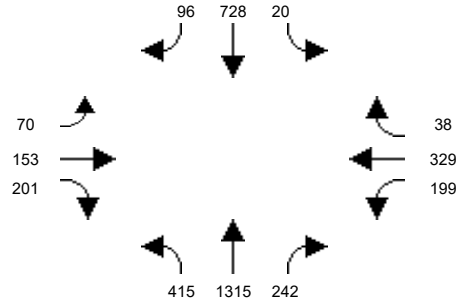
Intersection #104: Tustin Ranch Rd and Warner Ave



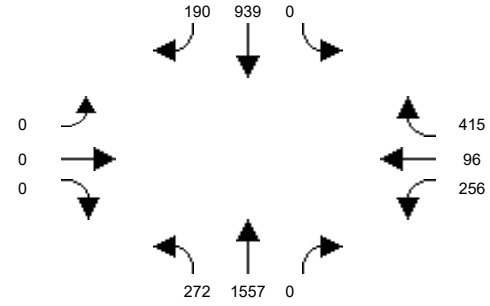
Intersection #105: Von Karman Ave and Barranca Pkwy



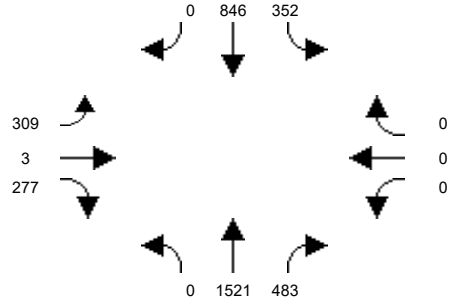
Intersection #106: Red Hill Avenue and El Camino Real



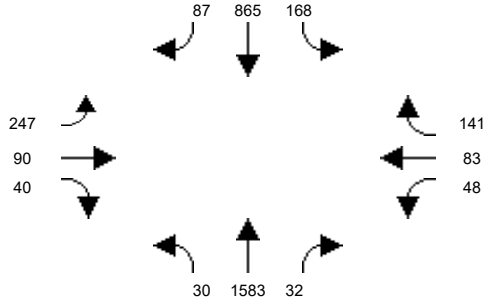
Intersection #107: Red Hill Avenue and I-5 NB Ramps



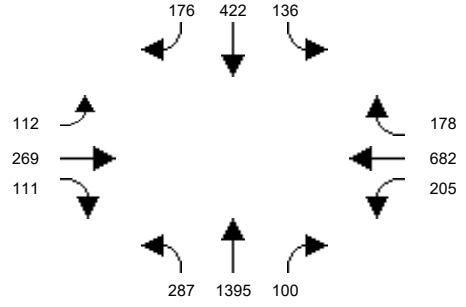
Intersection #108: Red Hill Avenue and I-5 SB Ramps



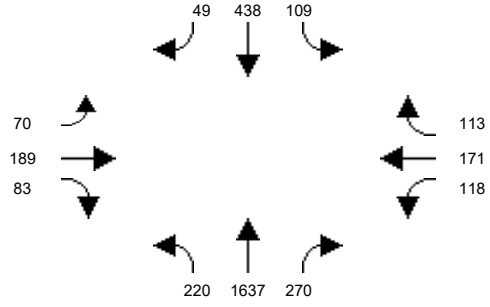
Intersection #109: Red Hill Avenue and Nisson Road



Intersection #110: Red Hill Avenue and Walnut Avenue



Intersection #111: Red Hill Avenue and Valencia Avenue

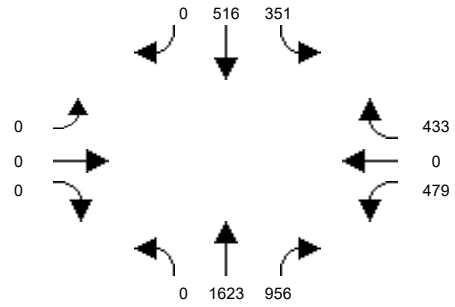


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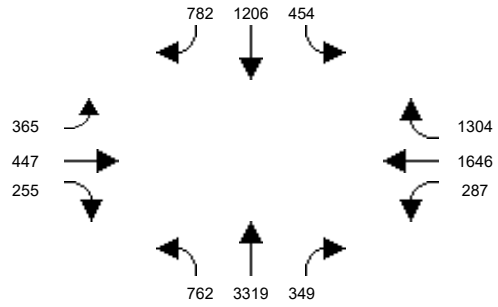
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2020 NP PM Peak Hour

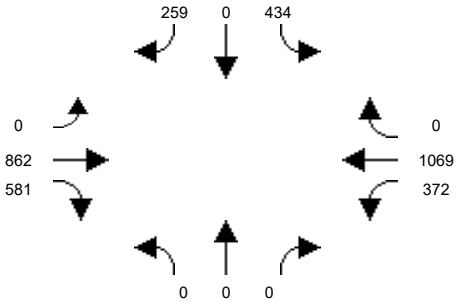
Intersection #112: Tustin Ranch Road and Warner Avenue North



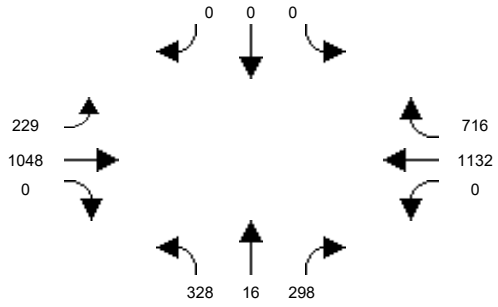
Intersection #113: Tustin Ranch Road and Walnut Avenue



Intersection #114: SR-55 SB Ramps and Irvine Boulevard



Intersection #115: SR-55 NB Ramps and Irvine Boulevard



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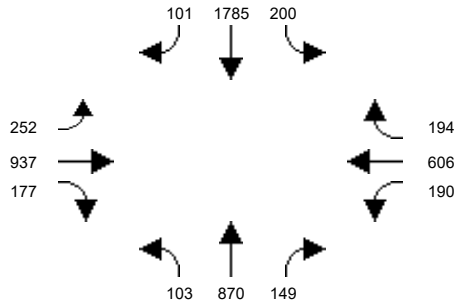
APPENDIX B.3

INTERSECTION VOLUMES –
2045 NP (AM PEAK HOUR)

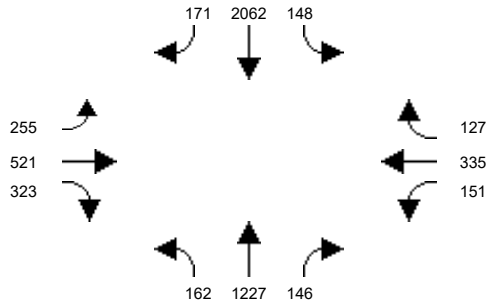
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP AM Peak Hour

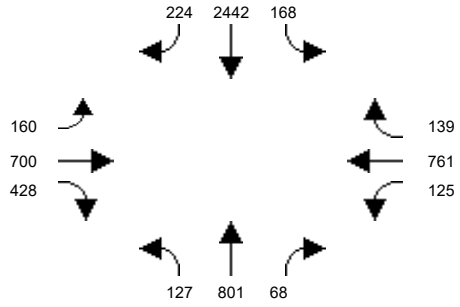
Intersection #1: Euclid St and 1st St



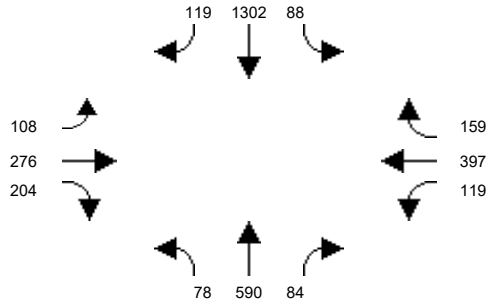
Intersection #2: Euclid St and McFadden Ave



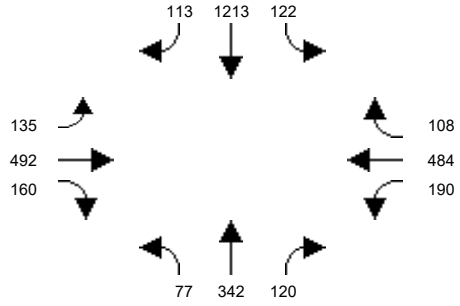
Intersection #3: Euclid St and Edinger Ave



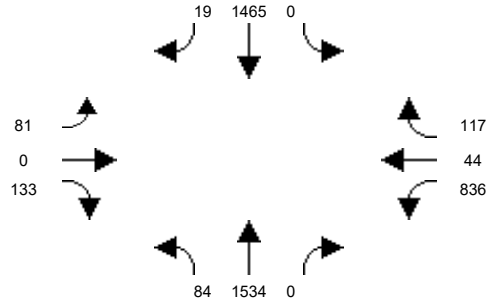
Intersection #4: Newhope St and Hazard Ave



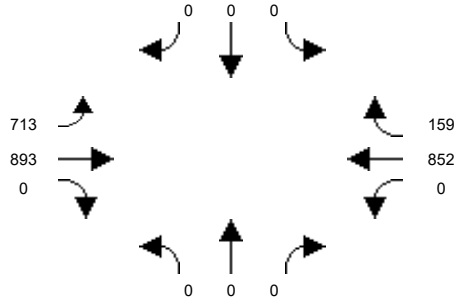
Intersection #5: Newhope St and McFadden Ave



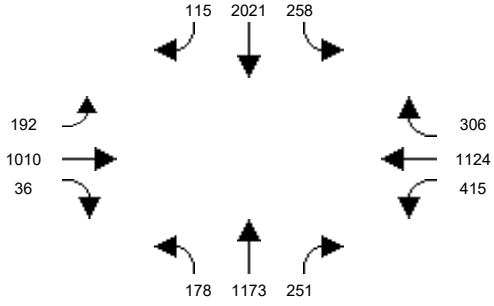
Intersection #6: Harbor Blvd and SR-22 WB Off-Ramp / Banner Dr



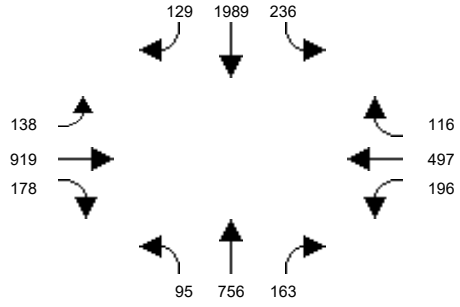
Intersection #7: Trask Avenue and SR-22 EB On-Ramp



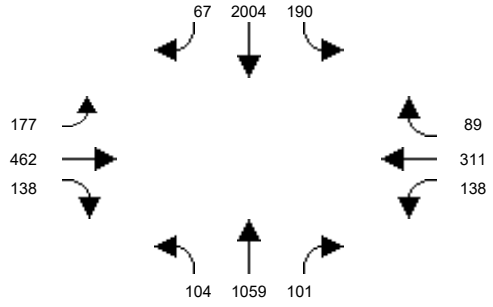
Intersection #8: Harbor Blvd and Westminster Ave



Intersection #9: Harbor Blvd and 1st St



Intersection #10: Harbor Blvd and McFadden Ave

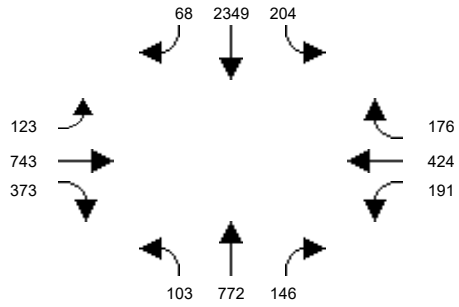


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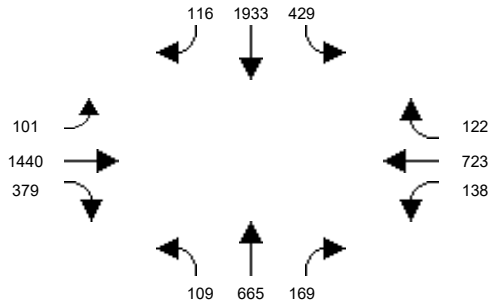
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP AM Peak Hour

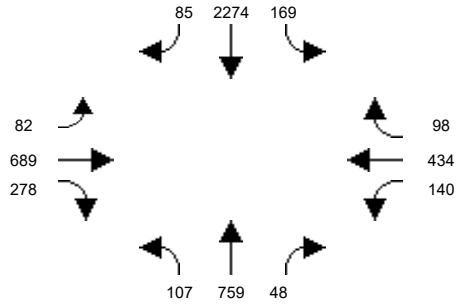
Intersection #11: Harbor Blvd and Edinger Ave



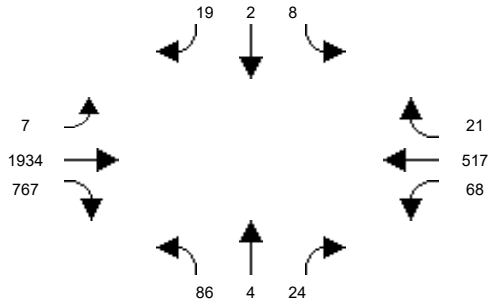
Intersection #12: Harbor Blvd and Warner Ave



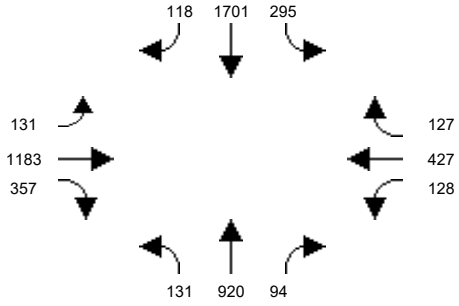
Intersection #13: Harbor Blvd and Segerstrom Ave



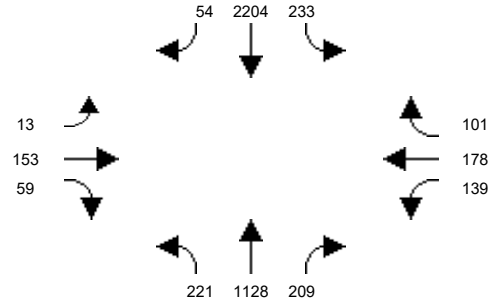
Intersection #14: MacArthur Blvd and Hyland Ave



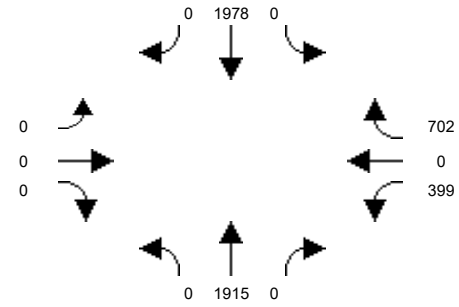
Intersection #15: MacArthur Blvd and Harbor Blvd



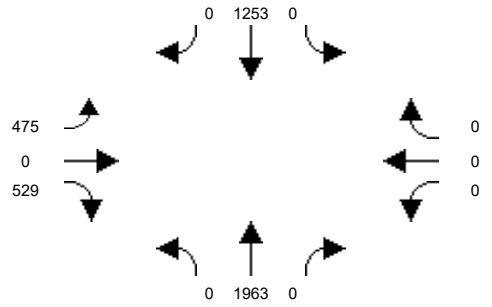
Intersection #16: Harbor Blvd and Sunflower Ave



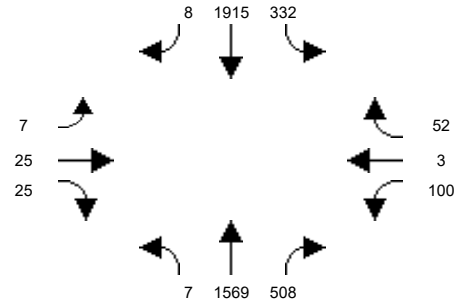
Intersection #17: Harbor Blvd and I-405 NB Off-Ramp



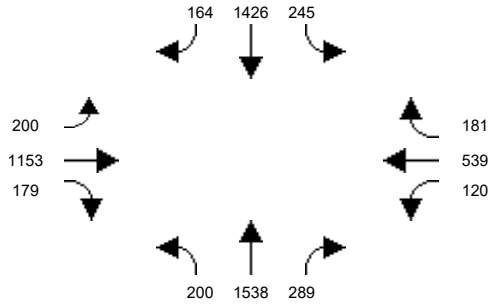
Intersection #18: Harbor Blvd and I-405 SB Off-Ramp



Intersection #19: Fairview St and Civic Center Dr



Intersection #20: Fairview St and 1st St

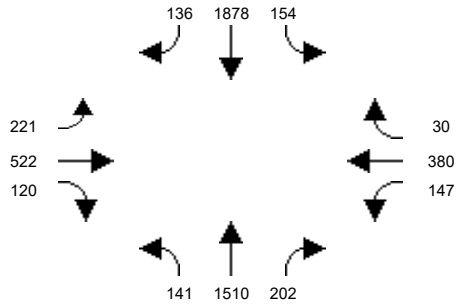


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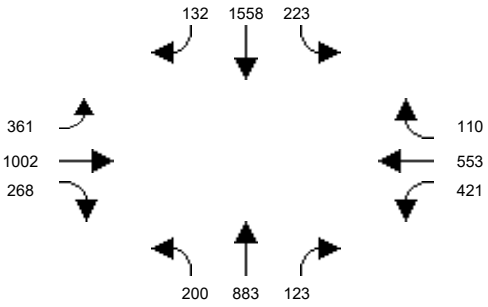
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP AM Peak Hour

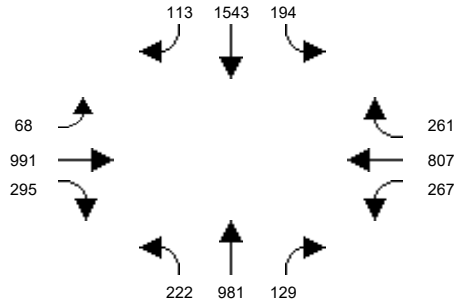
Intersection #21: Fairview St and McFadden Ave



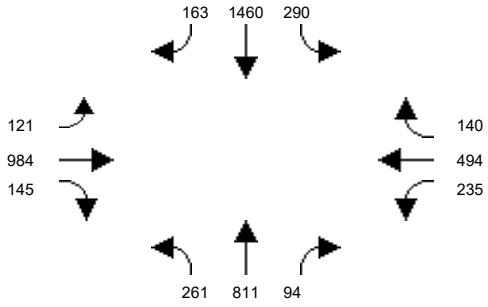
Intersection #22: Fairview St and Edinger Ave



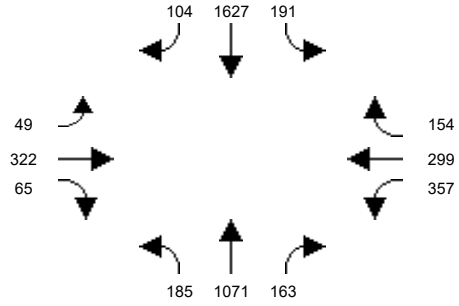
Intersection #23: Fairview St and Warner Ave



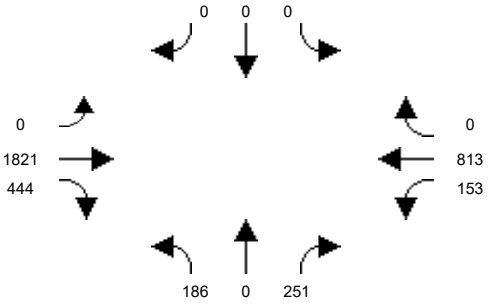
Intersection #24: Fairview St and MacArthur Blvd



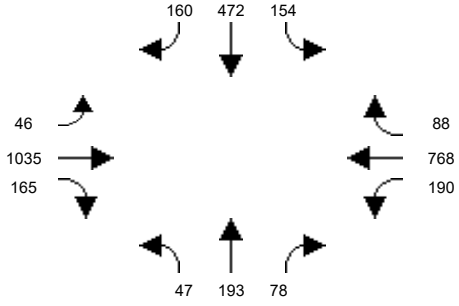
Intersection #25: Fairview Rd and Sunflower Ave



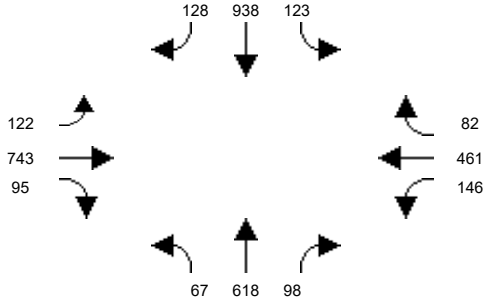
Intersection #26: Greenville St and Edinger Ave



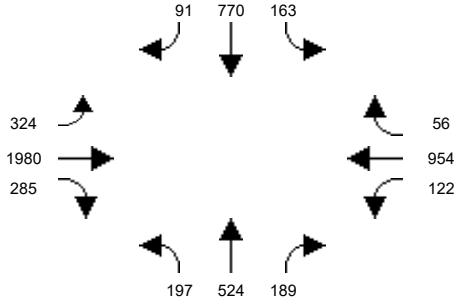
Intersection #27: Greenville St and Segerstrom Ave



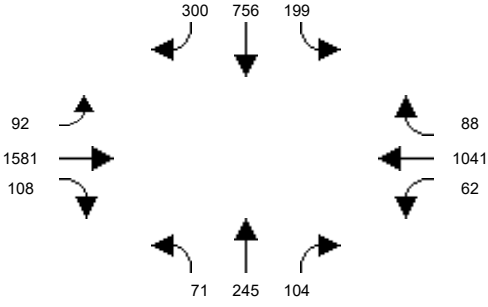
Intersection #28: Raitt St and McFadden Ave



Intersection #29: Raitt St and Edinger Ave



Intersection #30: Bear St and MacArthur Blvd

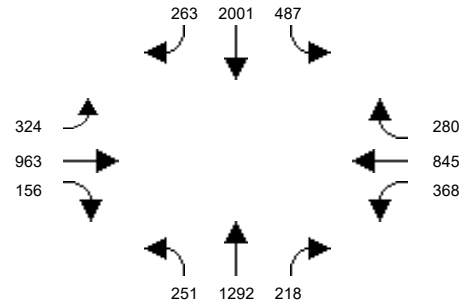


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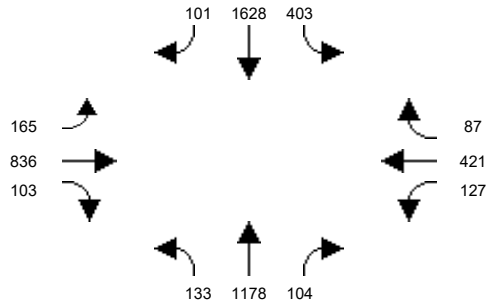
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP AM Peak Hour

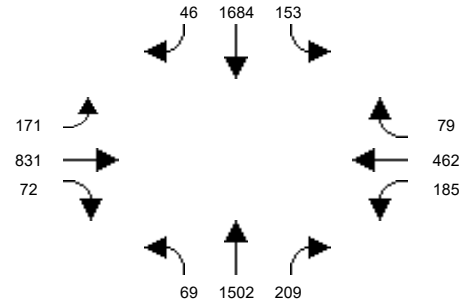
Intersection #31: Bristol St and 17th St



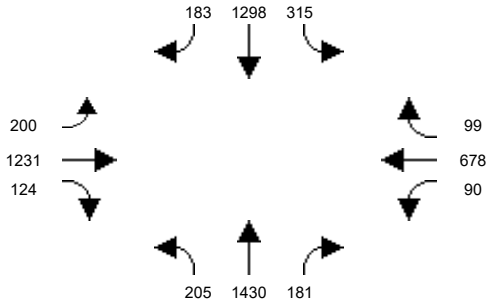
Intersection #32: Bristol St and Civic Center Dr



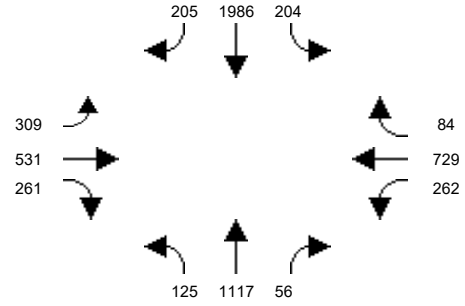
Intersection #33: Bristol St and Santa Ana Blvd



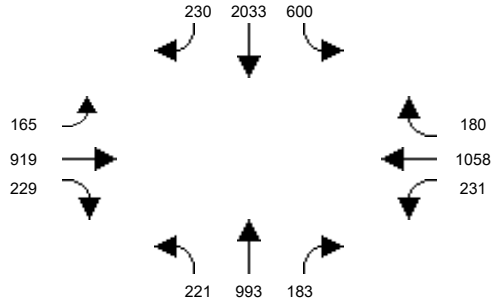
Intersection #34: Bristol St and 1st St



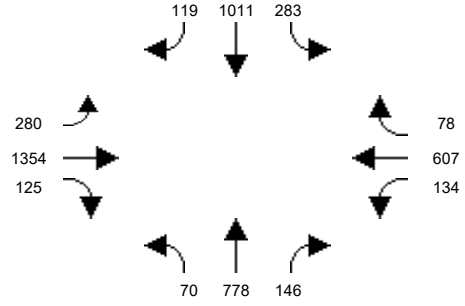
Intersection #35: Bristol St and McFadden Ave



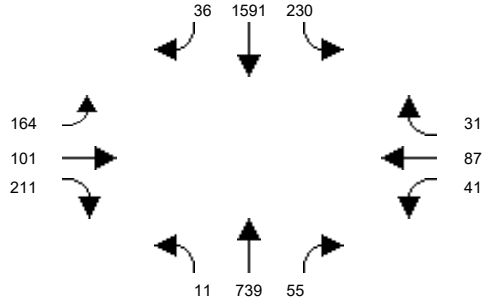
Intersection #36: Bristol St and Warner Ave



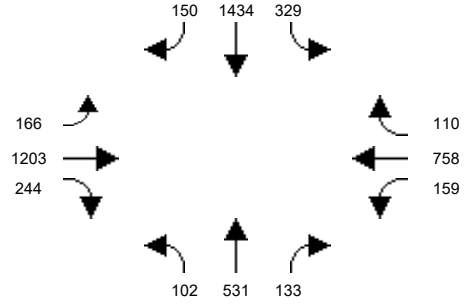
Intersection #37: Bristol St and Segerstrom Ave



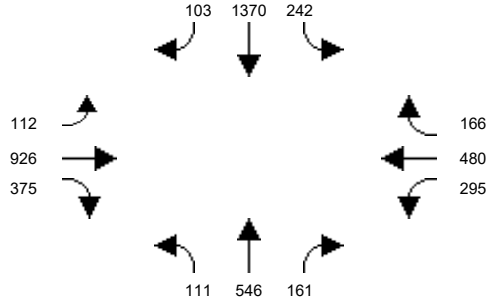
Intersection #38: Bristol St and Alton Ave



Intersection #39: Bristol St and MacArthur Blvd



Intersection #40: Bristol St and Sunflower Ave

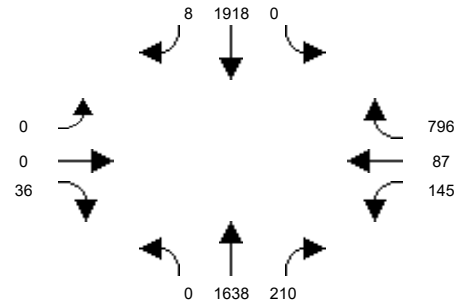


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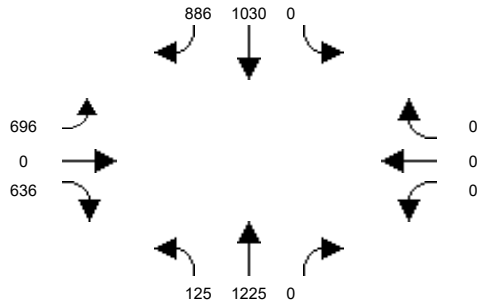
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP AM Peak Hour

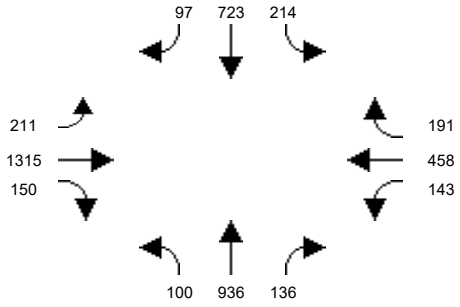
Intersection #41: Bristol St and I-405 NB Ramps



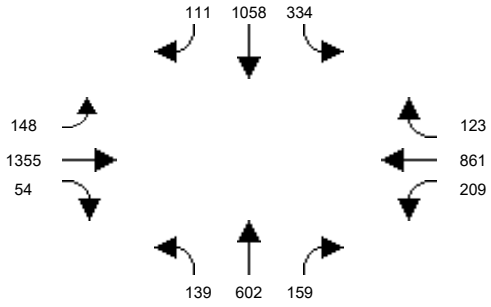
Intersection #42: Bristol St and I-405 SB Ramps



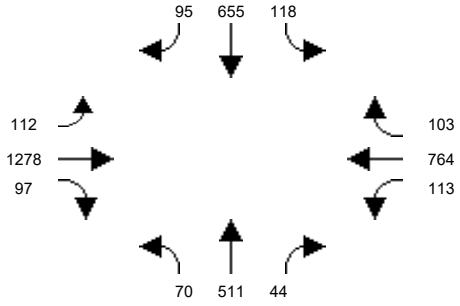
Intersection #43: Flower St and Santa Ana Blvd



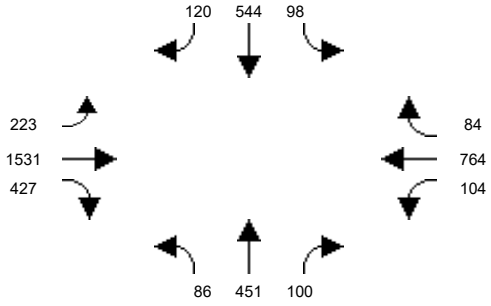
Intersection #44: Flower St and 1st St



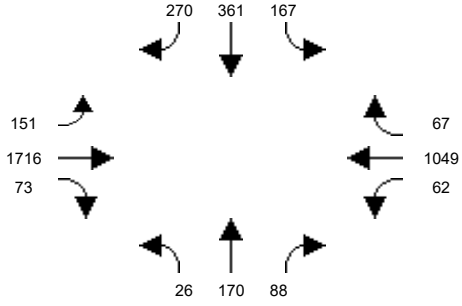
Intersection #45: Flower St and McFadden Ave



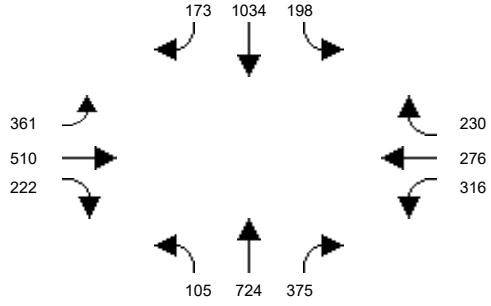
Intersection #46: Flower St and Segerstrom Ave



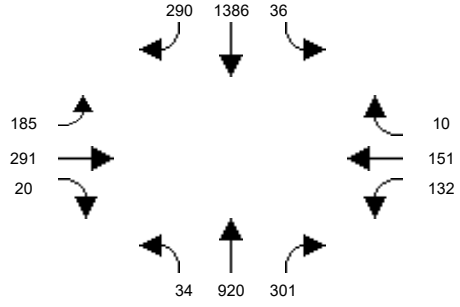
Intersection #47: Flower St and MacArthur Blvd



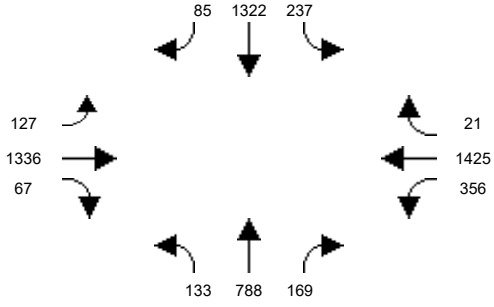
Intersection #48: Main St and La Veta Ave



Intersection #49: Main St and Mainplace Dr / Memory Ln



Intersection #50: Main St and 17th St

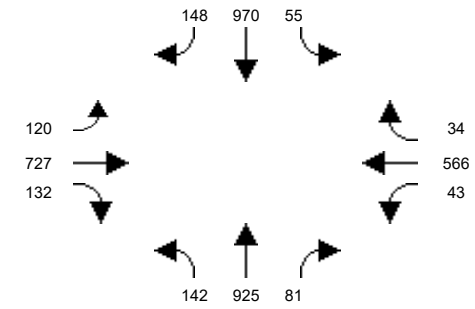


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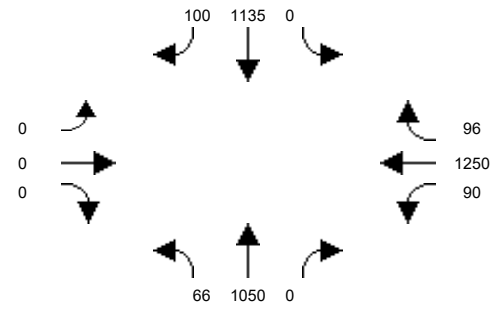
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP AM Peak Hour

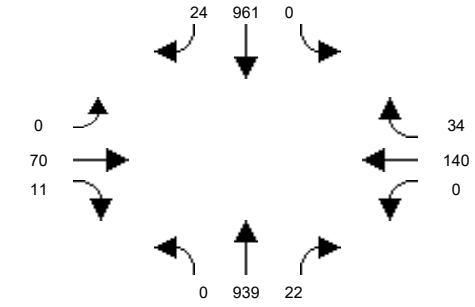
Intersection #51: Main St and Civic Center Dr



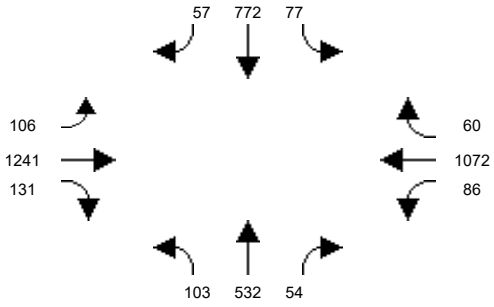
Intersection #52: Main St and Santa Ana Blvd



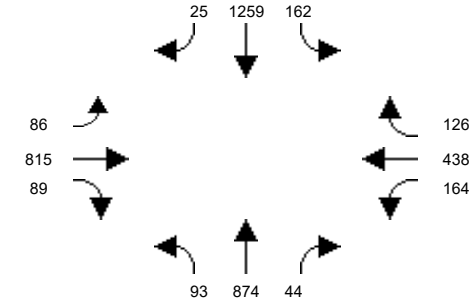
Intersection #53: Main St and 4th St



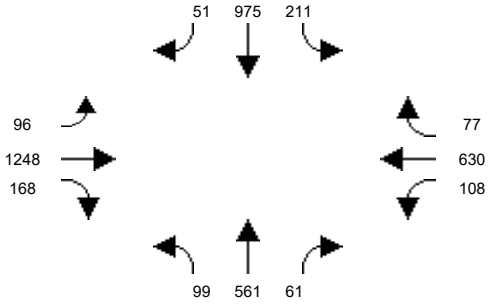
Intersection #54: Main St and 1st St



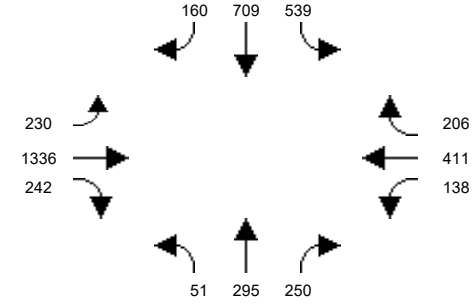
Intersection #55: Main St and McFadden Ave



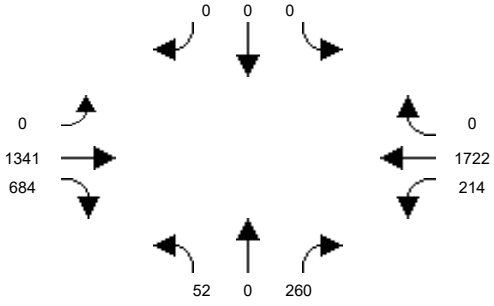
Intersection #56: Main St and Edinger Ave



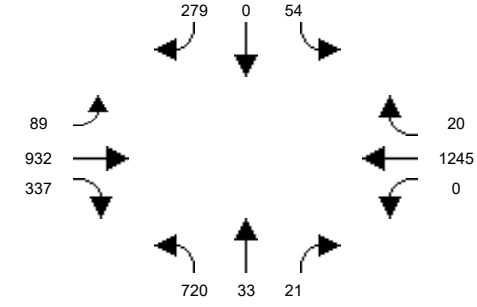
Intersection #57: Main St and MacArthur Blvd



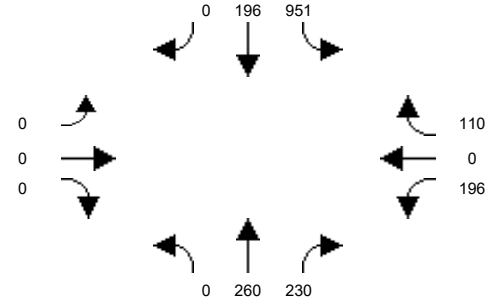
Intersection #58: Penn Wy and 17th St



Intersection #59: I-5 NB Off Ramps/17th Street



Intersection #60: Penn Wy and I-5 SB Ramps

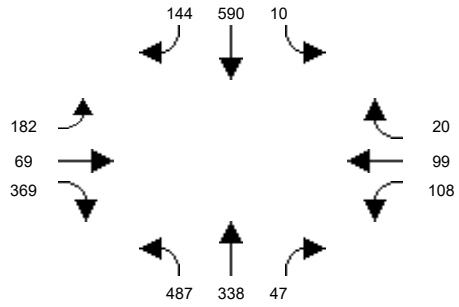


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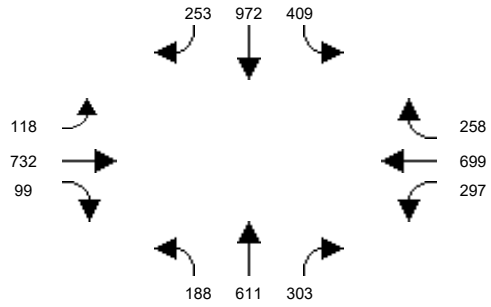
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP AM Peak Hour

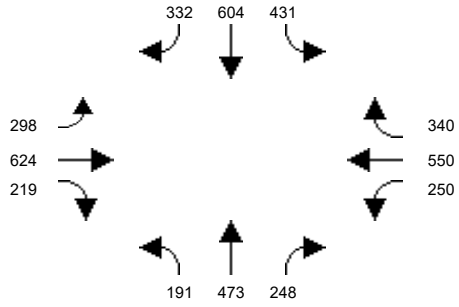
Intersection #61: Santiago St and Civic Center Dr



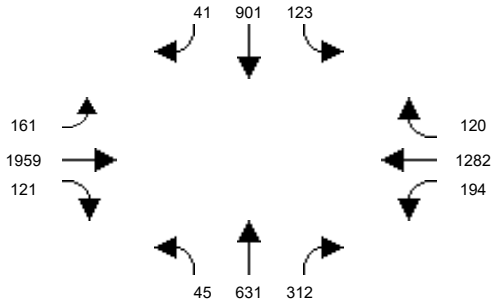
Intersection #62: Santiago St and Santa Ana Blvd



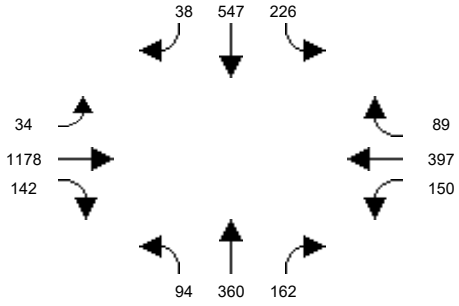
Intersection #63: Standard Ave and 4th St



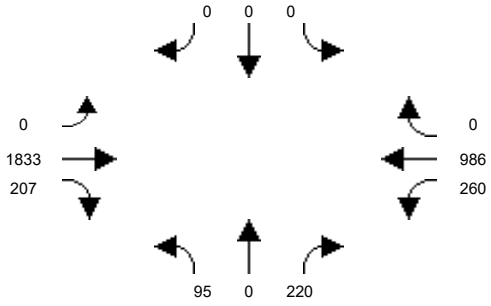
Intersection #64: Standard Ave and 1st St



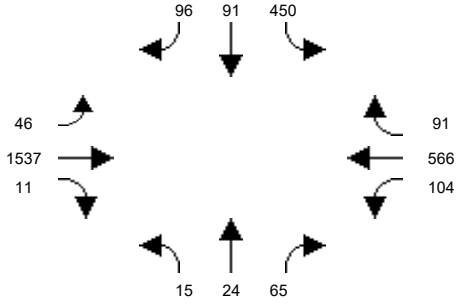
Intersection #65: Standard Ave and Mcfadden Ave



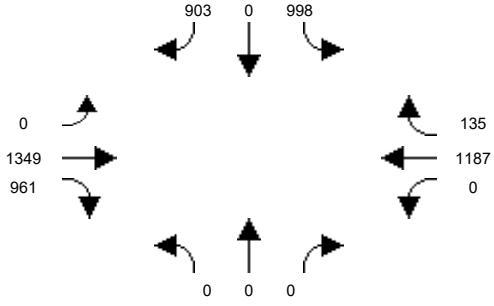
Intersection #66: Halladay St and Warner Ave



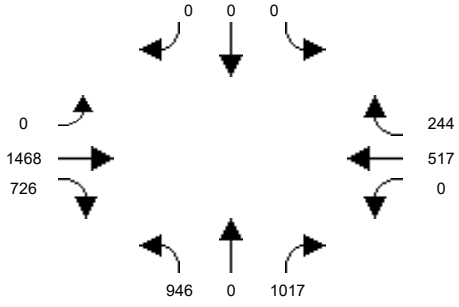
Intersection #67: Halladay St and Dyer Rd



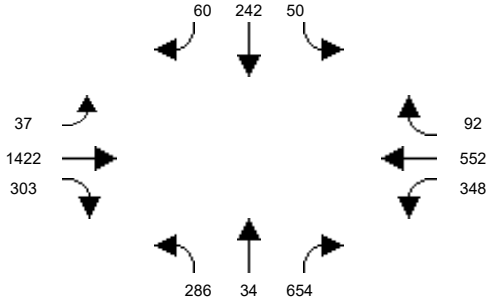
Intersection #68: SR-55 SB Ramps and MacArthur Blvd



Intersection #69: SR-55 NB Ramps and MacArthur Blvd



Intersection #70: SR-55 SB Ramps and Dyer Rd

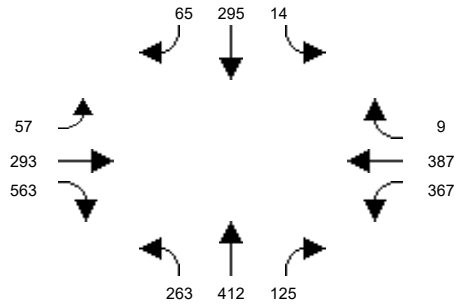


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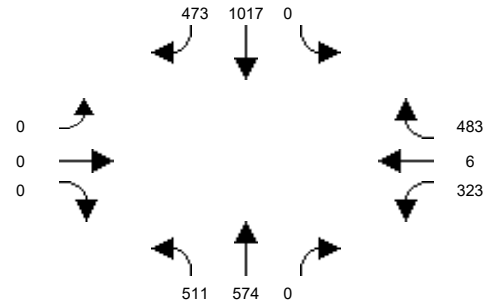
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP AM Peak Hour

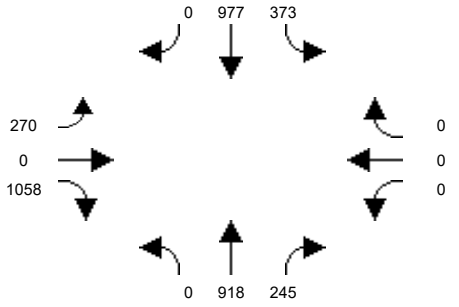
Intersection #71: Glassell St and La Veta Ave



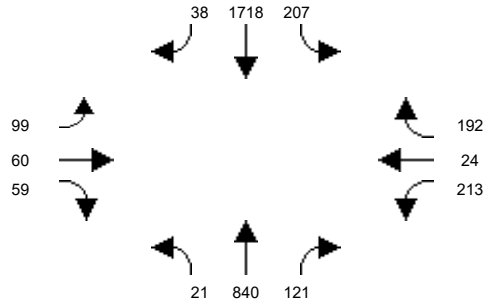
Intersection #72: Glassell St and SR-22 WB Ramps



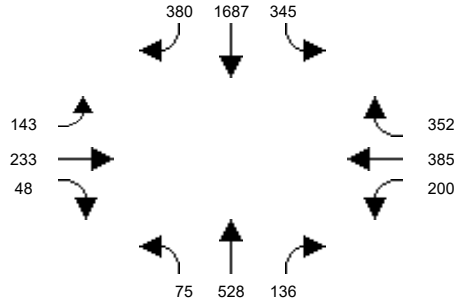
Intersection #73: Grand Ave / Glassell St and SR-22 EB Ramps



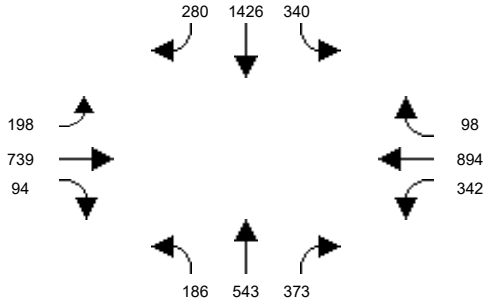
Intersection #74: Grand Ave and Fairhaven Ave



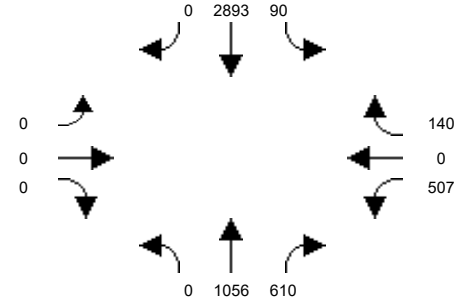
Intersection #75: Grand Ave and Santa Clara Ave



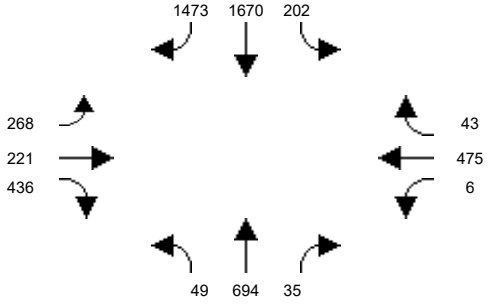
Intersection #76: Grand Ave and 17th St



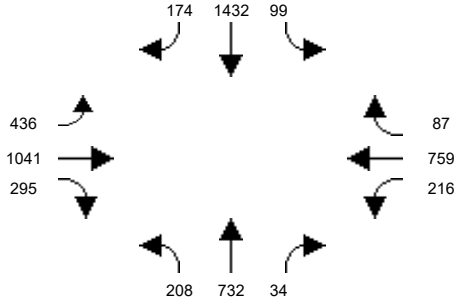
Intersection #77: Grand Ave and I-5 NB Ramps



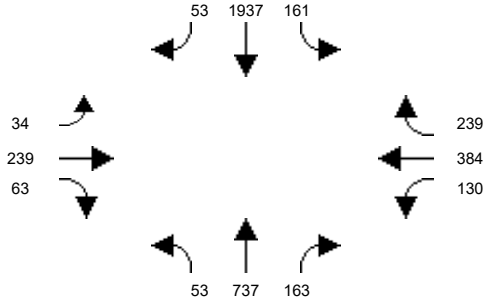
Intersection #78: Grand Ave and Santa Ana Blvd



Intersection #79: Grand Ave and 1st St



Intersection #80: Grand Ave and Chestnut Ave

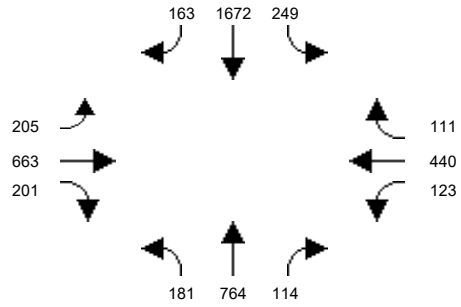


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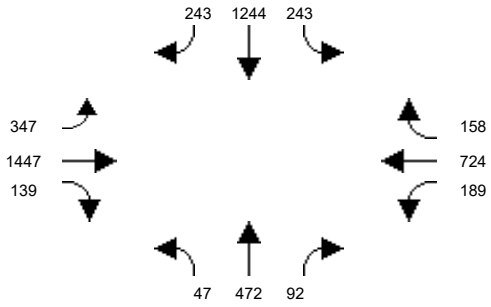
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP AM Peak Hour

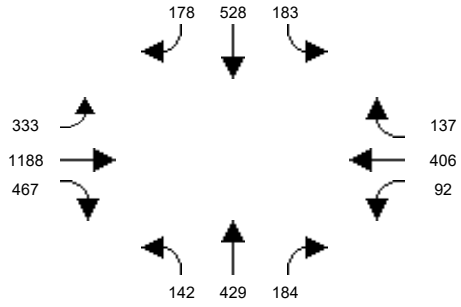
Intersection #81: Grand Ave and McFadden Ave



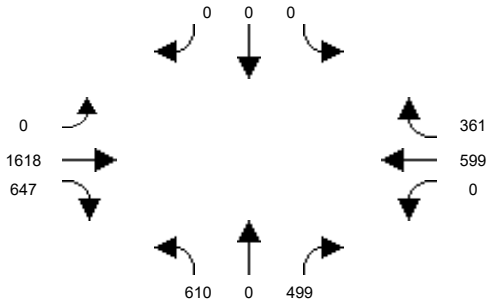
Intersection #82: Grand Ave and Edinger Ave



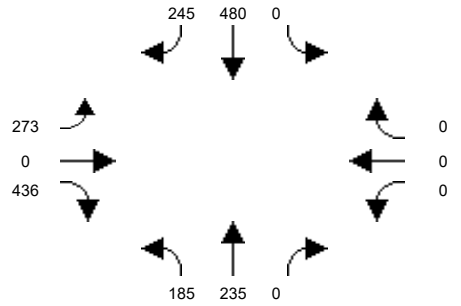
Intersection #83: Grand Ave and Warner Ave



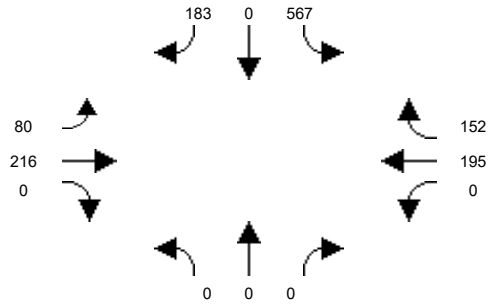
Intersection #84: SR-55 NB Ramps and Dyer Rd



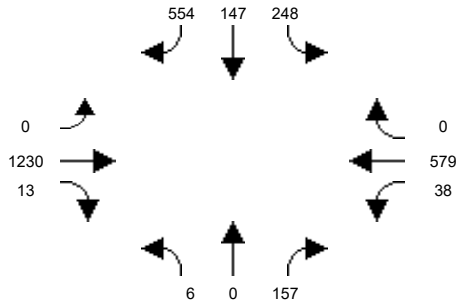
Intersection #85: Cambridge St and La Veta Ave



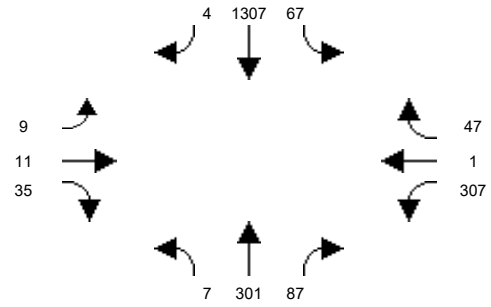
Intersection #86: Cambridge St and Fairhaven Ave



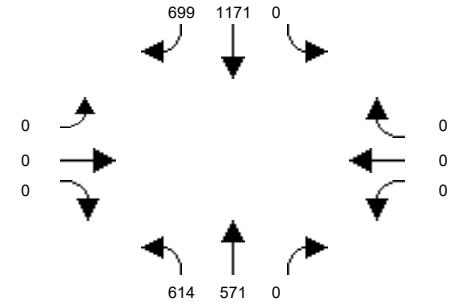
Intersection #87: Mabury St and 1st Street



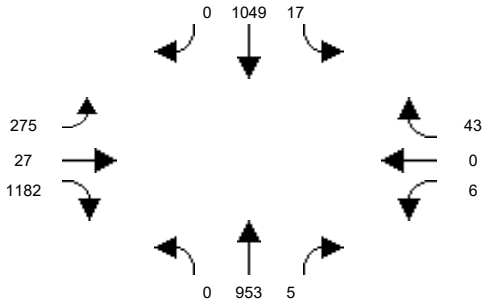
Intersection #88: Tustin St and La Veta Ave



Intersection #89: Tustin St and SR-22 WB On-Ramp



Intersection #90: Tustin St and SR-22 EB Off-Ramp / Seba Ave

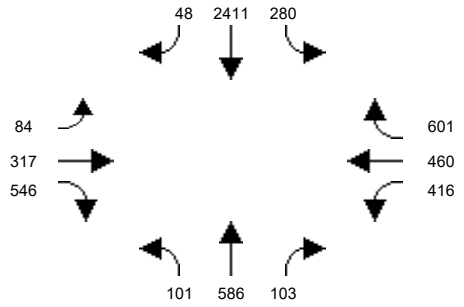


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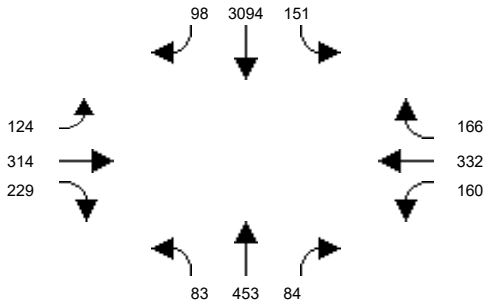
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP AM Peak Hour

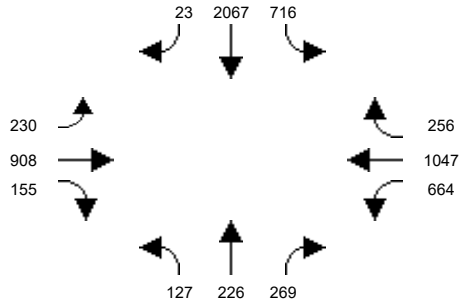
Intersection #91: Tustin Ave and Fairhaven Ave



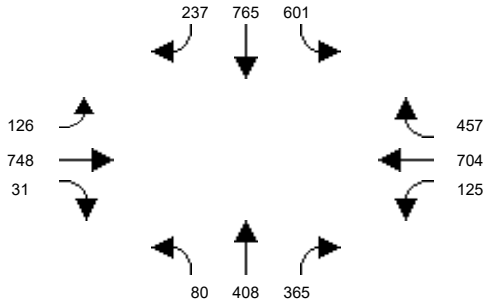
Intersection #92: Tustin Ave and Santa Clara Ave



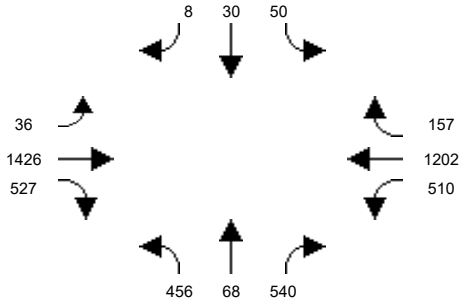
Intersection #93: Tustin Ave and 17th St



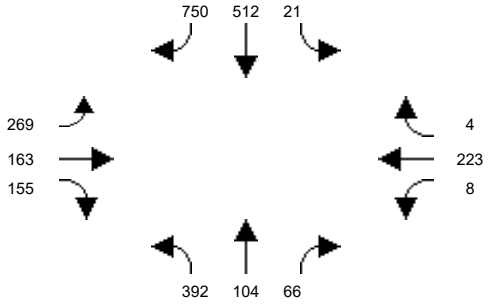
Intersection #94: Tustin Ave and 4th St



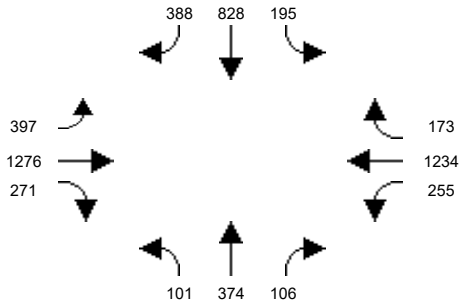
Intersection #95: SR-55 SB Ramps / Auto Mall Dr and Edinger Ave



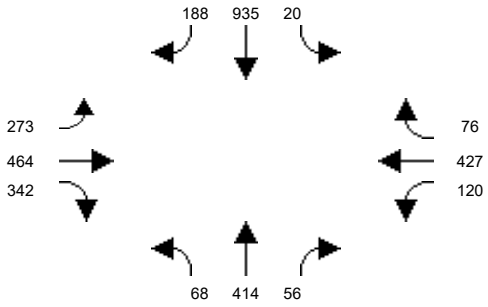
Intersection #96: SR-55 NB Ramps / Del Amo Ave and Newport Avenue



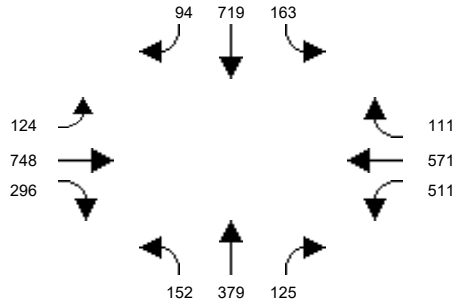
Intersection #97: Red Hill Ave and Edinger Ave



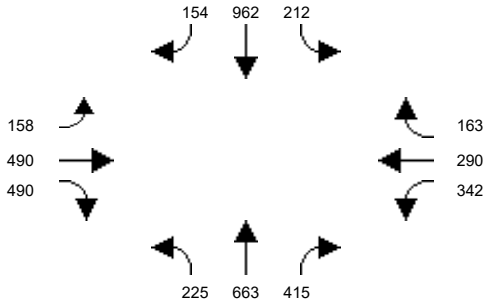
Intersection #98: Red Hill Ave and Warner Ave



Intersection #99: Red Hill Ave and Dyer Rd / barranca Pkwy



Intersection #100: Red Hill Ave and Alton Pkwy

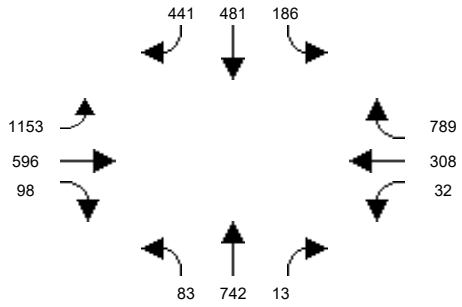


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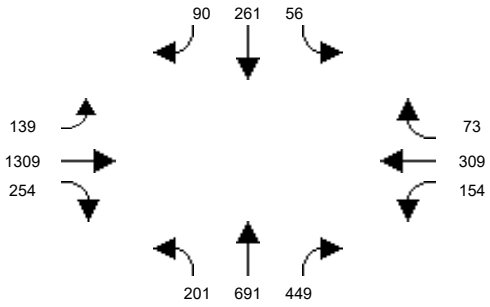
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP AM Peak Hour

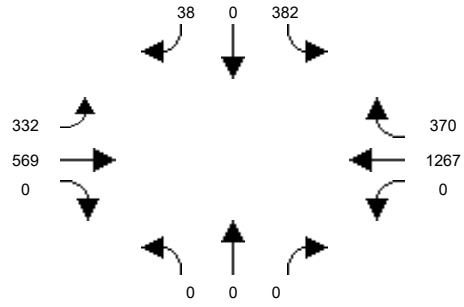
Intersection #101: Red Hill Ave and MacArthur Blvd



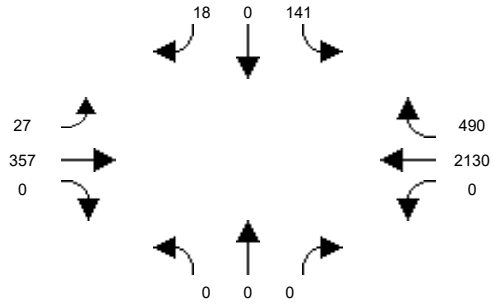
Intersection #102: Red Hill Ave and Main St



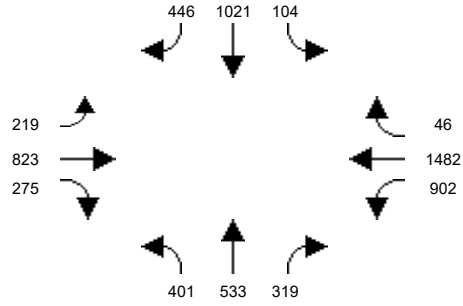
Intersection #103: I-5 SB Ramps and Santa Ana Blvd



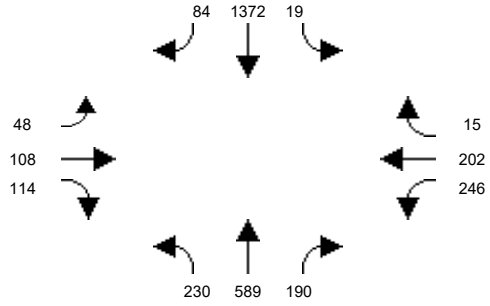
Intersection #104: Tustin Ranch Rd and Warner Ave



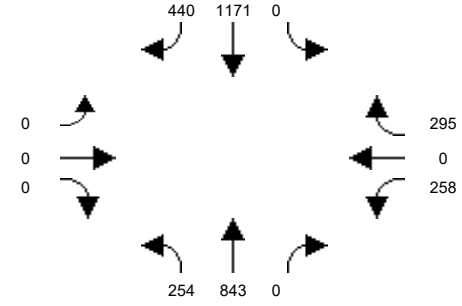
Intersection #105: Von Karman Ave and Barranca Pkwy



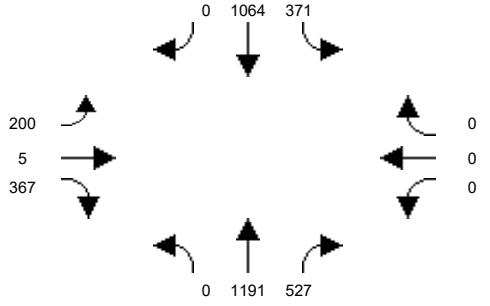
Intersection #106: Red Hill Avenue and El Camino Real



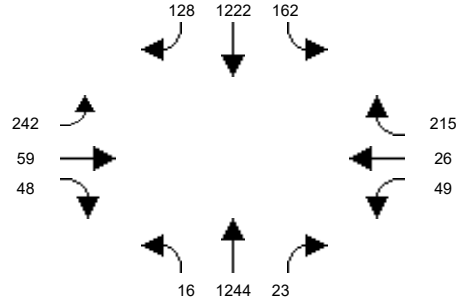
Intersection #107: Red Hill Avenue and I-5 NB Ramps



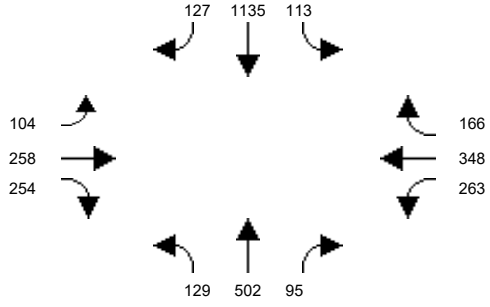
Intersection #108: Red Hill Avenue and I-5 SB Ramps



Intersection #109: Red Hill Avenue and Nisson Road



Intersection #110: Red Hill Avenue and Walnut Avenue

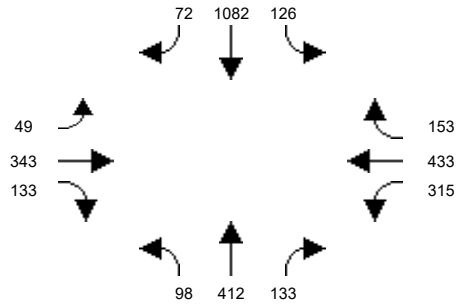


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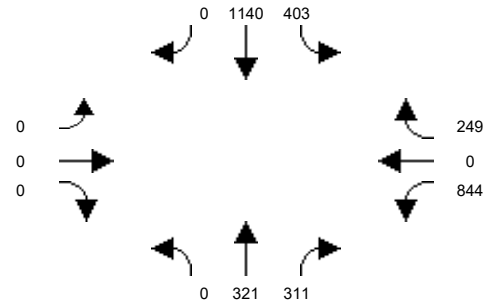
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP AM Peak Hour

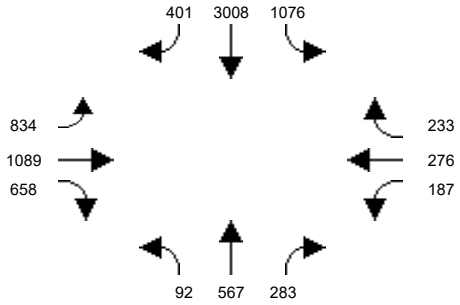
Intersection #111: Red Hill Avenue and Valencia Avenue



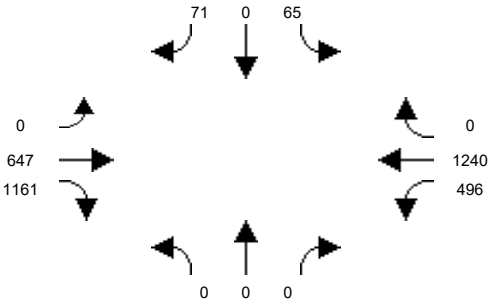
Intersection #112: Tustin Ranch Road and Warner Avenue North



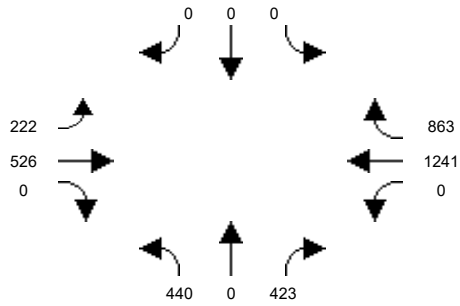
Intersection #113: Tustin Ranch Road and Walnut Avenue



Intersection #114: SR-55 SB Ramps and Irvine Boulevard



Intersection #115: SR-55 NB Ramps and Irvine Boulevard



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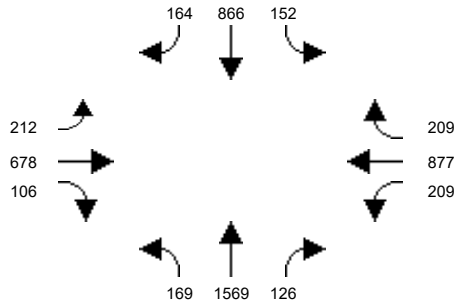
APPENDIX B.4

INTERSECTION VOLUMES –
2045 NP (PM PEAK HOUR)

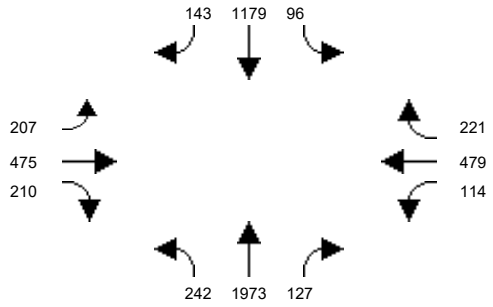
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP PM Peak Hour

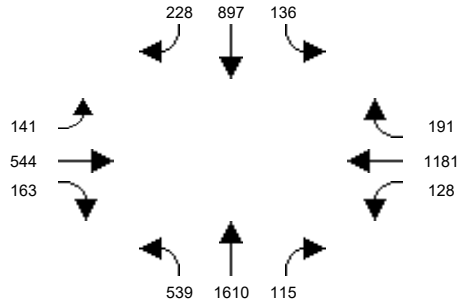
Intersection #1: Euclid St and 1st St



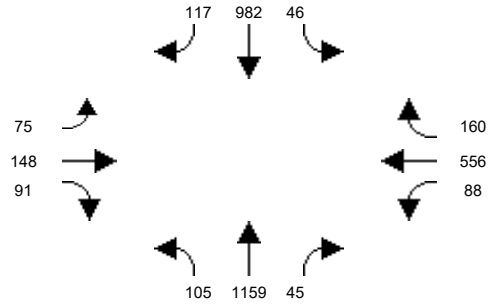
Intersection #2: Euclid St and McFadden Ave



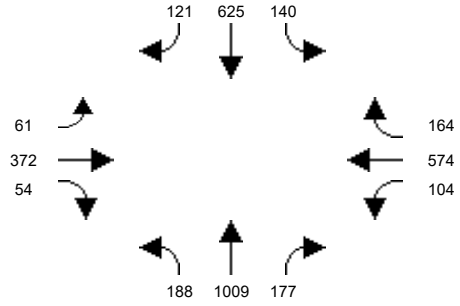
Intersection #3: Euclid St and Edinger Ave



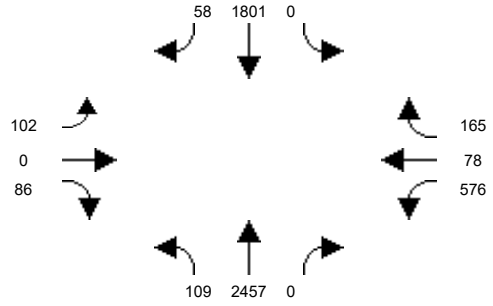
Intersection #4: Newhope St and Hazard Ave



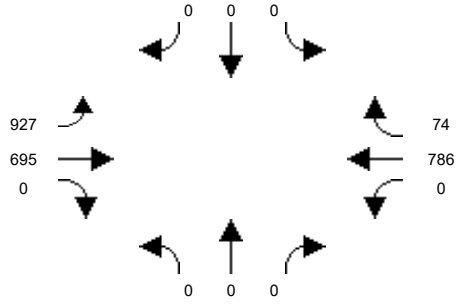
Intersection #5: Newhope St and McFadden Ave



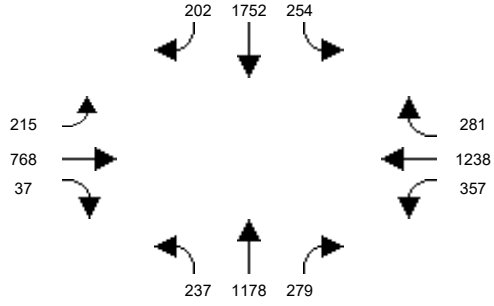
Intersection #6: Harbor Blvd and SR-22 WB Off-Ramp / Banner Dr



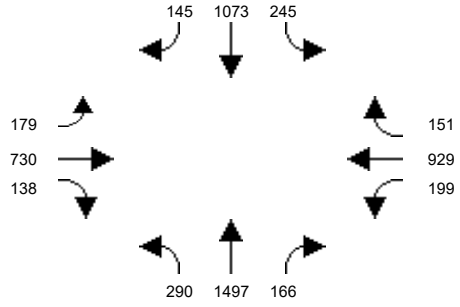
Intersection #7: Trask Avenue and SR-22 EB On-Ramp



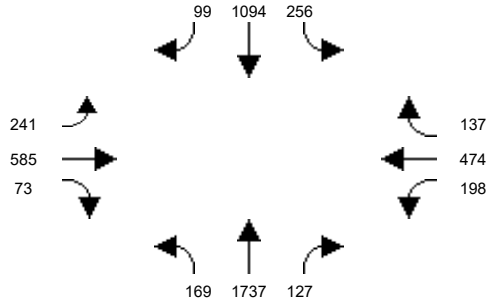
Intersection #8: Harbor Blvd and Westminster Ave



Intersection #9: Harbor Blvd and 1st St



Intersection #10: Harbor Blvd and McFadden Ave

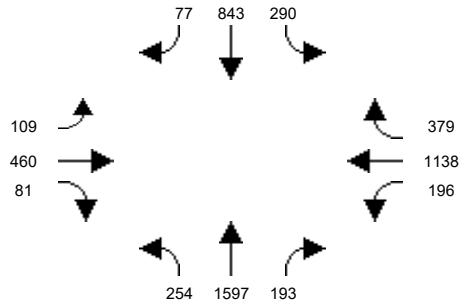


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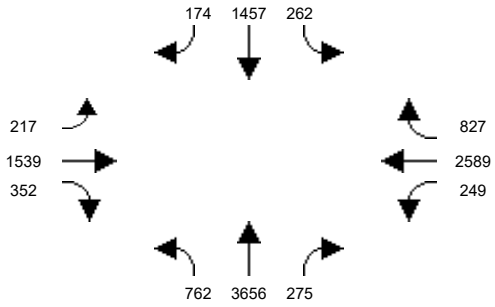
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP PM Peak Hour

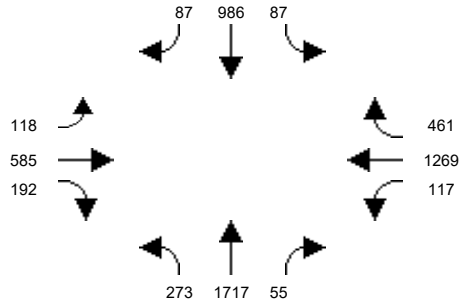
Intersection #11: Harbor Blvd and Edinger Ave



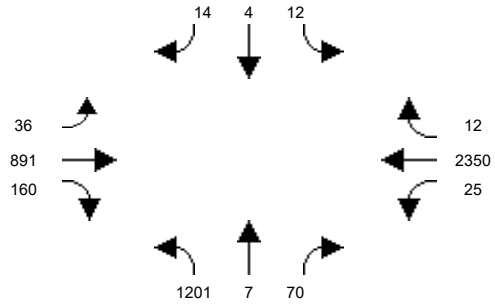
Intersection #12: Harbor Blvd and Warner Ave



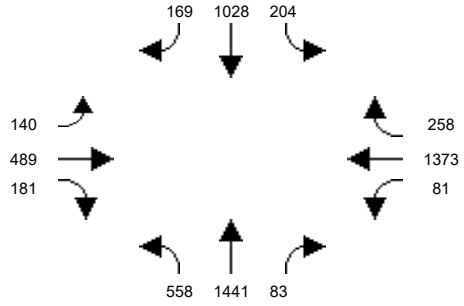
Intersection #13: Harbor Blvd and Segerstrom Ave



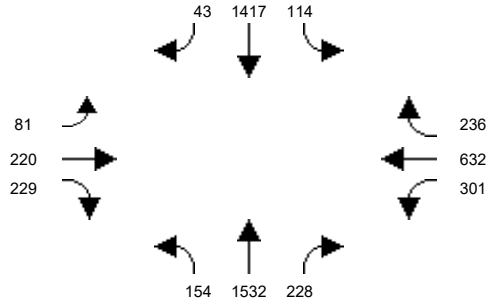
Intersection #14: MacArthur Blvd and Hyland Ave



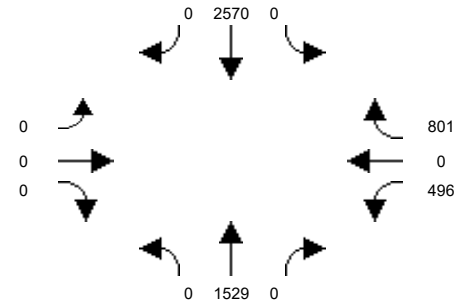
Intersection #15: MacArthur Blvd and Harbor Blvd



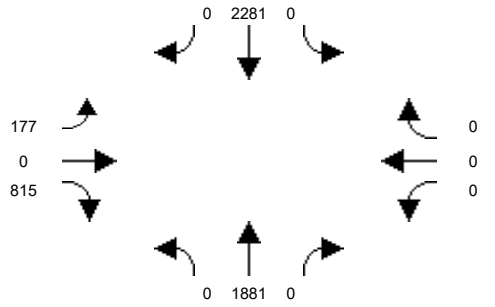
Intersection #16: Harbor Blvd and Sunflower Ave



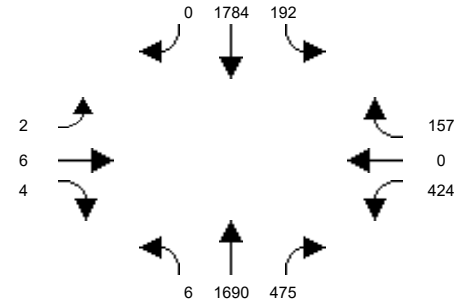
Intersection #17: Harbor Blvd and I-405 NB Off-Ramp



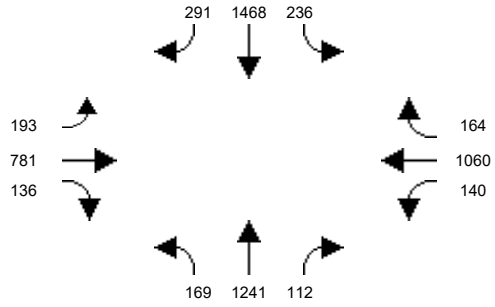
Intersection #18: Harbor Blvd and I-405 SB Off-Ramp



Intersection #19: Fairview St and Civic Center Dr



Intersection #20: Fairview St and 1st St

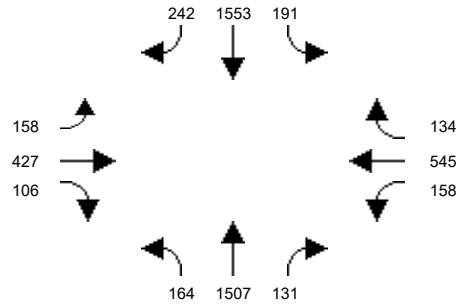


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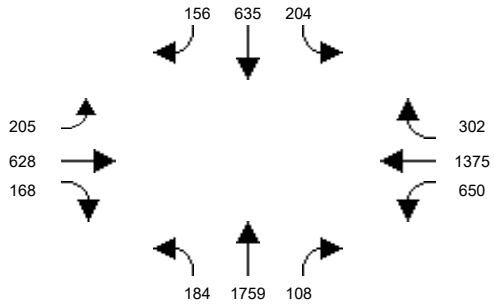
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP PM Peak Hour

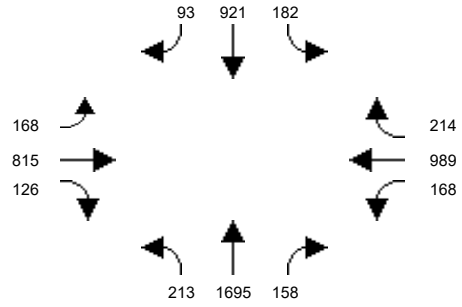
Intersection #21: Fairview St and McFadden Ave



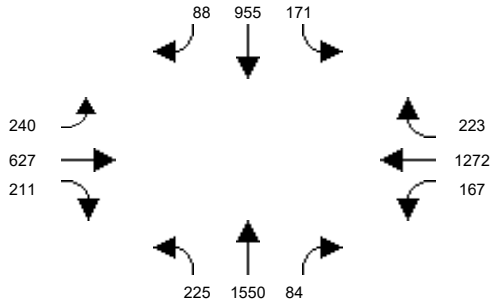
Intersection #22: Fairview St and Edinger Ave



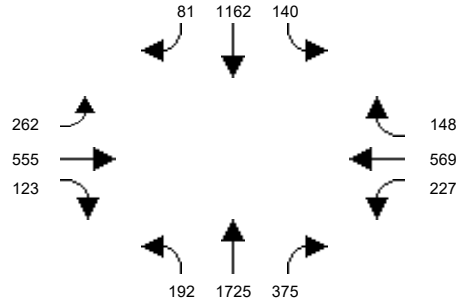
Intersection #23: Fairview St and Warner Ave



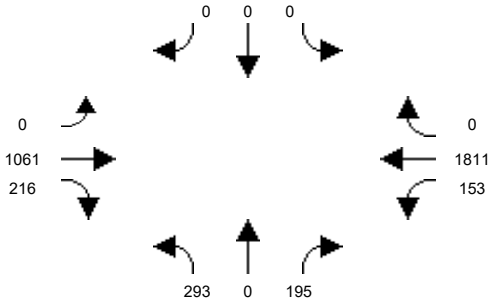
Intersection #24: Fairview St and MacArthur Blvd



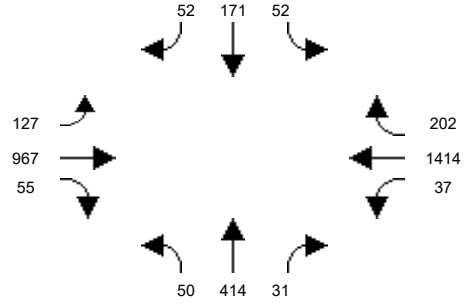
Intersection #25: Fairview Rd and Sunflower Ave



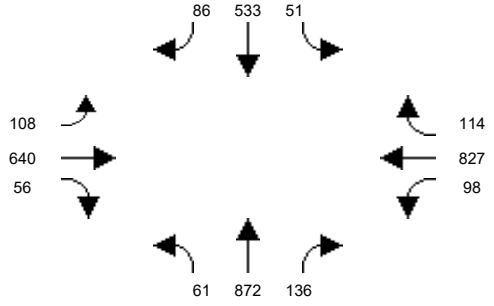
Intersection #26: Greenville St and Edinger Ave



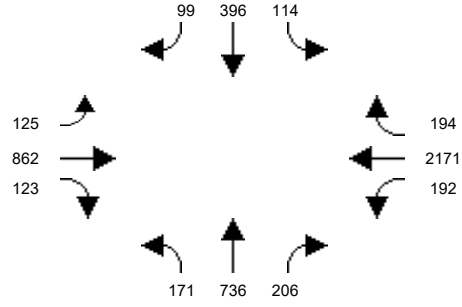
Intersection #27: Greenville St and Segerstrom Ave



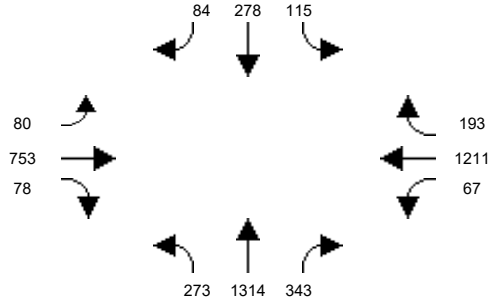
Intersection #28: Raitt St and McFadden Ave



Intersection #29: Raitt St and Edinger Ave



Intersection #30: Bear St and MacArthur Blvd

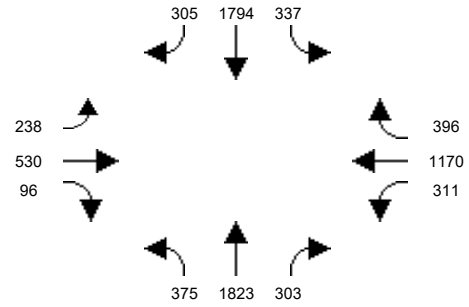


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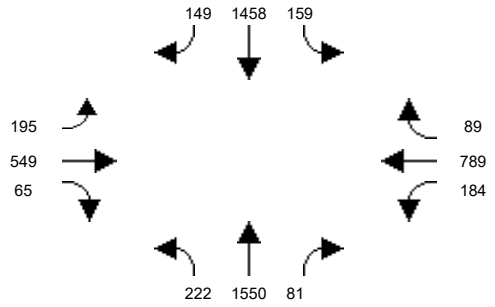
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP PM Peak Hour

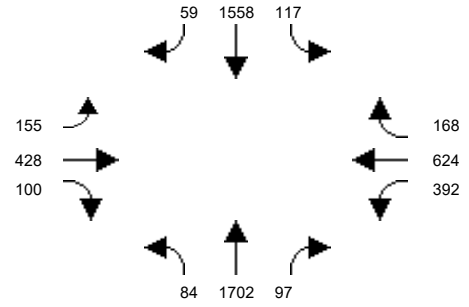
Intersection #31: Bristol St and 17th St



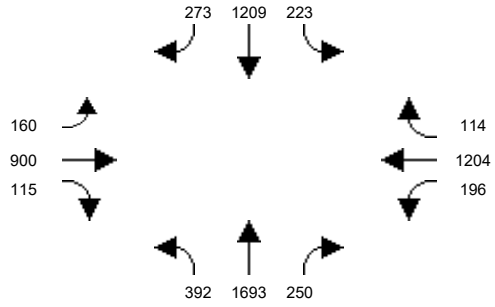
Intersection #32: Bristol St and Civic Center Dr



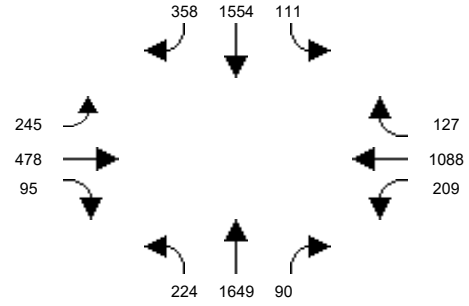
Intersection #33: Bristol St and Santa Ana Blvd



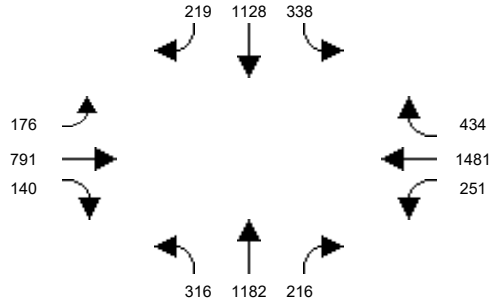
Intersection #34: Bristol St and 1st St



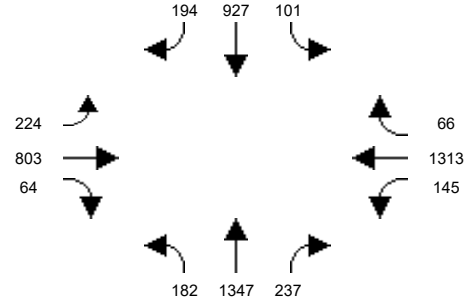
Intersection #35: Bristol St and McFadden Ave



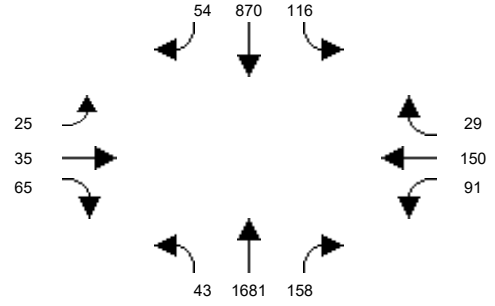
Intersection #36: Bristol St and Warner Ave



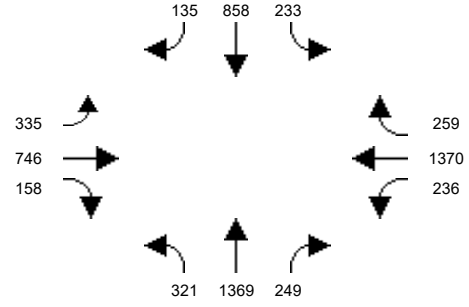
Intersection #37: Bristol St and Segerstrom Ave



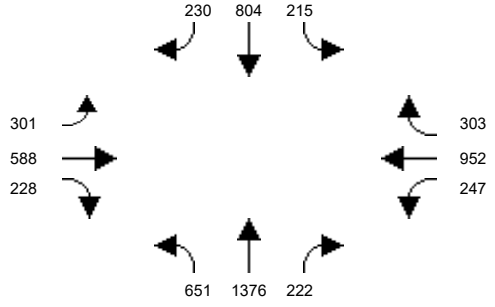
Intersection #38: Bristol St and Alton Ave



Intersection #39: Bristol St and MacArthur Blvd



Intersection #40: Bristol St and Sunflower Ave

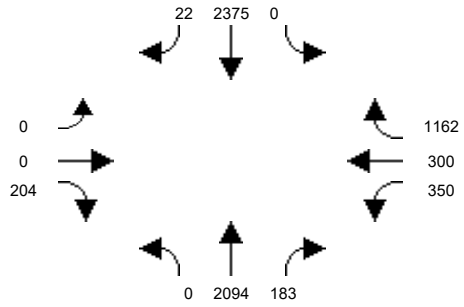


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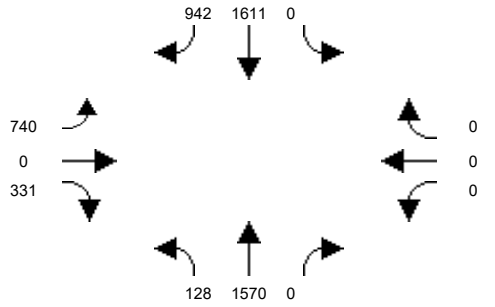
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP PM Peak Hour

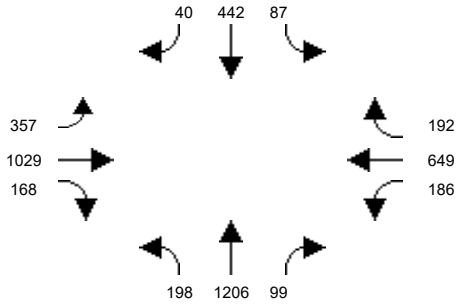
Intersection #41: Bristol St and I-405 NB Ramps



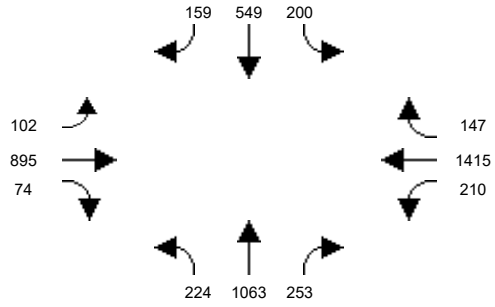
Intersection #42: Bristol St and I-405 SB Ramps



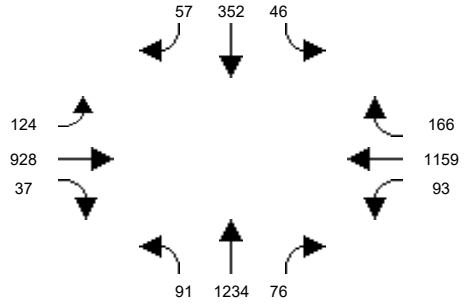
Intersection #43: Flower St and Santa Ana Blvd



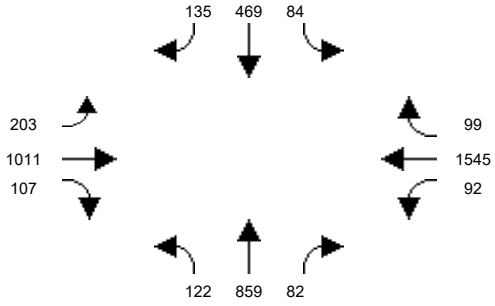
Intersection #44: Flower St and 1st St



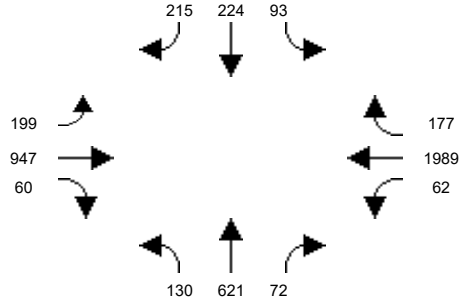
Intersection #45: Flower St and McFadden Ave



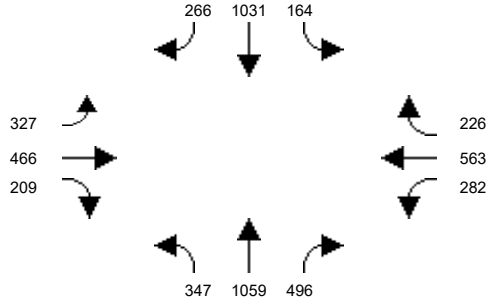
Intersection #46: Flower St and Segerstrom Ave



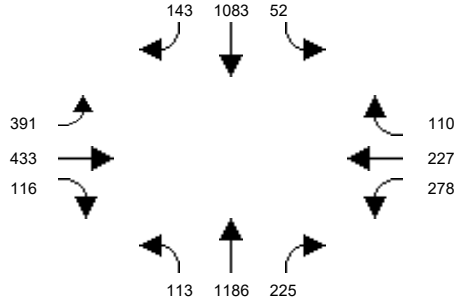
Intersection #47: Flower St and MacArthur Blvd



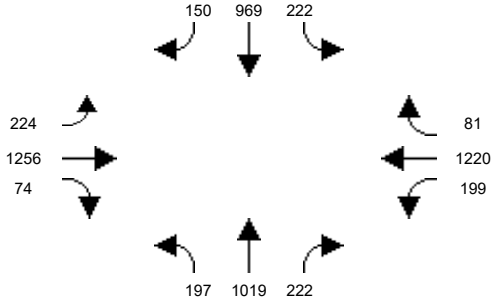
Intersection #48: Main St and La Veta Ave



Intersection #49: Main St and Mainplace Dr / Memory Ln



Intersection #50: Main St and 17th St

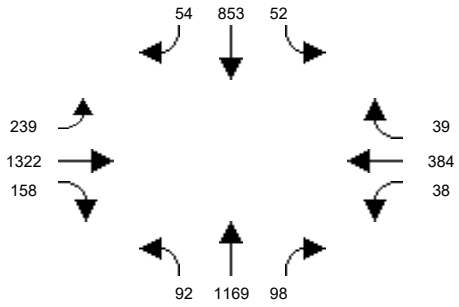


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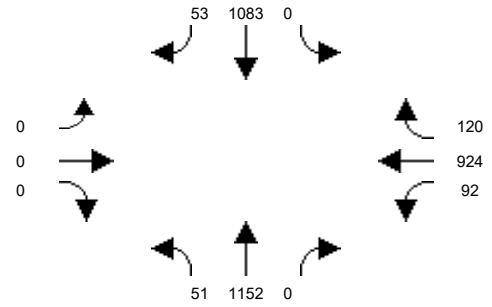
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP PM Peak Hour

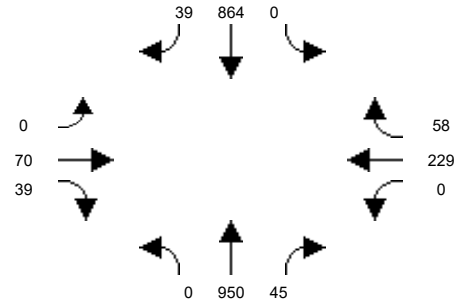
Intersection #51: Main St and Civic Center Dr



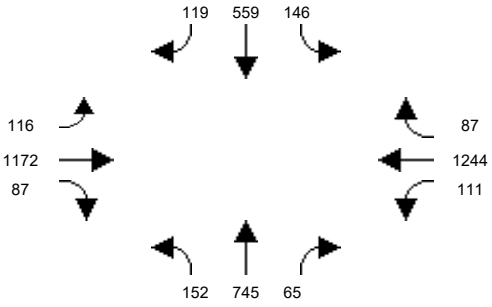
Intersection #52: Main St and Santa Ana Blvd



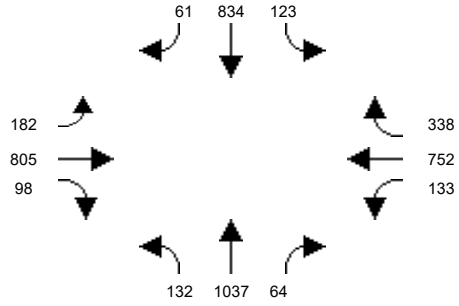
Intersection #53: Main St and 4th St



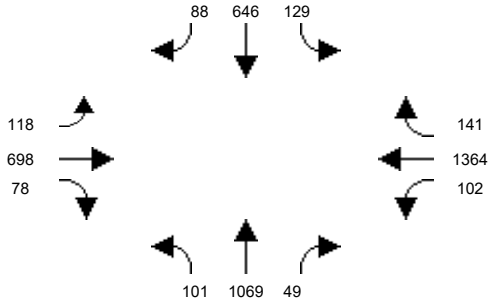
Intersection #54: Main St and 1st St



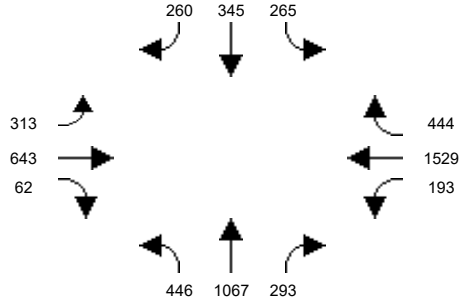
Intersection #55: Main St and McFadden Ave



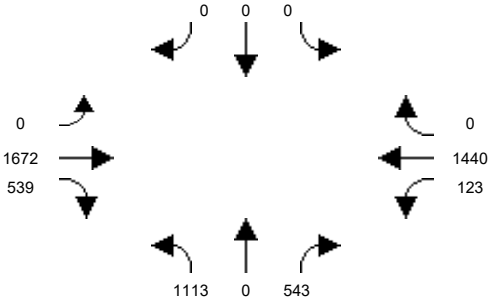
Intersection #56: Main St and Edinger Ave



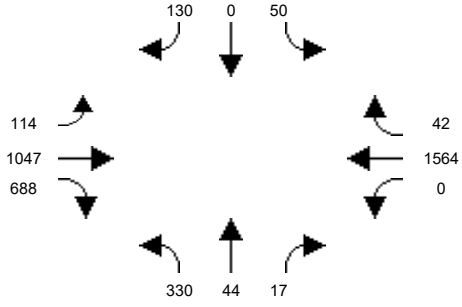
Intersection #57: Main St and MacArthur Blvd



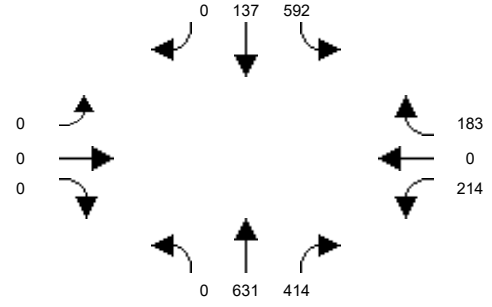
Intersection #58: Penn Wy and 17th St



Intersection #59: I-5 NB Off Ramps/17th Street



Intersection #60: Penn Wy and I-5 SB Ramps

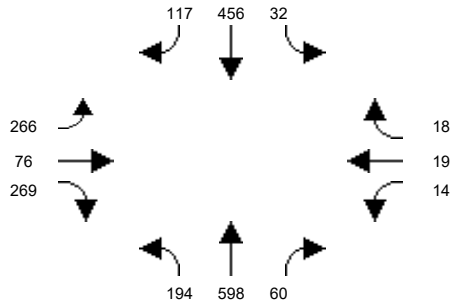


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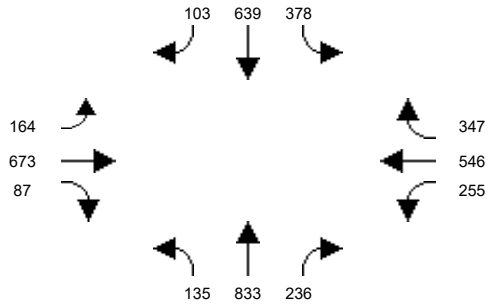
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP PM Peak Hour

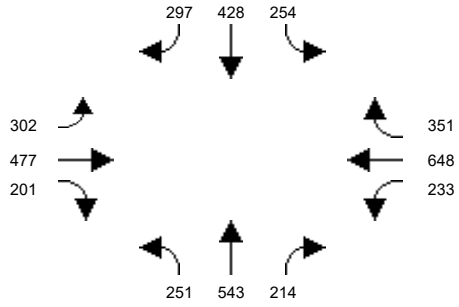
Intersection #61: Santiago St and Civic Center Dr



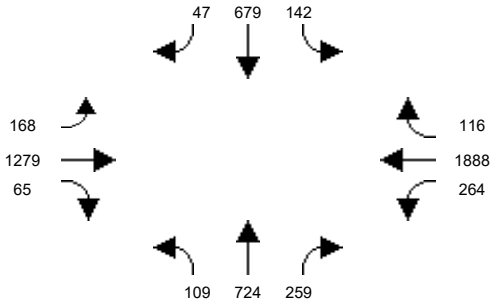
Intersection #62: Santiago St and Santa Ana Blvd



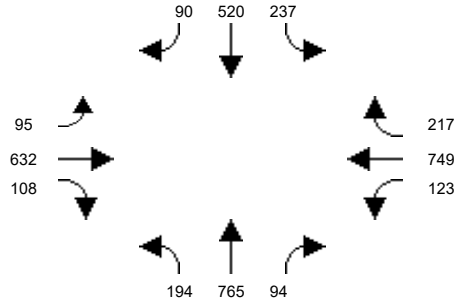
Intersection #63: Standard Ave and 4th St



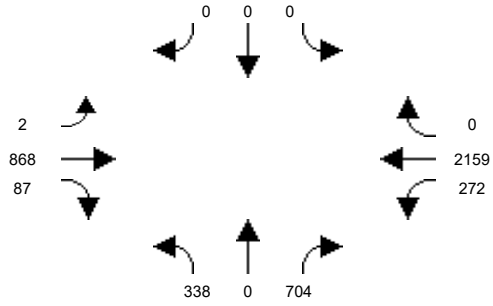
Intersection #64: Standard Ave and 1st St



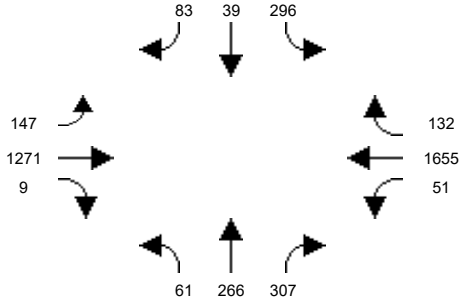
Intersection #65: Standard Ave and Mcfadden Ave



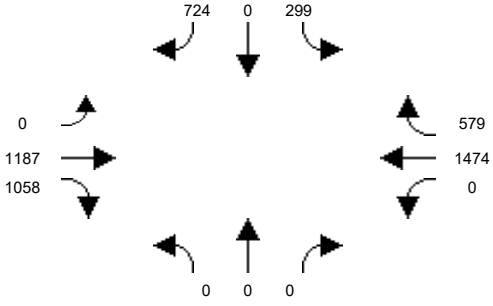
Intersection #66: Halladay St and Warner Ave



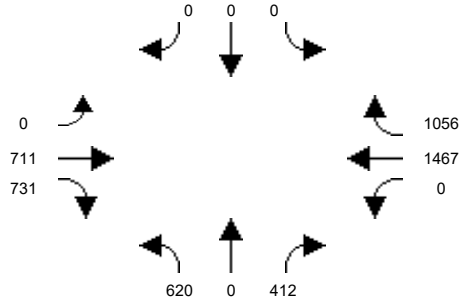
Intersection #67: Halladay St and Dyer Rd



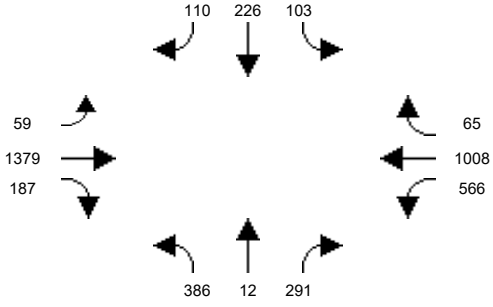
Intersection #68: SR-55 SB Ramps and MacArthur Blvd



Intersection #69: SR-55 NB Ramps and MacArthur Blvd



Intersection #70: SR-55 SB Ramps and Dyer Rd

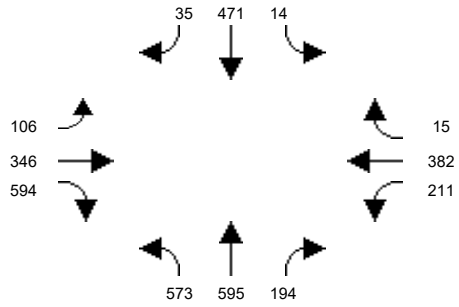


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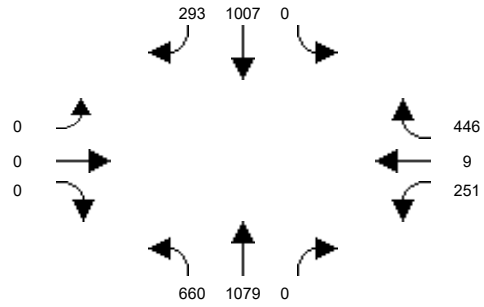
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP PM Peak Hour

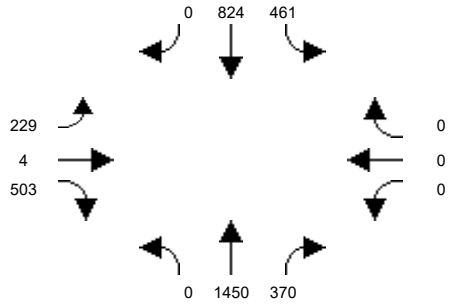
Intersection #71: Glassell St and La Veta Ave



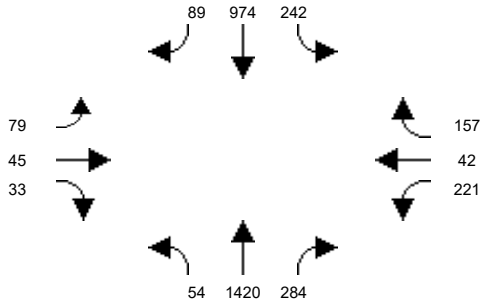
Intersection #72: Glassell St and SR-22 WB Ramps



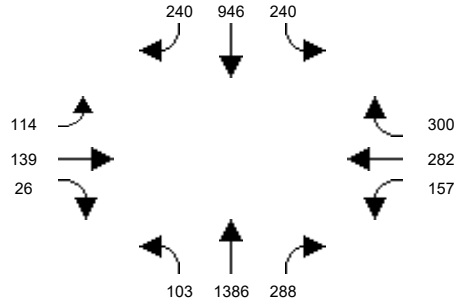
Intersection #73: Grand Ave / Glassell St and SR-22 EB Ramps



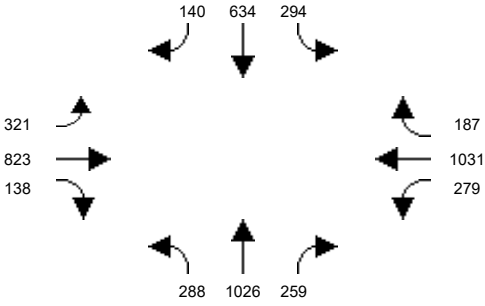
Intersection #74: Grand Ave and Fairhaven Ave



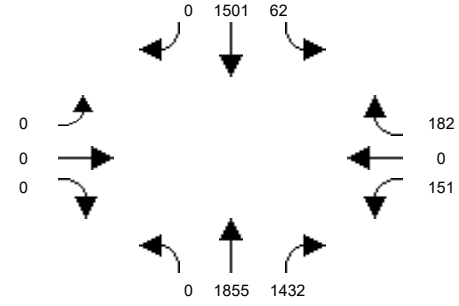
Intersection #75: Grand Ave and Santa Clara Ave



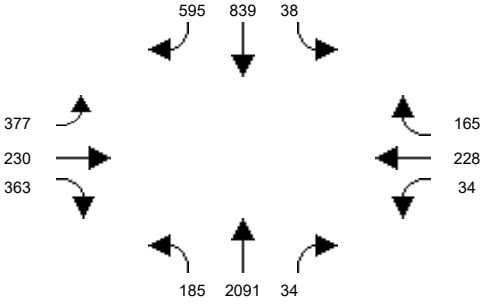
Intersection #76: Grand Ave and 17th St



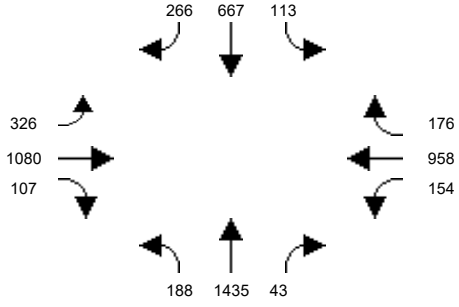
Intersection #77: Grand Ave and I-5 NB Ramps



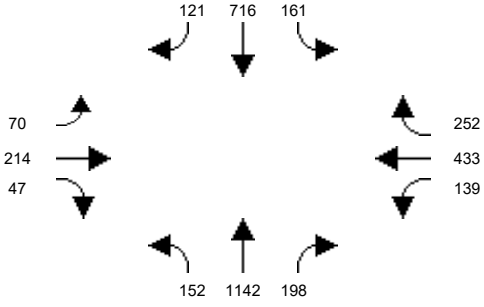
Intersection #78: Grand Ave and Santa Ana Blvd



Intersection #79: Grand Ave and 1st St



Intersection #80: Grand Ave and Chestnut Ave

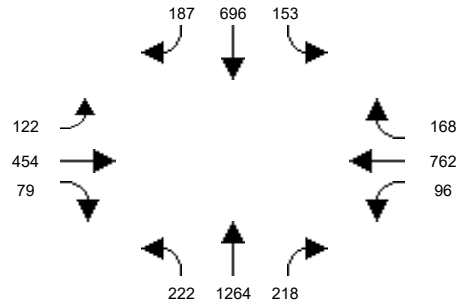


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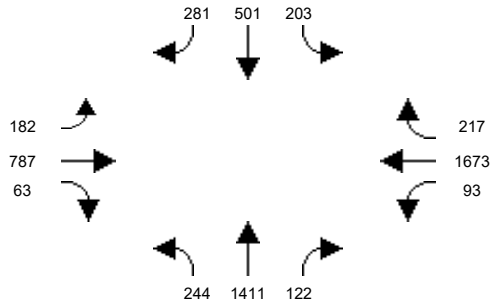
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP PM Peak Hour

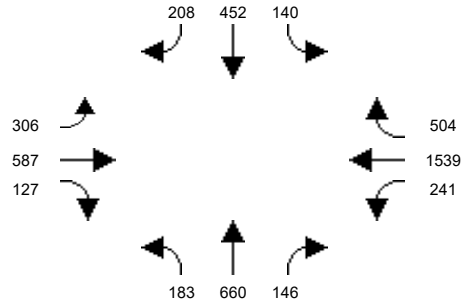
Intersection #81: Grand Ave and McFadden Ave



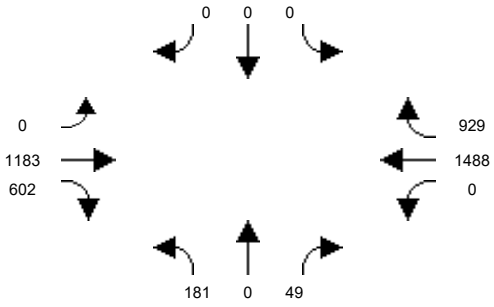
Intersection #82: Grand Ave and Edinger Ave



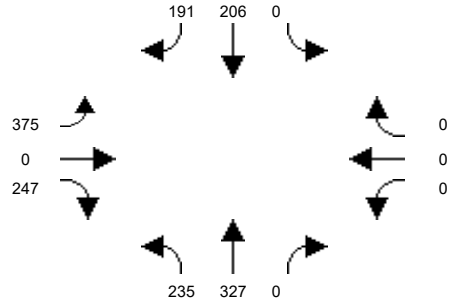
Intersection #83: Grand Ave and Warner Ave



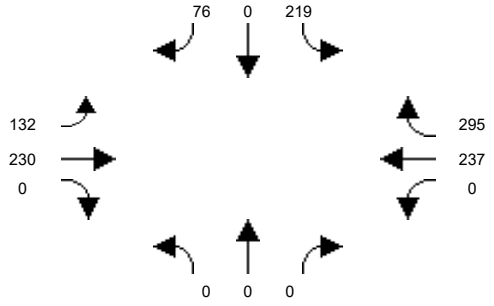
Intersection #84: SR-55 NB Ramps and Dyer Rd



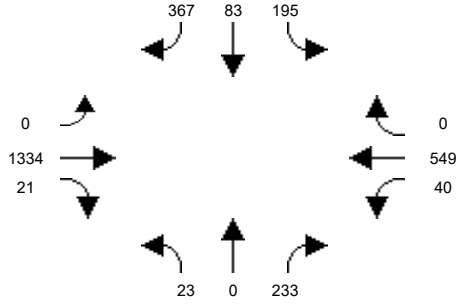
Intersection #85: Cambridge St and La Veta Ave



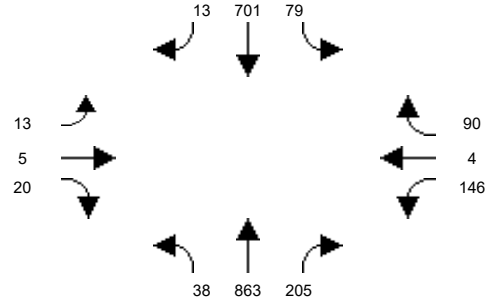
Intersection #86: Cambridge St and Fairhaven Ave



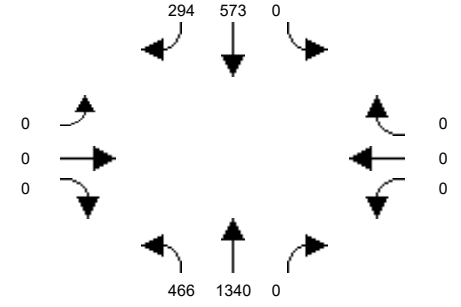
Intersection #87: Mabury St and 1st Street



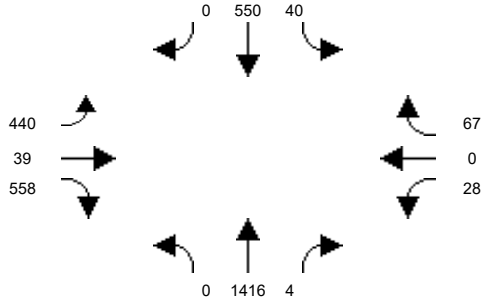
Intersection #88: Tustin St and La Veta Ave



Intersection #89: Tustin St and SR-22 WB On-Ramp



Intersection #90: Tustin St and SR-22 EB Off-Ramp / Seba Ave

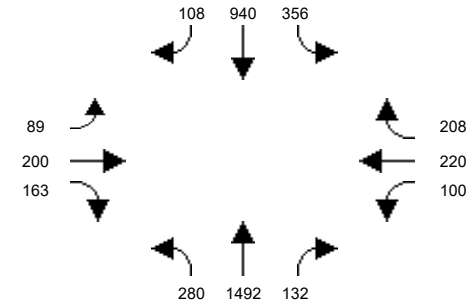


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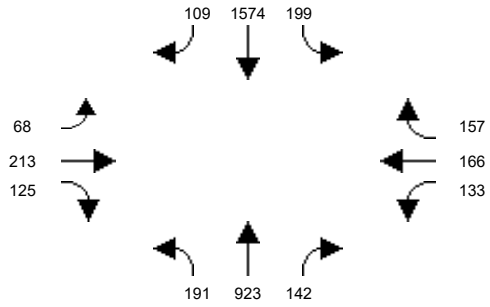
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP PM Peak Hour

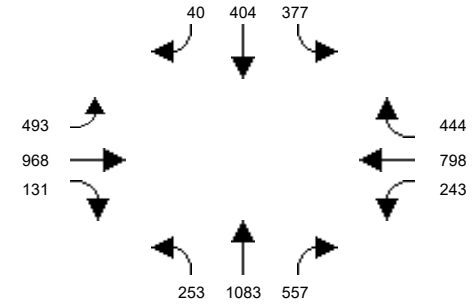
Intersection #91: Tustin Ave and Fairhaven Ave



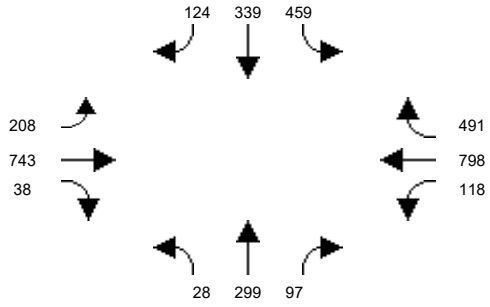
Intersection #92: Tustin Ave and Santa Clara Ave



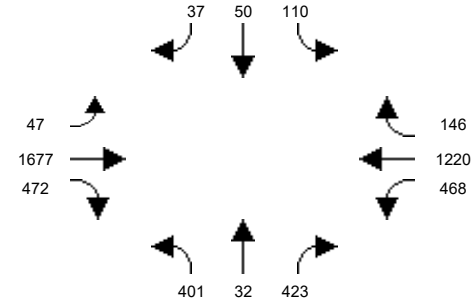
Intersection #93: Tustin Ave and 17th St



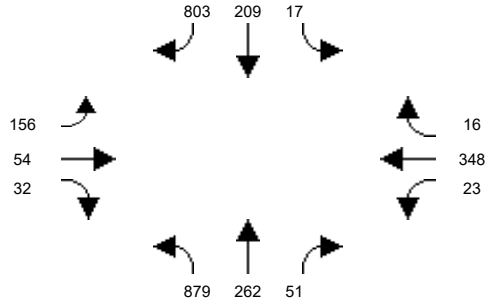
Intersection #94: Tustin Ave and 4th St



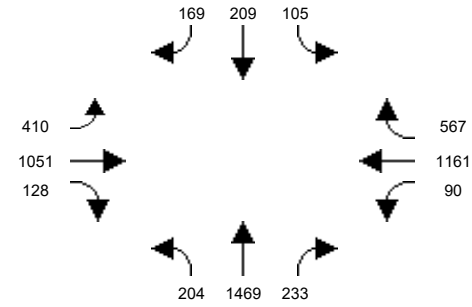
Intersection #95: SR-55 SB Ramps / Auto Mall Dr and Edinger Ave



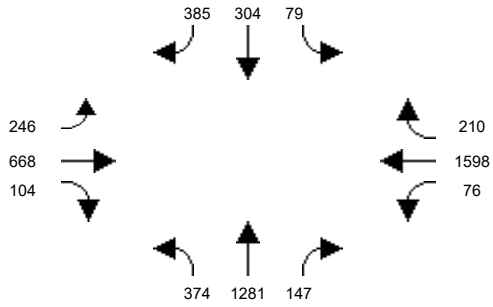
Intersection #96: SR-55 NB Ramps / Del Amo Ave and Newport Avenue



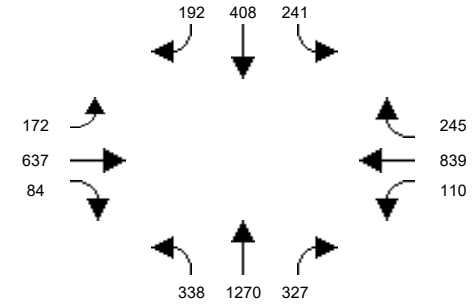
Intersection #97: Red Hill Ave and Edinger Ave



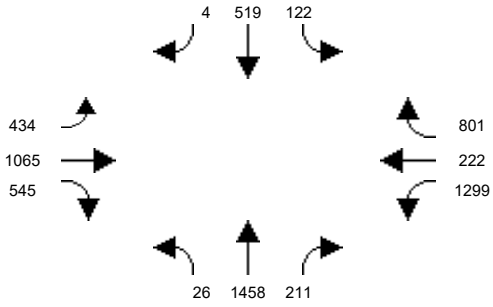
Intersection #98: Red Hill Ave and Warner Ave



Intersection #99: Red Hill Ave and Dyer Rd / barranca Pkwy



Intersection #100: Red Hill Ave and Alton Pkwy

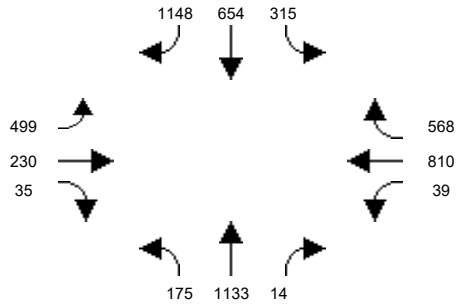


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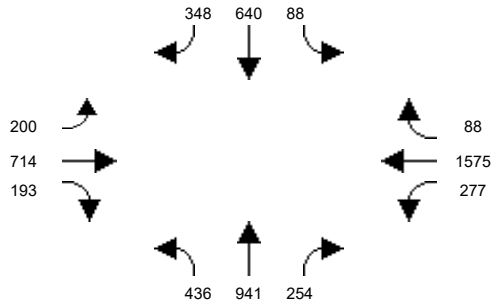
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP PM Peak Hour

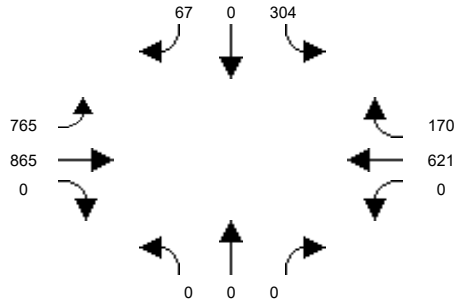
Intersection #101: Red Hill Ave and MacArthur Blvd



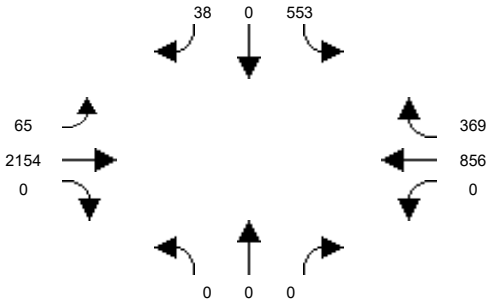
Intersection #102: Red Hill Ave and Main St



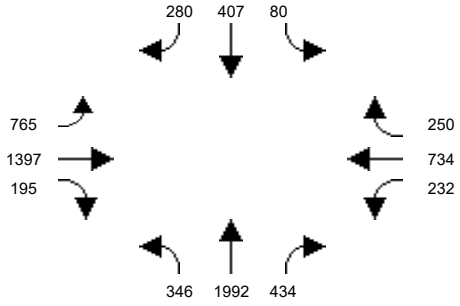
Intersection #103: I-5 SB Ramps and Santa Ana Blvd



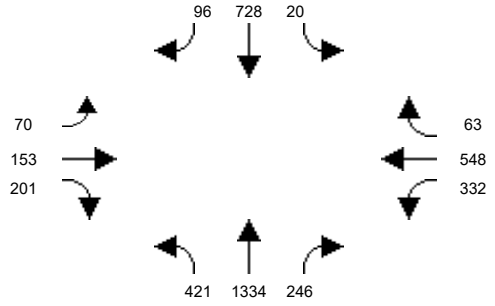
Intersection #104: Tustin Ranch Rd and Warner Ave



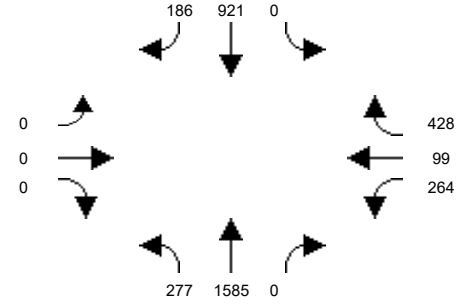
Intersection #105: Von Karman Ave and Barranca Pkwy



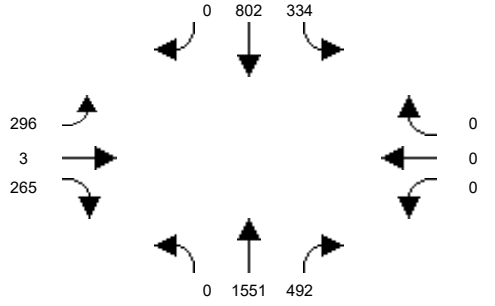
Intersection #106: Red Hill Avenue and El Camino Real



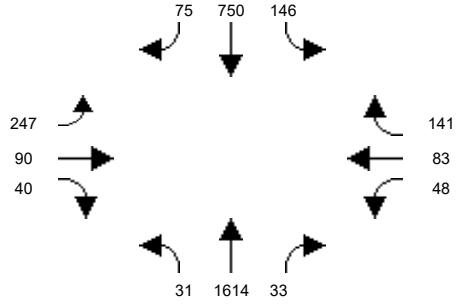
Intersection #107: Red Hill Avenue and I-5 NB Ramps



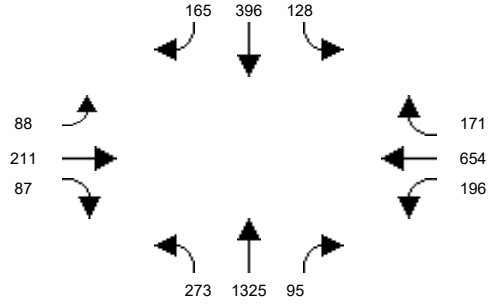
Intersection #108: Red Hill Avenue and I-5 SB Ramps



Intersection #109: Red Hill Avenue and Nisson Road



Intersection #110: Red Hill Avenue and Walnut Avenue

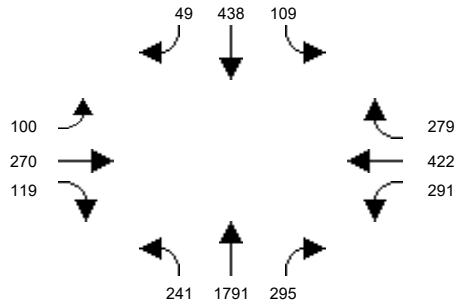


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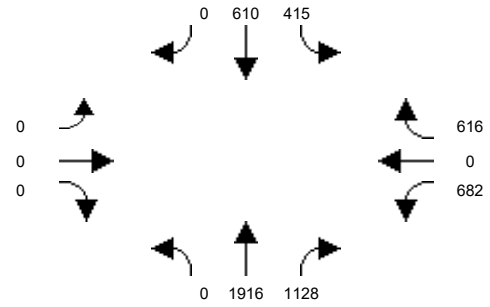
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 NP PM Peak Hour

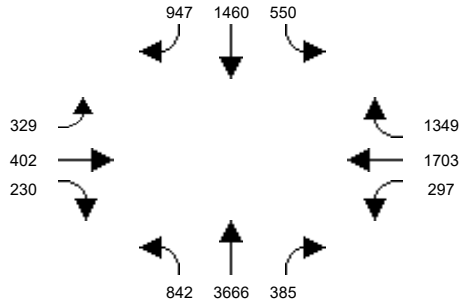
Intersection #111: Red Hill Avenue and Valencia Avenue



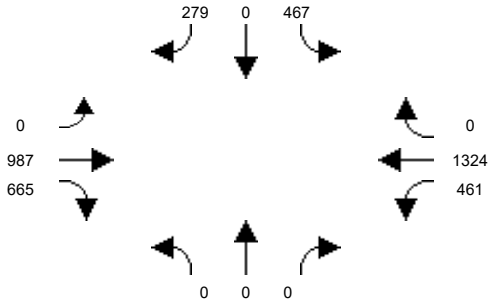
Intersection #112: Tustin Ranch Road and Warner Avenue North



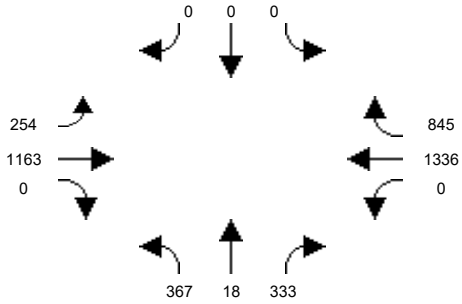
Intersection #113: Tustin Ranch Road and Walnut Avenue



Intersection #114: SR-55 SB Ramps and Irvine Boulevard



Intersection #115: SR-55 NB Ramps and Irvine Boulevard



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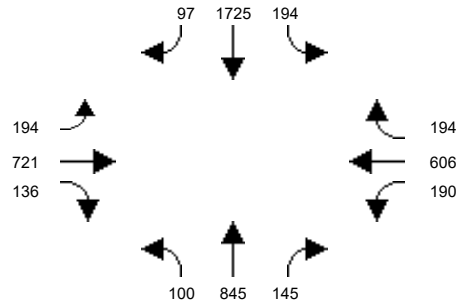
APPENDIX B.5

INTERSECTION VOLUMES –
2045 WP (AM PEAK HOUR)

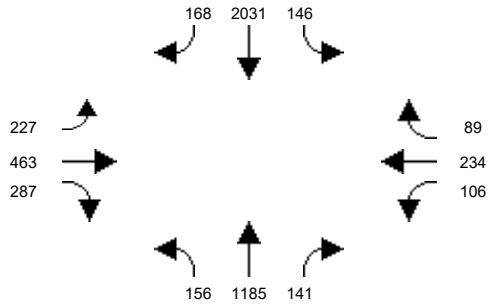
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP AM Peak Hour

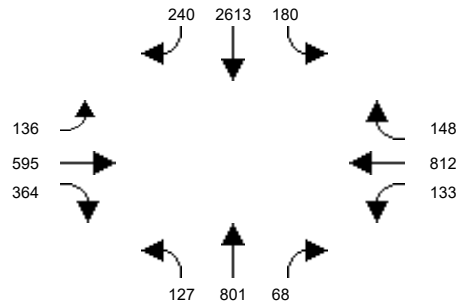
Intersection #1: Euclid St and 1st St



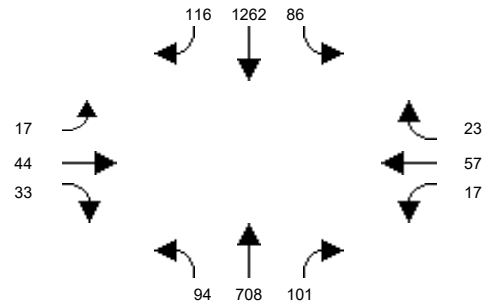
Intersection #2: Euclid St and McFadden Ave



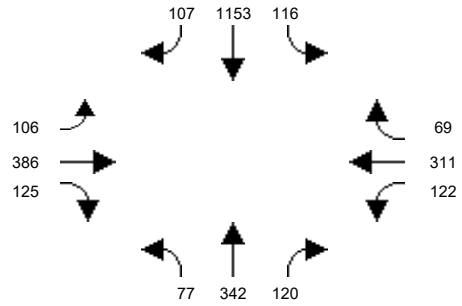
Intersection #3: Euclid St and Edinger Ave



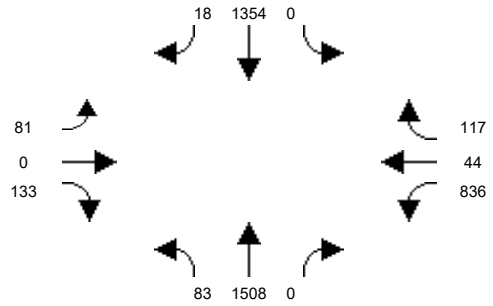
Intersection #4: Newhope St and Hazard Ave



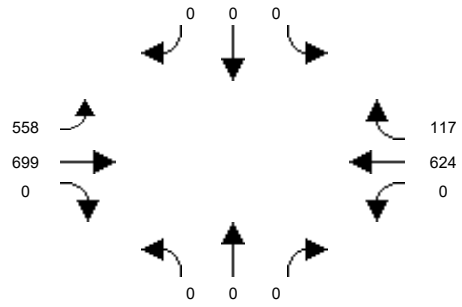
Intersection #5: Newhope St and McFadden Ave



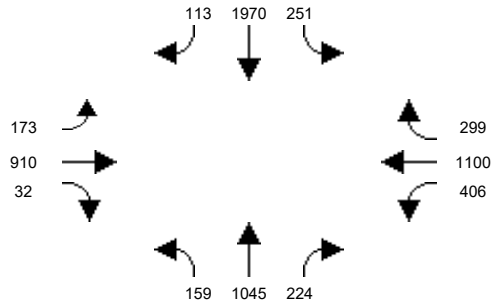
Intersection #6: Harbor Blvd and SR-22 WB Off-Ramp / Banner Dr



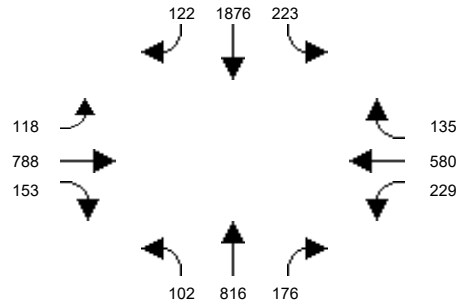
Intersection #7: Trask Avenue and SR-22 EB On-Ramp



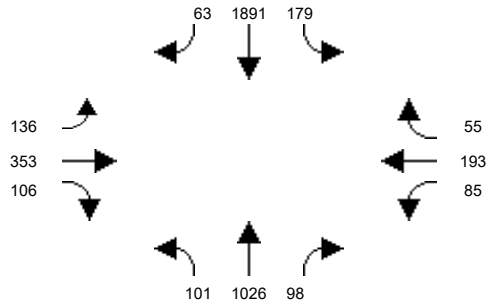
Intersection #8: Harbor Blvd and Westminster Ave



Intersection #9: Harbor Blvd and 1st St



Intersection #10: Harbor Blvd and McFadden Ave

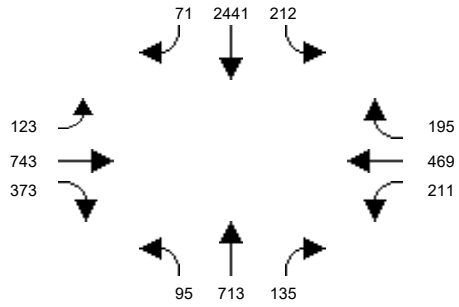


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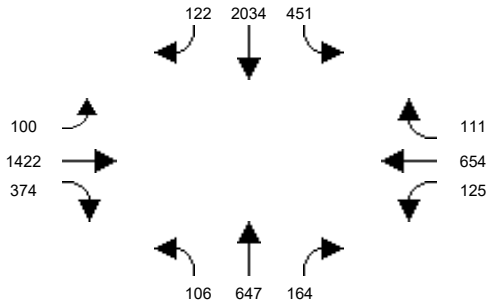
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP AM Peak Hour

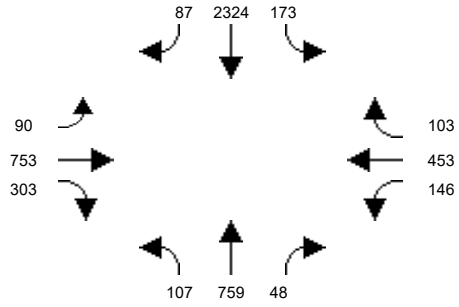
Intersection #11: Harbor Blvd and Edinger Ave



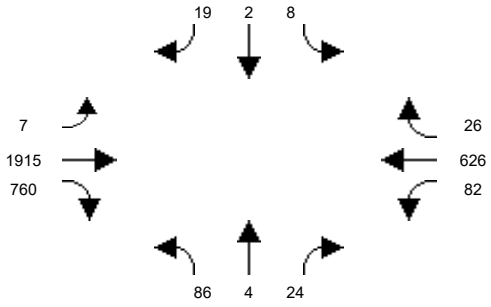
Intersection #12: Harbor Blvd and Warner Ave



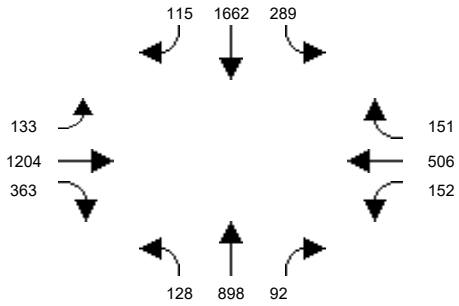
Intersection #13: Harbor Blvd and Segerstrom Ave



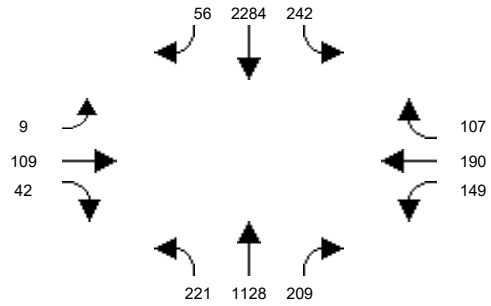
Intersection #14: MacArthur Blvd and Hyland Ave



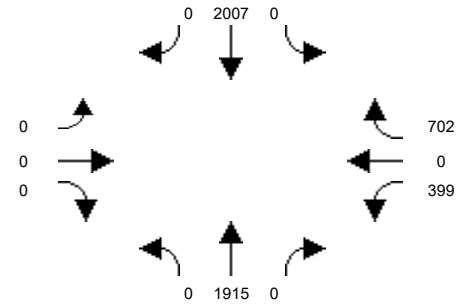
Intersection #15: MacArthur Blvd and Harbor Blvd



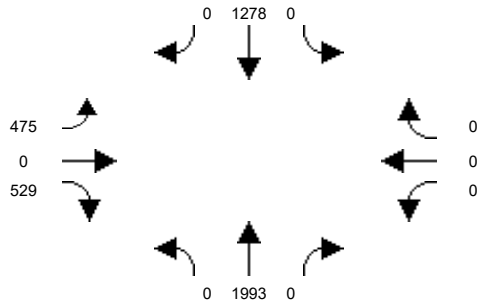
Intersection #16: Harbor Blvd and Sunflower Ave



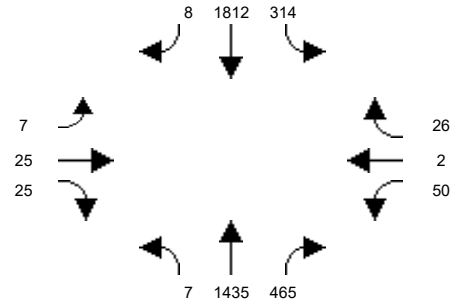
Intersection #17: Harbor Blvd and I-405 NB Off-Ramp



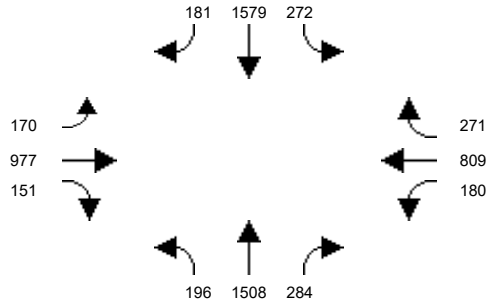
Intersection #18: Harbor Blvd and I-405 SB Off-Ramp



Intersection #19: Fairview St and Civic Center Dr



Intersection #20: Fairview St and 1st St

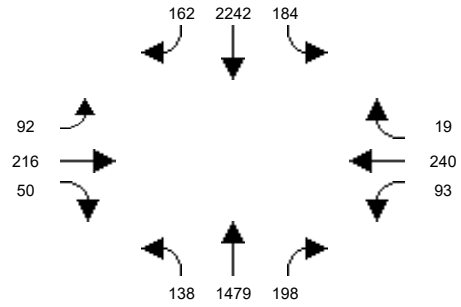


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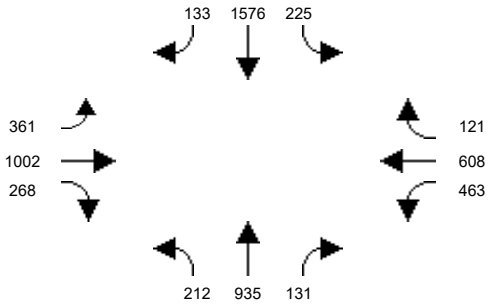
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP AM Peak Hour

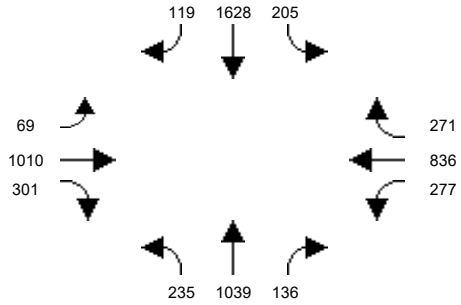
Intersection #21: Fairview St and McFadden Ave



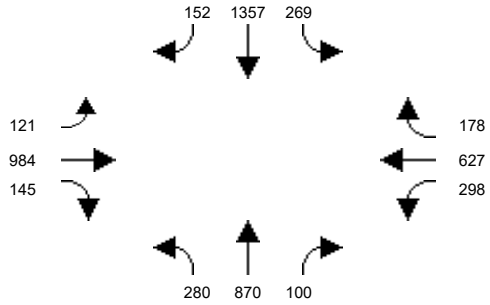
Intersection #22: Fairview St and Edinger Ave



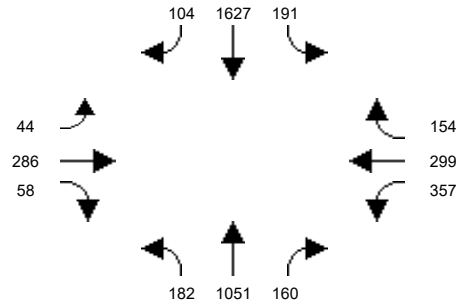
Intersection #23: Fairview St and Warner Ave



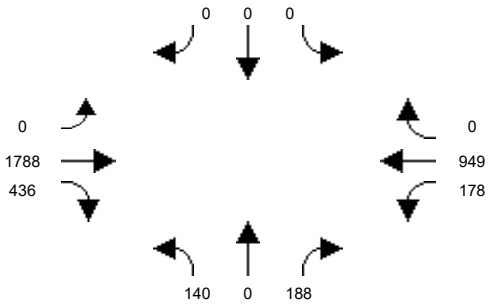
Intersection #24: Fairview St and MacArthur Blvd



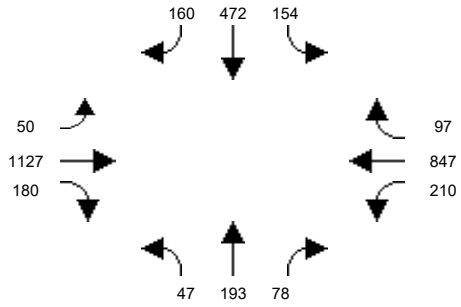
Intersection #25: Fairview Rd and Sunflower Ave



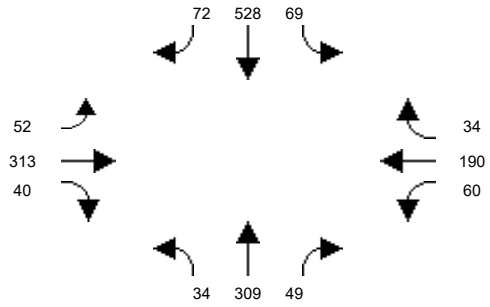
Intersection #26: Greenville St and Edinger Ave



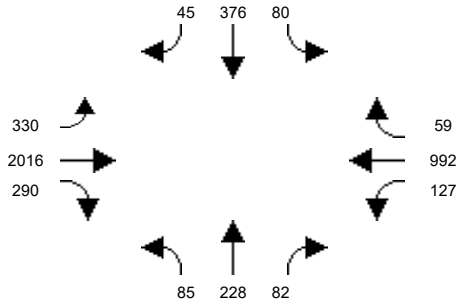
Intersection #27: Greenville St and Segerstrom Ave



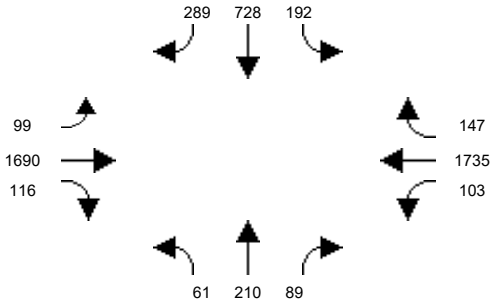
Intersection #28: Raitt St and McFadden Ave



Intersection #29: Raitt St and Edinger Ave



Intersection #30: Bear St and MacArthur Blvd

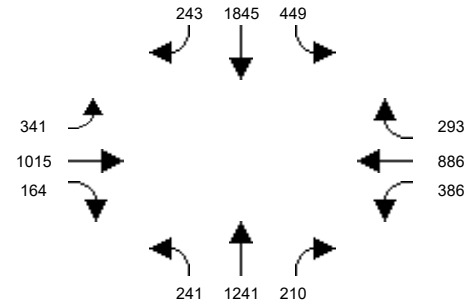


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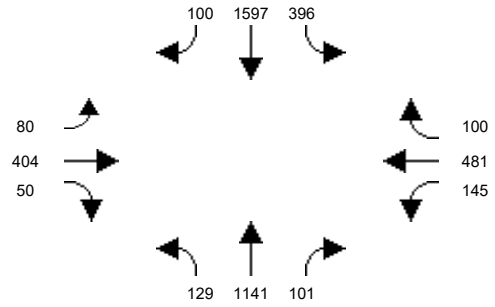
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP AM Peak Hour

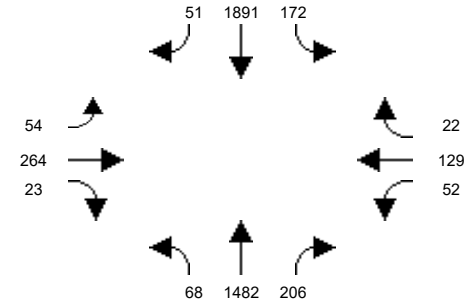
Intersection #31: Bristol St and 17th St



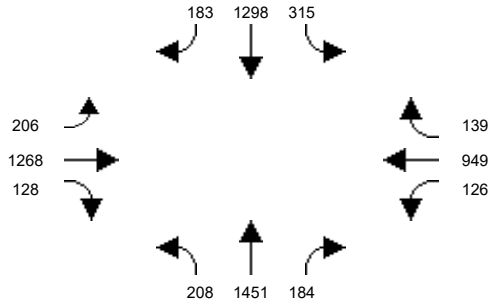
Intersection #32: Bristol St and Civic Center Dr



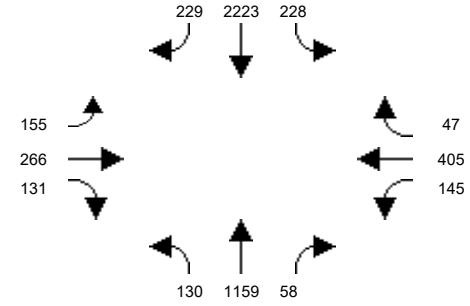
Intersection #33: Bristol St and Santa Ana Blvd



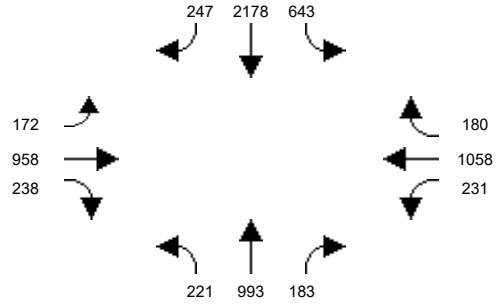
Intersection #34: Bristol St and 1st St



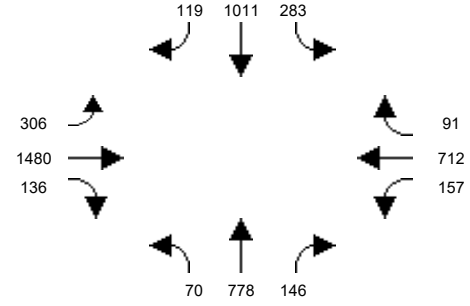
Intersection #35: Bristol St and McFadden Ave



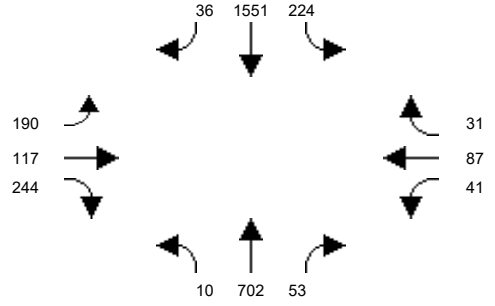
Intersection #36: Bristol St and Warner Ave



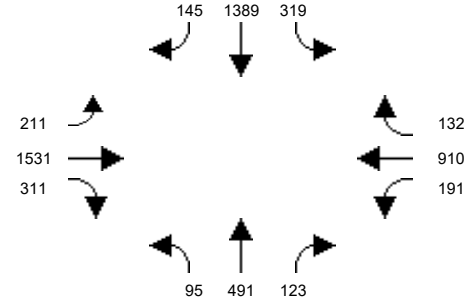
Intersection #37: Bristol St and Segerstrom Ave



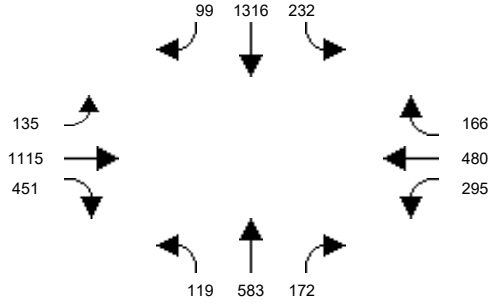
Intersection #38: Bristol St and Alton Ave



Intersection #39: Bristol St and MacArthur Blvd



Intersection #40: Bristol St and Sunflower Ave

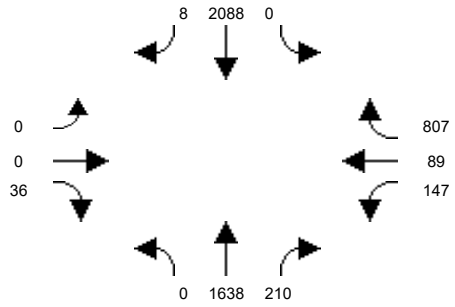


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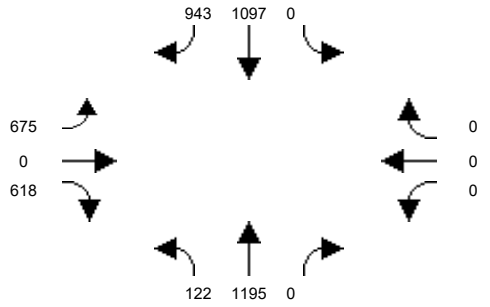
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP AM Peak Hour

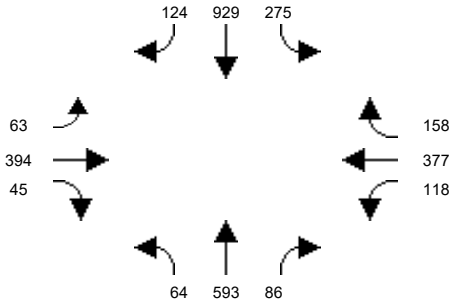
Intersection #41: Bristol St and I-405 NB Ramps



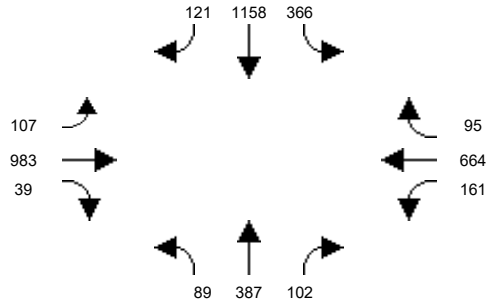
Intersection #42: Bristol St and I-405 SB Ramps



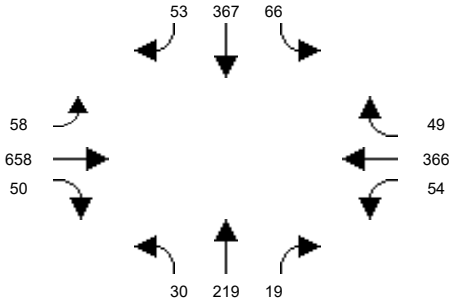
Intersection #43: Flower St and Santa Ana Blvd



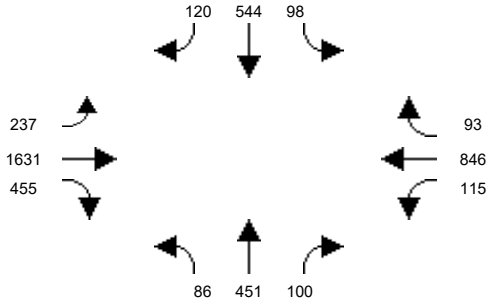
Intersection #44: Flower St and 1st St



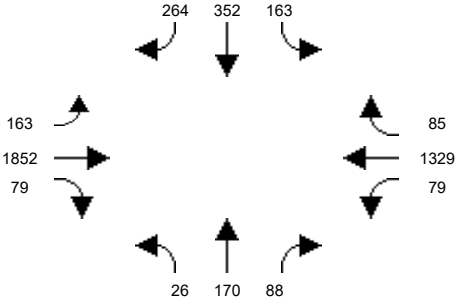
Intersection #45: Flower St and McFadden Ave



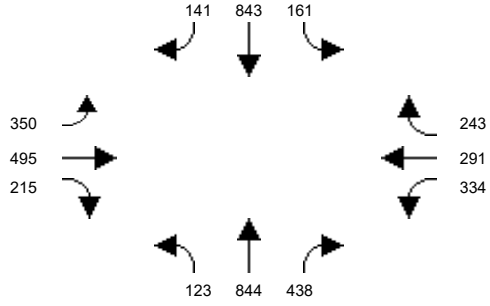
Intersection #46: Flower St and Segerstrom Ave



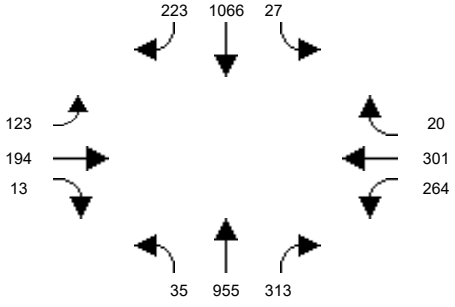
Intersection #47: Flower St and MacArthur Blvd



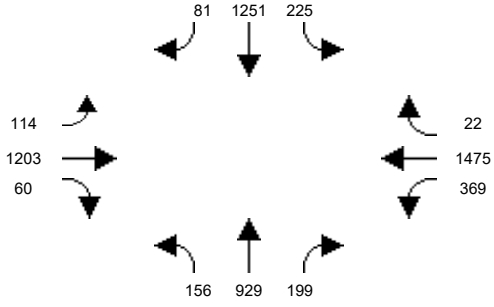
Intersection #48: Main St and La Veta Ave



Intersection #49: Main St and Mainplace Dr / Memory Ln



Intersection #50: Main St and 17th St

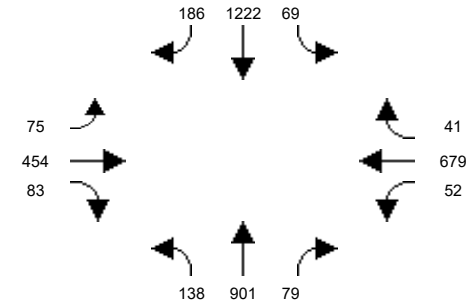


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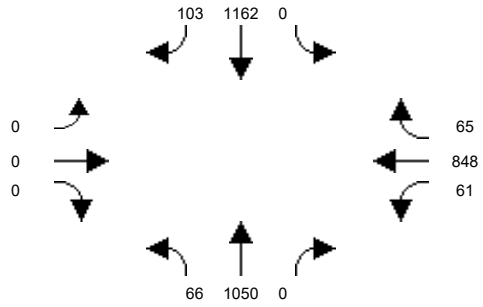
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP AM Peak Hour

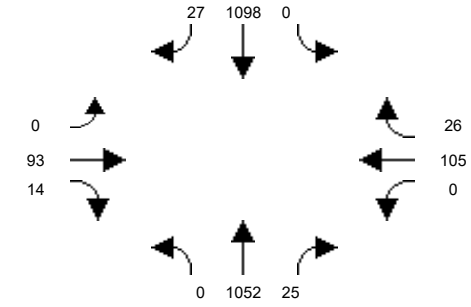
Intersection #51: Main St and Civic Center Dr



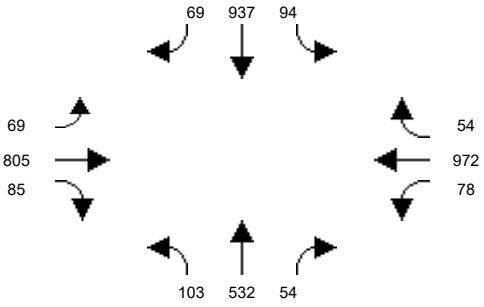
Intersection #52: Main St and Santa Ana Blvd



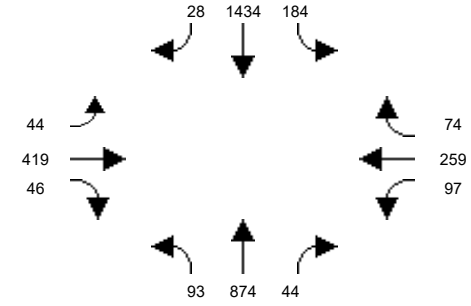
Intersection #53: Main St and 4th St



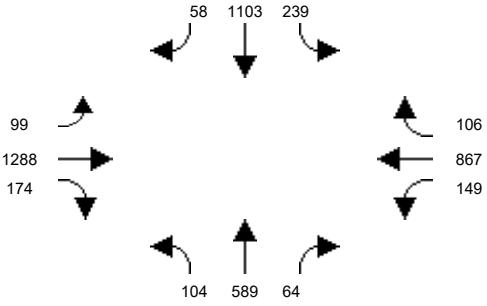
Intersection #54: Main St and 1st St



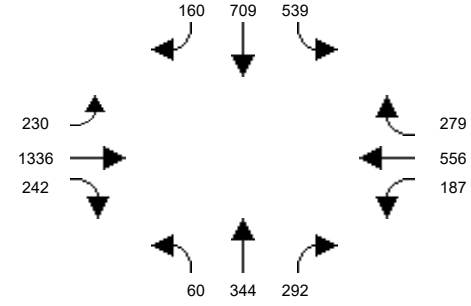
Intersection #55: Main St and McFadden Ave



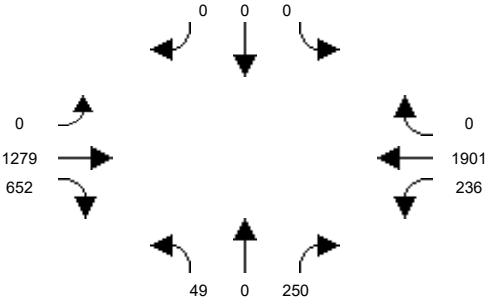
Intersection #56: Main St and Edinger Ave



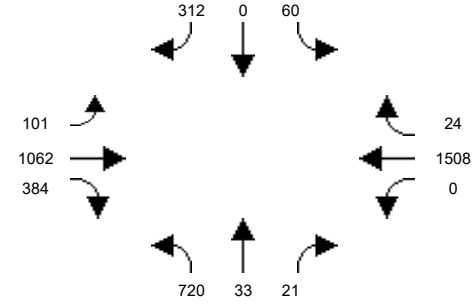
Intersection #57: Main St and MacArthur Blvd



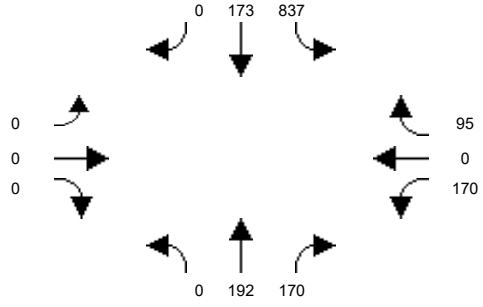
Intersection #58: Penn Wy and 17th St



Intersection #59: I-5 NB Off Ramps/17th Street



Intersection #60: Penn Wy and I-5 SB Ramps

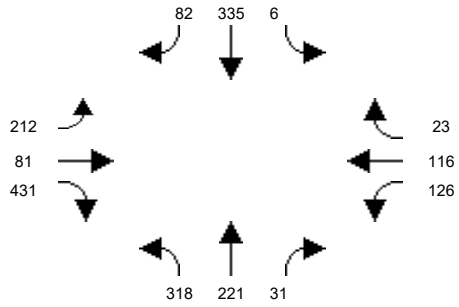


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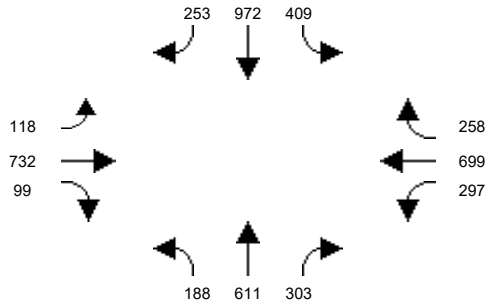
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP AM Peak Hour

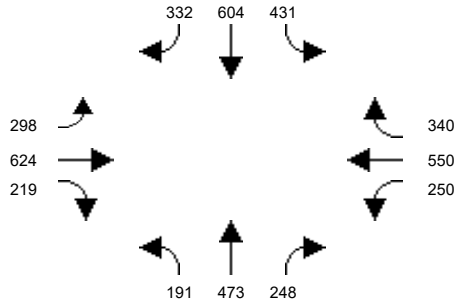
Intersection #61: Santiago St and Civic Center Dr



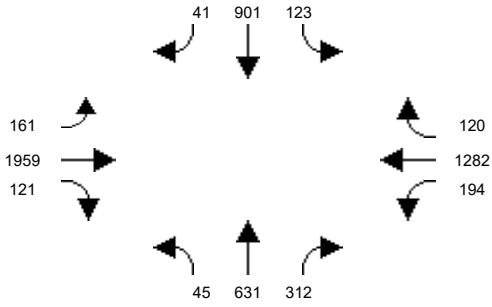
Intersection #62: Santiago St and Santa Ana Blvd



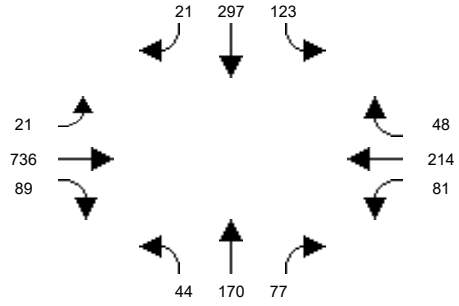
Intersection #63: Standard Ave and 4th St



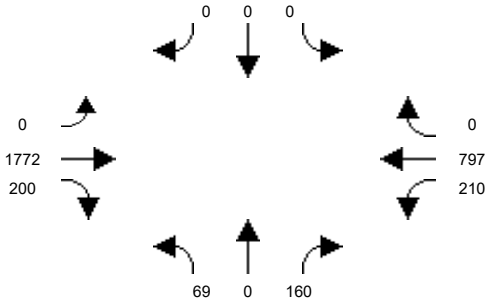
Intersection #64: Standard Ave and 1st St



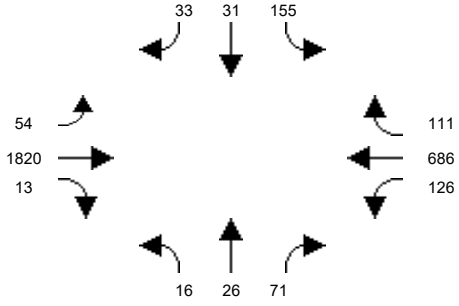
Intersection #65: Standard Ave and Mcfadden Ave



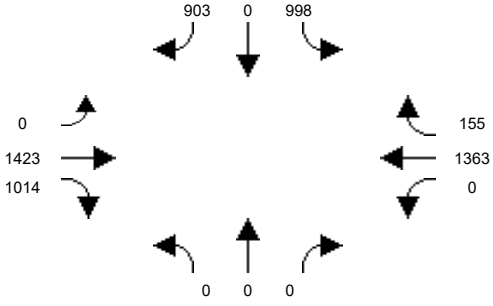
Intersection #66: Halladay St and Warner Ave



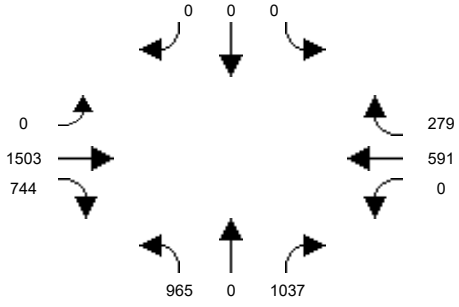
Intersection #67: Halladay St and Dyer Rd



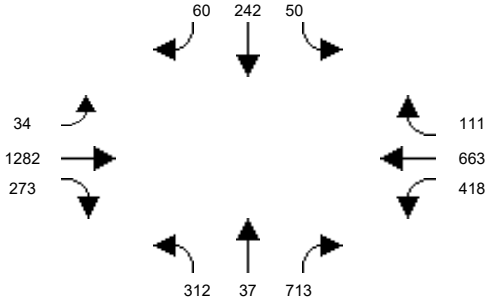
Intersection #68: SR-55 SB Ramps and MacArthur Blvd



Intersection #69: SR-55 NB Ramps and MacArthur Blvd



Intersection #70: SR-55 SB Ramps and Dyer Rd

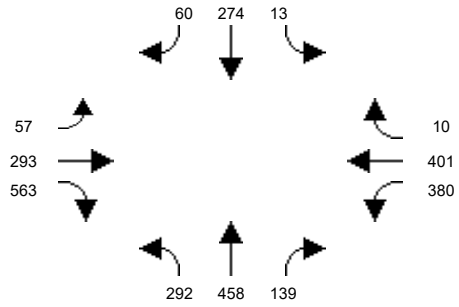


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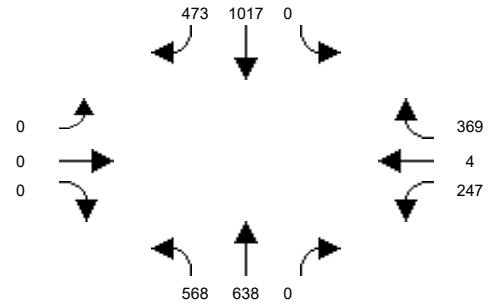
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP AM Peak Hour

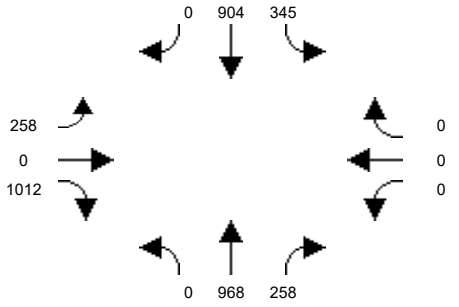
Intersection #71: Glassell St and La Veta Ave



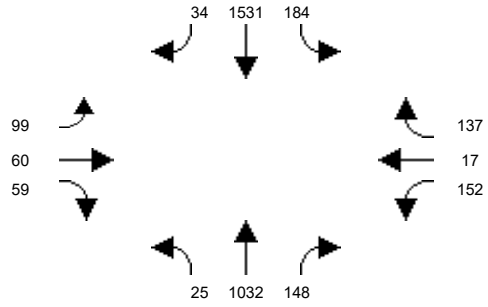
Intersection #72: Glassell St and SR-22 WB Ramps



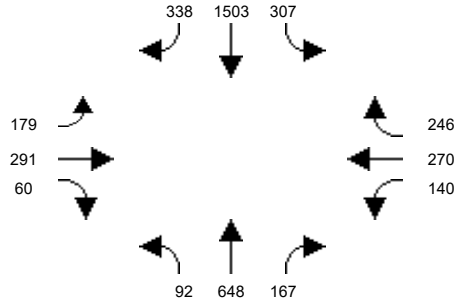
Intersection #73: Grand Ave / Glassell St and SR-22 EB Ramps



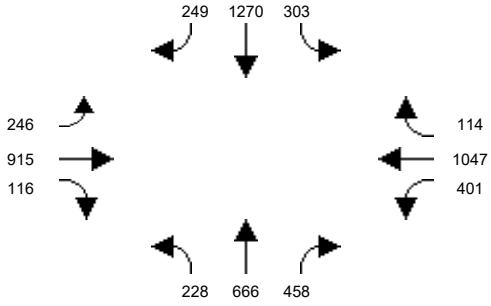
Intersection #74: Grand Ave and Fairhaven Ave



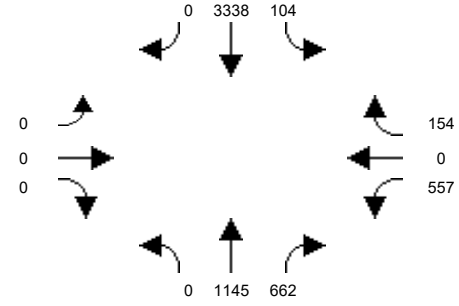
Intersection #75: Grand Ave and Santa Clara Ave



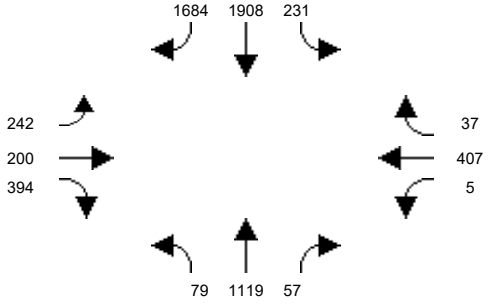
Intersection #76: Grand Ave and 17th St



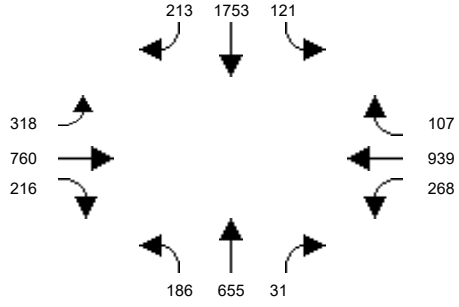
Intersection #77: Grand Ave and I-5 NB Ramps



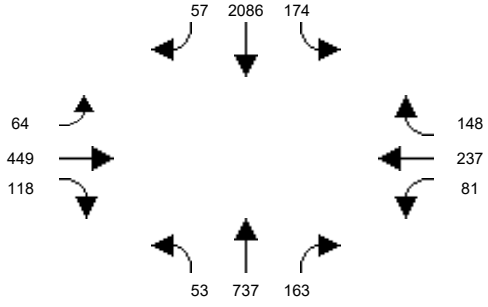
Intersection #78: Grand Ave and Santa Ana Blvd



Intersection #79: Grand Ave and 1st St



Intersection #80: Grand Ave and Chestnut Ave

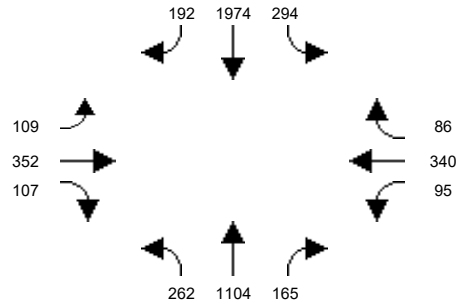


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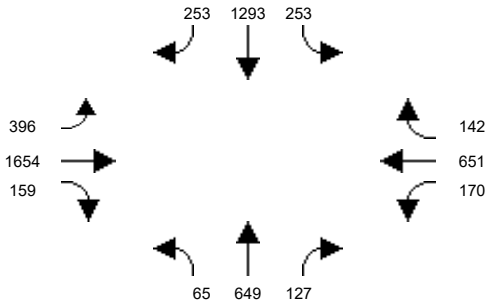
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP AM Peak Hour

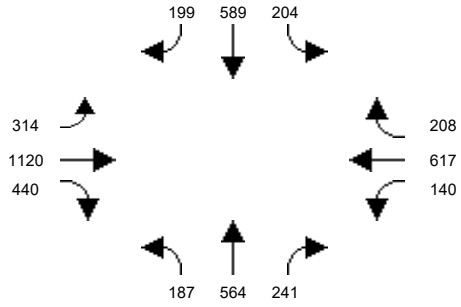
Intersection #81: Grand Ave and McFadden Ave



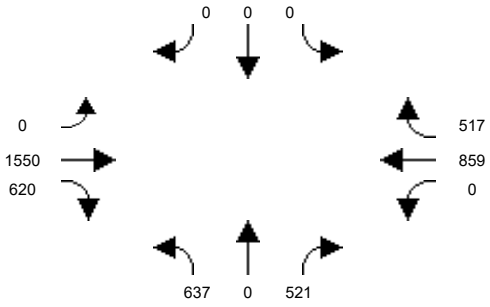
Intersection #82: Grand Ave and Edinger Ave



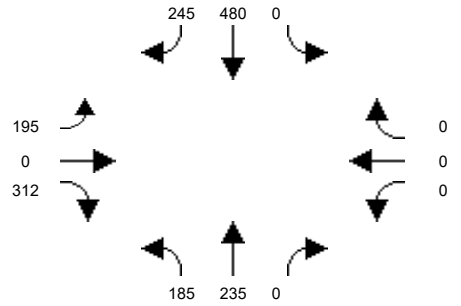
Intersection #83: Grand Ave and Warner Ave



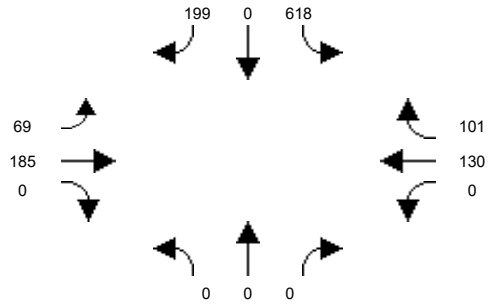
Intersection #84: SR-55 NB Ramps and Dyer Rd



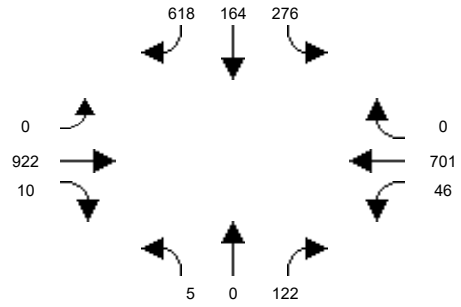
Intersection #85: Cambridge St and La Veta Ave



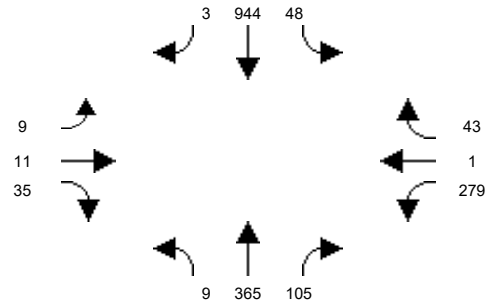
Intersection #86: Cambridge St and Fairhaven Ave



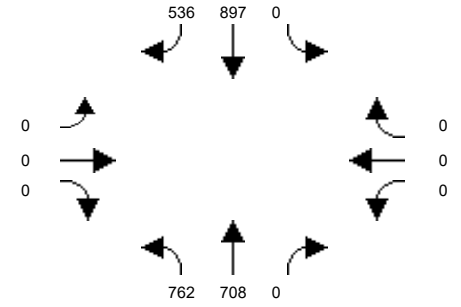
Intersection #87: Mabury St and 1st Street



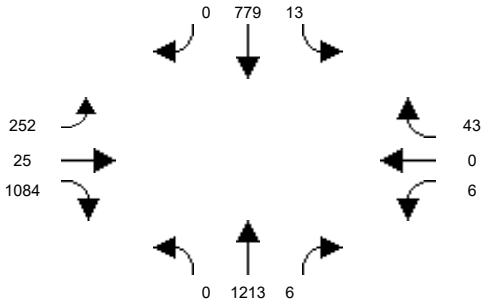
Intersection #88: Tustin St and La Veta Ave



Intersection #89: Tustin St and SR-22 WB On-Ramp



Intersection #90: Tustin St and SR-22 EB Off-Ramp / Seba Ave

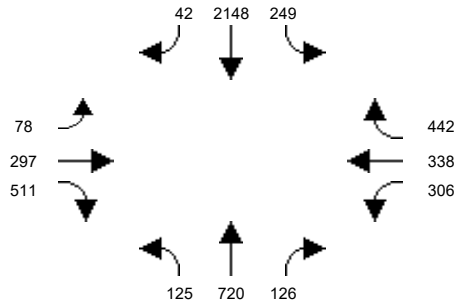


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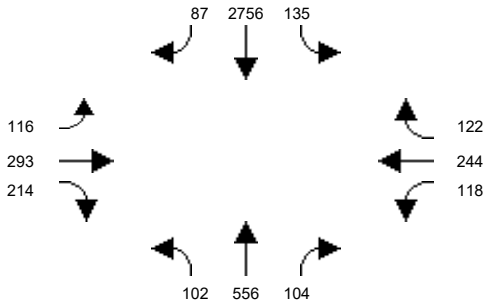
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP AM Peak Hour

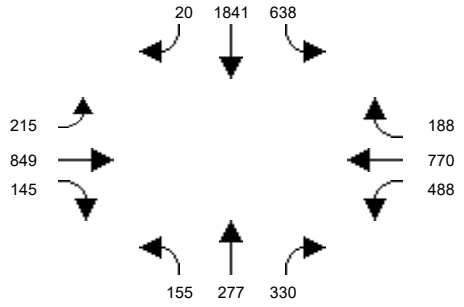
Intersection #91: Tustin Ave and Fairhaven Ave



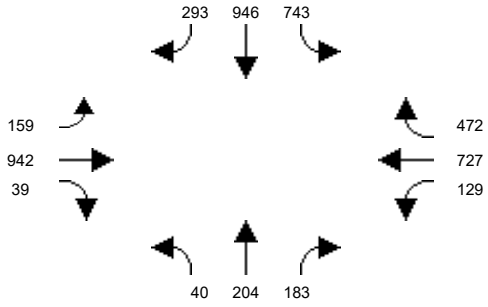
Intersection #92: Tustin Ave and Santa Clara Ave



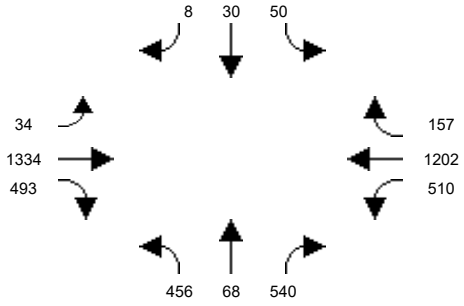
Intersection #93: Tustin Ave and 17th St



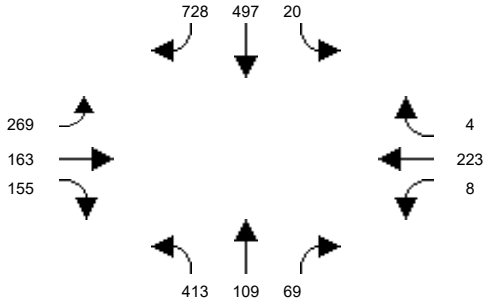
Intersection #94: Tustin Ave and 4th St



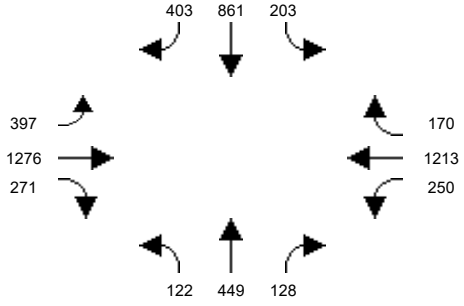
Intersection #95: SR-55 SB Ramps / Auto Mall Dr and Edinger Ave



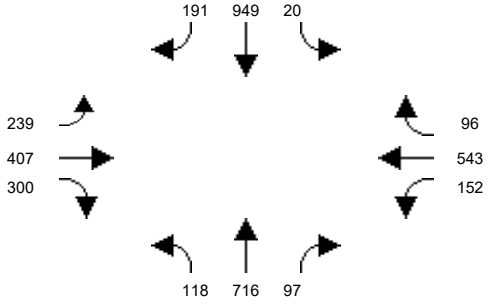
Intersection #96: SR-55 NB Ramps / Del Amo Ave and Newport Avenue



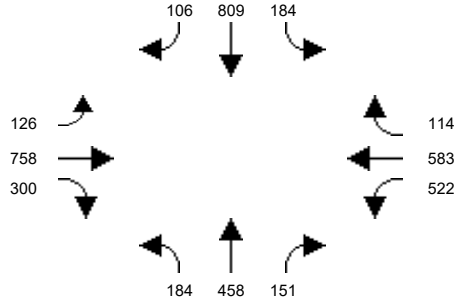
Intersection #97: Red Hill Ave and Edinger Ave



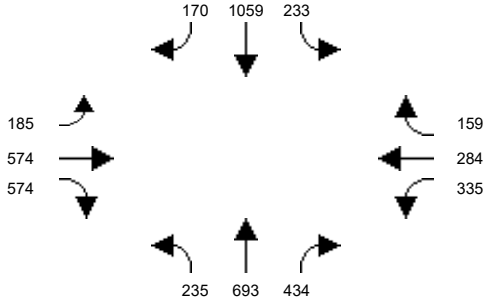
Intersection #98: Red Hill Ave and Warner Ave



Intersection #99: Red Hill Ave and Dyer Rd / barranca Pkwy



Intersection #100: Red Hill Ave and Alton Pkwy

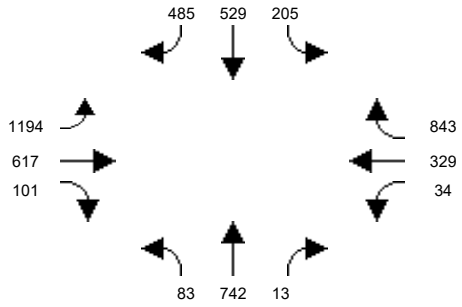


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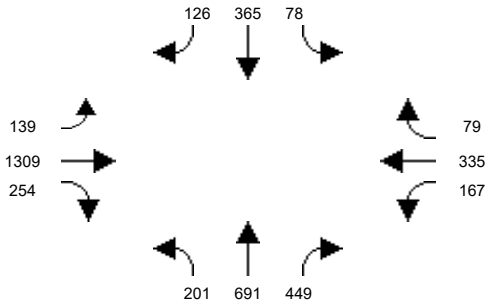
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP AM Peak Hour

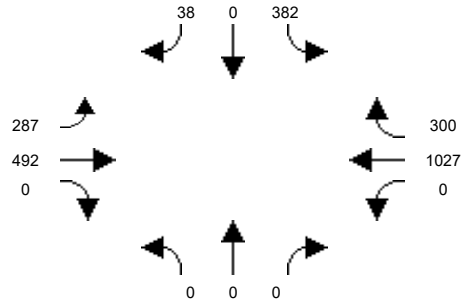
Intersection #101: Red Hill Ave and MacArthur Blvd



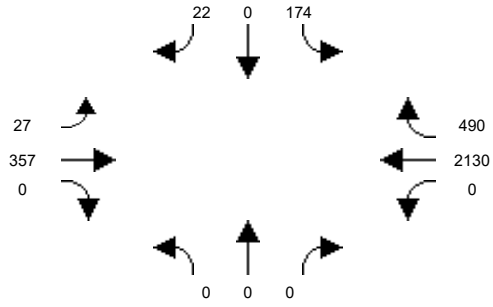
Intersection #102: Red Hill Ave and Main St



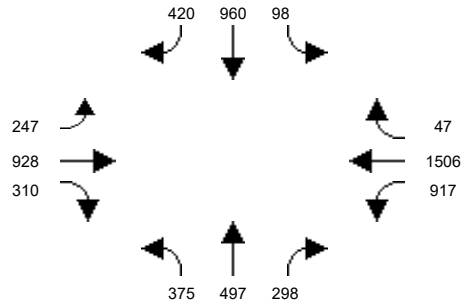
Intersection #103: I-5 SB Ramps and Santa Ana Blvd



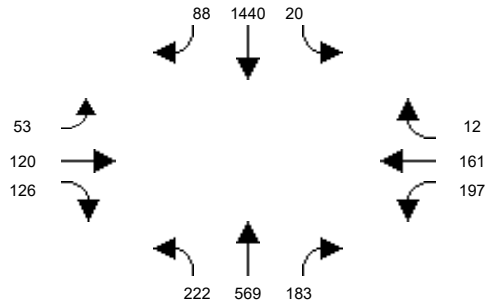
Intersection #104: Tustin Ranch Rd and Warner Ave



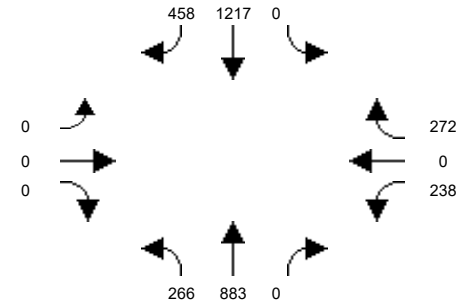
Intersection #105: Von Karman Ave and Barranca Pkwy



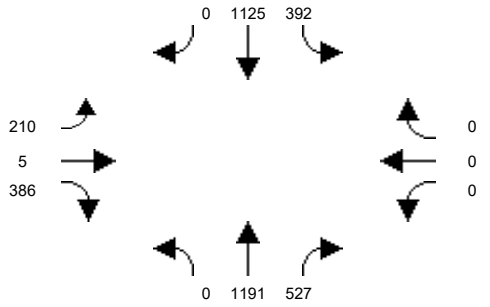
Intersection #106: Red Hill Avenue and El Camino Real



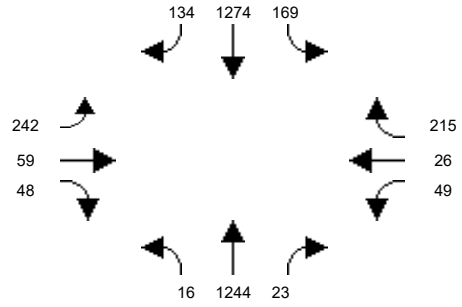
Intersection #107: Red Hill Avenue and I-5 NB Ramps



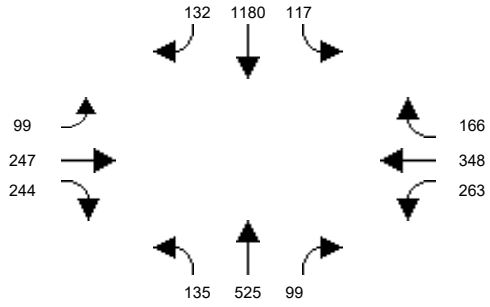
Intersection #108: Red Hill Avenue and I-5 SB Ramps



Intersection #109: Red Hill Avenue and Nisson Road



Intersection #110: Red Hill Avenue and Walnut Avenue

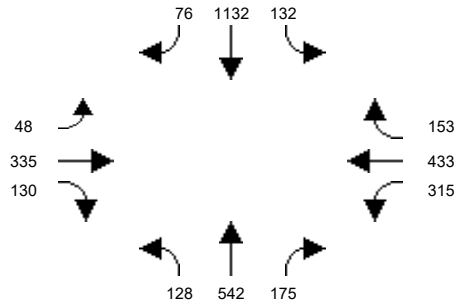


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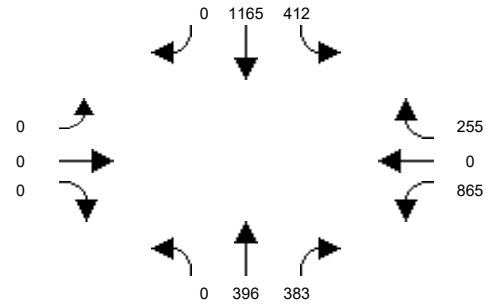
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP AM Peak Hour

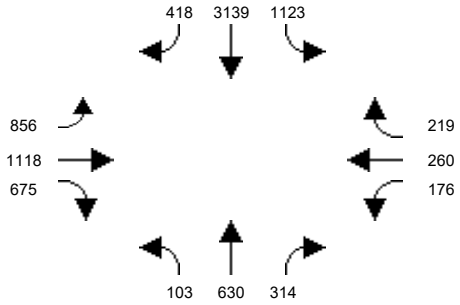
Intersection #111: Red Hill Avenue and Valencia Avenue



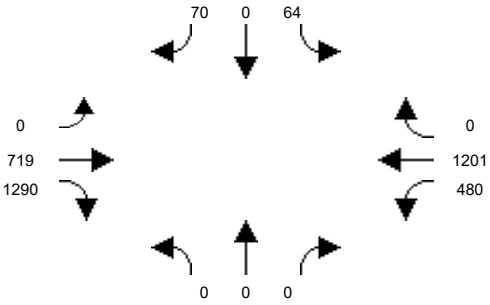
Intersection #112: Tustin Ranch Road and Warner Avenue North



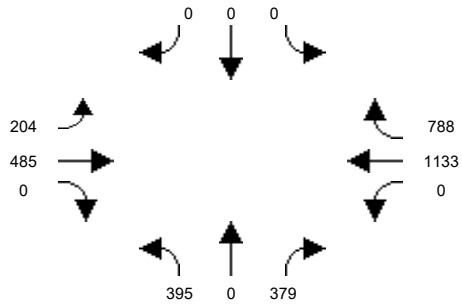
Intersection #113: Tustin Ranch Road and Walnut Avenue



Intersection #114: SR-55 SB Ramps and Irvine Boulevard



Intersection #115: SR-55 NB Ramps and Irvine Boulevard



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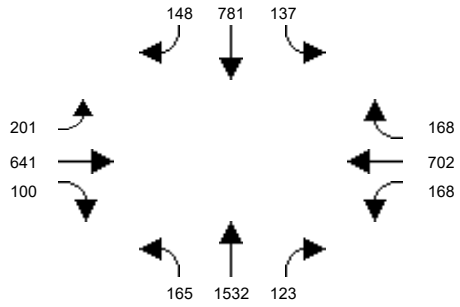
APPENDIX B.6

INTERSECTION VOLUMES –
2045 WP (PM PEAK HOUR)

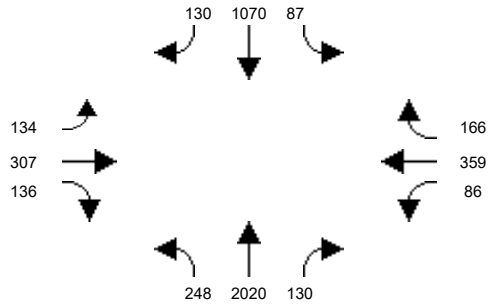
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP PM Peak Hour

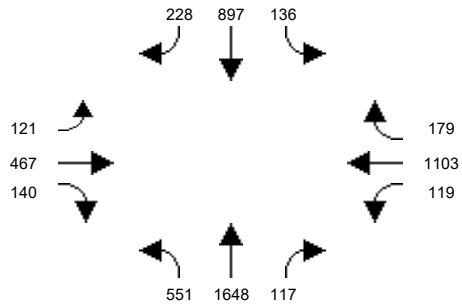
Intersection #1: Euclid St and 1st St



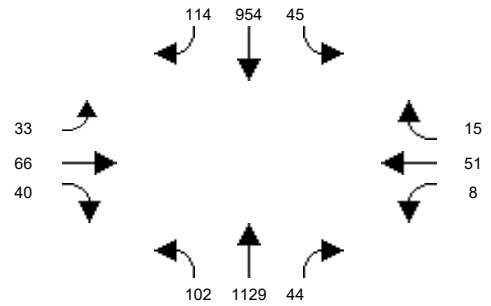
Intersection #2: Euclid St and McFadden Ave



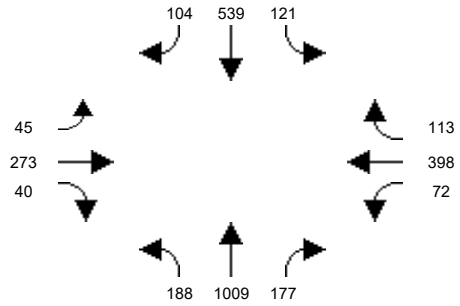
Intersection #3: Euclid St and Edinger Ave



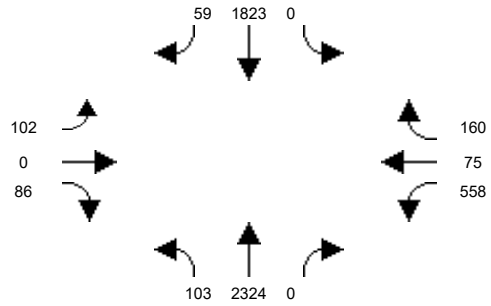
Intersection #4: Newhope St and Hazard Ave



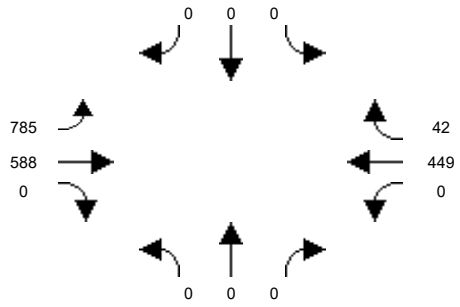
Intersection #5: Newhope St and McFadden Ave



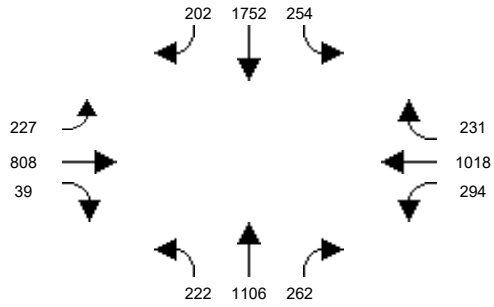
Intersection #6: Harbor Blvd and SR-22 WB Off-Ramp / Banner Dr



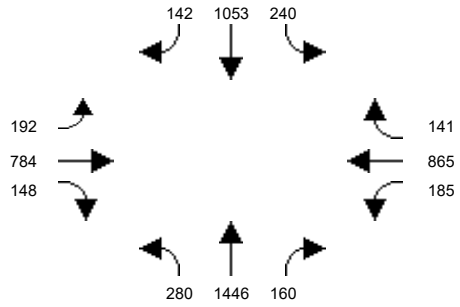
Intersection #7: Trask Avenue and SR-22 EB On-Ramp



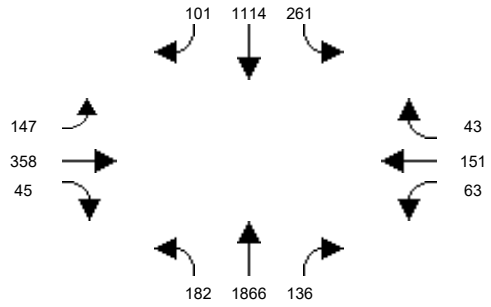
Intersection #8: Harbor Blvd and Westminster Ave



Intersection #9: Harbor Blvd and 1st St



Intersection #10: Harbor Blvd and McFadden Ave

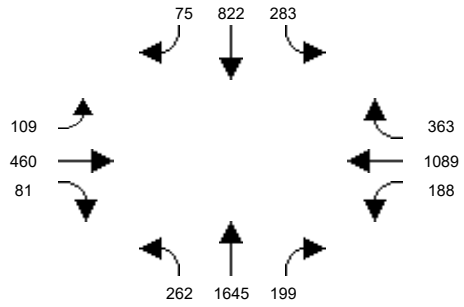


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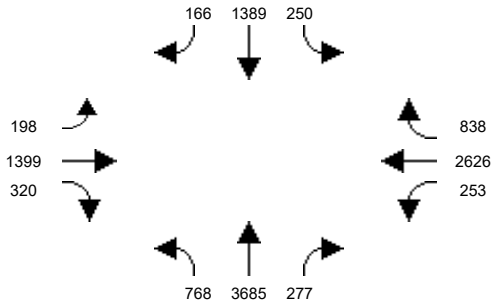
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP PM Peak Hour

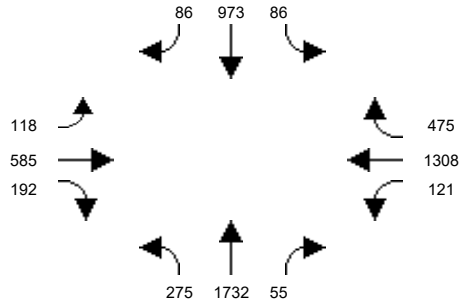
Intersection #11: Harbor Blvd and Edinger Ave



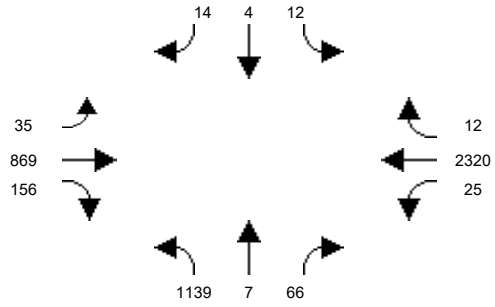
Intersection #12: Harbor Blvd and Warner Ave



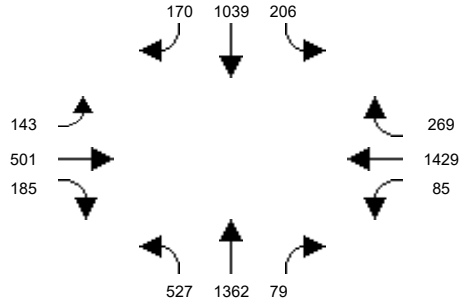
Intersection #13: Harbor Blvd and Segerstrom Ave



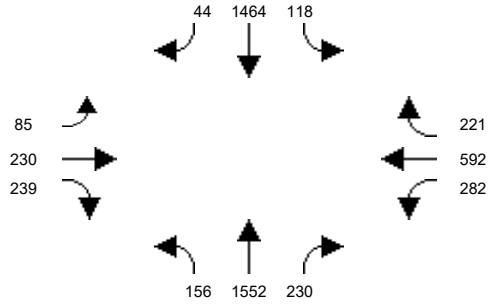
Intersection #14: MacArthur Blvd and Hyland Ave



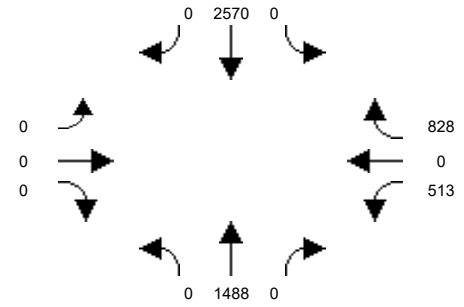
Intersection #15: MacArthur Blvd and Harbor Blvd



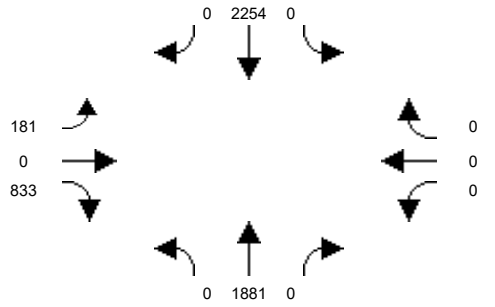
Intersection #16: Harbor Blvd and Sunflower Ave



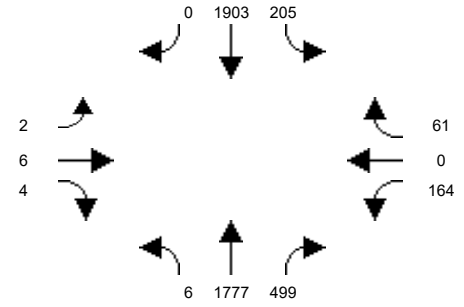
Intersection #17: Harbor Blvd and I-405 NB Off-Ramp



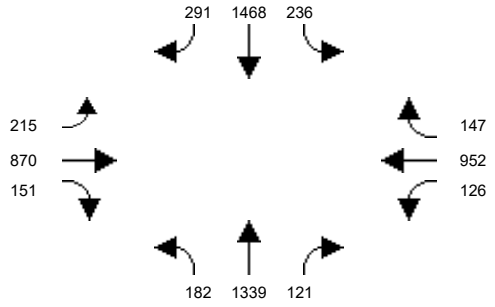
Intersection #18: Harbor Blvd and I-405 SB Off-Ramp



Intersection #19: Fairview St and Civic Center Dr



Intersection #20: Fairview St and 1st St

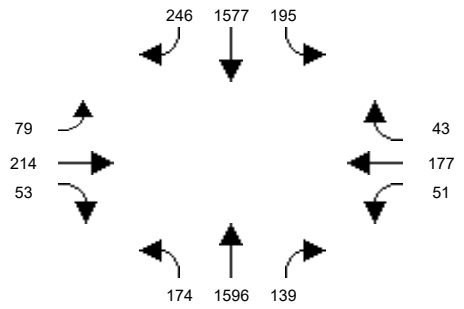


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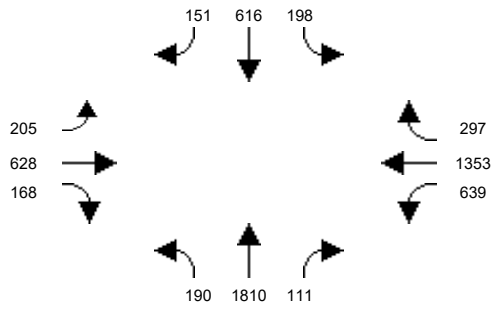
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP PM Peak Hour

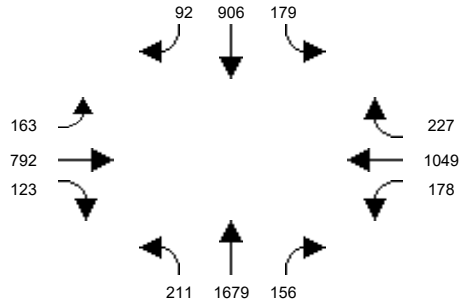
Intersection #21: Fairview St and McFadden Ave



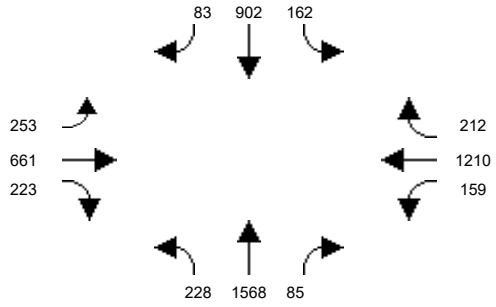
Intersection #22: Fairview St and Edinger Ave



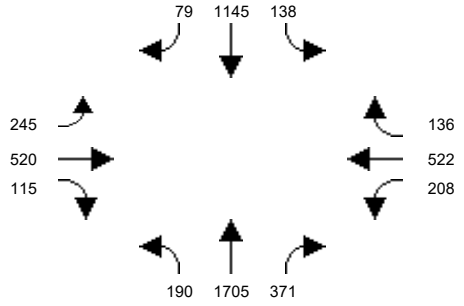
Intersection #23: Fairview St and Warner Ave



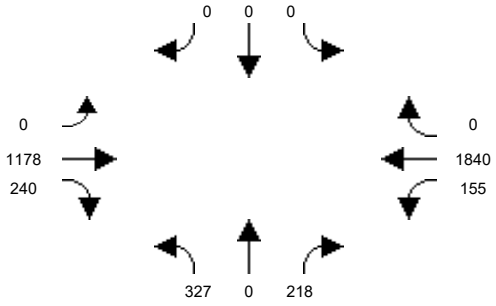
Intersection #24: Fairview St and MacArthur Blvd



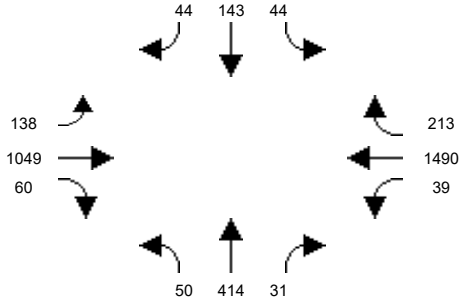
Intersection #25: Fairview Rd and Sunflower Ave



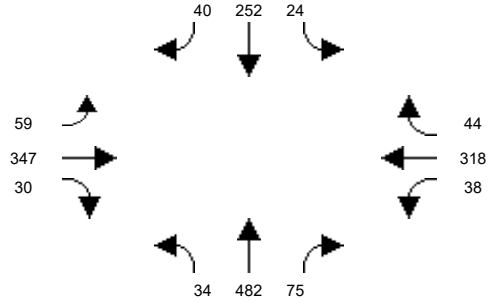
Intersection #26: Greenville St and Edinger Ave



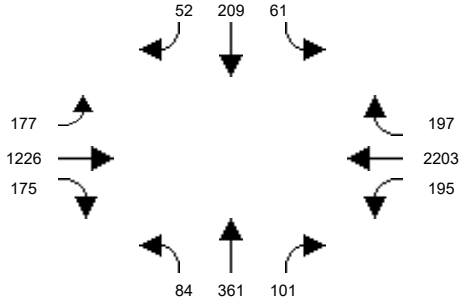
Intersection #27: Greenville St and Segerstrom Ave



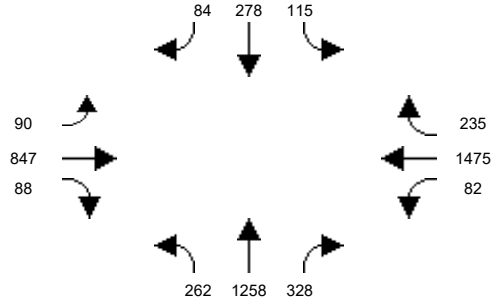
Intersection #28: Raitt St and McFadden Ave



Intersection #29: Raitt St and Edinger Ave



Intersection #30: Bear St and MacArthur Blvd

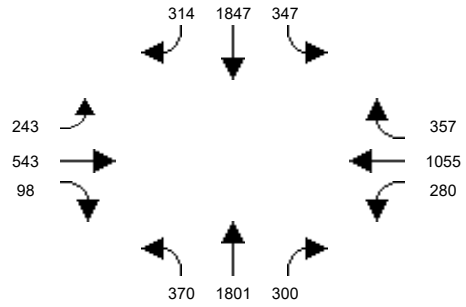


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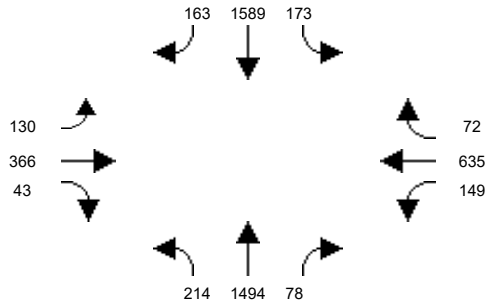
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP PM Peak Hour

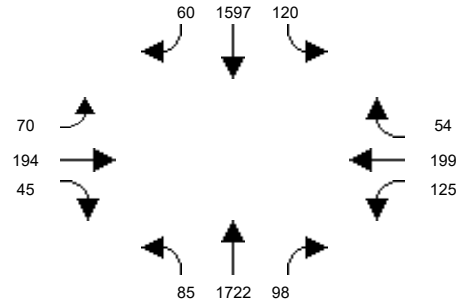
Intersection #31: Bristol St and 17th St



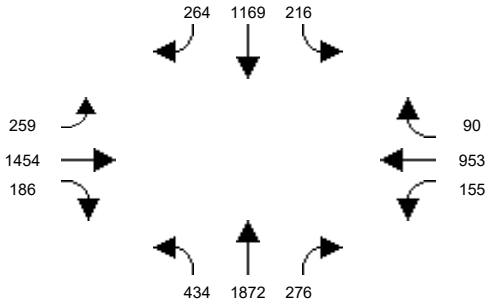
Intersection #32: Bristol St and Civic Center Dr



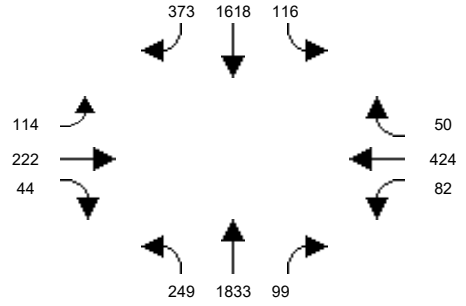
Intersection #33: Bristol St and Santa Ana Blvd



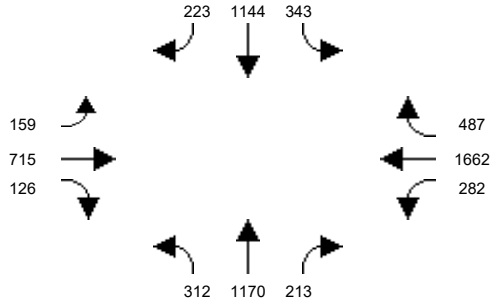
Intersection #34: Bristol St and 1st St



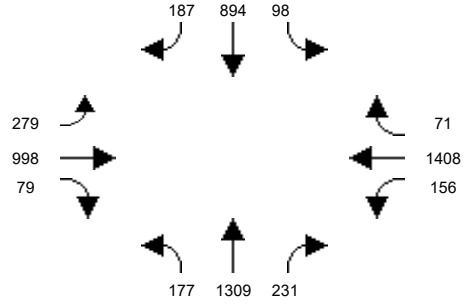
Intersection #35: Bristol St and McFadden Ave



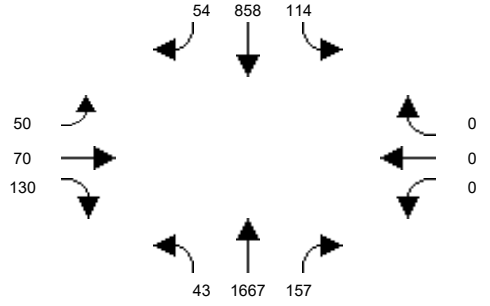
Intersection #36: Bristol St and Warner Ave



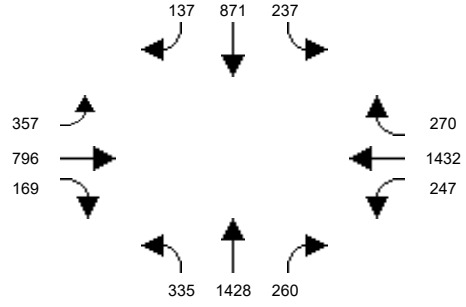
Intersection #37: Bristol St and Segerstrom Ave



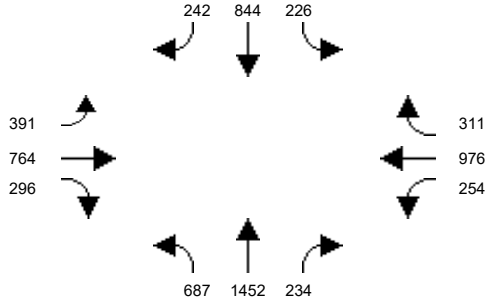
Intersection #38: Bristol St and Alton Ave



Intersection #39: Bristol St and MacArthur Blvd



Intersection #40: Bristol St and Sunflower Ave

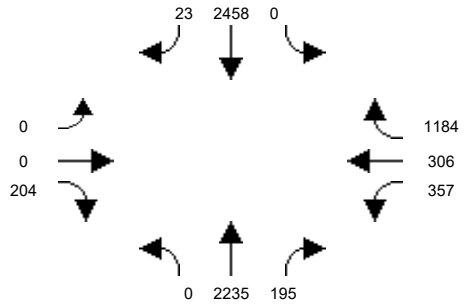


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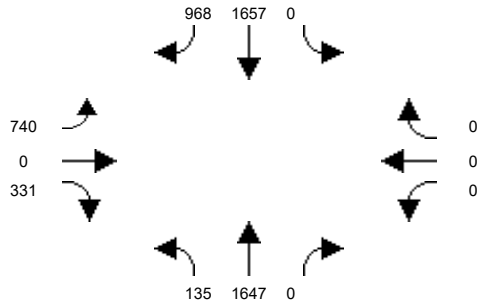
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP PM Peak Hour

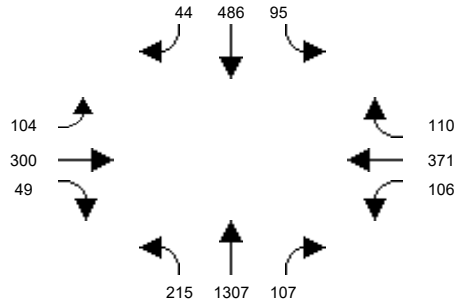
Intersection #41: Bristol St and I-405 NB Ramps



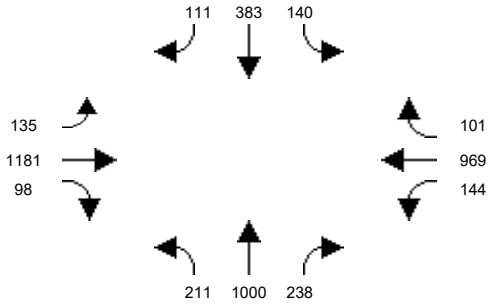
Intersection #42: Bristol St and I-405 SB Ramps



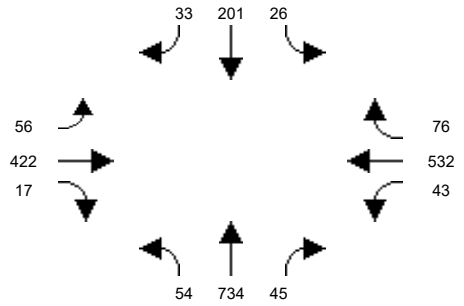
Intersection #43: Flower St and Santa Ana Blvd



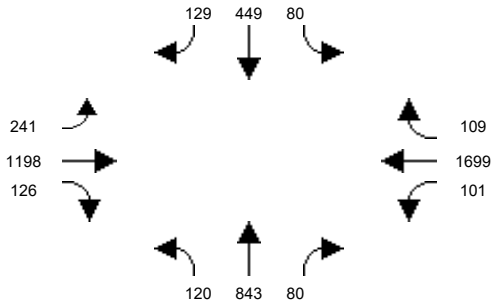
Intersection #44: Flower St and 1st St



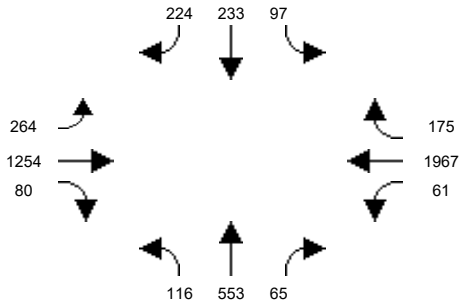
Intersection #45: Flower St and McFadden Ave



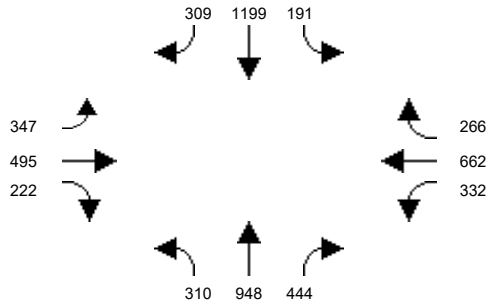
Intersection #46: Flower St and Segerstrom Ave



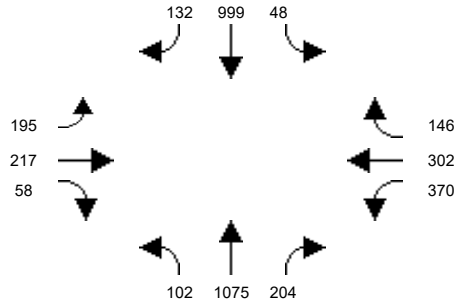
Intersection #47: Flower St and MacArthur Blvd



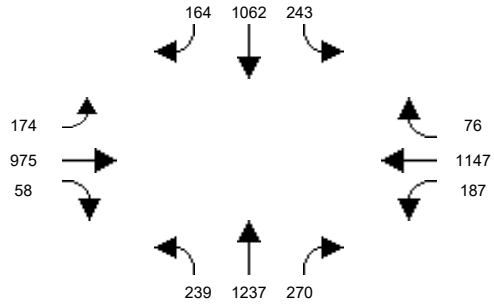
Intersection #48: Main St and La Veta Ave



Intersection #49: Main St and Mainplace Dr / Memory Ln



Intersection #50: Main St and 17th St

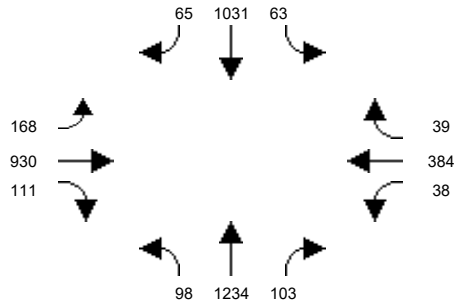


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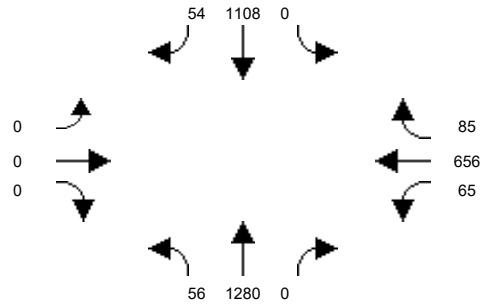
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP PM Peak Hour

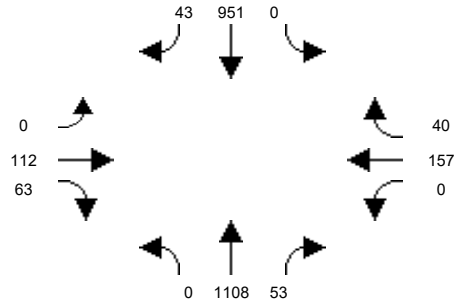
Intersection #51: Main St and Civic Center Dr



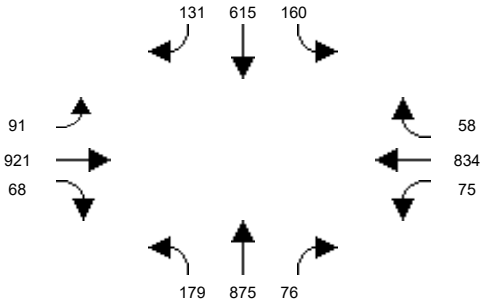
Intersection #52: Main St and Santa Ana Blvd



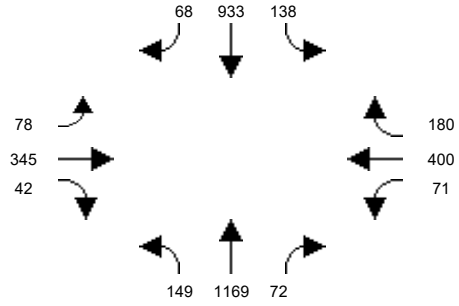
Intersection #53: Main St and 4th St



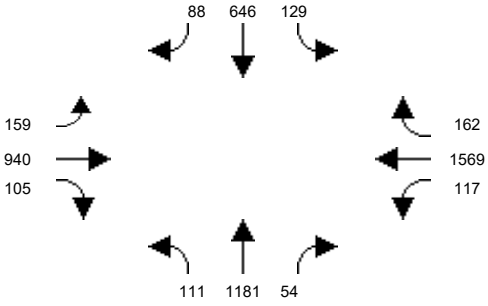
Intersection #54: Main St and 1st St



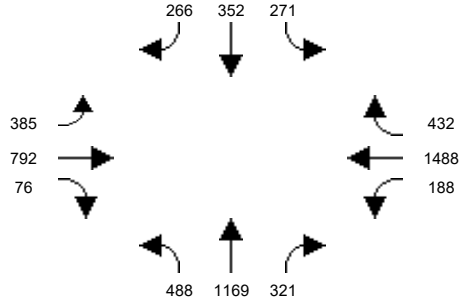
Intersection #55: Main St and McFadden Ave



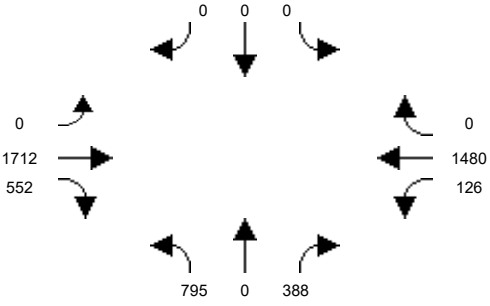
Intersection #56: Main St and Edinger Ave



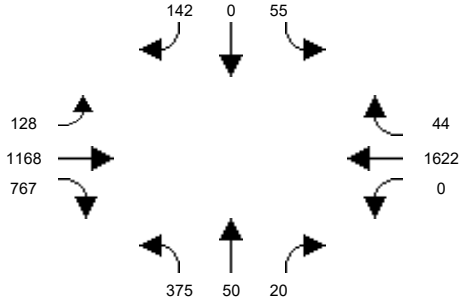
Intersection #57: Main St and MacArthur Blvd



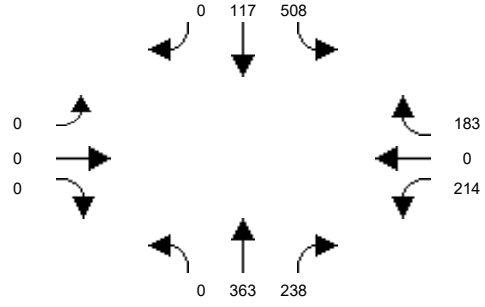
Intersection #58: Penn Wy and 17th St



Intersection #59: I-5 NB Off Ramps/17th Street



Intersection #60: Penn Wy and I-5 SB Ramps

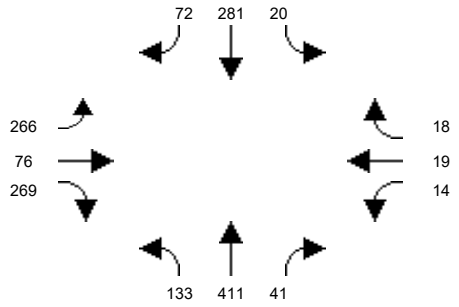


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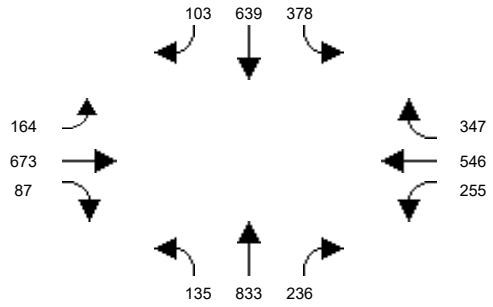
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP PM Peak Hour

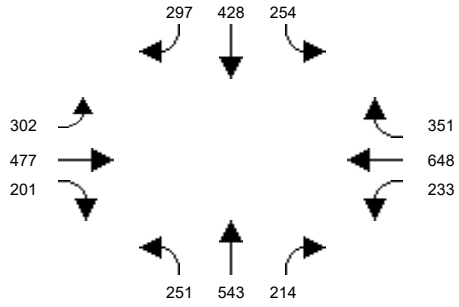
Intersection #61: Santiago St and Civic Center Dr



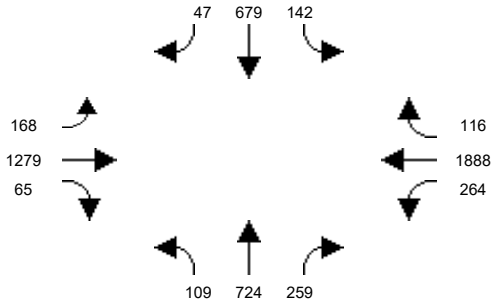
Intersection #62: Santiago St and Santa Ana Blvd



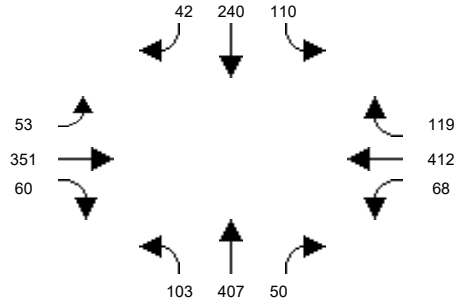
Intersection #63: Standard Ave and 4th St



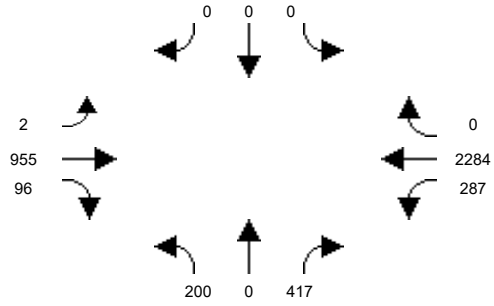
Intersection #64: Standard Ave and 1st St



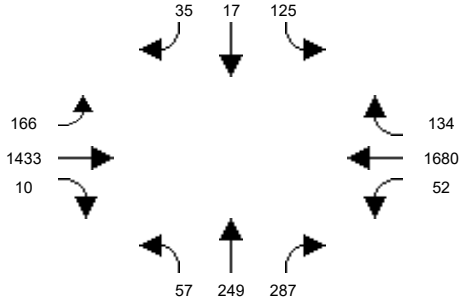
Intersection #65: Standard Ave and Mcfadden Ave



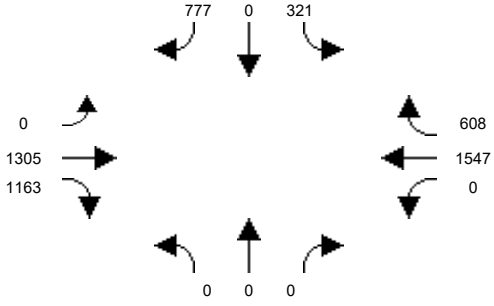
Intersection #66: Halladay St and Warner Ave



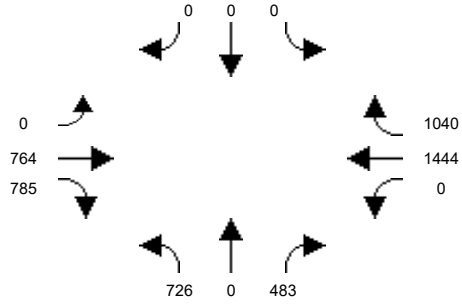
Intersection #67: Halladay St and Dyer Rd



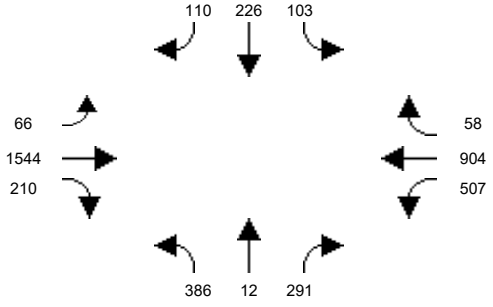
Intersection #68: SR-55 SB Ramps and MacArthur Blvd



Intersection #69: SR-55 NB Ramps and MacArthur Blvd



Intersection #70: SR-55 SB Ramps and Dyer Rd

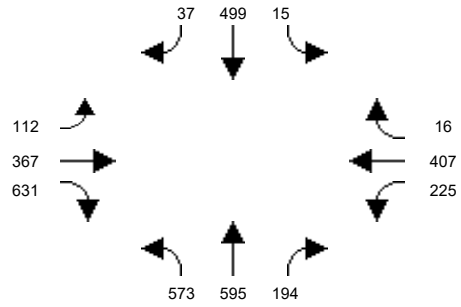


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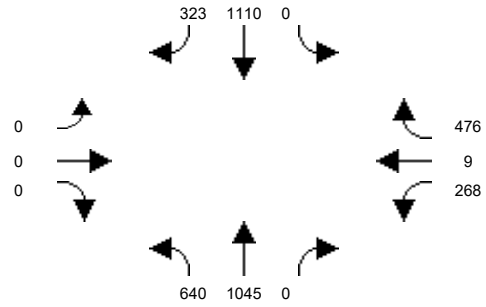
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP PM Peak Hour

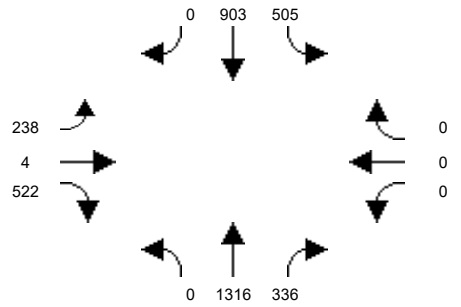
Intersection #71: Glassell St and La Veta Ave



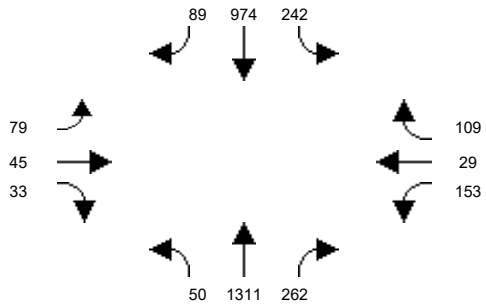
Intersection #72: Glassell St and SR-22 WB Ramps



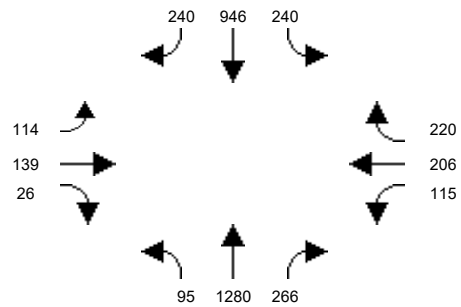
Intersection #73: Grand Ave / Glassell St and SR-22 EB Ramps



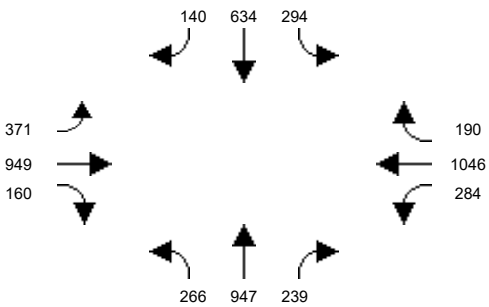
Intersection #74: Grand Ave and Fairhaven Ave



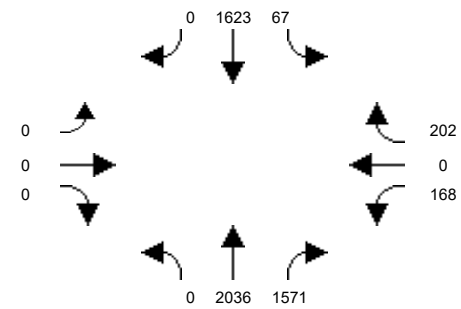
Intersection #75: Grand Ave and Santa Clara Ave



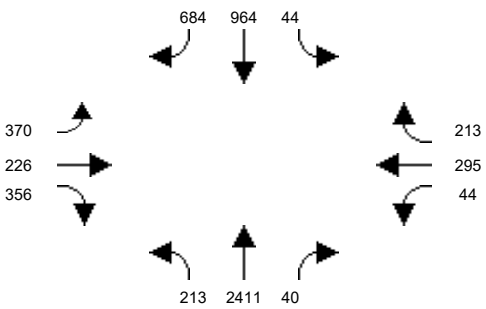
Intersection #76: Grand Ave and 17th St



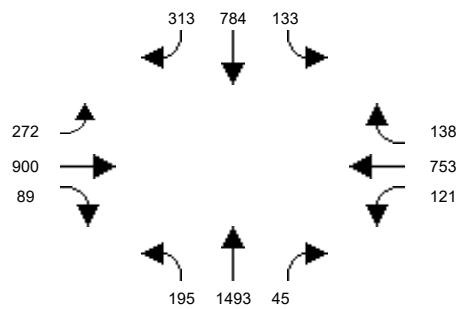
Intersection #77: Grand Ave and I-5 NB Ramps



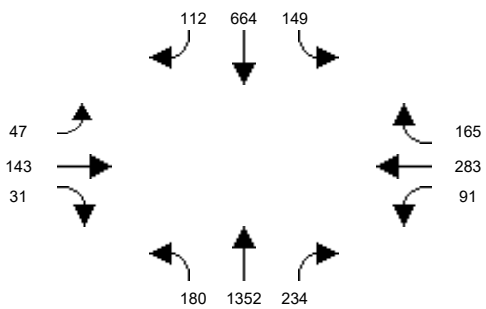
Intersection #78: Grand Ave and Santa Ana Blvd



Intersection #79: Grand Ave and 1st St



Intersection #80: Grand Ave and Chestnut Ave

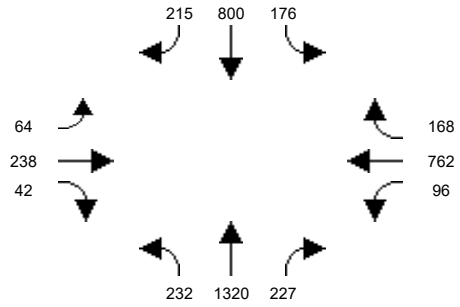


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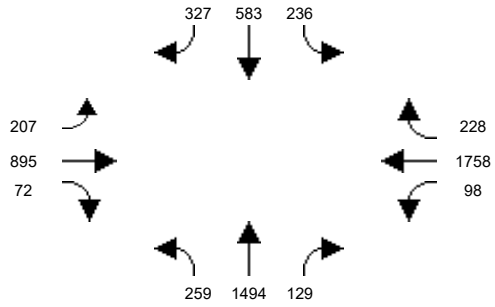
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP PM Peak Hour

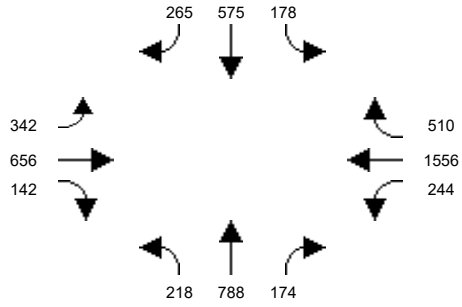
Intersection #81: Grand Ave and McFadden Ave



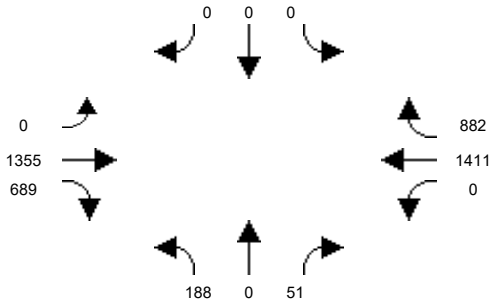
Intersection #82: Grand Ave and Edinger Ave



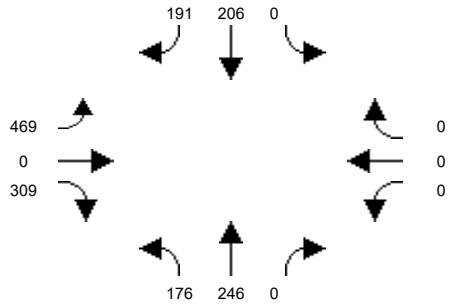
Intersection #83: Grand Ave and Warner Ave



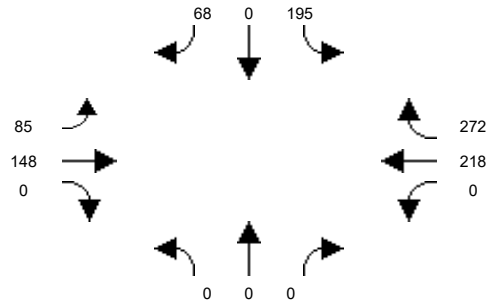
Intersection #84: SR-55 NB Ramps and Dyer Rd



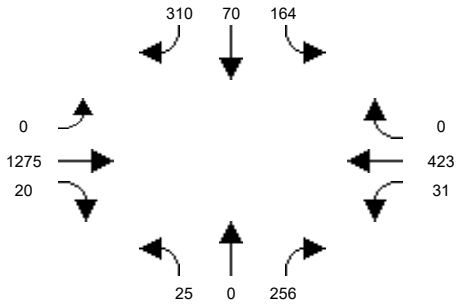
Intersection #85: Cambridge St and La Veta Ave



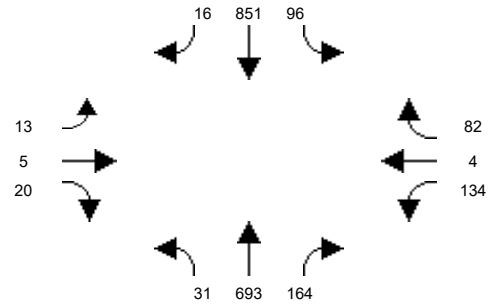
Intersection #86: Cambridge St and Fairhaven Ave



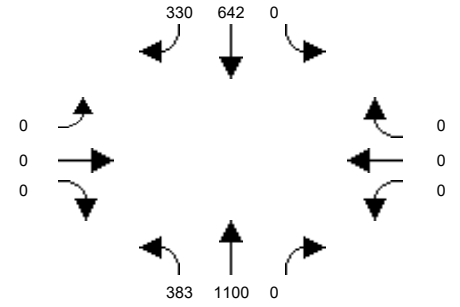
Intersection #87: Mabury St and 1st Street



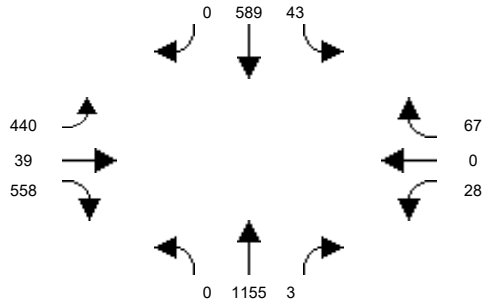
Intersection #88: Tustin St and La Veta Ave



Intersection #89: Tustin St and SR-22 WB On-Ramp



Intersection #90: Tustin St and SR-22 EB Off-Ramp / Seba Ave

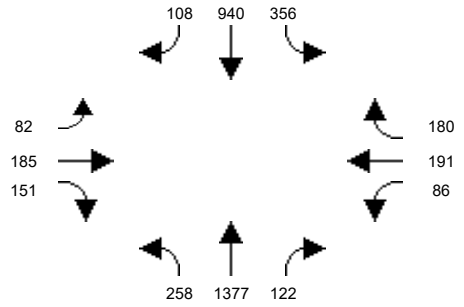


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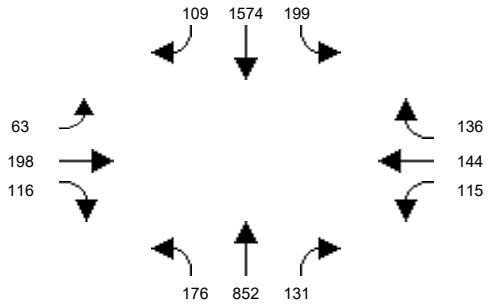
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP PM Peak Hour

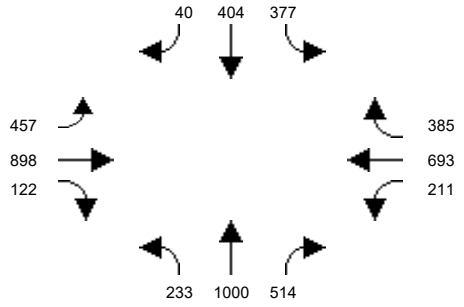
Intersection #91: Tustin Ave and Fairhaven Ave



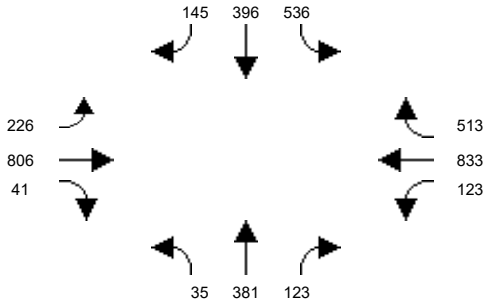
Intersection #92: Tustin Ave and Santa Clara Ave



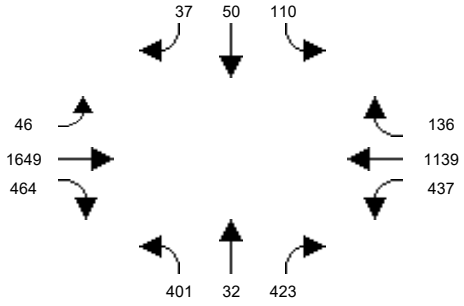
Intersection #93: Tustin Ave and 17th St



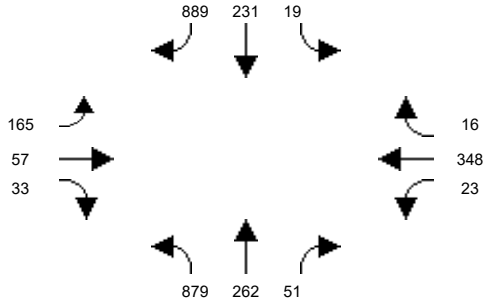
Intersection #94: Tustin Ave and 4th St



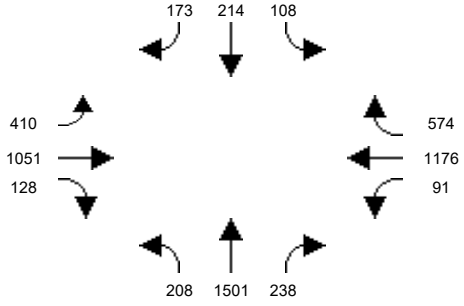
Intersection #95: SR-55 SB Ramps / Auto Mall Dr and Edinger Ave



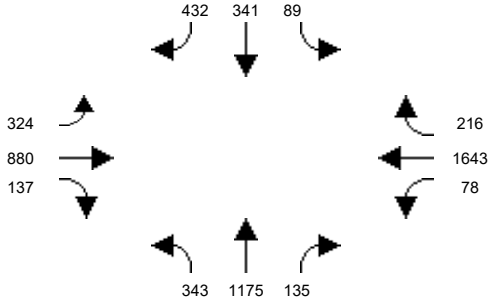
Intersection #96: SR-55 NB Ramps / Del Amo Ave and Newport Avenue



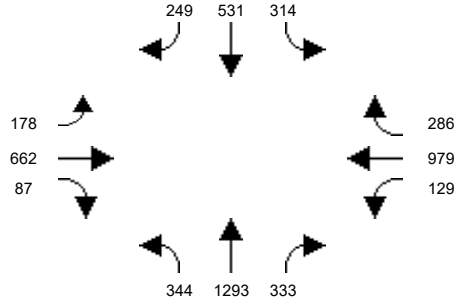
Intersection #97: Red Hill Ave and Edinger Ave



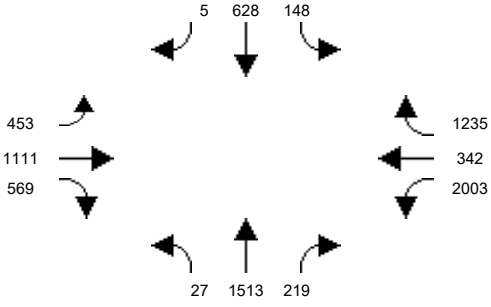
Intersection #98: Red Hill Ave and Warner Ave



Intersection #99: Red Hill Ave and Dyer Rd / barranca Pkwy



Intersection #100: Red Hill Ave and Alton Pkwy

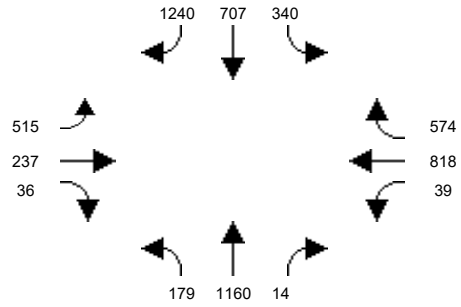


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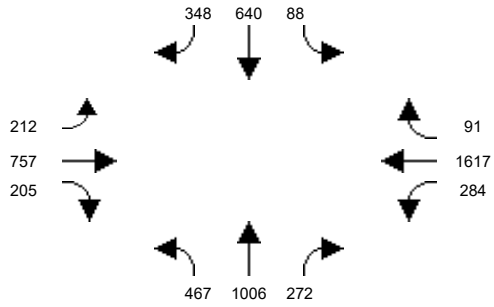
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP PM Peak Hour

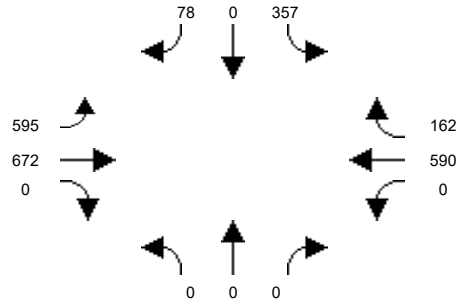
Intersection #101: Red Hill Ave and MacArthur Blvd



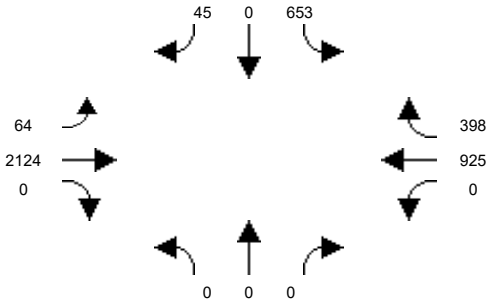
Intersection #102: Red Hill Ave and Main St



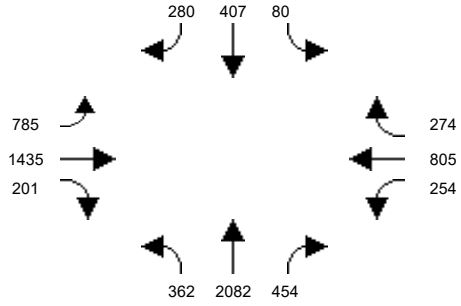
Intersection #103: I-5 SB Ramps and Santa Ana Blvd



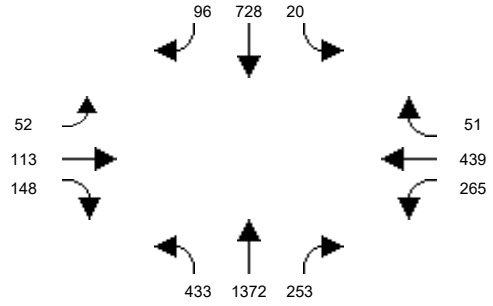
Intersection #104: Tustin Ranch Rd and Warner Ave



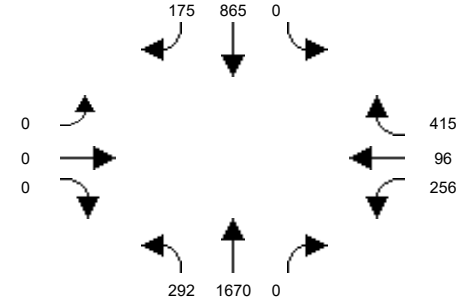
Intersection #105: Von Karman Ave and Barranca Pkwy



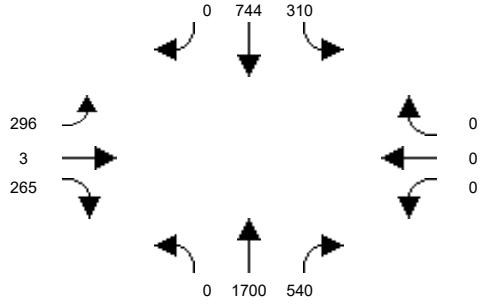
Intersection #106: Red Hill Avenue and El Camino Real



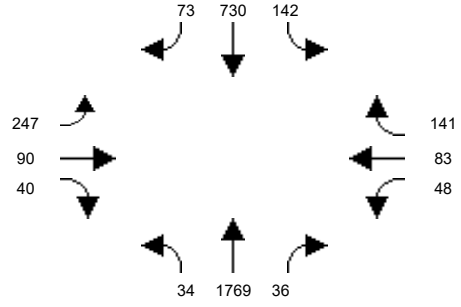
Intersection #107: Red Hill Avenue and I-5 NB Ramps



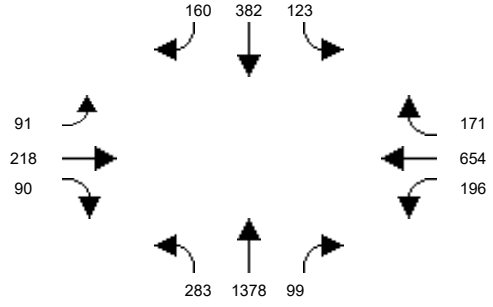
Intersection #108: Red Hill Avenue and I-5 SB Ramps



Intersection #109: Red Hill Avenue and Nisson Road



Intersection #110: Red Hill Avenue and Walnut Avenue

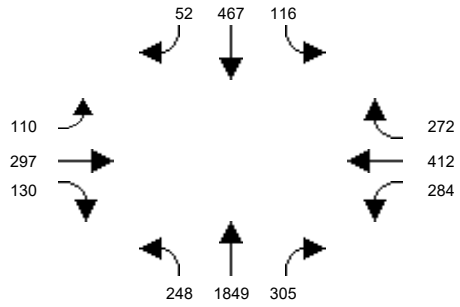


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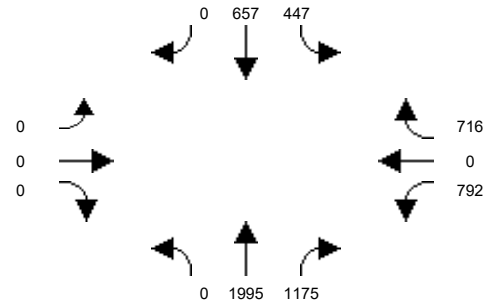
Santa Ana Circulation Element

Intersection Graphic Report
Entered Volume (Base Alternative)
2045 WP PM Peak Hour

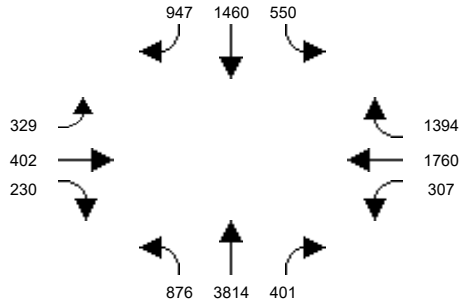
Intersection #111: Red Hill Avenue and Valencia Avenue



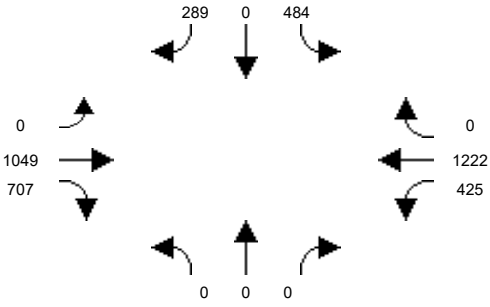
Intersection #112: Tustin Ranch Road and Warner Avenue North



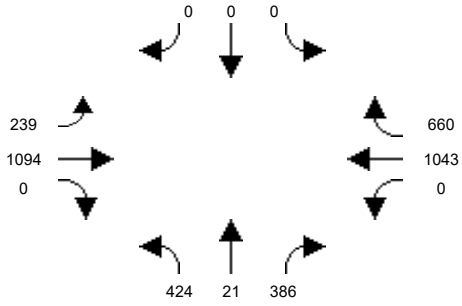
Intersection #113: Tustin Ranch Road and Walnut Avenue



Intersection #114: SR-55 SB Ramps and Irvine Boulevard



Intersection #115: SR-55 NB Ramps and Irvine Boulevard

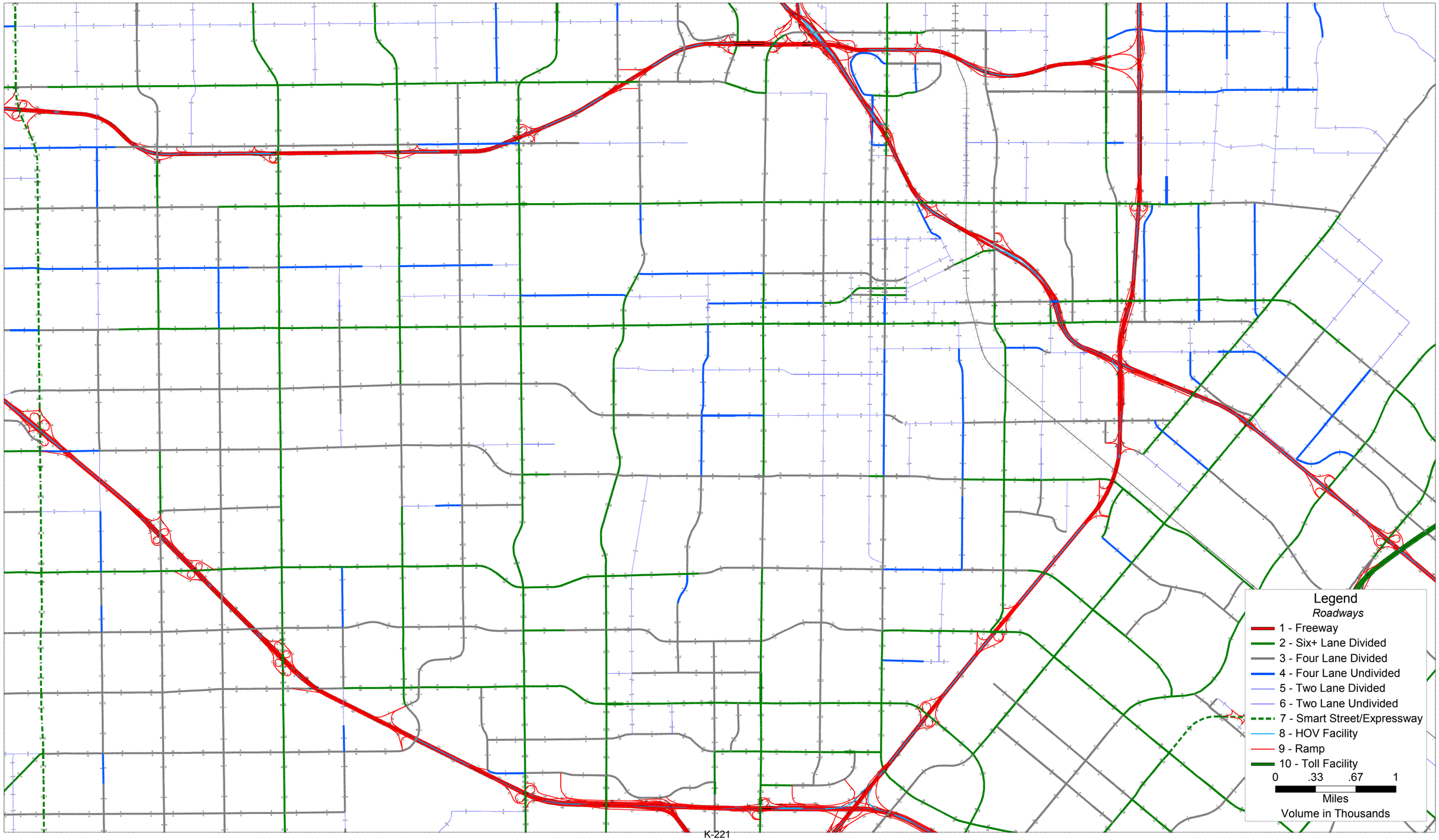


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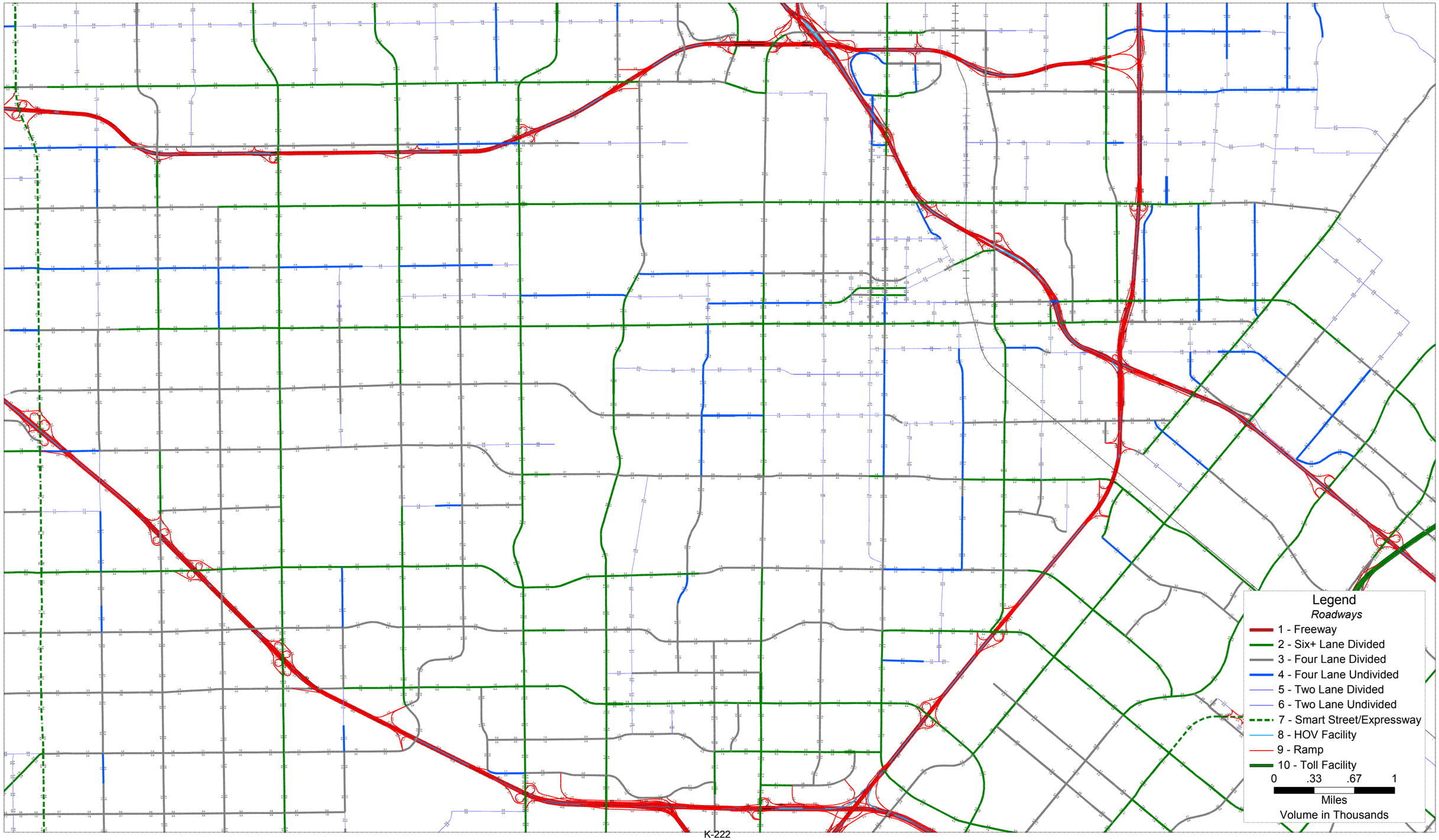
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APPENDIX C
OCTAM VOLUME PLOTS

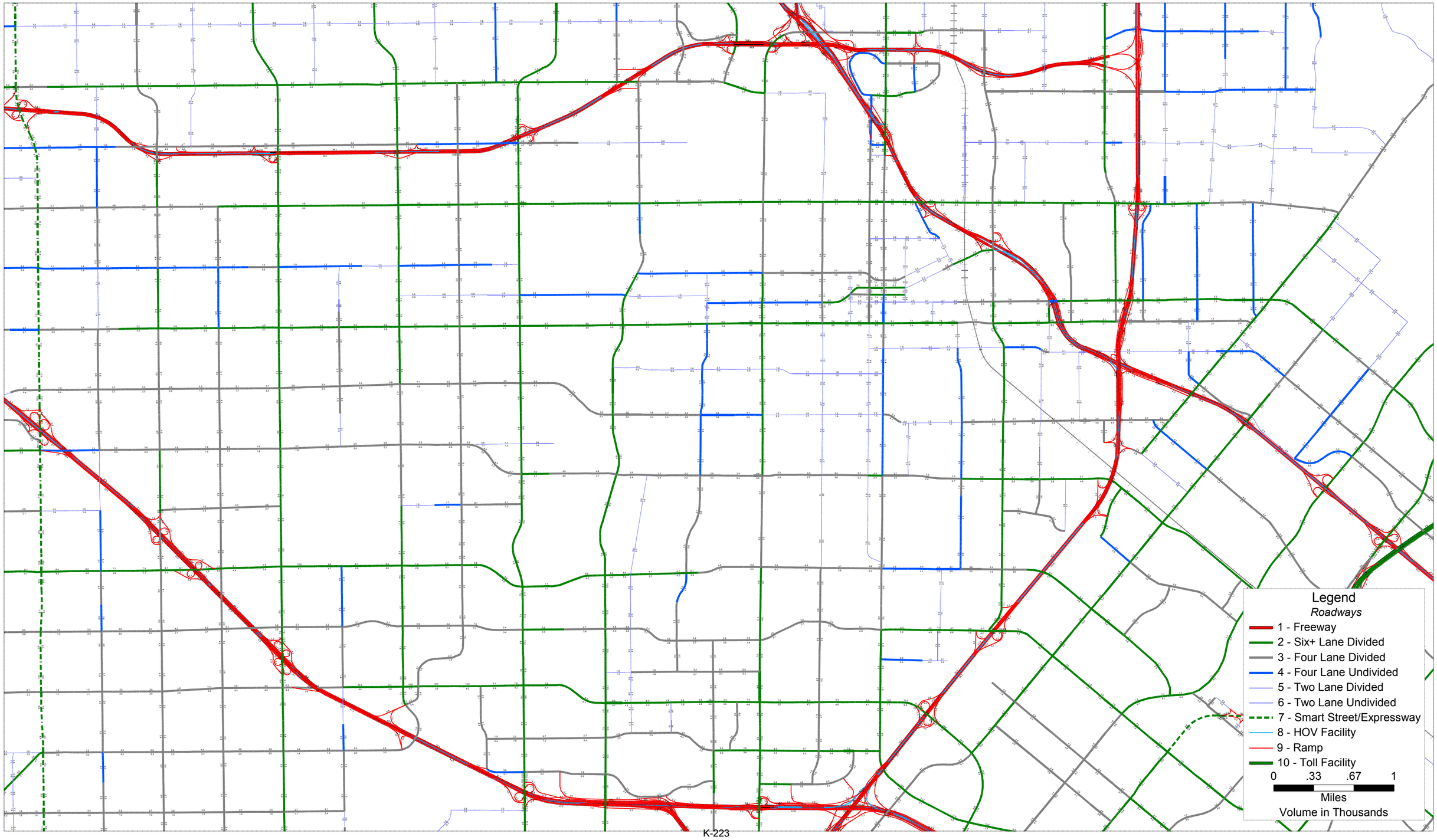
OCTAM 5.0 - Year 2016 Daily Forecasts
Volumes in Thousands
Raw Volumes - Not For Distribution



OCTAM 5.0 - Year 2016 AM Peak Period Forecasts
Volumes in Thousands
Raw Volumes - Not For Distribution

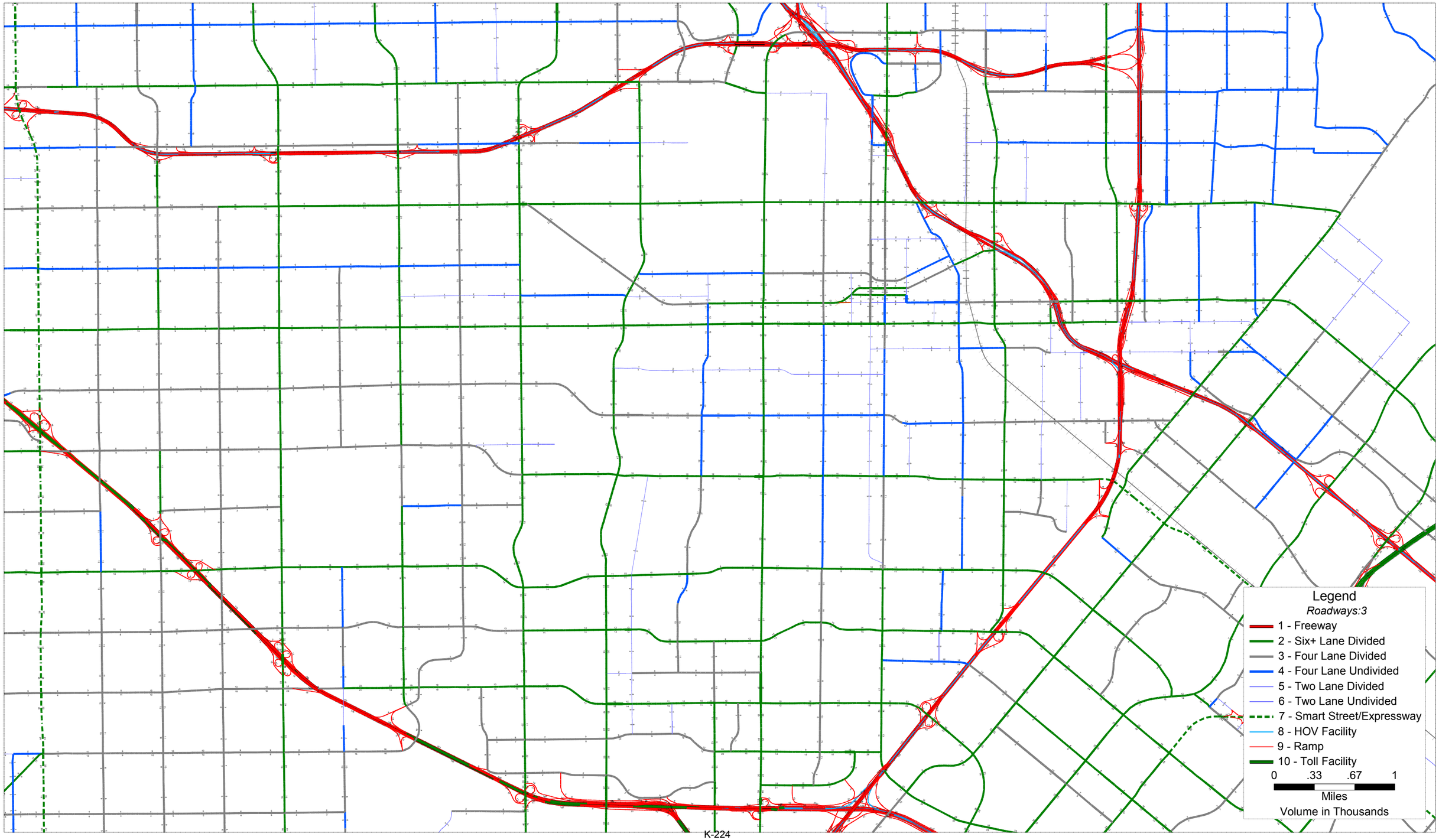


OCTAM 5.0 - Year 2016 PM Peak Period Forecasts
Volumes in Thousands
Raw Volumes - Not For Distribution

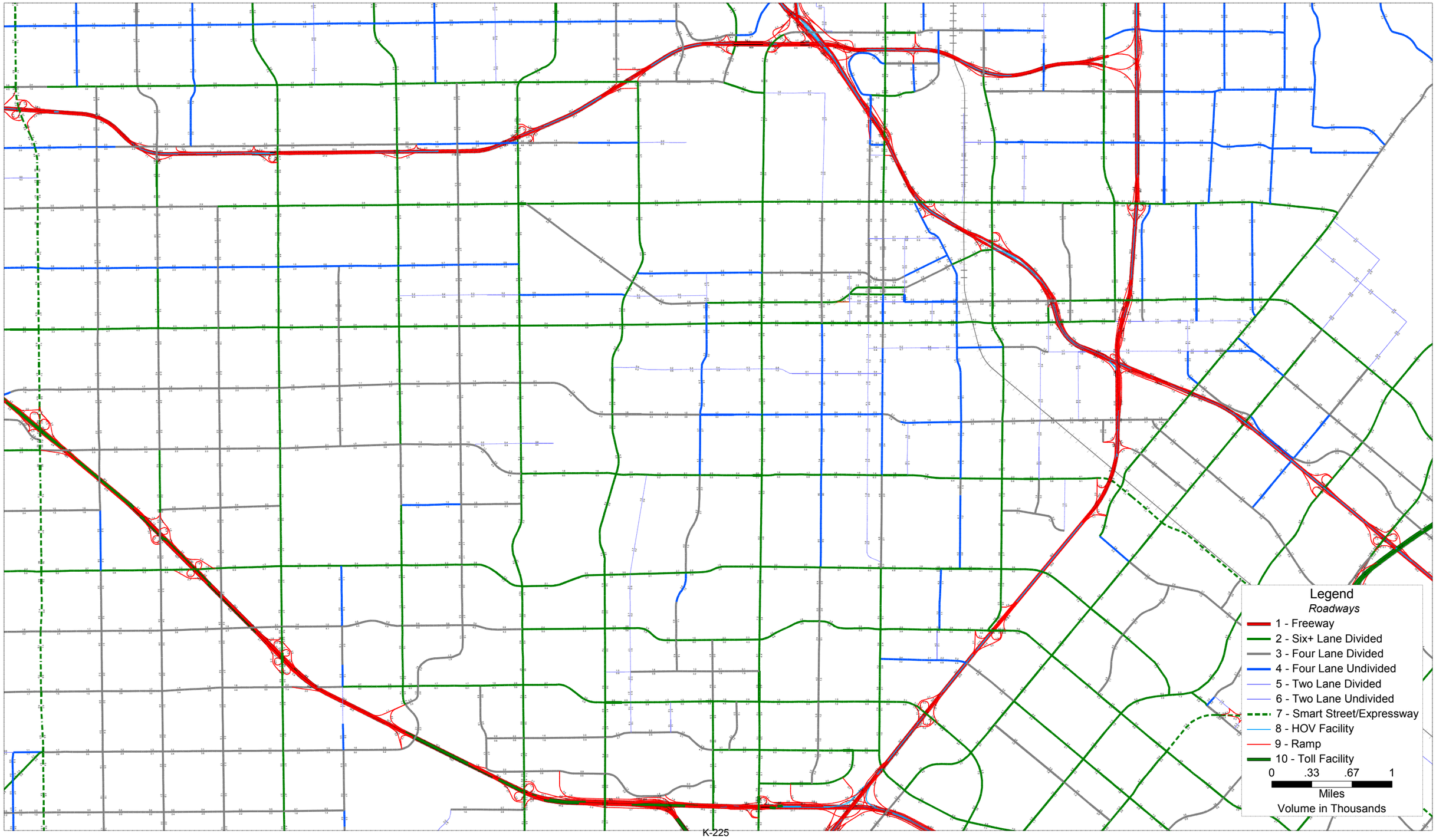


910

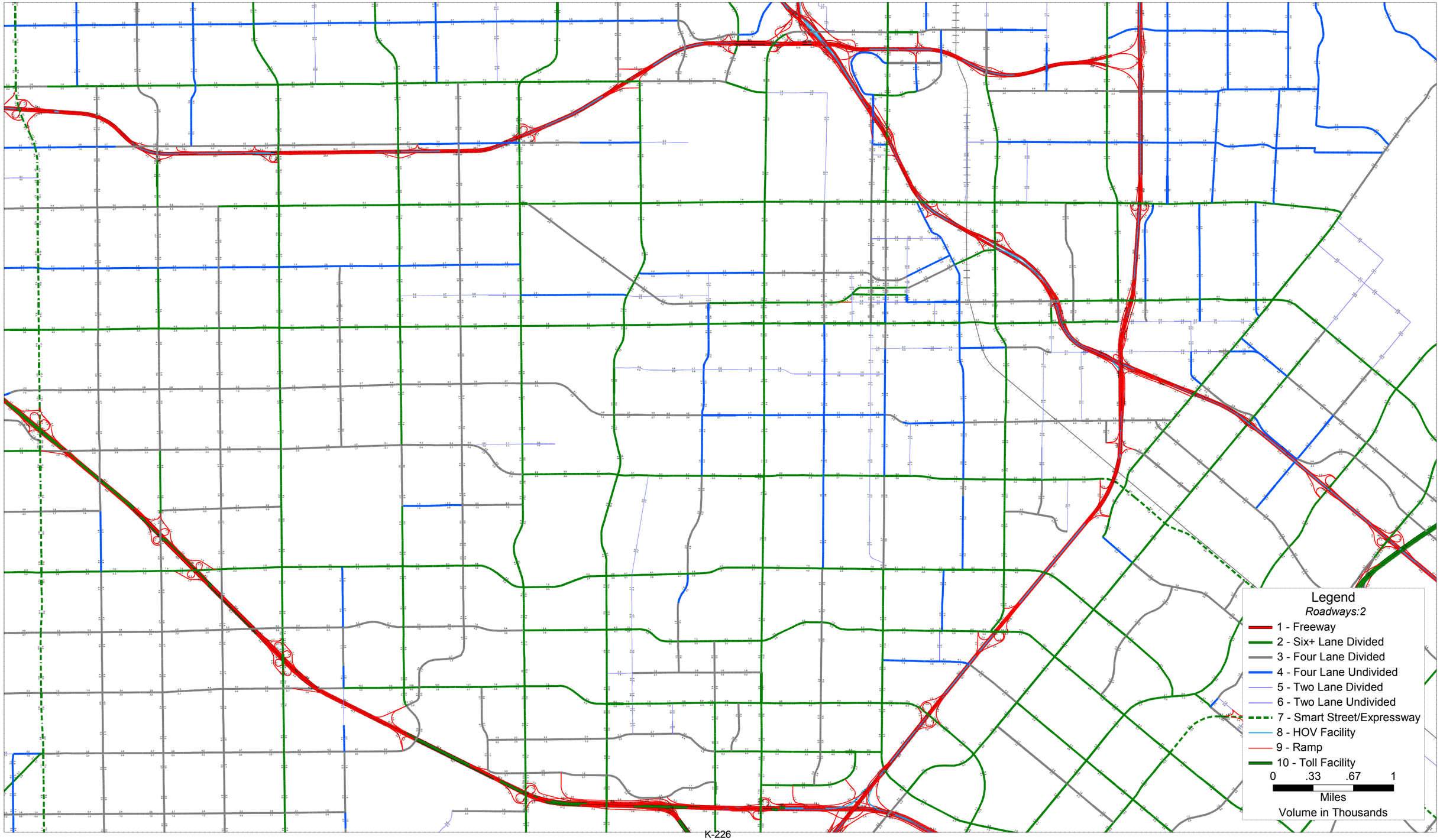
OCTAM 5.0 - Year 2045 SACE Baseline Daily Forecasts
Volumes in Thousands
Raw Volumes - Not For Distribution



OCTAM 5.0 - Year 2045 SACE Baseline AM Peak Period Forecasts
Volumes in Thousands
Raw Volumes - Not For Distribution



OCTAM 5.0 - Year 2045 SACE Baseline PM Peak Period Forecasts
Volumes in Thousands
Raw Volumes - Not For Distribution



APPENDIX D
ANALYSIS WORKSHEETS

APPENDIX D.1

ANALYSIS WORKSHEETS –
2020 NP (AM Peak Hour)

Santa Ana Circulation Element

Scenario Report

Scenario:	2020 NP AM Peak Hour
Command:	Default Command
Volume:	2020 NP AM
Geometry:	NP
Impact Fee:	Default Impact Fee
Trip Generation:	Default Trip Generation
Trip Distribution:	Default Trip Distribution
Paths:	Default Path
Routes:	Default Route
Configuration:	Default Configuration

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Santa Ana Circulation Element

Impact Analysis Report
Level Of Service

Intersection	Base LOS	Base Del/ Veh	Base V/ C	Future LOS	Future Del/ Veh	Future V/ C	Change in
# 1 Euclid St and 1st St	D	xxxxx	0.849	D	xxxxx	0.849	+ 0.000 V/C
# 2 Euclid St and McFadden Ave	E	xxxxx	0.909	E	xxxxx	0.909	+ 0.000 V/C
# 3 Euclid St and Edinger Ave	D	xxxxx	0.861	D	xxxxx	0.861	+ 0.000 V/C
# 4 Newhope St and Hazard Ave	C	xxxxx	0.709	C	xxxxx	0.709	+ 0.000 V/C
# 5 Newhope St and McFadden Ave	D	xxxxx	0.828	D	xxxxx	0.828	+ 0.000 V/C
# 6 Harbor Blvd and SR-22 WB Off-R	C	27.4	0.695	C	27.4	0.695	+ 0.000 D/V
# 7 Trask Avenue and SR-22 EB On-R	B	11.5	0.457	B	11.5	0.457	+ 0.000 D/V
# 8 Harbor Blvd and Westminster Av	D	xxxxx	0.820	D	xxxxx	0.820	+ 0.000 V/C
# 9 Harbor Blvd and 1st St	D	xxxxx	0.807	D	xxxxx	0.807	+ 0.000 V/C
# 10 Harbor Blvd and McFadden Ave	C	xxxxx	0.791	C	xxxxx	0.791	+ 0.000 V/C
# 11 Harbor Blvd and Edinger Ave	C	xxxxx	0.766	C	xxxxx	0.766	+ 0.000 V/C
# 12 Harbor Blvd and Warner Ave	D	xxxxx	0.808	D	xxxxx	0.808	+ 0.000 V/C
# 13 Harbor Blvd and Segerstrom Ave	E	xxxxx	0.978	E	xxxxx	0.978	+ 0.000 V/C
# 14 MacArthur Blvd and Hyland Ave	B	xxxxx	0.608	B	xxxxx	0.608	+ 0.000 V/C
# 15 MacArthur Blvd and Harbor Blvd	C	xxxxx	0.754	C	xxxxx	0.754	+ 0.000 V/C
# 16 Harbor Blvd and Sunflower Ave	C	xxxxx	0.702	C	xxxxx	0.702	+ 0.000 V/C
# 17 Harbor Blvd and I-405 WB Off-R	B	19.7	0.695	B	19.7	0.695	+ 0.000 D/V
# 18 Harbor Blvd and I-405 EB Off-R	B	13.2	0.524	B	13.2	0.524	+ 0.000 D/V
# 19 Fairview St and Civic Center D	B	xxxxx	0.667	B	xxxxx	0.667	+ 0.000 V/C
# 20 Fairview St and 1st St	D	xxxxx	0.872	D	xxxxx	0.872	+ 0.000 V/C
# 21 Fairview St and McFadden Ave	C	xxxxx	0.786	C	xxxxx	0.786	+ 0.000 V/C
# 22 Fairview St and Edinger Ave	C	xxxxx	0.753	C	xxxxx	0.753	+ 0.000 V/C
# 23 Fairview St and Warner Ave	D	xxxxx	0.850	D	xxxxx	0.850	+ 0.000 V/C
# 24 Fairview St and MacArthur Blvd	C	xxxxx	0.704	C	xxxxx	0.704	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Base Del/ Veh	Base V/ C	Future LOS	Future Del/ Veh	Future V/ C	Change in
# 25 Fairview Rd and Sunflower Ave	C	xxxxx	0.705	C	xxxxx	0.705	+ 0.000 V/C
# 26 Greenville St and Edinger Ave	A	xxxxx	0.563	A	xxxxx	0.563	+ 0.000 V/C
# 27 Greenville St and Segerstrom A	C	xxxxx	0.727	C	xxxxx	0.727	+ 0.000 V/C
# 28 Raitt St and McFadden Ave	C	xxxxx	0.715	C	xxxxx	0.715	+ 0.000 V/C
# 29 Raitt St and Edinger Ave	D	xxxxx	0.851	D	xxxxx	0.851	+ 0.000 V/C
# 30 Bear St and MacArthur Blvd	C	xxxxx	0.737	C	xxxxx	0.737	+ 0.000 V/C
# 31 Bristol St and 17th St	C	xxxxx	0.725	C	xxxxx	0.725	+ 0.000 V/C
# 32 Bristol St and Civic Center Dr	D	xxxxx	0.827	D	xxxxx	0.827	+ 0.000 V/C
# 33 Bristol St and Santa Ana Blvd	B	xxxxx	0.669	B	xxxxx	0.669	+ 0.000 V/C
# 34 Bristol St and 1st St	C	xxxxx	0.745	C	xxxxx	0.745	+ 0.000 V/C
# 35 Bristol St and McFadden Ave	C	xxxxx	0.749	C	xxxxx	0.749	+ 0.000 V/C
# 36 Bristol St and Warner Ave	C	xxxxx	0.767	C	xxxxx	0.767	+ 0.000 V/C
# 37 Bristol St and Segerstrom Ave	C	xxxxx	0.794	C	xxxxx	0.794	+ 0.000 V/C
# 38 Bristol St and Alton Ave	A	xxxxx	0.556	A	xxxxx	0.556	+ 0.000 V/C
# 39 Bristol St and MacArthur Blvd	C	xxxxx	0.738	C	xxxxx	0.738	+ 0.000 V/C
# 40 Bristol St and Sunflower Ave	B	xxxxx	0.635	B	xxxxx	0.635	+ 0.000 V/C
# 41 Bristol St and I-405 WB Ramps	B	19.1	0.500	B	19.1	0.500	+ 0.000 D/V
# 42 Bristol St and I-405 EB Ramps	C	22.3	0.607	C	22.3	0.607	+ 0.000 D/V
# 43 Flower St and Santa Ana Blvd	A	xxxxx	0.596	A	xxxxx	0.596	+ 0.000 V/C
# 44 Flower St and 1st St	C	xxxxx	0.795	C	xxxxx	0.795	+ 0.000 V/C
# 45 Flower St and McFadden Ave	D	xxxxx	0.819	D	xxxxx	0.819	+ 0.000 V/C
# 46 Flower St and Segerstrom Ave	C	xxxxx	0.769	C	xxxxx	0.769	+ 0.000 V/C
# 47 Flower St and MacArthur Blvd	B	xxxxx	0.698	B	xxxxx	0.698	+ 0.000 V/C
# 48 Main St and La Veta Ave	A	xxxxx	0.574	A	xxxxx	0.574	+ 0.000 V/C
# 49 Main St and Mainplace Dr / Mem	A	xxxxx	0.485	A	xxxxx	0.485	+ 0.000 V/C
# 50 Main St and 17th St	D	xxxxx	0.821	D	xxxxx	0.821	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Base Del/ Veh	Base V/ C	Future LOS	Future Del/ Veh	Future V/ C	Change in
# 51 Main St and Civic Center Dr	C	xxxxx	0.755	C	xxxxx	0.755	+ 0.000 V/C
# 52 Main St and Santa Ana Blvd	B	xxxxx	0.667	B	xxxxx	0.667	+ 0.000 V/C
# 53 Main St and 4th St	A	xxxxx	0.490	A	xxxxx	0.490	+ 0.000 V/C
# 54 Main St and 1st St	C	xxxxx	0.758	C	xxxxx	0.758	+ 0.000 V/C
# 55 Main St and McFadden Ave	E	xxxxx	0.902	E	xxxxx	0.902	+ 0.000 V/C
# 56 Main St and Edinger Ave	D	xxxxx	0.826	D	xxxxx	0.826	+ 0.000 V/C
# 57 Main St and MacArthur Blvd	C	xxxxx	0.735	C	xxxxx	0.735	+ 0.000 V/C
# 58 Penn Wy and 17th St	B	11.2	0.602	B	11.2	0.602	+ 0.000 D/V
# 59 I-5 NB Off Ramps/17th Street	C	31.2	0.740	C	31.2	0.740	+ 0.000 D/V
# 60 Penn Wy and I-5 SB Ramps	B	19.0	0.436	B	19.0	0.436	+ 0.000 D/V
# 61 Santiago St and Civic Center D	C	16.9	0.723	C	16.9	0.723	+ 0.000 V/C
# 62 Santiago St and Santa Ana Blvd	A	xxxxx	0.553	A	xxxxx	0.553	+ 0.000 V/C
# 64 Standard Ave and 1st St	D	xxxxx	0.803	D	xxxxx	0.803	+ 0.000 V/C
# 65 Standard Ave and Mcfadden Ave	B	xxxxx	0.609	B	xxxxx	0.609	+ 0.000 V/C
# 66 Halladay St and Warner Ave	B	xxxxx	0.659	B	xxxxx	0.659	+ 0.000 V/C
# 67 Halladay St and Dyer Rd	A	xxxxx	0.567	A	xxxxx	0.567	+ 0.000 V/C
# 68 SR-55 SB Ramps and MacArthur B	B	19.8	0.615	B	19.8	0.615	+ 0.000 D/V
# 69 SR-55 NB Ramps and MacArthur B	B	19.2	0.742	B	19.2	0.742	+ 0.000 D/V
# 70 SR-55 SB Ramps and Dyer Rd	C	23.7	0.552	C	23.7	0.552	+ 0.000 D/V
# 71 Glassell St and La Veta Ave	B	xxxxx	0.603	B	xxxxx	0.603	+ 0.000 V/C
# 72 Glassell St and SR-22 WB Ramps	C	28.5	0.741	C	28.5	0.741	+ 0.000 D/V
# 73 Grand Ave / Glassell St and SR	C	27.0	0.760	C	27.0	0.760	+ 0.000 D/V
# 74 Grand Ave and Fairhaven Ave	B	xxxxx	0.680	B	xxxxx	0.680	+ 0.000 V/C
# 75 Grand Ave and Santa Clara Ave	D	xxxxx	0.885	D	xxxxx	0.885	+ 0.000 V/C
# 76 Grand Ave and 17th St	C	xxxxx	0.797	C	xxxxx	0.797	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Del/ Veh	V/ C	Future LOS	Del/ Veh	V/ C	Change in
# 77 Grand Ave and I-5 NB Ramps	B	12.9	0.516	B	12.9	0.516	+ 0.000 D/V
# 78 Grand Ave and Santa Ana Blvd	C	23.8	0.662	C	23.8	0.662	+ 0.000 D/V
# 79 Grand Ave and 1st St	D	xxxxx	0.803	D	xxxxx	0.803	+ 0.000 V/C
# 80 Grand Ave and Chestnut Ave	C	xxxxx	0.705	C	xxxxx	0.705	+ 0.000 V/C
# 81 Grand Ave and McFadden Ave	D	xxxxx	0.829	D	xxxxx	0.829	+ 0.000 V/C
# 82 Grand Ave and Edinger Ave	C	xxxxx	0.763	C	xxxxx	0.763	+ 0.000 V/C
# 83 Grand Ave and Warner Ave	A	xxxxx	0.528	A	xxxxx	0.528	+ 0.000 V/C
# 84 SR-55 NB Ramps and Dyer Rd	B	16.3	0.530	B	16.3	0.530	+ 0.000 D/V
# 85 Cambridge St and La Veta Ave	C	17.3	0.799	C	17.3	0.799	+ 0.000 V/C
# 86 Cambridge St and Fairhaven Ave	A	xxxxx	0.559	A	xxxxx	0.559	+ 0.000 V/C
# 87 Mabury St and 1st Street	C	28.3	0.691	C	28.3	0.691	+ 0.000 D/V
# 88 Tustin St and La Veta Ave	A	xxxxx	0.521	A	xxxxx	0.521	+ 0.000 V/C
# 89 Tustin St and SR-22 WB On-Ramp	B	12.9	0.734	B	12.9	0.734	+ 0.000 D/V
# 90 Tustin St and SR-22 EB Off-Ram	C	24.2	0.731	C	24.2	0.731	+ 0.000 D/V
# 91 Tustin Ave and Fairhaven Ave	E	xxxxx	0.977	E	xxxxx	0.977	+ 0.000 V/C
# 92 Tustin Ave and Santa Clara Ave	D	xxxxx	0.866	D	xxxxx	0.866	+ 0.000 V/C
# 93 Tustin Ave and 17th St	C	xxxxx	0.748	C	xxxxx	0.748	+ 0.000 V/C
# 94 Tustin Ave and 4th St	C	xxxxx	0.701	C	xxxxx	0.701	+ 0.000 V/C
# 95 SR-55 SB Ramps / Auto Mall Dr	C	24.4	0.558	C	24.4	0.558	+ 0.000 D/V
# 96 SR-55 NB Ramps / Del Amo Ave a	C	24.0	0.463	C	24.0	0.463	+ 0.000 D/V
# 97 Red Hill Ave and Edinger Ave	A	xxxxx	0.569	A	xxxxx	0.569	+ 0.000 V/C
# 98 Red Hill Ave and Warner Ave	A	xxxxx	0.485	A	xxxxx	0.485	+ 0.000 V/C
# 99 Red Hill Ave and Dyer Rd / bar	A	xxxxx	0.575	A	xxxxx	0.575	+ 0.000 V/C
#100 Red Hill Ave and Alton Pkwy	A	xxxxx	0.524	A	xxxxx	0.524	+ 0.000 V/C
#101 Red Hill Ave and MacArthur Blv	B	xxxxx	0.664	B	xxxxx	0.664	+ 0.000 V/C
#102 Red Hill Ave and Main St	B	xxxxx	0.670	B	xxxxx	0.670	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Del/ Veh	V/ C	Future LOS	Del/ Veh	V/ C	Change in
#103 I-5 SB Ramps and Santa Ana Blv	B	17.4	0.514	B	17.4	0.514	+ 0.000 D/V
#104 Tustin Ranch Rd and Warner Ave	A	xxxxx	0.475	A	xxxxx	0.475	+ 0.000 V/C
#105 Von Karman Ave and Barranca Pk	C	xxxxx	0.762	C	xxxxx	0.762	+ 0.000 V/C
#106 Red Hill Avenue and El Camino	C	xxxxx	0.706	C	xxxxx	0.706	+ 0.000 V/C
#107 Red Hill Avenue and I-5 NB Ram	C	20.3	0.670	C	20.3	0.670	+ 0.000 D/V
#108 Red Hill Avenue and I-5 SB Ram	C	22.0	0.784	C	22.0	0.784	+ 0.000 D/V
#109 Red Hill Avenue and Nisson Roa	B	xxxxx	0.653	B	xxxxx	0.653	+ 0.000 V/C
#110 Red Hill Avenue and Walnut Ave	B	xxxxx	0.684	B	xxxxx	0.684	+ 0.000 V/C
#111 Red Hill Avenue and Valencia A	A	xxxxx	0.544	A	xxxxx	0.544	+ 0.000 V/C
#112 Tustin Ranch Road and Warner A	A	xxxxx	0.426	A	xxxxx	0.426	+ 0.000 V/C
#113 Tustin Ranch Road and Walnut A	F	xxxxx	1.193	F	xxxxx	1.193	+ 0.000 V/C
#114 SR-55 SB Ramps and Irvine Boul	C	23.2	0.970	C	23.2	0.970	+ 0.000 D/V
#115 SR-55 NB Ramps and Irvine Boul	B	19.8	0.797	B	19.8	0.797	+ 0.000 D/V

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #1 Euclid St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.849
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	65	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			WideBypass			WideBypass		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	124	1044	179	207	1844	104	218	811	153	184	586	188
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	124	1044	179	207	1844	104	218	811	153	184	586	188
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	124	1044	179	207	1844	104	218	811	153	184	586	188
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	124	1044	179	207	1844	104	218	811	153	184	586	188
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	124	1044	179	207	1844	104	218	811	153	184	586	188
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	1.00	2.56	0.44	1.00	2.84	0.16	1.00	2.52	0.48	1.00	2.27	0.73
Final Sat.:	1600	4297	703	1600	4744	256	1600	4238	762	1600	3834	1166
Capacity Analysis Module:												
Vol/Sat:	0.08	0.24	0.25	0.13	0.39	0.41	0.14	0.19	0.20	0.12	0.15	0.16
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #2 Euclid St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.909
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	92	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	145	1100	131	137	1906	158	227	463	287	166	368	140
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	145	1100	131	137	1906	158	227	463	287	166	368	140
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	145	1100	131	137	1906	158	227	463	287	166	368	140
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	145	1100	131	137	1906	158	227	463	287	166	368	140
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	145	1100	131	137	1906	158	227	463	287	166	368	140
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.04	1.00
Lanes:	1.00	2.68	0.32	1.00	2.77	0.23	1.00	1.23	0.77	1.00	1.45	0.55
Final Sat.:	1600	4489	511	1600	4633	367	1600	2075	1225	1600	2418	882
Capacity Analysis Module:												
Vol/Sat:	0.09	0.25	0.26	0.09	0.41	0.43	0.14	0.22	0.23	0.10	0.15	0.16
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #3 Euclid St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.861
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	69	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 0	2 0	2	1 0	1 0	2	0 1	1 0	2	0 1
Volume Module:												
Base Vol:	134	845	72	127	1842	169	172	753	460	100	609	111
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	134	845	72	127	1842	169	172	753	460	100	609	111
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	134	845	72	127	1842	169	172	753	460	100	609	111
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	134	845	72	127	1842	169	172	753	460	100	609	111
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	134	845	72	127	1842	169	172	753	460	100	609	111
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.76	0.24	2.00	2.75	0.25	1.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	3200	4623	377	3200	4597	403	1600	3400	1600	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.04	0.18	0.19	0.04	0.40	0.42	0.11	0.22	0.29	0.06	0.18	0.07
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #4 Newhope St and Hazard Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.709
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	38	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Prot+Permit			Prot+Permit			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	98	738	105	91	1341	123	78	199	147	51	170	68
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	98	738	105	91	1341	123	78	199	147	51	170	68
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	98	738	105	91	1341	123	78	199	147	51	170	68
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	98	738	105	91	1341	123	78	199	147	51	170	68
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	98	738	105	91	1341	123	78	199	147	51	170	68
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.03	1.00	1.00	1.05	1.00	1.00	1.04	1.00
Lanes:	1.00	1.75	0.25	1.00	1.83	0.17	1.00	1.15	0.85	1.00	1.43	0.57
Final Sat.:	1600	2901	399	1600	3031	269	1600	1940	1360	1600	2386	914
Capacity Analysis Module:												
Vol/Sat:	0.06	0.25	0.26	0.06	0.44	0.46	0.05	0.10	0.11	0.03	0.07	0.07
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #5 Newhope St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.828
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	59	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	110	491	173	119	1183	110	135	492	160	176	449	100
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	110	491	173	119	1183	110	135	492	160	176	449	100
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	110	491	173	119	1183	110	135	492	160	176	449	100
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	110	491	173	119	1183	110	135	492	160	176	449	100
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	110	491	173	119	1183	110	135	492	160	176	449	100
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	1.48	0.52	1.00	1.83	0.17	1.00	1.51	0.49	1.00	1.64	0.36
Final Sat.:	1600	2466	834	1600	3028	272	1600	2515	785	1600	2717	583
Capacity Analysis Module:												
Vol/Sat:	0.07	0.20	0.21	0.07	0.39	0.40	0.08	0.20	0.20	0.11	0.17	0.17
Crit Moves:	****					****		****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #6 Harbor Blvd and SR-22 WB Off-Ramp / Banner Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.695
Loss Time (sec):	5	Average Delay (sec/veh):	27.4
Optimal Cycle:	38	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	0	0 0	2 1	0	0 0	1! 0	0	1 1	0 0	1
Volume Module:												
Base Vol:	70	1274	0	0	1354	18	74	0	121	836	44	117
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	70	1274	0	0	1354	18	74	0	121	836	44	117
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	70	1274	0	0	1354	18	74	0	121	836	44	117
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	70	1274	0	0	1354	18	74	0	121	836	44	117
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	70	1274	0	0	1354	18	74	0	121	836	44	117
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	1.00	1.00	0.91	0.91	0.90	1.00	0.90	0.96	0.96	0.85
Lanes:	1.00	3.00	0.00	0.00	2.96	0.04	0.38	0.00	0.62	1.90	0.10	1.00
Final Sat.:	1805	5187	0	0	5109	68	648	0	1059	3448	181	1615
Capacity Analysis Module:												
Vol/Sat:	0.04	0.25	0.00	0.00	0.27	0.27	0.11	0.00	0.11	0.24	0.24	0.07
Crit Moves:	****				****				****		****	
Green/Cycle:	0.06	0.44	0.00	0.00	0.38	0.38	0.16	0.00	0.16	0.35	0.35	0.35
Volume/Cap:	0.70	0.56	0.00	0.00	0.70	0.70	0.70	0.00	0.70	0.70	0.70	0.21
Delay/Veh:	65.4	21.3	0.0	0.0	27.2	27.2	46.8	0.0	46.8	29.7	29.7	23.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	65.4	21.3	0.0	0.0	27.2	27.2	46.8	0.0	46.8	29.7	29.7	23.0
LOS by Move:	E	C	A	A	C	C	D	A	D	C	C	C
HCM2k85thQ:	6	16	0	0	21	21	11	0	11	19	19	4

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #7 Trask Avenue and SR-22 EB On-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.457
Loss Time (sec):	5	Average Delay (sec/veh):	11.5
Optimal Cycle:	23	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	0 0	0	0 0	0 0	0	2 0	1 0	0	0 0	1 1	0
Volume Module:												
Base Vol:	0	0	0	0	0	0	651	815	0	0	738	138
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	0	0	0	651	815	0	0	738	138
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	0	0	0	651	815	0	0	738	138
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	0	0	0	651	815	0	0	738	138
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	0	0	0	651	815	0	0	738	138
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	0.92	1.00	1.00	1.00	0.93	0.93
Lanes:	0.00	0.00	0.00	0.00	0.00	0.00	2.00	1.00	0.00	0.00	1.68	0.32
Final Sat.:	0	0	0	0	0	0	3502	1900	0	0	2968	555
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.00	0.00	0.00	0.19	0.43	0.00	0.00	0.25	0.25
Crit Moves:							****				****	
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.41	0.95	0.00	0.00	0.54	0.54
Volume/Cap:	0.00	0.00	0.00	0.00	0.00	0.00	0.46	0.45	0.00	0.00	0.46	0.46
Delay/Veh:	0.0	0.0	0.0	0.0	0.0	0.0	21.9	0.4	0.0	0.0	14.0	14.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	0.0	0.0	0.0	21.9	0.4	0.0	0.0	14.0	14.0
LOS by Move:	A	A	A	A	A	A	C	A	A	A	B	B
HCM2k85thQ:	0	0	0	0	0	0	12	4	0	0	13	13

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #8 Harbor Blvd and Westminster Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.820
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	56	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	2 1	0	1 0	3 0	1
Volume Module:												
Base Vol:	178	1173	251	212	1663	95	176	927	33	212	574	156
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	178	1173	251	212	1663	95	176	927	33	212	574	156
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	178	1173	251	212	1663	95	176	927	33	212	574	156
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	178	1173	251	212	1663	95	176	927	33	212	574	156
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	178	1173	251	212	1663	95	176	927	33	212	574	156
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	2.90	0.10	1.00	3.00	1.00
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	4835	165	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.11	0.23	0.16	0.13	0.33	0.06	0.11	0.19	0.20	0.13	0.11	0.10
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #9 Harbor Blvd and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.807
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	53	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	117	935	202	227	1914	124	140	932	181	245	621	145
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	117	935	202	227	1914	124	140	932	181	245	621	145
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	117	935	202	227	1914	124	140	932	181	245	621	145
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	117	935	202	227	1914	124	140	932	181	245	621	145
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	117	935	202	227	1914	124	140	932	181	245	621	145
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	2.00	2.51	0.49	2.00	2.43	0.57
Final Sat.:	1600	5100	1600	1600	5100	1600	3200	4219	781	3200	4091	909
Capacity Analysis Module:												
Vol/Sat:	0.07	0.18	0.13	0.14	0.38	0.08	0.04	0.22	0.23	0.08	0.15	0.16
Crit Moves:	****				****				****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #10 Harbor Blvd and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.791
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	50	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	104	1059	101	182	1919	64	167	435	130	190	430	123
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	104	1059	101	182	1919	64	167	435	130	190	430	123
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	104	1059	101	182	1919	64	167	435	130	190	430	123
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	104	1059	101	182	1919	64	167	435	130	190	430	123
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	104	1059	101	182	1919	64	167	435	130	190	430	123
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	2.00	3.00	1.00	2.00	2.90	0.10	1.00	1.54	0.46	1.00	1.56	0.44
Final Sat.:	3200	5100	1600	3200	4845	155	1600	2564	736	1600	2588	712
Capacity Analysis Module:												
Vol/Sat:	0.03	0.21	0.06	0.06	0.40	0.41	0.10	0.17	0.18	0.12	0.17	0.17
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #11 Harbor Blvd and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.766
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	46	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	2 0	2 1	0	2 0	3 0	1
Volume Module:												
Base Vol:	103	772	146	183	2105	61	107	644	323	221	491	204
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	103	772	146	183	2105	61	107	644	323	221	491	204
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	103	772	146	183	2105	61	107	644	323	221	491	204
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	103	772	146	183	2105	61	107	644	323	221	491	204
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	103	772	146	183	2105	61	107	644	323	221	491	204
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	2.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	3400	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.15	0.09	0.06	0.41	0.04	0.03	0.19	0.20	0.07	0.10	0.13
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #12 Harbor Blvd and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.808
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	54	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	115	701	178	457	2059	123	97	1385	364	151	792	134
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	115	701	178	457	2059	123	97	1385	364	151	792	134
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	115	701	178	457	2059	123	97	1385	364	151	792	134
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	115	701	178	457	2059	123	97	1385	364	151	792	134
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	115	701	178	457	2059	123	97	1385	364	151	792	134
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.39	0.61	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	4028	972	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.04	0.17	0.18	0.14	0.40	0.08	0.03	0.27	0.23	0.05	0.16	0.08
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #13 Harbor Blvd and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.978
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 0	2	0 1
Volume Module:												
Base Vol:	111	791	50	171	2299	86	88	737	297	117	362	82
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	111	791	50	171	2299	86	88	737	297	117	362	82
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	111	791	50	171	2299	86	88	737	297	117	362	82
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	111	791	50	171	2299	86	88	737	297	117	362	82
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	111	791	50	171	2299	86	88	737	297	117	362	82
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	2.82	0.18	1.00	2.89	0.11	1.00	1.43	0.57	1.00	2.00	1.00
Final Sat.:	3200	4715	285	1600	4827	173	1600	2381	919	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.17	0.18	0.11	0.48	0.50	0.06	0.31	0.32	0.07	0.11	0.05
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #14 MacArthur Blvd and Hyland Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.608
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	29	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	0 1	0	1 0	1 0	1	1 0	3 0	1	1 0	3 0	1
Volume Module:												
Base Vol:	69	3	19	8	2	19	7	1953	775	64	490	20
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	69	3	19	8	2	19	7	1953	775	64	490	20
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	69	3	19	8	2	19	7	1953	775	64	490	20
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	69	3	19	8	2	19	7	1953	775	64	490	20
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	69	3	19	8	2	19	7	1953	775	64	490	20
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	0.14	0.86	1.00	1.00	1.00	1.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	218	1382	1600	1700	1600	1600	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.02	0.01	0.01	0.01	0.00	0.01	0.00	0.38	0.48	0.04	0.10	0.01
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #15 MacArthur Blvd and Harbor Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.754
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	44	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	1 0	3 0	1	1 0	3 0	1
Volume Module:												
Base Vol:	128	898	92	316	1820	126	126	1141	344	133	443	132
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	128	898	92	316	1820	126	126	1141	344	133	443	132
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	128	898	92	316	1820	126	126	1141	344	133	443	132
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	128	898	92	316	1820	126	126	1141	344	133	443	132
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	128	898	92	316	1820	126	126	1141	344	133	443	132
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	1.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	1600	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.04	0.18	0.06	0.10	0.36	0.08	0.08	0.22	0.22	0.08	0.09	0.08
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #16 Harbor Blvd and Sunflower Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.702
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	37	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	1 0	1 1	0	1 1	0 1	0
Volume Module:												
Base Vol:	217	1107	205	246	2324	57	9	109	42	130	166	94
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	217	1107	205	246	2324	57	9	109	42	130	166	94
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	217	1107	205	246	2324	57	9	109	42	130	166	94
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	217	1107	205	246	2324	57	9	109	42	130	166	94
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	217	1107	205	246	2324	57	9	109	42	130	166	94
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.00	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	1.00	1.44	0.56	1.00	1.28	0.72
Final Sat.:	3200	5100	1600	3200	5100	1600	1600	2410	890	1603	2044	1152
Capacity Analysis Module:												
Vol/Sat:	0.07	0.22	0.13	0.08	0.46	0.04	0.01	0.05	0.05	0.08	0.08	0.08
Crit Moves:	****			****					****			****

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #17 Harbor Blvd and I-405 WB Off-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.695
Loss Time (sec):	5	Average Delay (sec/veh):	19.7
Optimal Cycle:	38	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 4	0 0	0	0 4	0 0	0	0 0	0 0	1	0 1!	0 1
Volume Module:												
Base Vol:	0	1567	0	0	2327	0	0	0	0	508	0	894
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1567	0	0	2327	0	0	0	0	508	0	894
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1567	0	0	2327	0	0	0	0	508	0	894
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1567	0	0	2327	0	0	0	0	508	0	894
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1567	0	0	2327	0	0	0	0	508	0	894
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	1.00	1.00	0.91	1.00	1.00	1.00	1.00	0.89	1.00	0.89
Lanes:	0.00	4.00	0.00	0.00	4.00	0.00	0.00	0.00	0.00	1.36	0.00	1.64
Final Sat.:	0	6916	0	0	6916	0	0	0	0	2298	0	2762
Capacity Analysis Module:												
Vol/Sat:	0.00	0.23	0.00	0.00	0.34	0.00	0.00	0.00	0.00	0.22	0.00	0.32
Crit Moves:	****				****							****
Green/Cycle:	0.00	0.48	0.00	0.00	0.48	0.00	0.00	0.00	0.00	0.47	0.00	0.47
Volume/Cap:	0.00	0.47	0.00	0.00	0.69	0.00	0.00	0.00	0.00	0.47	0.00	0.69
Delay/Veh:	0.0	17.3	0.0	0.0	20.7	0.0	0.0	0.0	0.0	18.4	0.0	22.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	17.3	0.0	0.0	20.7	0.0	0.0	0.0	0.0	18.4	0.0	22.2
LOS by Move:	A	B	A	A	C	A	A	A	A	B	A	C
HCM2k85thQ:	0	13	0	0	24	0	0	0	0	12	0	21

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #18 Harbor Blvd and I-405 EB Off-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.524
Loss Time (sec):	5	Average Delay (sec/veh):	13.2
Optimal Cycle:	26	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Ignore			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 3	0 1	0	0 4	0 0	1	0 1!	0 1	0	0 0	0 0
Volume Module:												
Base Vol:	0	1729	0	0	1504	0	385	0	429	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1729	0	0	1504	0	385	0	429	0	0	0
User Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1729	0	0	1504	0	385	0	429	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1729	0	0	1504	0	385	0	429	0	0	0
PCE Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1729	0	0	1504	0	385	0	429	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	1.00	1.00	0.91	1.00	0.90	1.00	0.90	1.00	1.00	1.00
Lanes:	0.00	3.00	1.00	0.00	4.00	0.00	1.47	0.00	1.53	0.00	0.00	0.00
Final Sat.:	0	5187	1900	0	6916	0	2518	0	2611	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.33	0.00	0.00	0.22	0.00	0.15	0.00	0.16	0.00	0.00	0.00
Crit Moves:		****			****				****			
Green/Cycle:	0.00	0.64	0.00	0.00	0.64	0.00	0.31	0.00	0.31	0.00	0.00	0.00
Volume/Cap:	0.00	0.52	0.00	0.00	0.34	0.00	0.49	0.00	0.52	0.00	0.00	0.00
Delay/Veh:	0.0	10.1	0.0	0.0	8.5	0.0	28.0	0.0	28.5	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	10.1	0.0	0.0	8.5	0.0	28.0	0.0	28.5	0.0	0.0	0.0
LOS by Move:	A	B	A	A	A	A	C	A	C	A	A	A
HCM2k85thQ:	0	16	0	0	9	0	10	0	11	0	0	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #19 Fairview St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.667
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	34	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	0 1	0	1 0	1 1	0	0 1
Volume Module:												
Base Vol:	6	1278	414	243	1402	6	6	23	23	300	9	155
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	6	1278	414	243	1402	6	6	23	23	300	9	155
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	6	1278	414	243	1402	6	6	23	23	300	9	155
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	6	1278	414	243	1402	6	6	23	23	300	9	155
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	6	1278	414	243	1402	6	6	23	23	300	9	155
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	2.27	0.73	1.00	2.99	0.01	0.23	0.89	0.88	1.94	0.06	1.00
Final Sat.:	1600	3826	1174	1600	4980	20	369	1415	1415	3107	93	1600
Capacity Analysis Module:												
Vol/Sat:	0.00	0.33	0.35	0.15	0.28	0.29	0.02	0.02	0.02	0.10	0.10	0.10
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #20 Fairview St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.872
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	73	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			WideBypass			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	2 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	157	1206	227	241	1401	161	214	1233	191	163	732	245
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	157	1206	227	241	1401	161	214	1233	191	163	732	245
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	157	1206	227	241	1401	161	214	1233	191	163	732	245
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	157	1206	227	241	1401	161	214	1233	191	163	732	245
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	157	1206	227	241	1401	161	214	1233	191	163	732	245
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	1.00	3.00	1.00	1.00	2.69	0.31	1.00	2.60	0.40	1.00	2.25	0.75
Final Sat.:	1600	5100	1600	1600	4505	495	1600	4356	644	1600	3796	1204
Capacity Analysis Module:												
Vol/Sat:	0.10	0.24	0.14	0.15	0.31	0.33	0.13	0.28	0.30	0.10	0.19	0.20
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #21 Fairview St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.786
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	49	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	121	1294	173	142	1727	125	221	522	120	194	500	40
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	121	1294	173	142	1727	125	221	522	120	194	500	40
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	121	1294	173	142	1727	125	221	522	120	194	500	40
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	121	1294	173	142	1727	125	221	522	120	194	500	40
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	121	1294	173	142	1727	125	221	522	120	194	500	40
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.03	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	1.63	0.37	1.00	1.85	0.15
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	2702	598	1600	3063	237
Capacity Analysis Module:												
Vol/Sat:	0.08	0.25	0.11	0.09	0.34	0.08	0.14	0.19	0.20	0.12	0.16	0.17
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #22 Fairview St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.753
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	44	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 0	2 0	2	1 0	2 0	2	1 0	2 0	2	1 0
Volume Module:												
Base Vol:	188	831	116	215	1503	127	237	657	176	421	553	110
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	188	831	116	215	1503	127	237	657	176	421	553	110
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	188	831	116	215	1503	127	237	657	176	421	553	110
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	188	831	116	215	1503	127	237	657	176	421	553	110
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	188	831	116	215	1503	127	237	657	176	421	553	110
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	2.00	2.63	0.37	2.00	2.77	0.23	2.00	2.37	0.63	2.00	2.50	0.50
Final Sat.:	3200	4412	588	3200	4626	374	3200	3986	1014	3200	4204	796
Capacity Analysis Module:												
Vol/Sat:	0.06	0.19	0.20	0.07	0.32	0.34	0.07	0.16	0.17	0.13	0.13	0.14
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #23 Fairview St and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.850
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	65	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	215	952	125	205	1628	119	69	1010	301	307	926	300
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	215	952	125	205	1628	119	69	1010	301	307	926	300
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	215	952	125	205	1628	119	69	1010	301	307	926	300
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	215	952	125	205	1628	119	69	1010	301	307	926	300
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	215	952	125	205	1628	119	69	1010	301	307	926	300
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	2.65	0.35	2.00	2.80	0.20	2.00	2.31	0.69	2.00	2.27	0.73
Final Sat.:	3200	4443	557	3200	4673	327	3200	3898	1102	3200	3825	1175
Capacity Analysis Module:												
Vol/Sat:	0.07	0.21	0.22	0.06	0.35	0.36	0.02	0.26	0.27	0.10	0.24	0.26
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #24 Fairview St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.704
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	38	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	248	771	89	307	1546	173	125	1019	150	235	494	140
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	248	771	89	307	1546	173	125	1019	150	235	494	140
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	248	771	89	307	1546	173	125	1019	150	235	494	140
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	248	771	89	307	1546	173	125	1019	150	235	494	140
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	248	771	89	307	1546	173	125	1019	150	235	494	140
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.69	0.31	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	4503	497	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.08	0.17	0.18	0.10	0.30	0.11	0.04	0.20	0.09	0.07	0.10	0.09
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #25 Fairview Rd and Sunflower Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.705
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	38	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	2 0	1 1	0	2 0	2 0	1
Volume Module:												
Base Vol:	185	1071	163	204	1738	111	49	322	65	306	256	132
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	185	1071	163	204	1738	111	49	322	65	306	256	132
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	185	1071	163	204	1738	111	49	322	65	306	256	132
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	185	1071	163	204	1738	111	49	322	65	306	256	132
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	185	1071	163	204	1738	111	49	322	65	306	256	132
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	2.82	0.18	2.00	1.66	0.34	2.00	2.00	1.00
Final Sat.:	3200	5100	1600	3200	4712	288	3200	2763	537	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.06	0.21	0.10	0.06	0.37	0.39	0.02	0.12	0.12	0.10	0.08	0.08
Crit Moves:	****					****		****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #26 Greenville St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.563
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	27	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	0 1	0 0	0	0 0	0 0	2	1 0	1 0	2	0 0
Volume Module:												
Base Vol:	186	0	251	0	0	0	0	960	234	172	915	0
Growth Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
Initial Bse:	186	0	251	0	0	0	0	960	234	172	915	0
User Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
PHF Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
PHF Volume:	186	0	251	0	0	0	0	960	234	172	915	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	186	0	251	0	0	0	0	960	234	172	915	0
PCE Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
MLF Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
FinalVolume:	186	0	251	0	0	0	0	960	234	172	915	0
Saturation Flow Module:												
Sat/Lane:	1600 1600		1600	1600 1600		1600	1600 1600		1600	1600 1600		1600
Adjustment:	1.00 1.06		1.00	1.00 1.06		1.00	1.00 1.05		1.00	1.00 1.06		1.00
Lanes:	1.00 0.00		1.00	0.00 0.00		0.00	0.00 2.41		0.59	1.00 2.00		0.00
Final Sat.:	1600	0	1600	0	0	0	0 4059		941	1600 3400		0
Capacity Analysis Module:												
Vol/Sat:	0.12 0.00		0.16	0.00 0.00		0.00	0.00 0.24		0.25	0.11 0.27		0.00
Crit Moves:			****						****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #27 Greenville St and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.727
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	40	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			WideBypass			WideBypass		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	0 1	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	47	193	78	154	472	160	33	739	118	164	662	76
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	47	193	78	154	472	160	33	739	118	164	662	76
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	47	193	78	154	472	160	33	739	118	164	662	76
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	47	193	78	154	472	160	33	739	118	164	662	76
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	47	193	78	154	472	160	33	739	118	164	662	76
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.03	1.00
Lanes:	1.00	1.42	0.58	1.00	1.00	1.00	1.00	1.72	0.28	1.00	1.79	0.21
Final Sat.:	1600	2379	921	1600	1700	1600	1600	2859	441	1600	2970	330
Capacity Analysis Module:												
Vol/Sat:	0.03	0.08	0.08	0.10	0.28	0.10	0.02	0.26	0.27	0.10	0.22	0.23
Crit Moves:	****				****				****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #28 Raitt St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.715
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	39	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	62	570	90	100	762	104	116	704	90	172	542	97
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	62	570	90	100	762	104	116	704	90	172	542	97
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	62	570	90	100	762	104	116	704	90	172	542	97
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	62	570	90	100	762	104	116	704	90	172	542	97
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	62	570	90	100	762	104	116	704	90	172	542	97
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	1.73	0.27	1.00	1.76	0.24	1.00	1.77	0.23	1.00	1.70	0.30
Final Sat.:	1600	2864	436	1600	2916	384	1600	2937	363	1600	2814	486
Capacity Analysis Module:												
Vol/Sat:	0.04	0.20	0.21	0.06	0.26	0.27	0.07	0.24	0.25	0.11	0.19	0.20
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #29 Raitt St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.851
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	65	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			WideBypass			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	188	501	181	152	716	85	165	1008	145	117	916	54
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	188	501	181	152	716	85	165	1008	145	117	916	54
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	188	501	181	152	716	85	165	1008	145	117	916	54
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	188	501	181	152	716	85	165	1008	145	117	916	54
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	188	501	181	152	716	85	165	1008	145	117	916	54
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.03	1.00
Lanes:	1.00	1.47	0.53	1.00	1.79	0.21	1.00	1.75	0.25	1.00	1.89	0.11
Final Sat.:	1600	2451	849	1600	2960	340	1600	2898	402	1600	3122	178
Capacity Analysis Module:												
Vol/Sat:	0.12	0.20	0.21	0.10	0.24	0.25	0.10	0.35	0.36	0.07	0.29	0.30
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #30 Bear St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.737
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	42	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	1	1 0	2 0	1	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	81	280	119	199	756	300	97	1663	114	70	1180	100
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	81	280	119	199	756	300	97	1663	114	70	1180	100
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	81	280	119	199	756	300	97	1663	114	70	1180	100
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	81	280	119	199	756	300	97	1663	114	70	1180	100
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	81	280	119	199	756	300	97	1663	114	70	1180	100
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.81	0.19	1.00	2.77	0.23
Final Sat.:	1600	3400	1600	1600	3400	1600	1600	4692	308	1600	4625	375
Capacity Analysis Module:												
Vol/Sat:	0.05	0.08	0.07	0.12	0.22	0.19	0.06	0.35	0.37	0.04	0.26	0.27
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #31 Bristol St and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.725
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	40	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	2 0	2 1	0	2 0	3 0	1
Volume Module:												
Base Vol:	192	988	167	335	1376	181	324	963	156	359	824	273
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	192	988	167	335	1376	181	324	963	156	359	824	273
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	192	988	167	335	1376	181	324	963	156	359	824	273
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	192	988	167	335	1376	181	324	963	156	359	824	273
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	192	988	167	335	1376	181	324	963	156	359	824	273
OvlAdjVol:												106
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	2.58	0.42	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	4331	669	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.06	0.19	0.10	0.10	0.27	0.11	0.10	0.22	0.23	0.11	0.16	0.17
OvlAdjV/S:												0.07
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #32 Bristol St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.827
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	58	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	123	1087	96	318	1284	80	149	755	93	121	401	83
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	123	1087	96	318	1284	80	149	755	93	121	401	83
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	123	1087	96	318	1284	80	149	755	93	121	401	83
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	123	1087	96	318	1284	80	149	755	93	121	401	83
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	123	1087	96	318	1284	80	149	755	93	121	401	83
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	2.76	0.24	1.00	2.82	0.18	1.00	1.78	0.22	1.00	1.66	0.34
Final Sat.:	1600	4610	390	1600	4718	282	1600	2949	351	1600	2751	549
Capacity Analysis Module:												
Vol/Sat:	0.08	0.24	0.25	0.20	0.27	0.28	0.09	0.26	0.26	0.08	0.15	0.15
Crit Moves:			****	****				****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #33 Bristol St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.669
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	34	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	65	1419	197	137	1507	41	83	403	35	96	240	41
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	65	1419	197	137	1507	41	83	403	35	96	240	41
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	65	1419	197	137	1507	41	83	403	35	96	240	41
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	65	1419	197	137	1507	41	83	403	35	96	240	41
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	65	1419	197	137	1507	41	83	403	35	96	240	41
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.03	1.00	1.00	1.04	1.00
Lanes:	1.00	2.63	0.37	1.00	2.92	0.08	1.00	1.84	0.16	1.00	1.71	0.29
Final Sat.:	1600	4415	585	1600	4873	127	1600	3044	256	1600	2833	467
Capacity Analysis Module:												
Vol/Sat:	0.04	0.32	0.34	0.09	0.31	0.32	0.05	0.13	0.14	0.06	0.08	0.09
Crit Moves:			****	****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #34 Bristol St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.745
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	43	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	183	1278	162	275	1133	160	206	1268	128	108	813	119
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	183	1278	162	275	1133	160	206	1268	128	108	813	119
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	183	1278	162	275	1133	160	206	1268	128	108	813	119
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	183	1278	162	275	1133	160	206	1268	128	108	813	119
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	183	1278	162	275	1133	160	206	1268	128	108	813	119
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	1.00	2.72	0.28	1.00	2.62	0.38
Final Sat.:	3200	5100	1600	3200	5100	1600	1600	4560	440	1600	4387	613
Capacity Analysis Module:												
Vol/Sat:	0.06	0.25	0.10	0.09	0.22	0.10	0.13	0.28	0.29	0.07	0.19	0.19
Crit Moves:		****			****				****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #35 Bristol St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.749
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	43	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	118	1054	53	161	1571	162	275	472	232	155	432	50
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	118	1054	53	161	1571	162	275	472	232	155	432	50
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	118	1054	53	161	1571	162	275	472	232	155	432	50
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	118	1054	53	161	1571	162	275	472	232	155	432	50
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	118	1054	53	161	1571	162	275	472	232	155	432	50
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.03	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	1.34	0.66	1.00	1.79	0.21
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	2245	1055	1600	2968	332
Capacity Analysis Module:												
Vol/Sat:	0.07	0.21	0.03	0.10	0.31	0.10	0.17	0.21	0.22	0.10	0.15	0.15
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #36 Bristol St and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.767
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	46	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 0	1	2 0	2 1	0	1 0	3 0	1	1 0	3 0	1
Volume Module:												
Base Vol:	192	862	159	386	1307	148	206	1149	286	188	860	146
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	192	862	159	386	1307	148	206	1149	286	188	860	146
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	192	862	159	386	1307	148	206	1149	286	188	860	146
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	192	862	159	386	1307	148	206	1149	286	188	860	146
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	192	862	159	386	1307	148	206	1149	286	188	860	146
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.00	1.00	2.00	2.69	0.31	1.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	3400	1600	3200	4512	488	1600	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.06	0.25	0.10	0.12	0.29	0.30	0.13	0.23	0.18	0.12	0.17	0.09
Crit Moves:		****		****				****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #37 Bristol St and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.794
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	51	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	65	725	136	283	1011	119	189	913	84	122	554	71
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	65	725	136	283	1011	119	189	913	84	122	554	71
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	65	725	136	283	1011	119	189	913	84	122	554	71
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	65	725	136	283	1011	119	189	913	84	122	554	71
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	65	725	136	283	1011	119	189	913	84	122	554	71
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.03	1.00	1.00	1.04	1.00
Lanes:	1.00	2.53	0.47	1.00	2.68	0.32	1.00	1.83	0.17	1.00	1.77	0.23
Final Sat.:	1600	4242	758	1600	4495	505	1600	3030	270	1600	2936	364
Capacity Analysis Module:												
Vol/Sat:	0.04	0.17	0.18	0.18	0.22	0.24	0.12	0.30	0.31	0.08	0.19	0.20
Crit Moves:			****	****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #38 Bristol St and Alton Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.556
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	26	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	3 0	1	1 0	1 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	9	628	47	227	1571	36	190	117	244	41	87	31
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	9	628	47	227	1571	36	190	117	244	41	87	31
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	9	628	47	227	1571	36	190	117	244	41	87	31
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	9	628	47	227	1571	36	190	117	244	41	87	31
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	9	628	47	227	1571	36	190	117	244	41	87	31
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	1.00	2.79	0.21	1.00	3.00	1.00	1.00	1.00	1.00	1.00	0.74	0.26
Final Sat.:	1600	4666	334	1600	5100	1600	1600	1700	1600	1600	1180	420
Capacity Analysis Module:												
Vol/Sat:	0.01	0.13	0.14	0.14	0.31	0.02	0.12	0.07	0.15	0.03	0.07	0.07
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #39 Bristol St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.738
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	42	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	92	478	120	326	1419	148	192	1394	283	191	910	132
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	92	478	120	326	1419	148	192	1394	283	191	910	132
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	92	478	120	326	1419	148	192	1394	283	191	910	132
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	92	478	120	326	1419	148	192	1394	283	191	910	132
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	92	478	120	326	1419	148	192	1394	283	191	910	132
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	2.72	0.28	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	4547	453	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.09	0.08	0.10	0.31	0.33	0.06	0.27	0.18	0.06	0.18	0.08
Crit Moves:	****					****		****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #40 Bristol St and Sunflower Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.635
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	31	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 1	2 0	3	0 1	2 0	2	1 1	2 0	3	0 1
Volume Module:												
Base Vol:	94	461	136	245	1388	104	114	942	381	246	400	138
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	94	461	136	245	1388	104	114	942	381	246	400	138
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	94	461	136	245	1388	104	114	942	381	246	400	138
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	94	461	136	245	1388	104	114	942	381	246	400	138
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	94	461	136	245	1388	104	114	942	381	246	400	138
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	2.85	1.15	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	4757	1843	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.09	0.09	0.08	0.27	0.07	0.04	0.20	0.21	0.08	0.08	0.09
Crit Moves:	****				****				****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #41 Bristol St and I-405 WB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.500
Loss Time (sec):	5	Average Delay (sec/veh):	19.1
Optimal Cycle:	25	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Ignore			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	4	0	1	0	0	4	1	0	0	0	0	2	1	1	1	0	2
Volume Module:												
Base Vol:	0	1489	191	0	1967	8	0	0	34	121	73	665
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1489	191	0	1967	8	0	0	34	121	73	665
User Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1489	0	0	1967	8	0	0	34	121	73	665
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1489	0	0	1967	8	0	0	34	121	73	665
PCE Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1489	0	0	1967	8	0	0	34	121	73	665
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	1.00	1.00	0.91	0.91	1.00	1.00	0.75	0.92	0.92	0.75
Lanes:	0.00	4.00	1.00	0.00	4.98	0.02	0.00	0.00	2.00	1.87	1.13	2.00
Final Sat.:	0	6916	1900	0	8601	35	0	0	2842	3276	1976	2842
Capacity Analysis Module:												
Vol/Sat:	0.00	0.22	0.00	0.00	0.23	0.23	0.00	0.00	0.01	0.04	0.04	0.23
Crit Moves:	****				****				****			****
Green/Cycle:	0.00	0.46	0.00	0.00	0.46	0.46	0.00	0.00	0.02	0.47	0.47	0.47
Volume/Cap:	0.00	0.47	0.00	0.00	0.50	0.50	0.00	0.00	0.50	0.08	0.08	0.50
Delay/Veh:	0.0	18.8	0.0	0.0	19.2	19.2	0.0	0.0	53.9	14.7	14.7	18.7
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	18.8	0.0	0.0	19.2	19.2	0.0	0.0	53.9	14.7	14.7	18.7
LOS by Move:	A	B	A	A	B	B	A	A	D	B	B	B
HCM2k85thQ:	0	13	0	0	14	14	0	0	2	2	2	13

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #42 Bristol St and I-405 EB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.607
Loss Time (sec):	5	Average Delay (sec/veh):	22.3
Optimal Cycle:	31	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Ignore			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	4 0	0	0 0	2	1 1	3 0	0	0 1	0 0	0	0 0
Volume Module:												
Base Vol:	113	1108	0	0	985	847	573	0	524	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	113	1108	0	0	985	847	573	0	524	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	113	1108	0	0	985	0	573	0	524	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	113	1108	0	0	985	0	573	0	524	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	113	1108	0	0	985	0	573	0	524	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	1.00	1.00	0.91	0.91	0.92	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	4.00	0.00	0.00	3.00	1.00	3.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	6916	0	0	5187	1729	5253	0	1615	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.06	0.16	0.00	0.00	0.19	0.00	0.11	0.00	0.32	0.00	0.00	0.00
Crit Moves:	****				****				****			
Green/Cycle:	0.10	0.42	0.00	0.00	0.31	0.00	0.53	0.00	0.53	0.00	0.00	0.00
Volume/Cap:	0.61	0.39	0.00	0.00	0.61	0.00	0.20	0.00	0.61	0.00	0.00	0.00
Delay/Veh:	48.6	20.4	0.0	0.0	29.8	0.0	12.2	0.0	17.3	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	48.6	20.4	0.0	0.0	29.8	0.0	12.2	0.0	17.3	0.0	0.0	0.0
LOS by Move:	D	C	A	A	C	A	B	A	B	A	A	A
HCM2k85thQ:	7	10	0	0	15	0	5	0	17	0	0	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #43 Flower St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.596
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	29	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Prot+Permit			Prot+Permit			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0 2	0	1	1 0 2	0	1	1 0 3	0	1	1 0 2	0	1
Volume Module:												
Base Vol:	87	811	118	168	568	76	114	710	81	101	323	135
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	87	811	118	168	568	76	114	710	81	101	323	135
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	87	811	118	168	568	76	114	710	81	101	323	135
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	87	811	118	168	568	76	114	710	81	101	323	135
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	87	811	118	168	568	76	114	710	81	101	323	135
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	3.00	1.00	1.00	2.00	1.00
Final Sat.:	1600	3400	1600	1600	3400	1600	1600	5100	1600	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.05	0.24	0.07	0.11	0.17	0.05	0.07	0.14	0.05	0.06	0.10	0.08
Crit Moves:		****			****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #44 Flower St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.795
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	51	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	1	1 0	1 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	139	602	159	175	554	58	165	1514	60	209	861	123
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	139	602	159	175	554	58	165	1514	60	209	861	123
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	139	602	159	175	554	58	165	1514	60	209	861	123
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	139	602	159	175	554	58	165	1514	60	209	861	123
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	139	602	159	175	554	58	165	1514	60	209	861	123
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	2.00	1.00	1.00	1.81	0.19	1.00	2.89	0.11	1.00	2.62	0.38
Final Sat.:	1600	3400	1600	1600	2997	303	1600	4817	183	1600	4400	600
Capacity Analysis Module:												
Vol/Sat:	0.09	0.18	0.10	0.11	0.18	0.19	0.10	0.31	0.33	0.13	0.20	0.21
Crit Moves:		****		****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #45 Flower St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.819
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	56	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	1 0	1 0	0	1 0	1 0	0	1 0	1 0	0	1 0
Volume Module:												
Base Vol:	60	438	38	66	367	53	51	581	44	64	432	58
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	60	438	38	66	367	53	51	581	44	64	432	58
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	60	438	38	66	367	53	51	581	44	64	432	58
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	60	438	38	66	367	53	51	581	44	64	432	58
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	60	438	38	66	367	53	51	581	44	64	432	58
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	0.92	0.08	1.00	0.87	0.13	1.00	0.93	0.07	1.00	0.88	0.12
Final Sat.:	1600	1472	128	1600	1398	202	1600	1487	113	1600	1411	189
Capacity Analysis Module:												
Vol/Sat:	0.04	0.30	0.30	0.04	0.26	0.26	0.03	0.39	0.39	0.04	0.31	0.31
Crit Moves:		****			****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #46 Flower St and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.769
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	46	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	63	331	73	98	544	120	155	1065	297	74	546	60
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	63	331	73	98	544	120	155	1065	297	74	546	60
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	63	331	73	98	544	120	155	1065	297	74	546	60
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	63	331	73	98	544	120	155	1065	297	74	546	60
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	63	331	73	98	544	120	155	1065	297	74	546	60
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.03	1.00
Lanes:	1.00	1.64	0.36	1.00	1.64	0.36	1.00	1.56	0.44	1.00	1.80	0.20
Final Sat.:	1600	2722	578	1600	2722	578	1600	2602	698	1600	2983	317
Capacity Analysis Module:												
Vol/Sat:	0.04	0.12	0.13	0.06	0.20	0.21	0.10	0.41	0.43	0.05	0.18	0.19
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #47 Flower St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.698
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	37	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	22	142	73	163	352	264	165	1879	80	54	909	58
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	22	142	73	163	352	264	165	1879	80	54	909	58
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	22	142	73	163	352	264	165	1879	80	54	909	58
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	22	142	73	163	352	264	165	1879	80	54	909	58
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	22	142	73	163	352	264	165	1879	80	54	909	58
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	1.32	0.68	1.00	1.14	0.86	1.00	2.88	0.12	1.00	2.82	0.18
Final Sat.:	1600	2213	1087	1600	1929	1371	1600	4804	196	1600	4712	288
Capacity Analysis Module:												
Vol/Sat:	0.01	0.06	0.07	0.10	0.18	0.19	0.10	0.39	0.41	0.03	0.19	0.20
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #48 Main St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.574
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	27	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	2 0	2 1	0	2 0	2 0	1
Volume Module:												
Base Vol:	84	579	300	198	1034	173	350	495	215	316	276	230
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	84	579	300	198	1034	173	350	495	215	316	276	230
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	84	579	300	198	1034	173	350	495	215	316	276	230
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	84	579	300	198	1034	173	350	495	215	316	276	230
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	84	579	300	198	1034	173	350	495	215	316	276	230
OvlAdjVol:			142									131
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	2.57	0.43	2.00	2.09	0.91	2.00	2.00	1.00
Final Sat.:	3200	5100	1600	3200	4312	688	3200	3546	1454	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.11	0.19	0.06	0.24	0.25	0.11	0.14	0.15	0.10	0.08	0.14
OvlAdjV/S:			0.09									0.08
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #49 Main St and Mainplace Dr / Memory Ln

Cycle (sec):	100	Critical Vol./Cap.(X):	0.485
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	23	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 0	2 0	2	1 0	2 0	2	1 0	2 0	2	0 1
Volume Module:												
Base Vol:	28	764	250	32	1244	260	148	233	16	198	226	15
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	28	764	250	32	1244	260	148	233	16	198	226	15
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	28	764	250	32	1244	260	148	233	16	198	226	15
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	28	764	250	32	1244	260	148	233	16	198	226	15
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	28	764	250	32	1244	260	148	233	16	198	226	15
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	2.26	0.74	2.00	2.48	0.52	2.00	2.81	0.19	2.00	2.00	1.00
Final Sat.:	3200	3817	1183	3200	4170	830	3200	4692	308	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.01	0.20	0.21	0.01	0.30	0.31	0.05	0.05	0.05	0.06	0.07	0.01
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #50 Main St and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.821
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	57	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 0	1	2 0	1 1	0	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	128	760	163	195	1086	70	116	1225	61	325	1300	19
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	128	760	163	195	1086	70	116	1225	61	325	1300	19
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	128	760	163	195	1086	70	116	1225	61	325	1300	19
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	128	760	163	195	1086	70	116	1225	61	325	1300	19
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	128	760	163	195	1086	70	116	1225	61	325	1300	19
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	2.00	2.00	1.00	2.00	1.88	0.12	2.00	2.86	0.14	2.00	2.96	0.04
Final Sat.:	3200	3400	1600	3200	3106	194	3200	4772	228	3200	4931	69
Capacity Analysis Module:												
Vol/Sat:	0.04	0.22	0.10	0.06	0.35	0.36	0.04	0.26	0.27	0.10	0.26	0.27
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #51 Main St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.755
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	44	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	138	901	79	59	1042	159	90	545	99	43	566	34
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	138	901	79	59	1042	159	90	545	99	43	566	34
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	138	901	79	59	1042	159	90	545	99	43	566	34
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	138	901	79	59	1042	159	90	545	99	43	566	34
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	138	901	79	59	1042	159	90	545	99	43	566	34
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.03	1.00
Lanes:	1.00	1.84	0.16	1.00	1.74	0.26	1.00	1.69	0.31	1.00	1.89	0.11
Final Sat.:	1600	3042	258	1600	2876	424	1600	2808	492	1600	3119	181
Capacity Analysis Module:												
Vol/Sat:	0.09	0.30	0.31	0.04	0.36	0.38	0.06	0.19	0.20	0.03	0.18	0.19
Crit Moves:	****					****	****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #52 Main St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.667
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	34	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	0	0 0	1 1	0	0 0	0 0	0	0 1	1 1	0
Volume Module:												
Base Vol:	68	1083	0	0	1107	98	0	0	0	61	848	65
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	68	1083	0	0	1107	98	0	0	0	61	848	65
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	68	1083	0	0	1107	98	0	0	0	61	848	65
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	68	1083	0	0	1107	98	0	0	0	61	848	65
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	68	1083	0	0	1107	98	0	0	0	61	848	65
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.03	1.00	1.00	1.06	1.00	1.00	1.02	1.00
Lanes:	1.00	2.00	0.00	0.00	1.84	0.16	0.00	0.00	0.00	0.19	2.61	0.20
Final Sat.:	1600	3400	0	0	3040	260	0	0	0	301	4279	320
Capacity Analysis Module:												
Vol/Sat:	0.04	0.32	0.00	0.00	0.36	0.38	0.00	0.00	0.00	0.04	0.20	0.20
Crit Moves:	****					****					****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #53 Main St and 4th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.490
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	23	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	1 1	0	0 0	1 1	0	0 0	0 1	0	0 0	0 1	0
Volume Module:												
Base Vol:	0	977	23	0	1098	27	0	93	14	0	114	28
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	977	23	0	1098	27	0	93	14	0	114	28
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	977	23	0	1098	27	0	93	14	0	114	28
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	977	23	0	1098	27	0	93	14	0	114	28
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	977	23	0	1098	27	0	93	14	0	114	28
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.00	1.95	0.05	0.00	1.95	0.05	0.00	0.87	0.13	0.00	0.80	0.20
Final Sat.:	0	3226	74	0	3223	77	0	1391	209	0	1285	315
Capacity Analysis Module:												
Vol/Sat:	0.00	0.30	0.31	0.00	0.34	0.35	0.00	0.07	0.07	0.00	0.09	0.09
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #54 Main St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.758
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	44	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	2	0 1	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	116	596	60	88	882	65	119	1393	147	88	1097	61
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	116	596	60	88	882	65	119	1393	147	88	1097	61
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	116	596	60	88	882	65	119	1393	147	88	1097	61
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	116	596	60	88	882	65	119	1393	147	88	1097	61
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	116	596	60	88	882	65	119	1393	147	88	1097	61
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.04	1.00
Lanes:	1.00	1.82	0.18	1.00	2.00	1.00	1.00	2.71	0.29	1.00	2.84	0.16
Final Sat.:	1600	3007	293	1600	3400	1600	1600	4542	458	1600	4747	253
Capacity Analysis Module:												
Vol/Sat:	0.07	0.20	0.20	0.06	0.26	0.04	0.07	0.31	0.32	0.06	0.23	0.24
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #55 Main St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.902
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	88	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Prot+Permit			Prot+Permit			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	0 1	0	1 0	1 0	1
Volume Module:												
Base Vol:	89	838	42	175	1364	27	42	396	43	140	373	107
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	89	838	42	175	1364	27	42	396	43	140	373	107
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	89	838	42	175	1364	27	42	396	43	140	373	107
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	89	838	42	175	1364	27	42	396	43	140	373	107
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	89	838	42	175	1364	27	42	396	43	140	373	107
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.00	1.00	1.00	1.06	1.00
Lanes:	1.00	1.90	0.10	1.00	1.96	0.04	1.00	0.90	0.10	1.00	1.00	1.00
Final Sat.:	1600	3147	153	1600	3238	62	1600	1443	157	1600	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.06	0.27	0.28	0.11	0.42	0.43	0.03	0.27	0.27	0.09	0.22	0.07
Crit Moves:	****					****		****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #56 Main St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.826
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	58	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	2	0 1	1 0	2	1 0
Volume Module:												
Base Vol:	109	617	67	228	1052	55	75	971	131	122	709	87
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	109	617	67	228	1052	55	75	971	131	122	709	87
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	109	617	67	228	1052	55	75	971	131	122	709	87
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	109	617	67	228	1052	55	75	971	131	122	709	87
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	109	617	67	228	1052	55	75	971	131	122	709	87
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.06	1.00	1.00	1.05	1.00
Lanes:	1.00	1.80	0.20	1.00	1.90	0.10	1.00	2.00	1.00	1.00	2.67	0.33
Final Sat.:	1600	2987	313	1600	3141	159	1600	3400	1600	1600	4475	525
Capacity Analysis Module:												
Vol/Sat:	0.07	0.21	0.21	0.14	0.33	0.35	0.05	0.29	0.08	0.08	0.16	0.17
Crit Moves:	****					****		****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #57 Main St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.735
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	41	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	54	311	264	645	848	192	240	1393	252	146	435	218
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	54	311	264	645	848	192	240	1393	252	146	435	218
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	54	311	264	645	848	192	240	1393	252	146	435	218
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	54	311	264	645	848	192	240	1393	252	146	435	218
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	54	311	264	645	848	192	240	1393	252	146	435	218
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.02	0.06	0.17	0.20	0.17	0.12	0.08	0.27	0.16	0.05	0.09	0.14
Crit Moves:			****	****				****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #58 Penn Wy and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.602
Loss Time (sec):	5	Average Delay (sec/veh):	11.2
Optimal Cycle:	30	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	0	0 2	0	0	0 0 0	0	0	2 1 0	1	0	3 0 0
Volume Module:												
Base Vol:	43	0	217	0	0	0	0	1238	631	214	1722	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	43	0	217	0	0	0	0	1238	631	214	1722	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	43	0	217	0	0	0	0	1238	631	214	1722	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	43	0	217	0	0	0	0	1238	631	214	1722	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	43	0	217	0	0	0	0	1238	631	214	1722	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.75	1.00	1.00	1.00	1.00	0.86	0.86	0.95	0.91	1.00
Lanes:	2.00	0.00	2.00	0.00	0.00	0.00	0.00	2.00	1.00	1.00	3.00	0.00
Final Sat.:	3502	0	2842	0	0	0	0	3282	1641	1805	5187	0
Capacity Analysis Module:												
Vol/Sat:	0.01	0.00	0.08	0.00	0.00	0.00	0.00	0.38	0.38	0.12	0.33	0.00
Crit Moves:			****					****		****		
Green/Cycle:	0.13	0.00	0.13	0.00	0.00	0.00	0.00	0.63	0.63	0.20	0.82	0.00
Volume/Cap:	0.10	0.00	0.60	0.00	0.00	0.00	0.00	0.60	0.61	0.60	0.40	0.00
Delay/Veh:	38.7	0.0	44.1	0.0	0.0	0.0	0.0	11.5	11.7	39.5	2.4	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	38.7	0.0	44.1	0.0	0.0	0.0	0.0	11.5	11.7	39.5	2.4	0.0
LOS by Move:	D	A	D	A	A	A	A	B	B	D	A	A
HCM2k85thQ:	1	0	7	0	0	0	0	19	20	10	8	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #59 I-5 NB Off Ramps/17th Street

Cycle (sec):	100	Critical Vol./Cap.(X):	0.740
Loss Time (sec):	5	Average Delay (sec/veh):	31.2
Optimal Cycle:	44	Level Of Service:	C

Street Name:	I-5 NB Off Ramps						17th Street					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Permitted		
Rights:	Include			Include			Ignore			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 1	0	0 1	1	0 0	0 1	1	0	3 0 1	0	0	2 1 0
Volume Module:												
Base Vol:	768	35	22	57	0	295	99	1040	376	0	1245	20
Growth Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
Initial Bse:	768	35	22	57	0	295	99	1040	376	0	1245	20
User Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		0.00	1.00 1.00		1.00
PHF Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		0.00	1.00 1.00		1.00
PHF Volume:	768	35	22	57	0	295	99	1040	0	0	1245	20
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	768	35	22	57	0	295	99	1040	0	0	1245	20
PCE Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		0.00	1.00 1.00		1.00
MLF Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		0.00	1.00 1.00		1.00
FinalVolume:	768	35	22	57	0	295	99	1040	0	0	1245	20
Saturation Flow Module:												
Sat/Lane:	1900 1900		1900	1900 1900		1900	1900 1900		1900	1900 1900		1900
Adjustment:	0.95 0.95		0.85	0.95 1.00		0.85	0.95 0.91		1.00	1.00 0.91		0.91
Lanes:	1.91 0.09		1.00	1.00 0.00		1.00	1.00 3.00		1.00	0.00 2.95		0.05
Final Sat.:	3467	158	1615	1805	0	1615	1805 5187		1900	0 5095		82
Capacity Analysis Module:												
Vol/Sat:	0.22 0.22		0.01	0.03 0.00		0.18	0.05 0.20		0.00	0.00 0.24		0.24
Crit Moves:	****					****	****				****	
Green/Cycle:	0.30 0.30		0.30	0.25 0.00		0.25	0.07 0.40		0.00	0.00 0.33		0.33
Volume/Cap:	0.74 0.74		0.05	0.13 0.00		0.74	0.74 0.50		0.00	0.00 0.74		0.74
Delay/Veh:	34.3 34.3		24.9	29.4 0.0		42.0	64.9 22.4		0.0	0.0 31.5		31.5
User DelAdj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
AdjDel/Veh:	34.3 34.3		24.9	29.4 0.0		42.0	64.9 22.4		0.0	0.0 31.5		31.5
LOS by Move:	C	C	C	C	A	D	E	C	A	A	C	C
HCM2k85thQ:	19	19	1	2	0	15	7	13	0	0	21	21

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #60 Penn Wy and I-5 SB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.436
Loss Time (sec):	5	Average Delay (sec/veh):	19.0
Optimal Cycle:	22	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 2	0 1	2	0 2	0 0	0	0 0	0 0	1	0 0	0 2
Volume Module:												
Base Vol:	0	158	140	799	165	0	0	0	0	179	0	100
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	158	140	799	165	0	0	0	0	179	0	100
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	158	140	799	165	0	0	0	0	179	0	100
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	158	140	799	165	0	0	0	0	179	0	100
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	158	140	799	165	0	0	0	0	179	0	100
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.95	0.85	0.92	0.95	1.00	1.00	1.00	1.00	0.95	1.00	0.75
Lanes:	0.00	2.00	1.00	2.00	2.00	0.00	0.00	0.00	0.00	1.00	0.00	2.00
Final Sat.:	0	3610	1615	3502	3610	0	0	0	0	1805	0	2842
Capacity Analysis Module:												
Vol/Sat:	0.00	0.04	0.09	0.23	0.05	0.00	0.00	0.00	0.00	0.10	0.00	0.04
Crit Moves:			****	****						****		
Green/Cycle:	0.00	0.20	0.20	0.52	0.72	0.00	0.00	0.00	0.00	0.23	0.00	0.75
Volume/Cap:	0.00	0.22	0.44	0.44	0.06	0.00	0.00	0.00	0.00	0.44	0.00	0.05
Delay/Veh:	0.0	33.7	36.1	14.9	4.0	0.0	0.0	0.0	0.0	33.9	0.0	3.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	33.7	36.1	14.9	4.0	0.0	0.0	0.0	0.0	33.9	0.0	3.2
LOS by Move:	A	C	D	B	A	A	A	A	A	C	A	A
HCM2k85thQ:	0	3	6	12	1	0	0	0	0	8	0	1

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #61 Santiago St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.723
Loss Time (sec):	5	Average Delay (sec/veh):	16.9
Optimal Cycle:	0	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	1 0	0 1	0	1 0	0 1	0	0 1	0 0	1	0 0	1! 0	0
Volume Module:												
Base Vol:	206	143	20	5	303	74	121	46	246	72	66	13
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	206	143	20	5	303	74	121	46	246	72	66	13
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	206	143	20	5	303	74	121	46	246	72	66	13
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	206	143	20	5	303	74	121	46	246	72	66	13
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	206	143	20	5	303	74	121	46	246	72	66	13
Saturation Flow Module:												
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	0.88	0.12	1.00	0.80	0.20	0.72	0.28	1.00	0.48	0.44	0.08
Final Sat.:	468	443	62	471	419	102	337	128	540	208	190	37
Capacity Analysis Module:												
Vol/Sat:	0.44	0.32	0.32	0.01	0.72	0.72	0.36	0.36	0.46	0.35	0.35	0.35
Crit Moves:	****				****				****		****	
Delay/Veh:	15.5	12.6	12.6	10.2	24.0	24.0	14.0	14.0	13.9	14.2	14.2	14.2
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	15.5	12.6	12.6	10.2	24.0	24.0	14.0	14.0	13.9	14.2	14.2	14.2
LOS by Move:	C	B	B	B	C	C	B	B	B	B	B	B
ApproachDel:	14.2			23.8			13.9			14.2		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	14.2			23.8			13.9			14.2		
LOS by Appr:	B			C			B			B		
AllWayAvgQ:	0.7	0.4	0.4	0.0	2.1	2.1	0.5	0.5	0.7	0.4	0.4	0.4

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #62 Santiago St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.553
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	26	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1 0	1	1 0	1 0	1	1 0	1 1	0	1 0	2 0	1
Volume Module:												
Base Vol:	24	71	56	311	161	133	48	474	14	108	807	271
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	24	71	56	311	161	133	48	474	14	108	807	271
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	24	71	56	311	161	133	48	474	14	108	807	271
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	24	71	56	311	161	133	48	474	14	108	807	271
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	24	71	56	311	161	133	48	474	14	108	807	271
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.03	1.00	1.00	1.06	1.00
Lanes:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.94	0.06	1.00	2.00	1.00
Final Sat.:	1600	1700	1600	1600	1700	1600	1600	3208	92	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.02	0.04	0.04	0.19	0.09	0.08	0.03	0.15	0.15	0.07	0.24	0.17
Crit Moves:		****			****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #64 Standard Ave and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.803
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	53	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0 1	0	0 0	1! 0	0	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	108	191	119	12	231	25	86	1317	214	64	1012	12
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	108	191	119	12	231	25	86	1317	214	64	1012	12
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	108	191	119	12	231	25	86	1317	214	64	1012	12
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	108	191	119	12	231	25	86	1317	214	64	1012	12
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	108	191	119	12	231	25	86	1317	214	64	1012	12
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.04	1.00	1.00	1.03	1.00
Lanes:	1.00	0.62	0.38	0.04	0.87	0.09	1.00	1.72	0.28	1.00	1.98	0.02
Final Sat.:	1600	986	614	72	1379	149	1600	2853	447	1600	3263	38
Capacity Analysis Module:												
Vol/Sat:	0.07	0.19	0.19	0.01	0.17	0.17	0.05	0.46	0.48	0.04	0.31	0.32
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #65 Standard Ave and Mcfadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.609
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	29	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	74	284	128	220	531	37	17	589	71	138	366	82
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	74	284	128	220	531	37	17	589	71	138	366	82
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	74	284	128	220	531	37	17	589	71	138	366	82
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	74	284	128	220	531	37	17	589	71	138	366	82
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	74	284	128	220	531	37	17	589	71	138	366	82
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	1.38	0.62	1.00	1.87	0.13	1.00	1.78	0.22	1.00	1.63	0.37
Final Sat.:	1600	2306	994	1600	3092	208	1600	2956	344	1600	2714	586
Capacity Analysis Module:												
Vol/Sat:	0.05	0.12	0.13	0.14	0.17	0.18	0.01	0.20	0.21	0.09	0.13	0.14
Crit Moves:			****	****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #66 Halladay St and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.659
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	33	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	0 1	0	0	0 0 0	0	0	1 1 0	0	1	1 0 0
Volume Module:												
Base Vol:	43	0	100	0	0	0	0	1283	145	160	607	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	43	0	100	0	0	0	0	1283	145	160	607	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	43	0	100	0	0	0	0	1283	145	160	607	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	43	0	100	0	0	0	0	1283	145	160	607	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	43	0	100	0	0	0	0	1283	145	160	607	0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.03	1.00	1.00	1.04	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	1.80	0.20	0.42	1.58	0.00
Final Sat.:	1600	0	1600	0	0	0	0	2975	325	668	2632	0
Capacity Analysis Module:												
Vol/Sat:	0.03	0.00	0.06	0.00	0.00	0.00	0.00	0.43	0.45	0.10	0.23	0.00
Crit Moves:			****						****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #67 Halladay St and Dyer Rd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.567
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	27	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	1 0	1 0	0	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	11	18	49	183	37	39	41	1375	10	115	626	101
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	11	18	49	183	37	39	41	1375	10	115	626	101
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	11	18	49	183	37	39	41	1375	10	115	626	101
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	11	18	49	183	37	39	41	1375	10	115	626	101
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	11	18	49	183	37	39	41	1375	10	115	626	101
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	0.27	0.73	1.00	0.49	0.51	1.00	2.98	0.02	1.00	2.58	0.42
Final Sat.:	1600	430	1170	1600	779	821	1600	4965	35	1600	4333	667
Capacity Analysis Module:												
Vol/Sat:	0.01	0.04	0.04	0.11	0.05	0.05	0.03	0.28	0.29	0.07	0.14	0.15
Crit Moves:			****	****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #68 SR-55 SB Ramps and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.615
Loss Time (sec):	5	Average Delay (sec/veh):	19.8
Optimal Cycle:	31	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Ignore			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	0	2	0	0	0	2	0	0	2	1	1	0	0	3	0	1
Volume Module:																		
Base Vol:	0	0	0	961	0	870	0	1441	1027	0	1275	145
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	961	0	870	0	1441	1027	0	1275	145
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Volume:	0	0	0	961	0	870	0	1441	0	0	1275	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	961	0	870	0	1441	0	0	1275	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
FinalVolume:	0	0	0	961	0	870	0	1441	0	0	1275	0
Saturation Flow Module:																		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.92	1.00	0.75	1.00	0.91	0.91	1.00	0.91	1.00
Lanes:	0.00	0.00	0.00	2.00	0.00	2.00	0.00	3.00	1.00	0.00	3.00	1.00
Final Sat.:	0	0	0	3502	0	2842	0	5187	1729	0	5187	1900
Capacity Analysis Module:																		
Vol/Sat:	0.00	0.00	0.00	0.27	0.00	0.31	0.00	0.28	0.00	0.00	0.25	0.00
Crit Moves:						****		****		****		
Green/Cycle:	0.00	0.00	0.00	0.50	0.00	0.50	0.00	0.45	0.00	0.00	0.45	0.00
Volume/Cap:	0.00	0.00	0.00	0.55	0.00	0.61	0.00	0.61	0.00	0.00	0.54	0.00
Delay/Veh:	0.0	0.0	0.0	17.7	0.0	19.0	0.0	21.3	0.0	0.0	20.2	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	17.7	0.0	19.0	0.0	21.3	0.0	0.0	20.2	0.0
LOS by Move:	A	A	A	B	A	B	A	C	A	A	C	A
HCM2k85thQ:	0	0	0	16	0	17	0	19	0	0	16	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #69 SR-55 NB Ramps and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.742
Loss Time (sec):	5	Average Delay (sec/veh):	19.2
Optimal Cycle:	44	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Ignore			Include			Ignore			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	0	0 1	0 0	0	0 0	0 0	2	0 2	0 0	3	0 1
Volume Module:												
Base Vol:	908	0	976	0	0	0	0	1609	796	0	517	244
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	908	0	976	0	0	0	0	1609	796	0	517	244
User Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Volume:	908	0	0	0	0	0	0	1609	0	0	517	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	908	0	0	0	0	0	0	1609	0	0	517	0
PCE Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
FinalVolume:	908	0	0	0	0	0	0	1609	0	0	517	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.88	1.00	0.91	1.00
Lanes:	2.00	0.00	1.00	0.00	0.00	0.00	0.00	2.00	2.00	0.00	3.00	1.00
Final Sat.:	3502	0	1900	0	0	0	0	3610	3344	0	5187	1900
Capacity Analysis Module:												
Vol/Sat:	0.26	0.00	0.00	0.00	0.00	0.00	0.00	0.45	0.00	0.00	0.10	0.00
Crit Moves:	****							****		****		
Green/Cycle:	0.35	0.00	0.00	0.00	0.00	0.00	0.00	0.60	0.00	0.00	0.60	0.00
Volume/Cap:	0.74	0.00	0.00	0.00	0.00	0.00	0.00	0.74	0.00	0.00	0.17	0.00
Delay/Veh:	31.1	0.0	0.0	0.0	0.0	0.0	0.0	15.8	0.0	0.0	8.9	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	31.1	0.0	0.0	0.0	0.0	0.0	0.0	15.8	0.0	0.0	8.9	0.0
LOS by Move:	C	A	A	A	A	A	A	B	A	A	A	A
HCM2k85thQ:	21	0	0	0	0	0	0	29	0	0	4	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #70 SR-55 SB Ramps and Dyer Rd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.552
Loss Time (sec):	5	Average Delay (sec/veh):	23.7
Optimal Cycle:	27	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Ovl			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 1	0	2	0 1	1	0 1	1 0	3	0 1	2 0	3	0 1
Volume Module:												
Base Vol:	221	26	505	50	242	60	35	1338	285	408	647	108
Growth Adj:	1.00 1.00	1.00		1.00 1.00	1.00		1.00 1.00	1.00		1.00 1.00	1.00	
Initial Bse:	221	26	505	50	242	60	35	1338	285	408	647	108
User Adj:	1.00 1.00	1.00		1.00 1.00	1.00		1.00 1.00	1.00		1.00 1.00	1.00	
PHF Adj:	1.00 1.00	1.00		1.00 1.00	1.00		1.00 1.00	1.00		1.00 1.00	1.00	
PHF Volume:	221	26	505	50	242	60	35	1338	285	408	647	108
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	221	26	505	50	242	60	35	1338	285	408	647	108
PCE Adj:	1.00 1.00	1.00		1.00 1.00	1.00		1.00 1.00	1.00		1.00 1.00	1.00	
MLF Adj:	1.00 1.00	1.00		1.00 1.00	1.00		1.00 1.00	1.00		1.00 1.00	1.00	
FinalVolume:	221	26	505	50	242	60	35	1338	285	408	647	108
Saturation Flow Module:												
Sat/Lane:	1900 1900	1900		1900 1900	1900		1900 1900	1900		1900 1900	1900	
Adjustment:	0.96 0.96	0.75		0.94 0.94	0.85		0.95 0.91	0.85		0.92 0.91	0.85	
Lanes:	1.79 0.21	2.00		0.34 1.66	1.00		1.00 3.00	1.00		2.00 3.00	1.00	
Final Sat.:	3254	383	2842	613	2968	1615	1805	5187	1615	3502	5187	1615
Capacity Analysis Module:												
Vol/Sat:	0.07 0.07	0.18		0.08 0.08	0.04		0.02 0.26	0.18		0.12 0.12	0.07	
Crit Moves:	****			****			****			****		
Green/Cycle:	0.12 0.12	0.33		0.15 0.15	0.15		0.09 0.47	0.47		0.21 0.59	0.59	
Volume/Cap:	0.55 0.55	0.53		0.55 0.55	0.25		0.21 0.55	0.38		0.55 0.21	0.11	
Delay/Veh:	42.7 42.7	27.5		40.8 40.8	38.3		42.7 19.4	17.5		36.1 9.7	9.2	
User DelAdj:	1.00 1.00	1.00		1.00 1.00	1.00		1.00 1.00	1.00		1.00 1.00	1.00	
AdjDel/Veh:	42.7 42.7	27.5		40.8 40.8	38.3		42.7 19.4	17.5		36.1 9.7	9.2	
LOS by Move:	D	D	C	D	D	D	D	B	B	D	A	A
HCM2k85thQ:	7	7	12	8	8	3	2	17	9	10	5	2

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #71 Glassell St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.603
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	29	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Ovl			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	1 0	1	1 0	1 1	0	1 0	1 0	1	1 0	1 1	0
Volume Module:												
Base Vol:	263	412	125	17	358	79	38	195	375	236	249	6
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	263	412	125	17	358	79	38	195	375	236	249	6
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	263	412	125	17	358	79	38	195	375	236	249	6
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	263	412	125	17	358	79	38	195	375	236	249	6
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	263	412	125	17	358	79	38	195	375	236	249	6
OvlAdjVol:			0						244			
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.03	1.00
Lanes:	2.00	1.00	1.00	1.00	1.64	0.36	1.00	1.00	1.00	1.00	1.95	0.05
Final Sat.:	3200	1700	1600	1600	2722	578	1600	1700	1600	1600	3225	75
Capacity Analysis Module:												
Vol/Sat:	0.08	0.24	0.08	0.01	0.13	0.14	0.02	0.11	0.23	0.15	0.08	0.08
OvlAdjV/S:			0.00						0.15			
Crit Moves:		****			****				****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #72 Glassell St and SR-22 WB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.741
Loss Time (sec):	5	Average Delay (sec/veh):	28.5
Optimal Cycle:	44	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	0	0 0	2 0	1	0 0	0 0	0	1 0	1! 0	1
Volume Module:												
Base Vol:	500	561	0	0	743	346	0	0	0	285	5	426
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	500	561	0	0	743	346	0	0	0	285	5	426
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	500	561	0	0	743	346	0	0	0	285	5	426
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	500	561	0	0	743	346	0	0	0	285	5	426
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	500	561	0	0	743	346	0	0	0	285	5	426
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.95	0.85	1.00	1.00	1.00	0.86	0.86	0.86
Lanes:	1.00	2.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	1.40	0.01	1.59
Final Sat.:	1805	3610	0	0	3610	1615	0	0	0	2270	23	2588
Capacity Analysis Module:												
Vol/Sat:	0.28	0.16	0.00	0.00	0.21	0.21	0.00	0.00	0.00	0.13	0.22	0.16
Crit Moves:	****				****						****	
Green/Cycle:	0.37	0.65	0.00	0.00	0.28	0.28	0.00	0.00	0.00	0.30	0.30	0.30
Volume/Cap:	0.74	0.24	0.00	0.00	0.74	0.77	0.00	0.00	0.00	0.42	0.74	0.55
Delay/Veh:	31.5	7.3	0.0	0.0	35.9	41.3	0.0	0.0	0.0	28.3	34.7	29.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	31.5	7.3	0.0	0.0	35.9	41.3	0.0	0.0	0.0	28.3	34.7	29.9
LOS by Move:	C	A	A	A	D	D	A	A	A	C	C	C
HCM2k85thQ:	22	6	0	0	19	17	0	0	0	8	17	11

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #73 Grand Ave / Glassell St and SR-22 EB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.760
Loss Time (sec):	5	Average Delay (sec/veh):	27.0
Optimal Cycle:	47	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 2	0 1	1	0 2	0 0	1	0 1!	0 1	0	0 0	0 0
Volume Module:												
Base Vol:	0	818	218	280	733	0	258	0	1012	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	818	218	280	733	0	258	0	1012	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	818	218	280	733	0	258	0	1012	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	818	218	280	733	0	258	0	1012	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	818	218	280	733	0	258	0	1012	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.95	0.85	0.95	0.95	1.00	0.87	1.00	0.87	1.00	1.00	1.00
Lanes:	0.00	2.00	1.00	1.00	2.00	0.00	1.20	0.00	1.80	0.00	0.00	0.00
Final Sat.:	0	3610	1615	1805	3610	0	1992	0	2974	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.23	0.13	0.16	0.20	0.00	0.13	0.00	0.34	0.00	0.00	0.00
Crit Moves:		****		****					****			
Green/Cycle:	0.00	0.30	0.30	0.20	0.50	0.00	0.45	0.00	0.45	0.00	0.00	0.00
Volume/Cap:	0.00	0.76	0.45	0.76	0.40	0.00	0.29	0.00	0.76	0.00	0.00	0.00
Delay/Veh:	0.0	35.0	29.1	46.4	15.7	0.0	17.6	0.0	25.2	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	35.0	29.1	46.4	15.7	0.0	17.6	0.0	25.2	0.0	0.0	0.0
LOS by Move:	A	D	C	D	B	A	B	A	C	A	A	A
HCM2k85thQ:	0	20	9	15	11	0	7	0	23	0	0	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #74 Grand Ave and Fairhaven Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.680
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	35	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	0 1	1 0	1	1 0	1 0	1	0 1
Volume Module:												
Base Vol:	18	737	106	174	1444	32	99	60	59	254	28	229
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	18	737	106	174	1444	32	99	60	59	254	28	229
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	18	737	106	174	1444	32	99	60	59	254	28	229
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	18	737	106	174	1444	32	99	60	59	254	28	229
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	18	737	106	174	1444	32	99	60	59	254	28	229
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	2.62	0.38	1.00	2.00	1.00	1.00	1.01	0.99	1.00	1.00	1.00
Final Sat.:	1600	4396	604	1600	3400	1600	1600	1713	1587	1600	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.01	0.17	0.18	0.11	0.42	0.02	0.06	0.04	0.04	0.16	0.02	0.14
Crit Moves:	****			****			****			****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #75 Grand Ave and Santa Clara Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.885
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	79	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			WideBypass			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	1 1	0	1 0	0 1	0	1 0	1 0	1
Volume Module:												
Base Vol:	66	463	119	290	1418	319	143	233	48	120	231	211
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	66	463	119	290	1418	319	143	233	48	120	231	211
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	66	463	119	290	1418	319	143	233	48	120	231	211
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	66	463	119	290	1418	319	143	233	48	120	231	211
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	66	463	119	290	1418	319	143	233	48	120	231	211
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.00	1.00	1.00	1.06	1.00
Lanes:	1.00	2.39	0.61	1.00	1.63	0.37	1.00	0.83	0.17	1.00	1.00	1.00
Final Sat.:	1600	4019	981	1600	2712	588	1600	1327	273	1600	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.04	0.12	0.12	0.18	0.52	0.54	0.09	0.18	0.18	0.08	0.14	0.13
Crit Moves:	****					****		****		****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #76 Grand Ave and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.797
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	51	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	0 1	2 0	2	1 0	2 0	2	1 0
Volume Module:												
Base Vol:	163	476	327	286	1198	235	224	835	106	309	807	88
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	163	476	327	286	1198	235	224	835	106	309	807	88
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	163	476	327	286	1198	235	224	835	106	309	807	88
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	163	476	327	286	1198	235	224	835	106	309	807	88
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	163	476	327	286	1198	235	224	835	106	309	807	88
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	2.00	2.66	0.34	2.00	2.71	0.29
Final Sat.:	1600	3400	1600	1600	3400	1600	3200	4459	541	3200	4528	472
Capacity Analysis Module:												
Vol/Sat:	0.10	0.14	0.20	0.18	0.35	0.15	0.07	0.19	0.20	0.10	0.18	0.19
Crit Moves:	****				****				****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #77 Grand Ave and I-5 NB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.516
Loss Time (sec):	5	Average Delay (sec/veh):	12.9
Optimal Cycle:	26	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Ignore			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	2 0	1	1 0	3 0	0	0 0	0 0	0 0	2 0	0 0	1
Volume Module:												
Base Vol:	0	831	480	58	1869	0	0	0	0	456	0	126
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	831	480	58	1869	0	0	0	0	456	0	126
User Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	831	0	58	1869	0	0	0	0	456	0	126
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	831	0	58	1869	0	0	0	0	456	0	126
PCE Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	831	0	58	1869	0	0	0	0	456	0	126
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.95	1.00	0.95	0.91	1.00	1.00	1.00	1.00	0.92	1.00	0.85
Lanes:	0.00	2.00	1.00	1.00	3.00	0.00	0.00	0.00	0.00	2.00	0.00	1.00
Final Sat.:	0	3610	1900	1805	5187	0	0	0	0	3502	0	1615
Capacity Analysis Module:												
Vol/Sat:	0.00	0.23	0.00	0.03	0.36	0.00	0.00	0.00	0.00	0.13	0.00	0.08
Crit Moves:	****				****					****		
Green/Cycle:	0.00	0.61	0.00	0.09	0.70	0.00	0.00	0.00	0.00	0.25	0.00	0.25
Volume/Cap:	0.00	0.38	0.00	0.38	0.52	0.00	0.00	0.00	0.00	0.52	0.00	0.31
Delay/Veh:	0.0	9.9	0.0	44.7	7.3	0.0	0.0	0.0	0.0	32.7	0.0	30.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	9.9	0.0	44.7	7.3	0.0	0.0	0.0	0.0	32.7	0.0	30.8
LOS by Move:	A	A	A	D	A	A	A	A	A	C	A	C
HCM2k85thQ:	0	10	0	3	15	0	0	0	0	10	0	5

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #78 Grand Ave and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.662
Loss Time (sec):	5	Average Delay (sec/veh):	23.8
Optimal Cycle:	35	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 0	2	2 0	1 0	2	0 1	0 1	0
Volume Module:												
Base Vol:	71	1003	51	141	1166	1029	263	217	428	5	373	34
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	71	1003	51	141	1166	1029	263	217	428	5	373	34
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	71	1003	51	141	1166	1029	263	217	428	5	373	34
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	71	1003	51	141	1166	1029	263	217	428	5	373	34
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	71	1003	51	141	1166	1029	263	217	428	5	373	34
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.90	0.90	0.95	0.95	0.75	0.92	1.00	0.75	0.94	0.94	0.94
Lanes:	1.00	2.85	0.15	1.00	2.00	2.00	2.00	1.00	2.00	0.02	1.81	0.17
Final Sat.:	1805	4901	249	1805	3610	2842	3502	1900	2842	43	3226	294
Capacity Analysis Module:												
Vol/Sat:	0.04	0.20	0.20	0.08	0.32	0.36	0.08	0.11	0.15	0.12	0.12	0.12
Crit Moves:	****				****				****			****
Green/Cycle:	0.06	0.40	0.40	0.15	0.49	0.72	0.23	0.23	0.23	0.17	0.17	0.17
Volume/Cap:	0.66	0.52	0.52	0.52	0.66	0.51	0.33	0.50	0.66	0.66	0.66	0.66
Delay/Veh:	60.4	23.1	23.1	40.8	20.3	6.5	32.5	34.6	37.7	41.2	41.2	41.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	60.4	23.1	23.1	40.8	20.3	6.5	32.5	34.6	37.7	41.2	41.2	41.2
LOS by Move:	E	C	C	D	C	A	C	C	D	D	D	D
HCM2k85thQ:	5	14	14	7	22	13	6	10	12	11	11	11

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #79 Grand Ave and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.803
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	53	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	3 0	1	2 0	2 1	0	2 0	1 1	0
Volume Module:												
Base Vol:	213	751	35	99	1432	174	332	793	225	247	867	99
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	213	751	35	99	1432	174	332	793	225	247	867	99
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	213	751	35	99	1432	174	332	793	225	247	867	99
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	213	751	35	99	1432	174	332	793	225	247	867	99
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	213	751	35	99	1432	174	332	793	225	247	867	99
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.03	1.00
Lanes:	2.00	2.87	0.13	2.00	3.00	1.00	2.00	2.34	0.66	2.00	1.80	0.20
Final Sat.:	3200	4786	214	3200	5100	1600	3200	3939	1061	3200	2972	328
Capacity Analysis Module:												
Vol/Sat:	0.07	0.16	0.16	0.03	0.28	0.11	0.10	0.20	0.21	0.08	0.29	0.30
Crit Moves:	****				****		****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #80 Grand Ave and Chestnut Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.705
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	38	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	53	737	163	149	1788	49	60	419	110	118	347	216
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	53	737	163	149	1788	49	60	419	110	118	347	216
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	53	737	163	149	1788	49	60	419	110	118	347	216
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	53	737	163	149	1788	49	60	419	110	118	347	216
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	53	737	163	149	1788	49	60	419	110	118	347	216
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	2.46	0.54	1.00	2.92	0.08	1.00	1.58	0.42	1.00	1.23	0.77
Final Sat.:	1600	4131	869	1600	4872	128	1600	2635	665	1600	2072	1228
Capacity Analysis Module:												
Vol/Sat:	0.03	0.18	0.19	0.09	0.37	0.38	0.04	0.16	0.17	0.07	0.17	0.18
Crit Moves:	****					****			****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #81 Grand Ave and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.829
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	59	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	171	722	108	237	1590	155	186	601	182	117	420	106
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	171	722	108	237	1590	155	186	601	182	117	420	106
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	171	722	108	237	1590	155	186	601	182	117	420	106
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	171	722	108	237	1590	155	186	601	182	117	420	106
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	171	722	108	237	1590	155	186	601	182	117	420	106
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	2.61	0.39	1.00	2.73	0.27	1.00	1.54	0.46	1.00	1.60	0.40
Final Sat.:	1600	4375	625	1600	4574	426	1600	2556	744	1600	2655	645
Capacity Analysis Module:												
Vol/Sat:	0.11	0.17	0.17	0.15	0.35	0.36	0.12	0.24	0.24	0.07	0.16	0.16
Crit Moves:	****					****		****		****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #82 Grand Ave and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.763
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	45	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	47	472	92	224	1146	224	319	1332	128	151	579	126
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	47	472	92	224	1146	224	319	1332	128	151	579	126
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	47	472	92	224	1146	224	319	1332	128	151	579	126
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	47	472	92	224	1146	224	319	1332	128	151	579	126
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	47	472	92	224	1146	224	319	1332	128	151	579	126
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.51	0.49	1.00	2.51	0.49	1.00	2.74	0.26	1.00	2.46	0.54
Final Sat.:	1600	4217	783	1600	4215	785	1600	4579	421	1600	4142	858
Capacity Analysis Module:												
Vol/Sat:	0.03	0.11	0.12	0.14	0.27	0.29	0.20	0.29	0.30	0.09	0.14	0.15
Crit Moves:	****					****			****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #83 Grand Ave and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.528
Loss Time (sec):	0	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	48	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	2 0	3 0	1	2 0	2 0	1
Volume Module:												
Base Vol:	169	510	218	199	574	194	256	914	359	88	388	131
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	169	510	218	199	574	194	256	914	359	88	388	131
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	169	510	218	199	574	194	256	914	359	88	388	131
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	169	510	218	199	574	194	256	914	359	88	388	131
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	169	510	218	199	574	194	256	914	359	88	388	131
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	2.10	0.90	1.00	2.24	0.76	2.00	3.00	1.00	2.00	2.00	1.00
Final Sat.:	1600	3563	1437	1600	3788	1213	3200	5100	1600	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.11	0.14	0.15	0.12	0.15	0.16	0.08	0.18	0.22	0.03	0.11	0.08
Crit Moves:		****	****						****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #84 SR-55 NB Ramps and Dyer Rd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.530
Loss Time (sec):	5	Average Delay (sec/veh):	16.3
Optimal Cycle:	26	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Ignore			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	1! 0	1	0	0	0	0	0	3	0	1	0	0	2	1	0
Volume Module:												
Base Vol:	557	0	456	0	0	0	0	1533	613	0	719	433
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	557	0	456	0	0	0	0	1533	613	0	719	433
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Volume:	557	0	456	0	0	0	0	1533	0	0	719	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	557	0	456	0	0	0	0	1533	0	0	719	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
FinalVolume:	557	0	456	0	0	0	0	1533	0	0	719	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.91	1.00	0.91	1.00	1.00	1.00	1.00	0.91	1.00	1.00	0.91	0.91
Lanes:	1.55	0.00	1.45	0.00	0.00	0.00	0.00	3.00	1.00	0.00	3.00	0.00
Final Sat.:	2673	0	2501	0	0	0	0	5187	1900	0	5187	0
Capacity Analysis Module:												
Vol/Sat:	0.21	0.00	0.18	0.00	0.00	0.00	0.00	0.30	0.00	0.00	0.14	0.00
Crit Moves:	****							****		****		
Green/Cycle:	0.39	0.00	0.39	0.00	0.00	0.00	0.00	0.56	0.00	0.00	0.56	0.00
Volume/Cap:	0.53	0.00	0.46	0.00	0.00	0.00	0.00	0.53	0.00	0.00	0.25	0.00
Delay/Veh:	23.6	0.0	22.7	0.0	0.0	0.0	0.0	14.1	0.0	0.0	11.4	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	23.6	0.0	22.7	0.0	0.0	0.0	0.0	14.1	0.0	0.0	11.4	0.0
LOS by Move:	C	A	C	A	A	A	A	B	A	A	B	A
HCM2k85thQ:	13	0	11	0	0	0	0	16	0	0	6	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #85 Cambridge St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.799
Loss Time (sec):	5	Average Delay (sec/veh):	17.3
Optimal Cycle:	0	Level Of Service:	C

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	T	R		L	T	R		L	T	R		L	T	R	
Control:	Stop Sign				Stop Sign				Stop Sign				Stop Sign			
Rights:	Include				Include				Include				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Lanes:	1	0	1	0	0	0	0	1	0	1	0	0	0	1	0	0
Volume Module:																
Base Vol:	160	204	0		0	480	245		117	0	187		0	0	0	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	160	204	0		0	480	245		117	0	187		0	0	0	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	160	204	0		0	480	245		117	0	187		0	0	0	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	160	204	0		0	480	245		117	0	187		0	0	0	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	160	204	0		0	480	245		117	0	187		0	0	0	
Saturation Flow Module:																
Adjustment:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Lanes:	1.00	1.00	0.00		0.00	1.00	1.00		1.00	0.00	1.00		0.00	0.00	0.00	
Final Sat.:	514	554	0		0	601	671		467	0	553		0	0	0	
Capacity Analysis Module:																
Vol/Sat:	0.31	0.37	xxxx		xxxx	0.80	0.37		0.25	xxxx	0.34		xxxx	xxxx	xxxx	
Crit Moves:		****				****					****					
Delay/Veh:	12.5	12.5	0.0		0.0	27.6	10.8		12.4	0.0	11.8		0.0	0.0	0.0	
Delay Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
AdjDel/Veh:	12.5	12.5	0.0		0.0	27.6	10.8		12.4	0.0	11.8		0.0	0.0	0.0	
LOS by Move:	B	B	*		*	D	B		B	*	B		*	*	*	
ApproachDel:	12.5				21.9				12.0				xxxxxx			
Delay Adj:	1.00				1.00				1.00				xxxxx			
ApprAdjDel:	12.5				21.9				12.0				xxxxxx			
LOS by Appr:	B				C				B				*			
AllWayAvgQ:	0.4	0.5	0.0		0.0	3.2	0.5		0.3	0.0	0.4		0.0	0.0	0.0	

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #86 Cambridge St and Fairhaven Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.559
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	26	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	0 0	0	1 0	0 0	1	1 0	2 0	0	0 0	1 1	0
Volume Module:												
Base Vol:	0	0	0	515	0	166	80	216	0	0	247	192
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	515	0	166	80	216	0	0	247	192
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	515	0	166	80	216	0	0	247	192
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	515	0	166	80	216	0	0	247	192
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	515	0	166	80	216	0	0	247	192
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	0.00	0.00	0.00	1.00	0.00	1.00	1.00	2.00	0.00	0.00	1.13	0.87
Final Sat.:	0	0	0	1600	0	1600	1600	3400	0	0	1900	1400
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.32	0.00	0.10	0.05	0.06	0.00	0.00	0.13	0.14
Crit Moves:				****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #87 Mabury St and 1st Street

Cycle (sec):	100	Critical Vol./Cap.(X):	0.691
Loss Time (sec):	0	Average Delay (sec/veh):	28.3
Optimal Cycle:	74	Level Of Service:	C

Street Name:	Mabury						1st Street					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	0 1	1 0	1!	0 1	0 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	10	0	244	238	141	533	0	1062	11	46	701	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	10	0	244	238	141	533	0	1062	11	46	701	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	10	0	244	238	141	533	0	1062	11	46	701	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	10	0	244	238	141	533	0	1062	11	46	701	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	10	0	244	238	141	533	0	1062	11	46	701	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	0.85	0.90	0.90	0.90	1.00	0.91	0.91	0.95	0.91	0.91
Lanes:	1.00	0.00	1.00	1.22	0.27	1.51	0.00	2.97	0.03	1.00	3.00	0.00
Final Sat.:	1805	0	1615	2097	458	2576	0	5129	53	1805	5187	0
Capacity Analysis Module:												
Vol/Sat:	0.01	0.00	0.15	0.11	0.31	0.21	0.00	0.21	0.21	0.03	0.14	0.00
Crit Moves:			****		****			****		****		
Green/Cycle:	0.22	0.00	0.22	0.45	0.45	0.45	0.00	0.30	0.30	0.04	0.34	0.00
Volume/Cap:	0.03	0.00	0.69	0.25	0.69	0.46	0.00	0.69	0.69	0.69	0.40	0.00
Delay/Veh:	30.7	0.0	41.8	17.4	23.8	19.6	0.0	32.3	32.3	74.4	25.6	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	30.7	0.0	41.8	17.4	23.8	19.6	0.0	32.3	32.3	74.4	25.6	0.0
LOS by Move:	C	A	D	B	C	B	A	C	C	E	C	A
HCM2k85thQ:	0	0	13	6	21	12	0	18	18	4	9	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #88 Tustin St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.521
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	25	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 1	0	0 1
Volume Module:												
Base Vol:	8	333	96	74	1452	4	9	11	35	447	2	69
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	8	333	96	74	1452	4	9	11	35	447	2	69
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	8	333	96	74	1452	4	9	11	35	447	2	69
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	8	333	96	74	1452	4	9	11	35	447	2	69
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	8	333	96	74	1452	4	9	11	35	447	2	69
OvlAdjVol:												0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	1.00	2.33	0.67	1.00	2.99	0.01	1.00	1.00	1.00	1.99	0.01	1.00
Final Sat.:	1600	3926	1074	1600	4987	13	1600	1700	1600	3186	14	1600
Capacity Analysis Module:												
Vol/Sat:	0.01	0.08	0.09	0.05	0.29	0.30	0.01	0.01	0.02	0.14	0.14	0.04
OvlAdjV/S:												0.00
Crit Moves:	****					****			****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #89 Tustin St and SR-22 WB On-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.734
Loss Time (sec):	5	Average Delay (sec/veh):	12.9
Optimal Cycle:	43	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0 2	0 0	0	0 2	1 0	0	0 0	0 0	0	0 0	0 0
Volume Module:												
Base Vol:	656	610	0	0	1395	833	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	656	610	0	0	1395	833	0	0	0	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	656	610	0	0	1395	833	0	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	656	610	0	0	1395	833	0	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	656	610	0	0	1395	833	0	0	0	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.95	1.00	1.00	0.86	0.86	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	2.00	2.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00
Final Sat.:	3502	3610	0	0	3264	1632	0	0	0	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.19	0.17	0.00	0.00	0.43	0.51	0.00	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****					****						
Green/Cycle:	0.26	0.95	0.00	0.00	0.69	0.69	0.00	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.73	0.18	0.00	0.00	0.61	0.73	0.00	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	37.3	0.2	0.0	0.0	8.4	10.5	0.0	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	37.3	0.2	0.0	0.0	8.4	10.5	0.0	0.0	0.0	0.0	0.0	0.0
LOS by Move:	D	A	A	A	A	B	A	A	A	A	A	A
HCM2k85thQ:	17	1	0	0	19	27	0	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #90 Tustin St and SR-22 EB Off-Ramp / Seba Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.731
Loss Time (sec):	5	Average Delay (sec/veh):	24.2
Optimal Cycle:	43	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 2	1 0	1	0 3	0 0	1	0 1!	0 1	0	0 1!	0 0
Volume Module:												
Base Vol:	0	996	5	22	1349	0	244	24	1051	6	0	43
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	996	5	22	1349	0	244	24	1051	6	0	43
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	996	5	22	1349	0	244	24	1051	6	0	43
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	996	5	22	1349	0	244	24	1051	6	0	43
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	996	5	22	1349	0	244	24	1051	6	0	43
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	0.91	0.16	0.91	1.00	0.87	0.87	0.87	0.88	1.00	0.88
Lanes:	0.00	2.99	0.01	1.00	3.00	0.00	1.18	0.04	1.78	0.12	0.00	0.88
Final Sat.:	0	5156	26	304	5187	0	1958	59	2954	204	0	1460
Capacity Analysis Module:												
Vol/Sat:	0.00	0.19	0.19	0.07	0.26	0.00	0.12	0.41	0.36	0.03	0.00	0.03
Crit Moves:					****			****		****		
Green/Cycle:	0.00	0.36	0.36	0.36	0.36	0.00	0.55	0.55	0.55	0.04	0.00	0.04
Volume/Cap:	0.00	0.54	0.54	0.20	0.73	0.00	0.22	0.73	0.64	0.73	0.00	0.73
Delay/Veh:	0.0	26.1	26.1	23.3	29.6	0.0	11.4	18.3	16.1	80.9	0.0	80.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	26.1	26.1	23.3	29.6	0.0	11.4	18.3	16.1	80.9	0.0	80.9
LOS by Move:	A	C	C	C	C	A	B	B	B	F	A	F
HCM2k85thQ:	0	14	14	1	21	0	5	24	20	5	0	5

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #91 Tustin Ave and Fairhaven Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.977
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	178	Level Of Service:	E

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	- T	- R		L	- T	- R		L	- T	- R		L	- T	- R	
Control:	Protected				Protected				Protected				Protected			
Rights:	Include				Include				Include				Ovl			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Y+R:	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0	
Lanes:	2	0 2	1	0	2	0 2	1	0	1	0 1	1	0	1	0 1	0	1
Volume Module:																
Base Vol:	89	514	90		235	2026	40		68	258	444		306	338	442	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	89	514	90		235	2026	40		68	258	444		306	338	442	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	89	514	90		235	2026	40		68	258	444		306	338	442	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	89	514	90		235	2026	40		68	258	444		306	338	442	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	89	514	90		235	2026	40		68	258	444		306	338	442	
OvlAdjVol:															325	
Saturation Flow Module:																
Sat/Lane:	1600	1600	1600		1600	1600	1600		1600	1600	1600		1600	1600	1600	
Adjustment:	1.00	1.05	1.00		1.00	1.04	1.00		1.00	1.06	1.00		1.00	1.06	1.00	
Lanes:	2.00	2.55	0.45		2.00	2.94	0.06		1.00	1.00	1.00		1.00	1.00	1.00	
Final Sat.:	3200	4285	715		3200	4907	93		1600	1700	1600		1600	1700	1600	
Capacity Analysis Module:																
Vol/Sat:	0.03	0.12	0.13		0.07	0.41	0.43		0.04	0.15	0.28		0.19	0.20	0.28	
OvlAdjV/S:															0.20	
Crit Moves:	****						****				****	****				

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #92 Tustin Ave and Santa Clara Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.866
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	71	Level Of Service:	D

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	- T	- R		L	- T	- R		L	- T	- R		L	- T	- R	
Control:	Protected				Protected				Permitted				Permitted			
Rights:	Include				Include				Include				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Y+R:	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0	
Lanes:	1	0 2	1	0	2	0 2	1	0	1	0 1	1	0	1	0 1	1	0
Volume Module:																
Base Vol:	73	397	74		127	2600	82		101	255	186		118	244	122	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	73	397	74		127	2600	82		101	255	186		118	244	122	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	73	397	74		127	2600	82		101	255	186		118	244	122	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	73	397	74		127	2600	82		101	255	186		118	244	122	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	73	397	74		127	2600	82		101	255	186		118	244	122	
Saturation Flow Module:																
Sat/Lane:	1600	1600	1600		1600	1600	1600		1600	1600	1600		1600	1600	1600	
Adjustment:	1.00	1.05	1.00		1.00	1.04	1.00		1.00	1.05	1.00		1.00	1.05	1.00	
Lanes:	1.00	2.53	0.47		2.00	2.91	0.09		1.00	1.16	0.84		1.00	1.33	0.67	
Final Sat.:	1600	4246	754		3200	4853	147		1600	1950	1350		1600	2233	1067	
Capacity Analysis Module:																
Vol/Sat:	0.05	0.09	0.10		0.04	0.54	0.56		0.06	0.13	0.14		0.07	0.11	0.11	
Crit Moves:	****						****				****	****				

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #93 Tustin Ave and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.748
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	43	Level Of Service:	C

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	- T	- R		L	- T	- R		L	- T	- R		L	- T	- R	
Control:	Protected				Protected				Protected				Protected			
Rights:	Ovl				Include				Include				Ovl			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Y+R:	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0	
Lanes:	2	0 2	0	1	2	0 2	1	0	2	0 3	0	1	2	0 3	0	1
Volume Module:																
Base Vol:	111	198	236		602	1737	19		187	738	126		488	770	188	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	111	198	236		602	1737	19		187	738	126		488	770	188	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	111	198	236		602	1737	19		187	738	126		488	770	188	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	111	198	236		602	1737	19		187	738	126		488	770	188	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	111	198	236		602	1737	19		187	738	126		488	770	188	
OvlAdjVol:			0												0	
Saturation Flow Module:																
Sat/Lane:	1600	1600	1600		1600	1600	1600		1600	1600	1600		1600	1600	1600	
Adjustment:	1.00	1.06	1.00		1.00	1.04	1.00		1.00	1.06	1.00		1.00	1.06	1.00	
Lanes:	2.00	2.00	1.00		2.00	2.97	0.03		2.00	3.00	1.00		2.00	3.00	1.00	
Final Sat.:	3200	3400	1600		3200	4948	52		3200	5100	1600		3200	5100	1600	
Capacity Analysis Module:																
Vol/Sat:	0.03	0.06	0.15		0.19	0.35	0.37		0.06	0.14	0.08		0.15	0.15	0.12	
OvlAdjV/S:			0.00												0.00	
Crit Moves:	****						****			****			****			

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #94 Tustin Ave and 4th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.701
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	37	Level Of Service:	C

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	- T	- R		L	- T	- R		L	- T	- R		L	- T	- R	
Control:	Protected				Protected				Protected				Protected			
Rights:	Include				Include				Include				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Y+R:	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0	
Lanes:	1	0 2	1	0	2	0 2	0	1	1	0 2	1	0	1	0 2	1	0
Volume Module:																
Base Vol:	48	245	219		664	845	262		114	676	28		103	579	376	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	48	245	219		664	845	262		114	676	28		103	579	376	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	48	245	219		664	845	262		114	676	28		103	579	376	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	48	245	219		664	845	262		114	676	28		103	579	376	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	48	245	219		664	845	262		114	676	28		103	579	376	
Saturation Flow Module:																
Sat/Lane:	1600	1600	1600		1600	1600	1600		1600	1600	1600		1600	1600	1600	
Adjustment:	1.00	1.06	1.00		1.00	1.06	1.00		1.00	1.04	1.00		1.00	1.06	1.00	
Lanes:	1.00	2.00	1.00		2.00	2.00	1.00		1.00	2.88	0.12		1.00	2.00	1.00	
Final Sat.:	1600	3400	1600		3200	3400	1600		1600	4809	191		1600	3400	1600	
Capacity Analysis Module:																
Vol/Sat:	0.03	0.07	0.14		0.21	0.25	0.16		0.07	0.14	0.15		0.06	0.17	0.24	
Crit Moves:			****		****				****						****	

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #95 SR-55 SB Ramps / Auto Mall Dr and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.558
Loss Time (sec):	5	Average Delay (sec/veh):	24.4
Optimal Cycle:	28	Level Of Service:	C

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	- T	- R		L	- T	- R		L	- T	- R		L	- T	- R	
Control:	Split Phase				Split Phase				Protected				Protected			
Rights:	Include				Include				Ignore				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Y+R:	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0	
Lanes:	1	1 0	0	2	1	0 1	0	1	1	0 3	0	1	2	0 2	1	0
Volume Module:																
Base Vol:	421	63	498		50	30	8		28	1104	408		399	940	123	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	421	63	498		50	30	8		28	1104	408		399	940	123	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	0.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	0.00		1.00	1.00	1.00	
PHF Volume:	421	63	498		50	30	8		28	1104	0		399	940	123	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	421	63	498		50	30	8		28	1104	0		399	940	123	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	0.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	0.00		1.00	1.00	1.00	
FinalVolume:	421	63	498		50	30	8		28	1104	0		399	940	123	
Saturation Flow Module:																
Sat/Lane:	1900	1900	1900		1900	1900	1900		1900	1900	1900		1900	1900	1900	
Adjustment:	0.96	0.96	0.75		0.95	1.00	0.85		0.95	0.91	1.00		0.92	0.89	0.89	
Lanes:	1.74	0.26	2.00		1.00	1.00	1.00		1.00	3.00	1.00		2.00	2.65	0.35	
Final Sat.:	3167	474	2842		1805	1900	1615		1805	5187	1900		3502	4509	590	
Capacity Analysis Module:																
Vol/Sat:	0.13	0.13	0.18		0.03	0.02	0.00		0.02	0.21	0.00		0.11	0.21	0.21	
Crit Moves:			****		****					****			****			
Green/Cycle:	0.31	0.31	0.31		0.05	0.05	0.05		0.04	0.38	0.00		0.20	0.55	0.55	
Volume/Cap:	0.42	0.42	0.56		0.56	0.32	0.10		0.38	0.56	0.00		0.56	0.38	0.38	
Delay/Veh:	27.4	27.4	29.3		54.0	47.8	45.9		50.1	24.6	0.0		36.7	13.1	13.1	
User DelAdj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
AdjDel/Veh:	27.4	27.4	29.3		54.0	47.8	45.9		50.1	24.6	0.0		36.7	13.1	13.1	
LOS by Move:	C	C	C		D	D	D		D	C	A		D	B	B	
HCM2k85thQ:	9	9	12		4	2	1		2	15	0		10	11	11	

Note: Queue reported is the number of cars per lane.
****

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #96 SR-55 NB Ramps / Del Amo Ave and Newport Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	0.463
Loss Time (sec):	5	Average Delay (sec/veh):	24.0
Optimal Cycle:	23	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	1 0	3 0	1	2 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	227	60	38	14	344	504	253	153	146	8	223	4
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	227	60	38	14	344	504	253	153	146	8	223	4
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	227	60	38	14	344	504	253	153	146	8	223	4
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	227	60	38	14	344	504	253	153	146	8	223	4
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	227	60	38	14	344	504	253	153	146	8	223	4
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.86	0.86	0.95	0.91	0.85	0.92	0.88	0.88	0.95	0.95	0.95
Lanes:	2.00	2.00	1.00	1.00	3.00	1.00	2.00	1.02	0.98	1.00	1.96	0.04
Final Sat.:	3502	3257	1629	1805	5187	1615	3502	1712	1634	1805	3536	63
Capacity Analysis Module:												
Vol/Sat:	0.06	0.02	0.02	0.01	0.07	0.31	0.07	0.09	0.09	0.00	0.06	0.06
Crit Moves:	****					****	****				****	
Green/Cycle:	0.14	0.49	0.49	0.16	0.52	0.67	0.16	0.28	0.28	0.01	0.14	0.14
Volume/Cap:	0.46	0.04	0.05	0.05	0.13	0.46	0.46	0.32	0.32	0.32	0.46	0.46
Delay/Veh:	40.2	13.1	13.1	35.3	12.5	8.0	39.0	28.8	28.8	56.2	40.5	40.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	40.2	13.1	13.1	35.3	12.5	8.0	39.0	28.8	28.8	56.2	40.5	40.5
LOS by Move:	D	B	B	D	B	A	D	C	C	E	D	D
HCM2k85thQ:	6	1	1	1	3	11	6	6	6	1	6	6

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #97 Red Hill Ave and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.569
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	27	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Ovl			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	81	299	85	226	960	450	190	611	130	224	1085	152
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	81	299	85	226	960	450	190	611	130	224	1085	152
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	81	299	85	226	960	450	190	611	130	224	1085	152
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	81	299	85	226	960	450	190	611	130	224	1085	152
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	81	299	85	226	960	450	190	611	130	224	1085	152
OvlAdjVol:						355			89			
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.06	0.05	0.07	0.19	0.28	0.06	0.12	0.08	0.07	0.21	0.10
OvlAdjV/S:						0.22			0.06			
Crit Moves:	****					****	****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #98 Red Hill Ave and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.485
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	23	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0 3	1 0	2	0 2	1 0	1	0 2	1 0	2	0 3	0 1
Volume Module:												
Base Vol:	56	339	46	19	891	179	202	343	253	98	349	62
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	56	339	46	19	891	179	202	343	253	98	349	62
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	56	339	46	19	891	179	202	343	253	98	349	62
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	56	339	46	19	891	179	202	343	253	98	349	62
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	56	339	46	19	891	179	202	343	253	98	349	62
OvlAdjVol:												53
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.52	0.48	2.00	2.50	0.50	1.00	2.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5935	765	3200	4197	803	1600	3400	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.02	0.06	0.06	0.01	0.21	0.22	0.13	0.10	0.16	0.03	0.07	0.04
OvlAdjV/S:												0.03
Crit Moves:	****					****	****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #99 Red Hill Ave and Dyer Rd / barranca Pkwy

Cycle (sec):	100	Critical Vol./Cap.(X):	0.575
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	27	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	4 0	1	2 0	4 0	1	2 0	3 1	0	2 0	4 0	1
Volume Module:												
Base Vol:	127	316	104	199	876	115	118	710	281	578	645	126
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	127	316	104	199	876	115	118	710	281	578	645	126
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	127	316	104	199	876	115	118	710	281	578	645	126
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	127	316	104	199	876	115	118	710	281	578	645	126
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	127	316	104	199	876	115	118	710	281	578	645	126
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	4.00	1.00	2.00	4.00	1.00	2.00	3.00	1.00	2.00	4.00	1.00
Final Sat.:	3200	6800	1600	3200	6800	1600	3200	5100	1600	3200	6800	1600
Capacity Analysis Module:												
Vol/Sat:	0.04	0.05	0.07	0.06	0.13	0.07	0.04	0.14	0.18	0.18	0.09	0.08
Crit Moves:	****				****				****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #100 Red Hill Ave and Alton Pkwy

Cycle (sec):	100	Critical Vol./Cap.(X):	0.524
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	25	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	2 0	1	2 0	1 0	1
Volume Module:												
Base Vol:	174	512	321	251	1141	183	9	28	28	223	189	106
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	174	512	321	251	1141	183	9	28	28	223	189	106
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	174	512	321	251	1141	183	9	28	28	223	189	106
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	174	512	321	251	1141	183	9	28	28	223	189	106
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	174	512	321	251	1141	183	9	28	28	223	189	106
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	2.00	1.00	2.00	1.00	1.00
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	3400	1600	3200	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.11	0.10	0.20	0.16	0.22	0.11	0.01	0.01	0.02	0.07	0.11	0.07
Crit Moves:			****	****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #101 Red Hill Ave and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.664
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	34	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ignore			Include			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0 2	1 0		2 0 3	0 1		2 0 3	0 1		1 0 3	0 1	
Volume Module:												
Base Vol:	64	569	10	208	537	492	1248	645	106	23	223	571
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	64	569	10	208	537	492	1248	645	106	23	223	571
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Volume:	64	569	10	208	537	0	1248	645	106	23	223	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	64	569	10	208	537	0	1248	645	106	23	223	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
FinalVolume:	64	569	10	208	537	0	1248	645	106	23	223	0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.95	0.05	2.00	3.00	1.00	2.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	4917	83	3200	5100	1600	3200	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.02	0.12	0.12	0.07	0.11	0.00	0.39	0.13	0.07	0.01	0.04	0.00
Crit Moves:		****			****			****			****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #102 Red Hill Ave and Main St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.670
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	34	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 0	1	2 0	2 0	1	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	162	556	361	67	313	108	139	1309	254	154	309	73
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	162	556	361	67	313	108	139	1309	254	154	309	73
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	162	556	361	67	313	108	139	1309	254	154	309	73
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	162	556	361	67	313	108	139	1309	254	154	309	73
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	162	556	361	67	313	108	139	1309	254	154	309	73
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	2.00	2.00	1.00	2.00	2.00	1.00	2.00	2.51	0.49	2.00	2.43	0.57
Final Sat.:	3200	3400	1600	3200	3400	1600	3200	4220	780	3200	4083	917
Capacity Analysis Module:												
Vol/Sat:	0.05	0.16	0.23	0.02	0.09	0.07	0.04	0.31	0.33	0.05	0.08	0.08
Crit Moves:			****	****					****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #103 I-5 SB Ramps and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.514
Loss Time (sec):	5	Average Delay (sec/veh):	17.4
Optimal Cycle:	25	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	0	2	0	0	2	0	3	0	2	1
Volume Module:												
Base Vol:	0	0	0	367	0	36	323	554	0	1130	330	
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	367	0	36	323	554	0	1130	330	
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	367	0	36	323	554	0	1130	330	
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	367	0	36	323	554	0	1130	330	
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	367	0	36	323	554	0	1130	330	
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.92	1.00	0.85	0.92	0.91	1.00	1.00	0.88	0.88
Lanes:	0.00	0.00	0.00	2.00	0.00	1.00	2.00	3.00	0.00	0.00	2.32	0.68
Final Sat.:	0	0	0	3502	0	1615	3502	5187	0	0	3878	1133
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.10	0.00	0.02	0.09	0.11	0.00	0.00	0.29	0.29
Crit Moves:				****			****				****	
Green/Cycle:	0.00	0.00	0.00	0.20	0.00	0.38	0.18	0.75	0.00	0.00	0.57	0.57
Volume/Cap:	0.00	0.00	0.00	0.51	0.00	0.06	0.51	0.14	0.00	0.00	0.51	0.51
Delay/Veh:	0.0	0.0	0.0	36.0	0.0	19.5	37.8	3.6	0.0	0.0	13.4	13.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

AdjDel/Veh:	0.0	0.0	0.0	36.0	0.0	19.5	37.8	3.6	0.0	0.0	13.4	13.4
LOS by Move:	A	A	A	D	A	B	D	A	A	A	B	B
HCM2k85thQ:	0	0	0	9	0	1	8	3	0	0	15	15

****

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #104 Tustin Ranch Rd and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.475
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	23	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	0	2	0	0	2	0	3	0	0	1
Volume Module:												
Base Vol:	0	0	0	265	0	33	18	238	0	0	1717	395
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	265	0	33	18	238	0	0	1717	395
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	265	0	33	18	238	0	0	1717	395
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	265	0	33	18	238	0	0	1717	395
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	265	0	33	18	238	0	0	1717	395
OvlAdjVol:												263
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	0.00	0.00	0.00	2.00	0.00	1.00	2.00	3.00	0.00	0.00	3.00	1.00
Final Sat.:	0	0	0	3200	0	1600	3200	5100	0	0	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.08	0.00	0.02	0.01	0.05	0.00	0.00	0.34	0.25
OvlAdjV/S:												0.16
Crit Moves:				****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #105 Von Karman Ave and Barranca Pkwy

Cycle (sec):	100	Critical Vol./Cap.(X):	0.762
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	45	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0 1	1 0		2 0 2			2 0 3	0 1		2 0 4	0 1	
Volume Module:												
Base Vol:	107	142	85	119	1164	509	130	488	163	769	1263	39
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	107	142	85	119	1164	509	130	488	163	769	1263	39
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	107	142	85	119	1164	509	130	488	163	769	1263	39
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	107	142	85	119	1164	509	130	488	163	769	1263	39
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	107	142	85	119	1164	509	130	488	163	769	1263	39
OvlAdjVol:						379						
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	1.25	0.75	2.00	2.00	2.00	2.00	3.00	1.00	2.00	4.00	1.00
Final Sat.:	3200	2102	1198	3200	3400	3200	3200	5100	1600	3200	6800	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.07	0.07	0.04	0.34	0.16	0.04	0.10	0.10	0.24	0.19	0.02
OvlAdjV/S:						0.12						
Crit Moves:	****			****			****			****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #106 Red Hill Avenue and El Camino Real

Cycle (sec):	100	Critical Vol./Cap.(X):	0.706
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	38	Level Of Service:	C

Street Name:	Red Hill Avenue						El Camino Real					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0 3	0 1	1	0 2	1 0	1	0 1	0 1	1	0 0	1 0
Volume Module:												
Base Vol:	306	785	253	18	1303	80	69	156	164	271	222	16
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	306	785	253	18	1303	80	69	156	164	271	222	16
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	306	785	253	18	1303	80	69	156	164	271	222	16
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	306	785	253	18	1303	80	69	156	164	271	222	16
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	306	785	253	18	1303	80	69	156	164	271	222	16
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	2.00	3.00	1.00	1.00	2.83	0.17	1.00	1.00	1.00	1.00	0.93	0.07
Final Sat.:	3200	5100	1600	1600	4722	278	1600	1700	1600	1600	1492	108
Capacity Analysis Module:												
Vol/Sat:	0.10	0.15	0.16	0.01	0.28	0.29	0.04	0.09	0.10	0.17	0.15	0.15
Crit Moves:	****					****			****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #107 Red Hill Avenue and I-5 NB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.670
Loss Time (sec):	5	Average Delay (sec/veh):	20.3
Optimal Cycle:	36	Level Of Service:	C

Street Name:	Red Hill Avenue						I-5 NB Ramps					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	0	0 0	3 0	1	0 0	0 0	0	2 0	0 0	1
Volume Module:												
Base Vol:	327	1084	0	0	1263	475	0	0	0	228	0	261
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	327	1084	0	0	1263	475	0	0	0	228	0	261
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	327	1084	0	0	1263	475	0	0	0	228	0	261
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	327	1084	0	0	1263	475	0	0	0	228	0	261
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	327	1084	0	0	1263	475	0	0	0	228	0	261
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	1.00	1.00	0.91	0.85	1.00	1.00	1.00	0.60	1.00	0.85
Lanes:	1.00	3.00	0.00	0.00	3.00	1.00	0.00	0.00	0.00	2.00	0.00	1.00
Final Sat.:	1805	5187	0	0	5187	1615	0	0	0	2278	0	1615
Capacity Analysis Module:												
Vol/Sat:	0.18	0.21	0.00	0.00	0.24	0.29	0.00	0.00	0.00	0.10	0.00	0.16
Crit Moves:	****					****						****
Green/Cycle:	0.27	0.71	0.00	0.00	0.44	0.44	0.00	0.00	0.00	0.24	0.00	0.24
Volume/Cap:	0.67	0.29	0.00	0.00	0.56	0.67	0.00	0.00	0.00	0.42	0.00	0.67
Delay/Veh:	36.1	5.4	0.0	0.0	21.1	24.8	0.0	0.0	0.0	32.5	0.0	38.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

AdjDel/Veh:	36.1	5.4	0.0	0.0	21.1	24.8	0.0	0.0	0.0	32.5	0.0	38.8
LOS by Move:	D	A	A	A	C	C	A	A	A	C	A	D
HCM2k85thQ:	15	7	0	0	16	19	0	0	0	5	0	13

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Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #108 Red Hill Avenue and I-5 SB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.784
Loss Time (sec):	5	Average Delay (sec/veh):	22.0
Optimal Cycle:	51	Level Of Service:	C

Street Name:	Red Hill Avenue						I-5 SB Ramps					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	4 0	1	1 0	3 0	0	0 1	0	0 1	0 0	0 0	0
Volume Module:												
Base Vol:	0	1243	550	385	1105	0	168	4	309	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1243	550	385	1105	0	168	4	309	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1243	550	385	1105	0	168	4	309	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1243	550	385	1105	0	168	4	309	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1243	550	385	1105	0	168	4	309	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	0.85	0.95	0.91	1.00	0.95	0.95	0.85	1.00	1.00	1.00
Lanes:	0.00	4.00	1.00	1.00	3.00	0.00	0.98	0.02	1.00	0.00	0.00	0.00
Final Sat.:	0	6916	1615	1805	5187	0	1759	42	1615	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.18	0.34	0.21	0.21	0.00	0.10	0.10	0.19	0.00	0.00	0.00
Crit Moves:			****	****					****			
Green/Cycle:	0.00	0.43	0.43	0.27	0.71	0.00	0.24	0.24	0.24	0.00	0.00	0.00
Volume/Cap:	0.00	0.41	0.78	0.78	0.30	0.00	0.39	0.39	0.78	0.00	0.00	0.00
Delay/Veh:	0.0	19.6	30.1	41.8	5.5	0.0	32.2	32.2	45.3	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	19.6	30.1	41.8	5.5	0.0	32.2	32.2	45.3	0.0	0.0	0.0
LOS by Move:	A	B	C	D	A	A	C	C	D	A	A	A
HCM2k85thQ:	0	11	24	19	7	0	7	7	16	0	0	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #109 Red Hill Avenue and Nisson Road

Cycle (sec):	100	Critical Vol./Cap.(X):	0.653
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	33	Level Of Service:	B

Street Name:	Red Hill Avenue						Nisson Road					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 1	0	1 0	2 1	0	1 0	0 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	17	1298	24	152	1144	120	242	59	48	49	26	215
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	17	1298	24	152	1144	120	242	59	48	49	26	215
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	17	1298	24	152	1144	120	242	59	48	49	26	215
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	17	1298	24	152	1144	120	242	59	48	49	26	215
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	17	1298	24	152	1144	120	242	59	48	49	26	215
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	3.93	0.07	1.00	2.72	0.28	1.00	0.55	0.45	1.00	0.11	0.89
Final Sat.:	1600	6584	116	1600	4544	456	1600	882	718	1600	173	1427
Capacity Analysis Module:												
Vol/Sat:	0.01	0.20	0.21	0.10	0.25	0.26	0.15	0.07	0.07	0.03	0.15	0.15
Crit Moves:			****	****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #110 Red Hill Avenue and Walnut Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	0.684
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	35	Level Of Service:	B

Street Name:	Red Hill Avenue						Walnut Avenue					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	2 1	0	1	0	2 1	0	1	0	1	1 0
Volume Module:												
Base Vol:	135	525	99	104	1046	117	160	398	392	192	254	121
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	135	525	99	104	1046	117	160	398	392	192	254	121
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	135	525	99	104	1046	117	160	398	392	192	254	121
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	135	525	99	104	1046	117	160	398	392	192	254	121
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	135	525	99	104	1046	117	160	398	392	192	254	121
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.05	1.00
Lanes:	1.00	2.52	0.48	1.00	2.70	0.30	1.00	1.01	0.99	2.00	1.35	0.65
Final Sat.:	1600	4238	762	1600	4517	483	1600	1712	1588	3200	2267	1033
Capacity Analysis Module:												
Vol/Sat:	0.08	0.12	0.13	0.07	0.23	0.24	0.10	0.23	0.25	0.06	0.11	0.12
Crit Moves:	****					****			****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #111 Red Hill Avenue and Valencia Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	0.544
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	26	Level Of Service:	A

Street Name:	Red Hill Avenue						Valencia Avenue					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 1	0	1 0	2 1	0	1 0	1 0	1	2 0	1 0	1
Volume Module:												
Base Vol:	77	325	105	120	1032	69	20	139	54	252	346	122
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	77	325	105	120	1032	69	20	139	54	252	346	122
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	77	325	105	120	1032	69	20	139	54	252	346	122
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	77	325	105	120	1032	69	20	139	54	252	346	122
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	77	325	105	120	1032	69	20	139	54	252	346	122
OvlAdjVol:			0									2
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	3.02	0.98	1.00	2.81	0.19	1.00	1.00	1.00	2.00	1.00	1.00
Final Sat.:	1600	5137	1563	1600	4699	301	1600	1700	1600	3200	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.05	0.06	0.07	0.08	0.22	0.23	0.01	0.08	0.03	0.08	0.20	0.08
OvlAdjV/S:			0.00									0.00
Crit Moves:	****					****	****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #112 Tustin Ranch Road and Warner Avenue North

Cycle (sec):	100	Critical Vol./Cap.(X):	0.426
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	21	Level Of Service:	A

Street Name:	Tustin Ranch Road						Warner Avenue North					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 3	0 1	2	0 3	0 0	0	0 0	0 0	3	0 0	0 2
Volume Module:												
Base Vol:	0	272	263	456	1289	0	0	0	0	591	0	174
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	272	263	456	1289	0	0	0	0	591	0	174
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	272	263	456	1289	0	0	0	0	591	0	174
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	272	263	456	1289	0	0	0	0	591	0	174
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	272	263	456	1289	0	0	0	0	591	0	174
OvlAdjVol:			66									0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	0.00	3.00	1.00	2.00	3.00	0.00	0.00	0.00	0.00	3.00	0.00	2.00
Final Sat.:	0	5100	1600	3200	5100	0	0	0	0	4800	0	3200
Capacity Analysis Module:												
Vol/Sat:	0.00	0.05	0.16	0.14	0.25	0.00	0.00	0.00	0.00	0.12	0.00	0.05
OvlAdjV/S:			0.04									0.00
Crit Moves:	****			****						****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #113 Tustin Ranch Road and Walnut Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	1.193
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Street Name:	Tustin Ranch Road						Walnut Avenue					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Ovl			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0	3	0	1	2	0	3	0	1	2	0	1	1	0	2	0	2	0	1
Volume Module:																				
Base Vol:	123	756	377	1045	2921	389	702	917	554	231	341	288
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	123	756	377	1045	2921	389	702	917	554	231	341	288
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	123	756	377	1045	2921	389	702	917	554	231	341	288
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	123	756	377	1045	2921	389	702	917	554	231	341	288
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	123	756	377	1045	2921	389	702	917	554	231	341	288
OvlAdjVol:			262			38						0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	1.25	0.75	2.00	2.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	2095	1205	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.04	0.15	0.24	0.33	0.57	0.24	0.22	0.44	0.46	0.07	0.10	0.18
OvlAdjV/S:			0.16			0.02						0.00
Crit Moves:	****			****					****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #114 SR-55 SB Ramps and Irvine Boulevard

Cycle (sec):	100	Critical Vol./Cap.(X):	0.970
Loss Time (sec):	5	Average Delay (sec/veh):	23.2
Optimal Cycle:	165	Level Of Service:	C

Street Name:	SR-55 SB Ramps						Irvine Boulevard					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	0	1	0	1! 0	1	0	0	2	1	0
Volume Module:												
Base Vol:	0	0	0	62	0	68	0	575	1032	425	1063	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	62	0	68	0	575	1032	425	1063	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	62	0	68	0	575	1032	425	1063	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	62	0	68	0	575	1032	425	1063	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	62	0	68	0	575	1032	425	1063	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.88	1.00	0.77	1.00	0.82	0.82	0.95	0.91	1.00
Lanes:	0.00	0.00	0.00	1.44	0.00	1.56	0.00	2.00	1.00	1.00	3.00	0.00
Final Sat.:	0	0	0	2415	0	2277	0	3126	1563	1805	5187	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.03	0.00	0.03	0.00	0.18	0.66	0.24	0.20	0.00
Crit Moves:				****					****	****		
Green/Cycle:	0.00	0.00	0.00	0.03	0.00	0.05	0.00	0.68	0.68	0.24	0.92	0.00
Volume/Cap:	0.00	0.00	0.00	0.86	0.00	0.58	0.00	0.27	0.97	0.97	0.22	0.00
Delay/Veh:	0.0	0.0	0.0	84.9	0.0	50.0	0.0	6.3	30.4	72.4	0.4	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	84.9	0.0	50.0	0.0	6.3	30.4	72.4	0.4	0.0
LOS by Move:	A	A	A	F	A	D	A	A	C	E	A	A

```
HCM2k85thQ:     0    0     0     5    0     4     0    6    54    27    2     0
********************************************************************************
Note: Queue reported is the number of cars per lane.
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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #115 SR-55 NB Ramps and Irvine Boulevard

Cycle (sec):	100	Critical Vol./Cap.(X):	0.797
Loss Time (sec):	5	Average Delay (sec/veh):	19.8
Optimal Cycle:	54	Level Of Service:	B

Street Name:	SR-55 NB Ramps						Irvine Boulevard					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	1! 0	1	0	0	0	0	1	0	3	0	0	0	0	2	1	0
Volume Module:																		
Base Vol:	410	0	394	0	0	0	187	444	0	0	1079	750
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	410	0	394	0	0	0	187	444	0	0	1079	750
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	410	0	394	0	0	0	187	444	0	0	1079	750
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	410	0	394	0	0	0	187	444	0	0	1079	750
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	410	0	394	0	0	0	187	444	0	0	1079	750
Saturation Flow Module:																		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.75	1.00	0.74	1.00	1.00	1.00	0.95	0.91	1.00	1.00	0.85	0.85
Lanes:	1.51	0.00	1.49	0.00	0.00	0.00	1.00	3.00	0.00	0.00	2.00	1.00
Final Sat.:	2135	0	2095	0	0	0	1805	5187	0	0	3247	1624
Capacity Analysis Module:																		
Vol/Sat:	0.19	0.00	0.19	0.00	0.00	0.00	0.10	0.09	0.00	0.00	0.33	0.46
Crit Moves:	****						****					****
Green/Cycle:	0.24	0.00	0.48	0.00	0.00	0.00	0.13	0.71	0.00	0.00	0.58	0.58
Volume/Cap:	0.80	0.00	0.39	0.00	0.00	0.00	0.80	0.12	0.00	0.00	0.57	0.80
Delay/Veh:	40.3	0.0	16.7	0.0	0.0	0.0	59.3	4.6	0.0	0.0	13.5	18.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	40.3	0.0	16.7	0.0	0.0	0.0	59.3	4.6	0.0	0.0	13.5	18.5
LOS by Move:	D	A	B	A	A	A	E	A	A	A	B	B
HCM2k85thQ:	15	0	8	0	0	0	12	2	0	0	18	31

Note: Queue reported is the number of cars per lane.

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APPENDIX D.2

ANALYSIS WORKSHEETS –
2020 NP (PM Peak Hour)

Santa Ana Circulation Element

Scenario Report

Scenario:	2020 NP PM Peak Hour
Command:	Default Command
Volume:	2020 NP PM
Geometry:	NP
Impact Fee:	Default Impact Fee
Trip Generation:	Default Trip Generation
Trip Distribution:	Default Trip Distribution
Paths:	Default Path
Routes:	Default Route
Configuration:	Default Configuration

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Santa Ana Circulation Element

Impact Analysis Report
Level Of Service

Intersection	Base LOS	Base Del/ Veh	Base V/ C	Future LOS	Future Del/ Veh	Future V/ C	Change in
# 1 Euclid St and 1st St	D	xxxxx	0.865	D	xxxxx	0.865	+ 0.000 V/C
# 2 Euclid St and McFadden Ave	D	xxxxx	0.833	D	xxxxx	0.833	+ 0.000 V/C
# 3 Euclid St and Edinger Ave	C	xxxxx	0.787	C	xxxxx	0.787	+ 0.000 V/C
# 4 Newhope St and Hazard Ave	B	xxxxx	0.642	B	xxxxx	0.642	+ 0.000 V/C
# 5 Newhope St and McFadden Ave	D	xxxxx	0.838	D	xxxxx	0.838	+ 0.000 V/C
# 6 Harbor Blvd and SR-22 WB Off-R	C	23.6	0.703	C	23.6	0.703	+ 0.000 D/V
# 7 Trask Avenue and SR-22 EB On-R	B	12.4	0.448	B	12.4	0.448	+ 0.000 D/V
# 8 Harbor Blvd and Westminster Av	C	xxxxx	0.792	C	xxxxx	0.792	+ 0.000 V/C
# 9 Harbor Blvd and 1st St	D	xxxxx	0.833	D	xxxxx	0.833	+ 0.000 V/C
# 10 Harbor Blvd and McFadden Ave	D	xxxxx	0.809	D	xxxxx	0.809	+ 0.000 V/C
# 11 Harbor Blvd and Edinger Ave	B	xxxxx	0.697	B	xxxxx	0.697	+ 0.000 V/C
# 12 Harbor Blvd and Warner Ave	F	xxxxx	1.581	F	xxxxx	1.581	+ 0.000 V/C
# 13 Harbor Blvd and Segerstrom Ave	D	xxxxx	0.897	D	xxxxx	0.897	+ 0.000 V/C
# 14 MacArthur Blvd and Hyland Ave	E	xxxxx	0.917	E	xxxxx	0.917	+ 0.000 V/C
# 15 MacArthur Blvd and Harbor Blvd	D	xxxxx	0.819	D	xxxxx	0.819	+ 0.000 V/C
# 16 Harbor Blvd and Sunflower Ave	C	xxxxx	0.780	C	xxxxx	0.780	+ 0.000 V/C
# 17 Harbor Blvd and I-405 WB Off-R	C	22.9	0.812	C	22.9	0.812	+ 0.000 D/V
# 18 Harbor Blvd and I-405 EB Off-R	B	16.2	0.630	B	16.2	0.630	+ 0.000 D/V
# 19 Fairview St and Civic Center D	B	xxxxx	0.662	B	xxxxx	0.662	+ 0.000 V/C
# 20 Fairview St and 1st St	D	xxxxx	0.883	D	xxxxx	0.883	+ 0.000 V/C
# 21 Fairview St and McFadden Ave	C	xxxxx	0.798	C	xxxxx	0.798	+ 0.000 V/C
# 22 Fairview St and Edinger Ave	D	xxxxx	0.811	D	xxxxx	0.811	+ 0.000 V/C
# 23 Fairview St and Warner Ave	D	xxxxx	0.845	D	xxxxx	0.845	+ 0.000 V/C
# 24 Fairview St and MacArthur Blvd	D	xxxxx	0.826	D	xxxxx	0.826	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Base Del/ Veh	Base V/ C	Future LOS	Future Del/ Veh	Future V/ C	Change in
# 25 Fairview Rd and Sunflower Ave	C	xxxxx	0.708	C	xxxxx	0.708	+ 0.000 V/C
# 26 Greenville St and Edinger Ave	A	xxxxx	0.586	A	xxxxx	0.586	+ 0.000 V/C
# 27 Greenville St and Segerstrom A	B	xxxxx	0.686	B	xxxxx	0.686	+ 0.000 V/C
# 28 Raitt St and McFadden Ave	C	xxxxx	0.722	C	xxxxx	0.722	+ 0.000 V/C
# 29 Raitt St and Edinger Ave	D	xxxxx	0.864	D	xxxxx	0.864	+ 0.000 V/C
# 30 Bear St and MacArthur Blvd	D	xxxxx	0.868	D	xxxxx	0.868	+ 0.000 V/C
# 31 Bristol St and 17th St	B	xxxxx	0.692	B	xxxxx	0.692	+ 0.000 V/C
# 32 Bristol St and Civic Center Dr	D	xxxxx	0.832	D	xxxxx	0.832	+ 0.000 V/C
# 33 Bristol St and Santa Ana Blvd	B	xxxxx	0.657	B	xxxxx	0.657	+ 0.000 V/C
# 34 Bristol St and 1st St	C	xxxxx	0.779	C	xxxxx	0.779	+ 0.000 V/C
# 35 Bristol St and McFadden Ave	C	xxxxx	0.752	C	xxxxx	0.752	+ 0.000 V/C
# 36 Bristol St and Warner Ave	C	xxxxx	0.790	C	xxxxx	0.790	+ 0.000 V/C
# 37 Bristol St and Segerstrom Ave	E	xxxxx	0.909	E	xxxxx	0.909	+ 0.000 V/C
# 38 Bristol St and Alton Ave	B	xxxxx	0.686	B	xxxxx	0.686	+ 0.000 V/C
# 39 Bristol St and MacArthur Blvd	D	xxxxx	0.812	D	xxxxx	0.812	+ 0.000 V/C
# 40 Bristol St and Sunflower Ave	B	xxxxx	0.689	B	xxxxx	0.689	+ 0.000 V/C
# 41 Bristol St and I-405 WB Ramps	C	28.2	0.829	C	28.2	0.829	+ 0.000 D/V
# 42 Bristol St and I-405 EB Ramps	B	18.4	0.595	B	18.4	0.595	+ 0.000 D/V
# 43 Flower St and Santa Ana Blvd	A	xxxxx	0.571	A	xxxxx	0.571	+ 0.000 V/C
# 44 Flower St and 1st St	C	xxxxx	0.752	C	xxxxx	0.752	+ 0.000 V/C
# 45 Flower St and McFadden Ave	E	xxxxx	0.920	E	xxxxx	0.920	+ 0.000 V/C
# 46 Flower St and Segerstrom Ave	D	xxxxx	0.897	D	xxxxx	0.897	+ 0.000 V/C
# 47 Flower St and MacArthur Blvd	E	xxxxx	0.906	E	xxxxx	0.906	+ 0.000 V/C
# 48 Main St and La Veta Ave	B	xxxxx	0.628	B	xxxxx	0.628	+ 0.000 V/C
# 49 Main St and Mainplace Dr / Mem	A	xxxxx	0.471	A	xxxxx	0.471	+ 0.000 V/C
# 50 Main St and 17th St	C	xxxxx	0.732	C	xxxxx	0.732	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Base Del/ Veh	Base V/ C	Future LOS	Future Del/ Veh	Future V/ C	Change in
# 51 Main St and Civic Center Dr	D	xxxxx	0.801	D	xxxxx	0.801	+ 0.000 V/C
# 52 Main St and Santa Ana Blvd	B	xxxxx	0.607	B	xxxxx	0.607	+ 0.000 V/C
# 53 Main St and 4th St	A	xxxxx	0.537	A	xxxxx	0.537	+ 0.000 V/C
# 54 Main St and 1st St	C	xxxxx	0.799	C	xxxxx	0.799	+ 0.000 V/C
# 55 Main St and McFadden Ave	D	xxxxx	0.845	D	xxxxx	0.845	+ 0.000 V/C
# 56 Main St and Edinger Ave	D	xxxxx	0.847	D	xxxxx	0.847	+ 0.000 V/C
# 57 Main St and MacArthur Blvd	C	xxxxx	0.777	C	xxxxx	0.777	+ 0.000 V/C
# 58 Penn Wy and 17th St	C	20.7	0.770	C	20.7	0.770	+ 0.000 D/V
# 59 I-5 NB Off Ramps/17th Street	B	19.5	0.589	B	19.5	0.589	+ 0.000 D/V
# 60 Penn Wy and I-5 SB Ramps	C	22.0	0.423	C	22.0	0.423	+ 0.000 D/V
# 61 Santiago St and Civic Center D	C	17.1	0.674	C	17.1	0.674	+ 0.000 V/C
# 62 Santiago St and Santa Ana Blvd	A	xxxxx	0.569	A	xxxxx	0.569	+ 0.000 V/C
# 64 Standard Ave and 1st St	D	xxxxx	0.842	D	xxxxx	0.842	+ 0.000 V/C
# 65 Standard Ave and Mcfadden Ave	B	xxxxx	0.682	B	xxxxx	0.682	+ 0.000 V/C
# 66 Halladay St and Warner Ave	B	xxxxx	0.677	B	xxxxx	0.677	+ 0.000 V/C
# 67 Halladay St and Dyer Rd	C	xxxxx	0.784	C	xxxxx	0.784	+ 0.000 V/C
# 68 SR-55 SB Ramps and MacArthur B	B	17.3	0.569	B	17.3	0.569	+ 0.000 D/V
# 69 SR-55 NB Ramps and MacArthur B	B	14.8	0.497	B	14.8	0.497	+ 0.000 D/V
# 70 SR-55 SB Ramps and Dyer Rd	C	25.7	0.669	C	25.7	0.669	+ 0.000 D/V
# 71 Glassell St and La Veta Ave	B	xxxxx	0.620	B	xxxxx	0.620	+ 0.000 V/C
# 72 Glassell St and SR-22 WB Ramps	C	24.8	0.766	C	24.8	0.766	+ 0.000 D/V
# 73 Grand Ave / Glassell St and SR	C	26.5	0.812	C	26.5	0.812	+ 0.000 D/V
# 74 Grand Ave and Fairhaven Ave	B	xxxxx	0.675	B	xxxxx	0.675	+ 0.000 V/C
# 75 Grand Ave and Santa Clara Ave	C	xxxxx	0.748	C	xxxxx	0.748	+ 0.000 V/C
# 76 Grand Ave and 17th St	D	xxxxx	0.828	D	xxxxx	0.828	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Del/ Veh	V/ C	Future LOS	Del/ Veh	V/ C	Change in
# 77 Grand Ave and I-5 NB Ramps	A	10.0	0.518	A	10.0	0.518	+ 0.000 D/V
# 78 Grand Ave and Santa Ana Blvd	C	21.1	0.605	C	21.1	0.605	+ 0.000 D/V
# 79 Grand Ave and 1st St	C	xxxxx	0.793	C	xxxxx	0.793	+ 0.000 V/C
# 80 Grand Ave and Chestnut Ave	B	xxxxx	0.615	B	xxxxx	0.615	+ 0.000 V/C
# 81 Grand Ave and McFadden Ave	C	xxxxx	0.759	C	xxxxx	0.759	+ 0.000 V/C
# 82 Grand Ave and Edinger Ave	E	xxxxx	0.909	E	xxxxx	0.909	+ 0.000 V/C
# 83 Grand Ave and Warner Ave	C	xxxxx	0.704	C	xxxxx	0.704	+ 0.000 V/C
# 84 SR-55 NB Ramps and Dyer Rd	A	5.7	0.376	A	5.7	0.376	+ 0.000 D/V
# 85 Cambridge St and La Veta Ave	B	12.5	0.459	B	12.5	0.459	+ 0.000 V/C
# 86 Cambridge St and Fairhaven Ave	A	xxxxx	0.488	A	xxxxx	0.488	+ 0.000 V/C
# 87 Mabury St and 1st Street	C	26.1	0.703	C	26.1	0.703	+ 0.000 D/V
# 88 Tustin St and La Veta Ave	A	xxxxx	0.444	A	xxxxx	0.444	+ 0.000 V/C
# 89 Tustin St and SR-22 WB On-Ramp	B	12.1	0.446	B	12.1	0.446	+ 0.000 D/V
# 90 Tustin St and SR-22 EB Off-Ram	C	23.7	0.719	C	23.7	0.719	+ 0.000 D/V
# 91 Tustin Ave and Fairhaven Ave	B	xxxxx	0.614	B	xxxxx	0.614	+ 0.000 V/C
# 92 Tustin Ave and Santa Clara Ave	B	xxxxx	0.647	B	xxxxx	0.647	+ 0.000 V/C
# 93 Tustin Ave and 17th St	C	xxxxx	0.717	C	xxxxx	0.717	+ 0.000 V/C
# 94 Tustin Ave and 4th St	B	xxxxx	0.692	B	xxxxx	0.692	+ 0.000 V/C
# 95 SR-55 SB Ramps / Auto Mall Dr	C	25.1	0.583	C	25.1	0.583	+ 0.000 D/V
# 96 SR-55 NB Ramps / Del Amo Ave a	C	27.4	0.525	C	27.4	0.525	+ 0.000 D/V
# 97 Red Hill Ave and Edinger Ave	B	xxxxx	0.667	B	xxxxx	0.667	+ 0.000 V/C
# 98 Red Hill Ave and Warner Ave	A	xxxxx	0.588	A	xxxxx	0.588	+ 0.000 V/C
# 99 Red Hill Ave and Dyer Rd / bar	A	xxxxx	0.563	A	xxxxx	0.563	+ 0.000 V/C
#100 Red Hill Ave and Alton Pkwy	B	xxxxx	0.671	B	xxxxx	0.671	+ 0.000 V/C
#101 Red Hill Ave and MacArthur Blv	B	xxxxx	0.657	B	xxxxx	0.657	+ 0.000 V/C
#102 Red Hill Ave and Main St	C	xxxxx	0.774	C	xxxxx	0.774	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Del/ Veh	V/ C	Future LOS	Del/ Veh	V/ C	Change in
#103 I-5 SB Ramps and Santa Ana Blv	B	18.9	0.431	B	18.9	0.431	+ 0.000 D/V
#104 Tustin Ranch Rd and Warner Ave	B	xxxxx	0.606	B	xxxxx	0.606	+ 0.000 V/C
#105 Von Karman Ave and Barranca Pk	D	xxxxx	0.874	D	xxxxx	0.874	+ 0.000 V/C
#106 Red Hill Avenue and El Camino	B	xxxxx	0.624	B	xxxxx	0.624	+ 0.000 V/C
#107 Red Hill Avenue and I-5 NB Ram	C	23.3	0.626	C	23.3	0.626	+ 0.000 D/V
#108 Red Hill Avenue and I-5 SB Ram	C	22.8	0.708	C	22.8	0.708	+ 0.000 D/V
#109 Red Hill Avenue and Nisson Roa	C	xxxxx	0.702	C	xxxxx	0.702	+ 0.000 V/C
#110 Red Hill Avenue and Walnut Ave	C	xxxxx	0.785	C	xxxxx	0.785	+ 0.000 V/C
#111 Red Hill Avenue and Valencia A	A	xxxxx	0.549	A	xxxxx	0.549	+ 0.000 V/C
#112 Tustin Ranch Road and Warner A	C	xxxxx	0.757	C	xxxxx	0.757	+ 0.000 V/C
#113 Tustin Ranch Road and Walnut A	F	xxxxx	1.630	F	xxxxx	1.630	+ 0.000 V/C
#114 SR-55 SB Ramps and Irvine Boul	C	21.5	0.794	C	21.5	0.794	+ 0.000 D/V
#115 SR-55 NB Ramps and Irvine Boul	B	19.5	0.809	B	19.5	0.809	+ 0.000 D/V

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #1 Euclid St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.865
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	70	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			WideBypass			WideBypass		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	159	1477	119	185	1053	199	212	678	106	217	909	217
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	159	1477	119	185	1053	199	212	678	106	217	909	217
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	159	1477	119	185	1053	199	212	678	106	217	909	217
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	159	1477	119	185	1053	199	212	678	106	217	909	217
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	159	1477	119	185	1053	199	212	678	106	217	909	217
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.78	0.22	1.00	2.52	0.48	1.00	2.59	0.41	1.00	2.42	0.58
Final Sat.:	1600	4642	358	1600	4237	763	1600	4351	649	1600	4075	925
Capacity Analysis Module:												
Vol/Sat:	0.10	0.32	0.33	0.12	0.25	0.26	0.13	0.16	0.16	0.14	0.22	0.23
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #2 Euclid St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.833
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	60	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	175	1426	92	110	1354	164	183	419	185	127	532	246
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	175	1426	92	110	1354	164	183	419	185	127	532	246
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	175	1426	92	110	1354	164	183	419	185	127	532	246
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	175	1426	92	110	1354	164	183	419	185	127	532	246
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	175	1426	92	110	1354	164	183	419	185	127	532	246
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.82	0.18	1.00	2.68	0.32	1.00	1.39	0.61	1.00	1.37	0.63
Final Sat.:	1600	4709	291	1600	4481	519	1600	2320	980	1600	2288	1012
Capacity Analysis Module:												
Vol/Sat:	0.11	0.30	0.32	0.07	0.30	0.32	0.11	0.18	0.19	0.08	0.23	0.24
Crit Moves:	****					****	****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #3 Euclid St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.787
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	49	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 0	2 0	2	1 0	1 0	2	0 1	1 0	2	0 1
Volume Module:												
Base Vol:	526	1572	112	122	803	204	161	622	186	91	840	136
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	526	1572	112	122	803	204	161	622	186	91	840	136
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	526	1572	112	122	803	204	161	622	186	91	840	136
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	526	1572	112	122	803	204	161	622	186	91	840	136
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	526	1572	112	122	803	204	161	622	186	91	840	136
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.80	0.20	2.00	2.39	0.61	1.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	3200	4681	319	3200	4028	972	1600	3400	1600	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.16	0.34	0.35	0.04	0.20	0.21	0.10	0.18	0.12	0.06	0.25	0.09
Crit Moves:			****	****			****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #4 Newhope St and Hazard Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.642
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	32	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Prot+Permit			Prot+Permit			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	116	1281	50	49	1038	124	108	214	131	32	202	58
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	116	1281	50	49	1038	124	108	214	131	32	202	58
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	116	1281	50	49	1038	124	108	214	131	32	202	58
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	116	1281	50	49	1038	124	108	214	131	32	202	58
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	116	1281	50	49	1038	124	108	214	131	32	202	58
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.05	1.00	1.00	1.04	1.00
Lanes:	1.00	1.92	0.08	1.00	1.79	0.21	1.00	1.24	0.76	1.00	1.55	0.45
Final Sat.:	1600	3180	120	1600	2959	341	1600	2085	1215	1600	2586	714
Capacity Analysis Module:												
Vol/Sat:	0.07	0.40	0.42	0.03	0.35	0.36	0.07	0.10	0.11	0.02	0.08	0.08
Crit Moves:			****	****			****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #5 Newhope St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.838
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	61	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	202	1084	190	164	733	142	81	496	72	107	589	168
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	202	1084	190	164	733	142	81	496	72	107	589	168
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	202	1084	190	164	733	142	81	496	72	107	589	168
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	202	1084	190	164	733	142	81	496	72	107	589	168
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	202	1084	190	164	733	142	81	496	72	107	589	168
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	1.70	0.30	1.00	1.68	0.32	1.00	1.75	0.25	1.00	1.56	0.44
Final Sat.:	1600	2823	477	1600	2781	519	1600	2894	406	1600	2590	710
Capacity Analysis Module:												
Vol/Sat:	0.13	0.38	0.40	0.10	0.26	0.27	0.05	0.17	0.18	0.07	0.23	0.24
Crit Moves:		****	****				****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #6 Harbor Blvd and SR-22 WB Off-Ramp / Banner Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.703
Loss Time (sec):	5	Average Delay (sec/veh):	23.6
Optimal Cycle:	39	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	0	0 0	2	1 0	0 0	1! 0	0	1 1	0 0	1
Volume Module:												
Base Vol:	88	1992	0	0	1519	49	93	0	78	593	80	170
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	88	1992	0	0	1519	49	93	0	78	593	80	170
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	88	1992	0	0	1519	49	93	0	78	593	80	170
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	88	1992	0	0	1519	49	93	0	78	593	80	170
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	88	1992	0	0	1519	49	93	0	78	593	80	170
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	1.00	1.00	0.91	0.91	0.91	1.00	0.91	0.96	0.96	0.85
Lanes:	1.00	3.00	0.00	0.00	2.91	0.09	0.54	0.00	0.46	1.76	0.24	1.00
Final Sat.:	1805	5187	0	0	5000	161	944	0	792	3208	433	1615
Capacity Analysis Module:												
Vol/Sat:	0.05	0.38	0.00	0.00	0.30	0.30	0.10	0.00	0.10	0.18	0.18	0.11
Crit Moves:		****			****			****			****	
Green/Cycle:	0.08	0.55	0.00	0.00	0.47	0.47	0.14	0.00	0.14	0.26	0.26	0.26
Volume/Cap:	0.64	0.70	0.00	0.00	0.64	0.64	0.70	0.00	0.70	0.70	0.70	0.40
Delay/Veh:	55.1	17.5	0.0	0.0	20.7	20.7	49.9	0.0	49.9	35.7	35.7	31.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	55.1	17.5	0.0	0.0	20.7	20.7	49.9	0.0	49.9	35.7	35.7	31.0
LOS by Move:	E	B	A	A	C	C	D	A	D	D	D	C
HCM2k85thQ:	6	25	0	0	21	21	10	0	10	16	16	7

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #7 Trask Avenue and SR-22 EB On-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.448
Loss Time (sec):	5	Average Delay (sec/veh):	12.4
Optimal Cycle:	23	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	0	0 0	0 0	0	0 0	2 0	1	0 0	0 0	1	1 0
Volume Module:												
Base Vol:	0	0	0	0	0	0	838	628	0	0	606	57
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	0	0	0	838	628	0	0	606	57
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	0	0	0	838	628	0	0	606	57
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	0	0	0	838	628	0	0	606	57
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	0	0	0	838	628	0	0	606	57
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	0.92	1.00	1.00	1.00	0.94	0.94
Lanes:	0.00	0.00	0.00	0.00	0.00	0.00	2.00	1.00	0.00	0.00	1.83	0.17
Final Sat.:	0	0	0	0	0	0	3502	1900	0	0	3257	306
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.00	0.00	0.00	0.24	0.33	0.00	0.00	0.19	0.19
Crit Moves:							****				****	
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.53	0.95	0.00	0.00	0.42	0.42
Volume/Cap:	0.00	0.00	0.00	0.00	0.00	0.00	0.45	0.35	0.00	0.00	0.45	0.45
Delay/Veh:	0.0	0.0	0.0	0.0	0.0	0.0	14.4	0.3	0.0	0.0	21.2	21.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	0.0	0.0	0.0	14.4	0.3	0.0	0.0	21.2	21.2
LOS by Move:	A	A	A	A	A	A	B	A	A	A	C	C
HCM2k85thQ:	0	0	0	0	0	0	12	3	0	0	12	12

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #8 Harbor Blvd and Westminster Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.792
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	50	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	2 1	0	1 0	3 0	1
Volume Module:												
Base Vol:	276	1374	325	207	1430	165	187	667	32	230	797	181
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	276	1374	325	207	1430	165	187	667	32	230	797	181
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	276	1374	325	207	1430	165	187	667	32	230	797	181
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	276	1374	325	207	1430	165	187	667	32	230	797	181
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	276	1374	325	207	1430	165	187	667	32	230	797	181
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	2.86	0.14	1.00	3.00	1.00
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	4780	220	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.17	0.27	0.20	0.13	0.28	0.10	0.12	0.14	0.15	0.14	0.16	0.11
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #9 Harbor Blvd and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.833
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	60	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	287	1480	164	278	1218	164	188	766	145	230	1073	175
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	287	1480	164	278	1218	164	188	766	145	230	1073	175
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	287	1480	164	278	1218	164	188	766	145	230	1073	175
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	287	1480	164	278	1218	164	188	766	145	230	1073	175
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	287	1480	164	278	1218	164	188	766	145	230	1073	175
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	2.00	2.52	0.48	2.00	2.58	0.42
Final Sat.:	1600	5100	1600	1600	5100	1600	3200	4236	764	3200	4327	673
Capacity Analysis Module:												
Vol/Sat:	0.18	0.29	0.10	0.17	0.24	0.10	0.06	0.18	0.19	0.07	0.25	0.26
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #10 Harbor Blvd and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.809
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	54	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	167	1716	125	298	1273	115	214	520	65	211	506	146
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	167	1716	125	298	1273	115	214	520	65	211	506	146
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	167	1716	125	298	1273	115	214	520	65	211	506	146
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	167	1716	125	298	1273	115	214	520	65	211	506	146
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	167	1716	125	298	1273	115	214	520	65	211	506	146
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	2.00	3.00	1.00	2.00	2.75	0.25	1.00	1.78	0.22	1.00	1.55	0.45
Final Sat.:	3200	5100	1600	3200	4602	398	1600	2944	356	1600	2583	717
Capacity Analysis Module:												
Vol/Sat:	0.05	0.34	0.08	0.09	0.28	0.29	0.13	0.18	0.18	0.13	0.20	0.20
Crit Moves:		****			****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #11 Harbor Blvd and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.697
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	37	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	2 0	2 1	0	2 0	3 0	1
Volume Module:												
Base Vol:	282	1773	214	283	822	75	134	564	100	140	813	271
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	282	1773	214	283	822	75	134	564	100	140	813	271
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	282	1773	214	283	822	75	134	564	100	140	813	271
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	282	1773	214	283	822	75	134	564	100	140	813	271
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	282	1773	214	283	822	75	134	564	100	140	813	271
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	2.55	0.45	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	4277	723	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.09	0.35	0.13	0.09	0.16	0.05	0.04	0.13	0.14	0.04	0.16	0.17
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #12 Harbor Blvd and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	1.581
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0 2	1 0		2 0 3	0 1		2 0 3	0 1		2 0 3	0 1	
Volume Module:												
Base Vol:	774	3713	279	274	1525	182	241	1707	390	260	2700	862
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	774	3713	279	274	1525	182	241	1707	390	260	2700	862
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	774	3713	279	274	1525	182	241	1707	390	260	2700	862
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	774	3713	279	274	1525	182	241	1707	390	260	2700	862
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	774	3713	279	274	1525	182	241	1707	390	260	2700	862
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.79	0.21	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	4665	335	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.24	0.80	0.83	0.09	0.30	0.11	0.08	0.33	0.24	0.08	0.53	0.54
Crit Moves:		****	****				****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #13 Harbor Blvd and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.897
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	85	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	1 0	2 1	0	1 0	1 1	0	1 0	2 0	1
Volume Module:												
Base Vol:	299	1881	60	91	1027	91	111	552	181	99	1074	390
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	299	1881	60	91	1027	91	111	552	181	99	1074	390
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	299	1881	60	91	1027	91	111	552	181	99	1074	390
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	299	1881	60	91	1027	91	111	552	181	99	1074	390
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	299	1881	60	91	1027	91	111	552	181	99	1074	390
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	2.91	0.09	1.00	2.76	0.24	1.00	1.51	0.49	1.00	2.00	1.00
Final Sat.:	3200	4852	148	1600	4609	391	1600	2510	790	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.09	0.39	0.40	0.06	0.22	0.23	0.07	0.22	0.23	0.06	0.32	0.24
Crit Moves:			****	****			****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #14 MacArthur Blvd and Hyland Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.917
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	98	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	0 1	0	1 0	1 0	1	1 0	3 0	1	1 0	3 0	1
Volume Module:												
Base Vol:	1201	7	70	12	4	14	36	891	160	25	2350	12
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	1201	7	70	12	4	14	36	891	160	25	2350	12
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	1201	7	70	12	4	14	36	891	160	25	2350	12
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	1201	7	70	12	4	14	36	891	160	25	2350	12
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	1201	7	70	12	4	14	36	891	160	25	2350	12
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	0.09	0.91	1.00	1.00	1.00	1.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	145	1455	1600	1700	1600	1600	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.38	0.05	0.05	0.01	0.00	0.01	0.02	0.17	0.10	0.02	0.46	0.01
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #15 MacArthur Blvd and Harbor Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.819
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	56	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	1 0	3 0	1	1 0	3 0	1
Volume Module:												
Base Vol:	588	1519	88	213	1073	176	140	489	181	87	1467	276
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	588	1519	88	213	1073	176	140	489	181	87	1467	276
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	588	1519	88	213	1073	176	140	489	181	87	1467	276
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	588	1519	88	213	1073	176	140	489	181	87	1467	276
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	588	1519	88	213	1073	176	140	489	181	87	1467	276
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	1.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	1600	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.18	0.30	0.06	0.07	0.21	0.11	0.09	0.10	0.11	0.05	0.29	0.17
Crit Moves:	****				****		****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #16 Harbor Blvd and Sunflower Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.780
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	48	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	1 0	1 1	0	1 1	0 1	0
Volume Module:												
Base Vol:	178	1771	263	112	1393	42	67	182	189	254	533	199
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	178	1771	263	112	1393	42	67	182	189	254	533	199
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	178	1771	263	112	1393	42	67	182	189	254	533	199
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	178	1771	263	112	1393	42	67	182	189	254	533	199
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	178	1771	263	112	1393	42	67	182	189	254	533	199
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	1.00	1.00	1.00	1.00	1.39	0.61
Final Sat.:	3200	5100	1600	3200	5100	1600	1600	1700	1600	1600	2322	974
Capacity Analysis Module:												
Vol/Sat:	0.06	0.35	0.16	0.04	0.27	0.03	0.04	0.11	0.12	0.16	0.23	0.20
Crit Moves:		****			****				****		****	

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #17 Harbor Blvd and I-405 WB Off-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.812
Loss Time (sec):	5	Average Delay (sec/veh):	22.9
Optimal Cycle:	57	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 4	0 0	0	0 4	0 0	0	0 0	0 0	1	0 1!	0 1
Volume Module:												
Base Vol:	0	1529	0	0	2475	0	0	0	0	701	0	1132
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1529	0	0	2475	0	0	0	0	701	0	1132
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1529	0	0	2475	0	0	0	0	701	0	1132
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1529	0	0	2475	0	0	0	0	701	0	1132
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1529	0	0	2475	0	0	0	0	701	0	1132
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	1.00	1.00	0.91	1.00	1.00	1.00	1.00	0.89	1.00	0.89
Lanes:	0.00	4.00	0.00	0.00	4.00	0.00	0.00	0.00	0.00	1.38	0.00	1.62
Final Sat.:	0	6916	0	0	6916	0	0	0	0	2337	0	2735
Capacity Analysis Module:												
Vol/Sat:	0.00	0.22	0.00	0.00	0.36	0.00	0.00	0.00	0.00	0.30	0.00	0.41
Crit Moves:	****				****							****
Green/Cycle:	0.00	0.44	0.00	0.00	0.44	0.00	0.00	0.00	0.00	0.51	0.00	0.51
Volume/Cap:	0.00	0.50	0.00	0.00	0.81	0.00	0.00	0.00	0.00	0.59	0.00	0.81
Delay/Veh:	0.0	20.2	0.0	0.0	26.1	0.0	0.0	0.0	0.0	17.5	0.0	22.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	20.2	0.0	0.0	26.1	0.0	0.0	0.0	0.0	17.5	0.0	22.9
LOS by Move:	A	C	A	A	C	A	A	A	A	B	A	C
HCM2k85thQ:	0	14	0	0	29	0	0	0	0	17	0	29

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #18 Harbor Blvd and I-405 EB Off-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.630
Loss Time (sec):	5	Average Delay (sec/veh):	16.2
Optimal Cycle:	32	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Ignore			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 3	0 1	0	0 4	0 0	1	0 1!	0 1	0	0 0	0 0
Volume Module:												
Base Vol:	0	1861	0	0	2147	0	157	0	722	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1861	0	0	2147	0	157	0	722	0	0	0
User Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1861	0	0	2147	0	157	0	722	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1861	0	0	2147	0	157	0	722	0	0	0
PCE Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1861	0	0	2147	0	157	0	722	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	1.00	1.00	0.91	1.00	0.87	1.00	0.87	1.00	1.00	1.00
Lanes:	0.00	3.00	1.00	0.00	4.00	0.00	1.18	0.00	1.82	0.00	0.00	0.00
Final Sat.:	0	5187	1900	0	6916	0	1946	0	3008	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.36	0.00	0.00	0.31	0.00	0.08	0.00	0.24	0.00	0.00	0.00
Crit Moves:		****			****				****			
Green/Cycle:	0.00	0.57	0.00	0.00	0.57	0.00	0.38	0.00	0.38	0.00	0.00	0.00
Volume/Cap:	0.00	0.63	0.00	0.00	0.55	0.00	0.21	0.00	0.63	0.00	0.00	0.00
Delay/Veh:	0.0	14.9	0.0	0.0	13.6	0.0	20.9	0.0	26.2	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	14.9	0.0	0.0	13.6	0.0	20.9	0.0	26.2	0.0	0.0	0.0
LOS by Move:	A	B	A	A	B	A	C	A	C	A	A	A
HCM2k85thQ:	0	21	0	0	17	0	4	0	16	0	0	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #19 Fairview St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.662
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	34	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	0 1	0	1 0	1 1	0	0 1
Volume Module:												
Base Vol:	5	1430	402	144	1338	0	2	5	4	438	0	162
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	5	1430	402	144	1338	0	2	5	4	438	0	162
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	5	1430	402	144	1338	0	2	5	4	438	0	162
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	5	1430	402	144	1338	0	2	5	4	438	0	162
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	5	1430	402	144	1338	0	2	5	4	438	0	162
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.00	1.00	1.00	1.06	1.00
Lanes:	1.00	2.34	0.66	1.00	3.00	0.00	0.36	0.91	0.73	2.00	0.00	1.00
Final Sat.:	1600	3947	1053	1600	5000	0	582	1455	1164	3200	0	1600
Capacity Analysis Module:												
Vol/Sat:	0.00	0.36	0.38	0.09	0.27	0.00	0.00	0.00	0.00	0.14	0.00	0.10
Crit Moves:			****	****				****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #20 Fairview St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.883
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	78	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			WideBypass			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	2 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	149	1094	99	203	1264	251	215	870	151	159	1204	186
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	149	1094	99	203	1264	251	215	870	151	159	1204	186
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	149	1094	99	203	1264	251	215	870	151	159	1204	186
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	149	1094	99	203	1264	251	215	870	151	159	1204	186
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	149	1094	99	203	1264	251	215	870	151	159	1204	186
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	3.00	1.00	1.00	2.50	0.50	1.00	2.56	0.44	1.00	2.60	0.40
Final Sat.:	1600	5100	1600	1600	4205	795	1600	4290	710	1600	4358	642
Capacity Analysis Module:												
Vol/Sat:	0.09	0.21	0.06	0.13	0.30	0.32	0.13	0.20	0.21	0.10	0.28	0.29
Crit Moves:	****					****	****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #21 Fairview St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.798
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	52	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	159	1462	127	155	1257	196	244	659	163	158	545	134
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	159	1462	127	155	1257	196	244	659	163	158	545	134
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	159	1462	127	155	1257	196	244	659	163	158	545	134
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	159	1462	127	155	1257	196	244	659	163	158	545	134
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	159	1462	127	155	1257	196	244	659	163	158	545	134
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	1.60	0.40	1.00	1.61	0.39
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	2665	635	1600	2668	632
Capacity Analysis Module:												
Vol/Sat:	0.10	0.29	0.08	0.10	0.25	0.12	0.15	0.25	0.26	0.10	0.20	0.21
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #22 Fairview St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.811
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	54	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	188	1793	110	198	616	151	230	703	188	373	789	173
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	188	1793	110	198	616	151	230	703	188	373	789	173
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	188	1793	110	198	616	151	230	703	188	373	789	173
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	188	1793	110	198	616	151	230	703	188	373	789	173
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	188	1793	110	198	616	151	230	703	188	373	789	173
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	2.00	2.83	0.17	2.00	2.41	0.59	2.00	2.37	0.63	2.00	2.46	0.54
Final Sat.:	3200	4723	277	3200	4055	945	3200	3987	1013	3200	4137	863
Capacity Analysis Module:												
Vol/Sat:	0.06	0.38	0.40	0.06	0.15	0.16	0.07	0.18	0.19	0.12	0.19	0.20
Crit Moves:			****	****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #23 Fairview St and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.845
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	63	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	217	1728	161	188	951	96	197	955	148	188	1108	240
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	217	1728	161	188	951	96	197	955	148	188	1108	240
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	217	1728	161	188	951	96	197	955	148	188	1108	240
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	217	1728	161	188	951	96	197	955	148	188	1108	240
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	217	1728	161	188	951	96	197	955	148	188	1108	240
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	2.00	2.74	0.26	2.00	2.72	0.28	2.00	2.60	0.40	2.00	2.47	0.53
Final Sat.:	3200	4591	409	3200	4560	440	3200	4356	644	3200	4145	855
Capacity Analysis Module:												
Vol/Sat:	0.07	0.38	0.39	0.06	0.21	0.22	0.06	0.22	0.23	0.06	0.27	0.28
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #24 Fairview St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.826
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	58	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	243	1672	91	171	955	88	280	731	246	179	1365	239
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	243	1672	91	171	955	88	280	731	246	179	1365	239
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	243	1672	91	171	955	88	280	731	246	179	1365	239
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	243	1672	91	171	955	88	280	731	246	179	1365	239
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	243	1672	91	171	955	88	280	731	246	179	1365	239
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.85	0.15	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	4752	248	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.08	0.35	0.37	0.05	0.19	0.06	0.09	0.14	0.15	0.06	0.27	0.15
Crit Moves:			****	****			****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #25 Fairview Rd and Sunflower Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.708
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	38	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	2 0	1 1	0	2 0	2 0	1
Volume Module:												
Base Vol:	201	1806	393	144	1196	83	203	430	95	265	664	173
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	201	1806	393	144	1196	83	203	430	95	265	664	173
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	201	1806	393	144	1196	83	203	430	95	265	664	173
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	201	1806	393	144	1196	83	203	430	95	265	664	173
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	201	1806	393	144	1196	83	203	430	95	265	664	173
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	2.81	0.19	2.00	1.64	0.36	2.00	2.00	1.00
Final Sat.:	3200	5100	1600	3200	4689	311	3200	2721	579	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.06	0.35	0.25	0.05	0.26	0.27	0.06	0.16	0.16	0.08	0.20	0.11
Crit Moves:		****			****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #26 Greenville St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.586
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	28	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	0 1	0 0	0	0 0	0 0	2	1 0	1 0	2	0 0
Volume Module:												
Base Vol:	327	0	218	0	0	0	0	982	200	95	1128	0
Growth Adj:	1.00 1.00	1.00		1.00 1.00	1.00		1.00 1.00	1.00		1.00 1.00	1.00	
Initial Bse:	327	0	218	0	0	0	0	982	200	95	1128	0
User Adj:	1.00 1.00	1.00		1.00 1.00	1.00		1.00 1.00	1.00		1.00 1.00	1.00	
PHF Adj:	1.00 1.00	1.00		1.00 1.00	1.00		1.00 1.00	1.00		1.00 1.00	1.00	
PHF Volume:	327	0	218	0	0	0	0	982	200	95	1128	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	327	0	218	0	0	0	0	982	200	95	1128	0
PCE Adj:	1.00 1.00	1.00		1.00 1.00	1.00		1.00 1.00	1.00		1.00 1.00	1.00	
MLF Adj:	1.00 1.00	1.00		1.00 1.00	1.00		1.00 1.00	1.00		1.00 1.00	1.00	
FinalVolume:	327	0	218	0	0	0	0	982	200	95	1128	0
Saturation Flow Module:												
Sat/Lane:	1600 1600	1600		1600 1600	1600		1600 1600	1600		1600 1600	1600	
Adjustment:	1.00 1.06	1.00		1.00 1.06	1.00		1.00 1.05	1.00		1.00 1.06	1.00	
Lanes:	1.00 0.00	1.00		0.00 0.00	0.00		0.00 2.49	0.51		1.00 2.00	0.00	
Final Sat.:	1600	0	1600	0	0	0	0	4188	812	1600	3400	0
Capacity Analysis Module:												
Vol/Sat:	0.20 0.00	0.14		0.00 0.00	0.00		0.00 0.23	0.25		0.06 0.33	0.00	
Crit Moves:	****						****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #27 Greenville St and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.686
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	36	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			WideBypass			WideBypass		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	0 1	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	57	476	36	61	200	61	108	823	47	27	1037	148
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	57	476	36	61	200	61	108	823	47	27	1037	148
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	57	476	36	61	200	61	108	823	47	27	1037	148
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	57	476	36	61	200	61	108	823	47	27	1037	148
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	57	476	36	61	200	61	108	823	47	27	1037	148
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.06	1.00	1.00	1.03	1.00	1.00	1.04	1.00
Lanes:	1.00	1.86	0.14	1.00	1.00	1.00	1.00	1.89	0.11	1.00	1.75	0.25
Final Sat.:	1600	3075	225	1600	1700	1600	1600	3127	173	1600	2900	400
Capacity Analysis Module:												
Vol/Sat:	0.04	0.15	0.16	0.04	0.12	0.04	0.07	0.26	0.27	0.02	0.36	0.37
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #28 Raitt St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.722
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	40	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	58	835	130	45	474	76	126	747	65	88	742	102
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	58	835	130	45	474	76	126	747	65	88	742	102
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	58	835	130	45	474	76	126	747	65	88	742	102
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	58	835	130	45	474	76	126	747	65	88	742	102
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	58	835	130	45	474	76	126	747	65	88	742	102
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.03	1.00	1.00	1.04	1.00
Lanes:	1.00	1.73	0.27	1.00	1.72	0.28	1.00	1.84	0.16	1.00	1.76	0.24
Final Sat.:	1600	2869	431	1600	2858	442	1600	3044	256	1600	2913	387
Capacity Analysis Module:												
Vol/Sat:	0.04	0.29	0.30	0.03	0.17	0.17	0.08	0.25	0.25	0.06	0.25	0.26
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #29 Raitt St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.864
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	70	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			WideBypass			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	190	816	229	111	384	96	139	961	137	86	972	87
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	190	816	229	111	384	96	139	961	137	86	972	87
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	190	816	229	111	384	96	139	961	137	86	972	87
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	190	816	229	111	384	96	139	961	137	86	972	87
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	190	816	229	111	384	96	139	961	137	86	972	87
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.03	1.00
Lanes:	1.00	1.56	0.44	1.00	1.60	0.40	1.00	1.75	0.25	1.00	1.84	0.16
Final Sat.:	1600	2599	701	1600	2660	640	1600	2901	399	1600	3037	263
Capacity Analysis Module:												
Vol/Sat:	0.12	0.31	0.33	0.07	0.14	0.15	0.09	0.33	0.34	0.05	0.32	0.33
Crit Moves:		****	****				****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #30 Bear St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.868
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	71	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	1	1 0	2 0	1	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	200	962	251	112	271	82	97	910	94	93	1673	266
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	200	962	251	112	271	82	97	910	94	93	1673	266
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	200	962	251	112	271	82	97	910	94	93	1673	266
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	200	962	251	112	271	82	97	910	94	93	1673	266
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	200	962	251	112	271	82	97	910	94	93	1673	266
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.72	0.28	1.00	2.59	0.41
Final Sat.:	1600	3400	1600	1600	3400	1600	1600	4551	449	1600	4342	658
Capacity Analysis Module:												
Vol/Sat:	0.13	0.28	0.16	0.07	0.08	0.05	0.06	0.20	0.21	0.06	0.39	0.40
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #31 Bristol St and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.692
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	36	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	2 0	2 1	0	2 0	3 0	1
Volume Module:												
Base Vol:	236	1148	191	243	1293	220	336	750	136	284	1069	362
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	236	1148	191	243	1293	220	336	750	136	284	1069	362
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	236	1148	191	243	1293	220	336	750	136	284	1069	362
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	236	1148	191	243	1293	220	336	750	136	284	1069	362
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	236	1148	191	243	1293	220	336	750	136	284	1069	362
OvlAdjVol:												241
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	2.54	0.46	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	4263	737	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.07	0.23	0.12	0.08	0.25	0.14	0.11	0.18	0.18	0.09	0.21	0.23
OvlAdjV/S:												0.15
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #32 Bristol St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.832
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	60	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	200	1399	73	136	1250	128	190	534	63	169	723	82
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	200	1399	73	136	1250	128	190	534	63	169	723	82
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	200	1399	73	136	1250	128	190	534	63	169	723	82
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	200	1399	73	136	1250	128	190	534	63	169	723	82
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	200	1399	73	136	1250	128	190	534	63	169	723	82
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.03	1.00	1.00	1.03	1.00
Lanes:	1.00	2.85	0.15	1.00	2.72	0.28	1.00	1.79	0.21	1.00	1.80	0.20
Final Sat.:	1600	4762	238	1600	4554	446	1600	2962	338	1600	2974	326
Capacity Analysis Module:												
Vol/Sat:	0.13	0.29	0.31	0.09	0.27	0.29	0.12	0.18	0.19	0.11	0.24	0.25
Crit Moves:	****					****	****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #33 Bristol St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.657
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	33	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	75	1524	87	105	1402	53	59	163	38	233	371	100
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	75	1524	87	105	1402	53	59	163	38	233	371	100
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	75	1524	87	105	1402	53	59	163	38	233	371	100
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	75	1524	87	105	1402	53	59	163	38	233	371	100
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	75	1524	87	105	1402	53	59	163	38	233	371	100
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	2.84	0.16	1.00	2.89	0.11	1.00	1.62	0.38	1.00	1.58	0.42
Final Sat.:	1600	4741	259	1600	4825	175	1600	2695	605	1600	2621	679
Capacity Analysis Module:												
Vol/Sat:	0.05	0.32	0.34	0.07	0.29	0.30	0.04	0.06	0.06	0.15	0.14	0.15
Crit Moves:			****	****				****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #34 Bristol St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.779
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	48	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	286	1235	182	206	1115	252	189	1062	136	217	1330	126
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	286	1235	182	206	1115	252	189	1062	136	217	1330	126
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	286	1235	182	206	1115	252	189	1062	136	217	1330	126
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	286	1235	182	206	1115	252	189	1062	136	217	1330	126
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	286	1235	182	206	1115	252	189	1062	136	217	1330	126
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	1.00	2.66	0.34	1.00	2.74	0.26
Final Sat.:	3200	5100	1600	3200	5100	1600	1600	4455	545	1600	4585	415
Capacity Analysis Module:												
Vol/Sat:	0.09	0.24	0.11	0.06	0.22	0.16	0.12	0.24	0.25	0.14	0.29	0.30
Crit Moves:	****			****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #35 Bristol St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.752
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	44	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	213	1566	85	84	1171	270	262	512	102	97	504	59
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	213	1566	85	84	1171	270	262	512	102	97	504	59
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	213	1566	85	84	1171	270	262	512	102	97	504	59
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	213	1566	85	84	1171	270	262	512	102	97	504	59
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	213	1566	85	84	1171	270	262	512	102	97	504	59
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.03	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	1.67	0.33	1.00	1.79	0.21
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	2768	532	1600	2965	335
Capacity Analysis Module:												
Vol/Sat:	0.13	0.31	0.05	0.05	0.23	0.17	0.16	0.18	0.19	0.06	0.17	0.18
Crit Moves:	****				****		****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #36 Bristol St and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.790
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	50	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 0	1	2 0	2 1	0	1 0	3 0	1	1 0	3 0	1
Volume Module:												
Base Vol:	309	1157	211	251	838	163	193	868	153	173	1021	299
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	309	1157	211	251	838	163	193	868	153	173	1021	299
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	309	1157	211	251	838	163	193	868	153	173	1021	299
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	309	1157	211	251	838	163	193	868	153	173	1021	299
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	309	1157	211	251	838	163	193	868	153	173	1021	299
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.00	1.00	2.00	2.51	0.49	1.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	3400	1600	3200	4218	782	1600	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.10	0.34	0.13	0.08	0.20	0.21	0.12	0.17	0.10	0.11	0.20	0.19
Crit Moves:		****			****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #37 Bristol St and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.909
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	92	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	179	1322	233	92	843	176	200	716	57	119	1074	54
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	179	1322	233	92	843	176	200	716	57	119	1074	54
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	179	1322	233	92	843	176	200	716	57	119	1074	54
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	179	1322	233	92	843	176	200	716	57	119	1074	54
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	179	1322	233	92	843	176	200	716	57	119	1074	54
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.03	1.00	1.00	1.03	1.00
Lanes:	1.00	2.55	0.45	1.00	2.48	0.52	1.00	1.85	0.15	1.00	1.90	0.10
Final Sat.:	1600	4281	719	1600	4171	829	1600	3064	236	1600	3147	153
Capacity Analysis Module:												
Vol/Sat:	0.11	0.31	0.32	0.06	0.20	0.21	0.13	0.23	0.24	0.07	0.34	0.35
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #38 Bristol St and Alton Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.686
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	36	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	3 0	1	1 0	1 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	43	1681	158	119	896	56	75	105	195	91	150	29
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	43	1681	158	119	896	56	75	105	195	91	150	29
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	43	1681	158	119	896	56	75	105	195	91	150	29
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	43	1681	158	119	896	56	75	105	195	91	150	29
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	43	1681	158	119	896	56	75	105	195	91	150	29
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	1.00	2.74	0.26	1.00	3.00	1.00	1.00	1.00	1.00	1.00	0.84	0.16
Final Sat.:	1600	4588	412	1600	5100	1600	1600	1700	1600	1600	1341	259
Capacity Analysis Module:												
Vol/Sat:	0.03	0.37	0.38	0.07	0.18	0.04	0.05	0.06	0.12	0.06	0.11	0.11
Crit Moves:			****	****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #39 Bristol St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.812
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	55	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	335	1428	260	223	821	129	357	796	169	265	1536	290
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	335	1428	260	223	821	129	357	796	169	265	1536	290
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	335	1428	260	223	821	129	357	796	169	265	1536	290
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	335	1428	260	223	821	129	357	796	169	265	1536	290
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	335	1428	260	223	821	129	357	796	169	265	1536	290
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	2.59	0.41	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	4348	652	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.10	0.28	0.16	0.07	0.19	0.20	0.11	0.16	0.11	0.08	0.30	0.18
Crit Moves:		****			****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #40 Bristol St and Sunflower Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.689
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	36	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 1	2 0	3	0 1	2 0	2	1 1	2 0	3	0 1
Volume Module:												
Base Vol:	645	1363	220	201	750	215	293	573	222	263	1012	322
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	645	1363	220	201	750	215	293	573	222	263	1012	322
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	645	1363	220	201	750	215	293	573	222	263	1012	322
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	645	1363	220	201	750	215	293	573	222	263	1012	322
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	645	1363	220	201	750	215	293	573	222	263	1012	322
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	2.88	1.12	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	4813	1787	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.20	0.27	0.14	0.06	0.15	0.13	0.09	0.12	0.12	0.08	0.20	0.20
Crit Moves:	****				****		****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #41 Bristol St and I-405 WB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.829
Loss Time (sec):	5	Average Delay (sec/veh):	28.2
Optimal Cycle:	61	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Ignore			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 4	0 1	0	0 4	1 0	0	0 0	0 2	1	1 1	0 2
Volume Module:												
Base Vol:	0	2094	183	0	2148	20	0	0	194	357	306	1184
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	2094	183	0	2148	20	0	0	194	357	306	1184
User Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	2094	0	0	2148	20	0	0	194	357	306	1184
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	2094	0	0	2148	20	0	0	194	357	306	1184
PCE Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	2094	0	0	2148	20	0	0	194	357	306	1184
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	1.00	1.00	0.91	0.91	1.00	1.00	0.75	0.93	0.93	0.75
Lanes:	0.00	4.00	1.00	0.00	4.95	0.05	0.00	0.00	2.00	1.62	1.38	2.00
Final Sat.:	0	6916	1900	0	8557	80	0	0	2842	2840	2434	2842
Capacity Analysis Module:												
Vol/Sat:	0.00	0.30	0.00	0.00	0.25	0.25	0.00	0.00	0.07	0.13	0.13	0.42
Crit Moves:		****		****					****			****
Green/Cycle:	0.00	0.37	0.00	0.00	0.37	0.37	0.00	0.00	0.08	0.50	0.50	0.50
Volume/Cap:	0.00	0.83	0.00	0.00	0.69	0.69	0.00	0.00	0.83	0.25	0.25	0.83
Delay/Veh:	0.0	31.3	0.0	0.0	27.5	27.5	0.0	0.0	66.4	14.2	14.2	25.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	31.3	0.0	0.0	27.5	27.5	0.0	0.0	66.4	14.2	14.2	25.4
LOS by Move:	A	C	A	A	C	C	A	A	E	B	B	C
HCM2k85thQ:	0	27	0	0	20	20	0	0	9	6	6	29

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #42 Bristol St and I-405 EB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.595
Loss Time (sec):	5	Average Delay (sec/veh):	18.4
Optimal Cycle:	30	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Ignore			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	4 0	0	0 0	2 1	1	3 0	0 0	1	0 0	0 0	0
Volume Module:												
Base Vol:	127	1555	0	0	1506	880	740	0	331	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	127	1555	0	0	1506	880	740	0	331	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	127	1555	0	0	1506	0	740	0	331	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	127	1555	0	0	1506	0	740	0	331	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	127	1555	0	0	1506	0	740	0	331	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	1.00	1.00	0.91	0.91	0.92	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	4.00	0.00	0.00	3.00	1.00	3.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	6916	0	0	5187	1729	5253	0	1615	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.07	0.22	0.00	0.00	0.29	0.00	0.14	0.00	0.20	0.00	0.00	0.00
Crit Moves:	****				****				****			
Green/Cycle:	0.12	0.61	0.00	0.00	0.49	0.00	0.34	0.00	0.34	0.00	0.00	0.00
Volume/Cap:	0.60	0.37	0.00	0.00	0.60	0.00	0.41	0.00	0.60	0.00	0.00	0.00
Delay/Veh:	46.3	10.1	0.0	0.0	18.9	0.0	25.2	0.0	28.8	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	46.3	10.1	0.0	0.0	18.9	0.0	25.2	0.0	28.8	0.0	0.0	0.0
LOS by Move:	D	B	A	A	B	A	C	A	C	A	A	A
HCM2k85thQ:	7	10	0	0	19	0	9	0	14	0	0	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #43 Flower St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.571
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	27	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Prot+Permit			Prot+Permit			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	1	1 0	2 0	1	1 0	3 0	1	1 0	2 0	1
Volume Module:												
Base Vol:	124	754	62	104	530	48	134	386	63	146	510	151
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	124	754	62	104	530	48	134	386	63	146	510	151
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	124	754	62	104	530	48	134	386	63	146	510	151
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	124	754	62	104	530	48	134	386	63	146	510	151
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	124	754	62	104	530	48	134	386	63	146	510	151
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	3.00	1.00	1.00	2.00	1.00
Final Sat.:	1600	3400	1600	1600	3400	1600	1600	5100	1600	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.08	0.22	0.04	0.07	0.16	0.03	0.08	0.08	0.04	0.09	0.15	0.09
Crit Moves:		****			****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #44 Flower St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.752
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	44	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	1	1 0	1 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	132	625	149	170	466	135	139	1217	101	210	1415	147
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	132	625	149	170	466	135	139	1217	101	210	1415	147
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	132	625	149	170	466	135	139	1217	101	210	1415	147
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	132	625	149	170	466	135	139	1217	101	210	1415	147
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	132	625	149	170	466	135	139	1217	101	210	1415	147
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.00	1.00	1.00	1.55	0.45	1.00	2.77	0.23	1.00	2.72	0.28
Final Sat.:	1600	3400	1600	1600	2581	719	1600	4632	368	1600	4548	452
Capacity Analysis Module:												
Vol/Sat:	0.08	0.18	0.09	0.11	0.18	0.19	0.09	0.26	0.27	0.13	0.31	0.33
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #45 Flower St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.920
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	100	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	1 0	1 0	0	1 0	1 0	0	1 0	1 0	0	1 0
Volume Module:												
Base Vol:	42	567	35	46	352	57	64	478	19	48	595	85
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	42	567	35	46	352	57	64	478	19	48	595	85
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	42	567	35	46	352	57	64	478	19	48	595	85
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	42	567	35	46	352	57	64	478	19	48	595	85
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	42	567	35	46	352	57	64	478	19	48	595	85
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	0.94	0.06	1.00	0.86	0.14	1.00	0.96	0.04	1.00	0.88	0.12
Final Sat.:	1600	1507	93	1600	1377	223	1600	1539	61	1600	1400	200
Capacity Analysis Module:												
Vol/Sat:	0.03	0.38	0.38	0.03	0.26	0.26	0.04	0.31	0.31	0.03	0.43	0.43
Crit Moves:		****			****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #46 Flower St and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.897
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	85	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	120	843	80	66	369	106	158	786	83	75	1258	81
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	120	843	80	66	369	106	158	786	83	75	1258	81
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	120	843	80	66	369	106	158	786	83	75	1258	81
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	120	843	80	66	369	106	158	786	83	75	1258	81
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	120	843	80	66	369	106	158	786	83	75	1258	81
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.03	1.00	1.00	1.03	1.00
Lanes:	1.00	1.83	0.17	1.00	1.55	0.45	1.00	1.81	0.19	1.00	1.88	0.12
Final Sat.:	1600	3023	277	1600	2586	714	1600	2994	306	1600	3106	194
Capacity Analysis Module:												
Vol/Sat:	0.08	0.28	0.29	0.04	0.14	0.15	0.10	0.26	0.27	0.05	0.40	0.42
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #47 Flower St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.906
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	90	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	133	634	74	85	205	197	209	994	63	62	1989	177
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	133	634	74	85	205	197	209	994	63	62	1989	177
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	133	634	74	85	205	197	209	994	63	62	1989	177
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	133	634	74	85	205	197	209	994	63	62	1989	177
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	133	634	74	85	205	197	209	994	63	62	1989	177
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	1.79	0.21	1.00	1.02	0.98	1.00	2.82	0.18	1.00	2.75	0.25
Final Sat.:	1600	2966	334	1600	1732	1568	1600	4714	286	1600	4608	392
Capacity Analysis Module:												
Vol/Sat:	0.08	0.21	0.22	0.05	0.12	0.13	0.13	0.21	0.22	0.04	0.43	0.45
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #48 Main St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.628
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	31	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	2 0	2 1	0	2 0	2 0	1
Volume Module:												
Base Vol:	307	938	439	145	911	235	296	422	189	257	513	206
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	307	938	439	145	911	235	296	422	189	257	513	206
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	307	938	439	145	911	235	296	422	189	257	513	206
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	307	938	439	145	911	235	296	422	189	257	513	206
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	307	938	439	145	911	235	296	422	189	257	513	206
OvlAdjVol:			310									134
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	2.38	0.62	2.00	2.07	0.93	2.00	2.00	1.00
Final Sat.:	3200	5100	1600	3200	4016	984	3200	3515	1485	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.10	0.18	0.27	0.05	0.23	0.24	0.09	0.12	0.13	0.08	0.15	0.13
OvlAdjV/S:			0.19									0.08
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #49 Main St and Mainplace Dr / Memory Ln

Cycle (sec):	100	Critical Vol./Cap.(X):	0.471
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	22	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 0	2 0	2	1 0	2 0	2	1 0	2 0	2	0 1
Volume Module:												
Base Vol:	90	945	179	51	1055	139	293	325	87	185	151	73
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	90	945	179	51	1055	139	293	325	87	185	151	73
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	90	945	179	51	1055	139	293	325	87	185	151	73
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	90	945	179	51	1055	139	293	325	87	185	151	73
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	90	945	179	51	1055	139	293	325	87	185	151	73
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	2.52	0.48	2.00	2.65	0.35	2.00	2.37	0.63	2.00	2.00	1.00
Final Sat.:	3200	4236	764	3200	4441	559	3200	3986	1014	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.22	0.23	0.02	0.24	0.25	0.09	0.08	0.09	0.06	0.04	0.05
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #50 Main St and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.732
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	41	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 0	1	2 0	1 1	0	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	197	1019	222	173	757	117	221	1237	73	178	1089	72
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	197	1019	222	173	757	117	221	1237	73	178	1089	72
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	197	1019	222	173	757	117	221	1237	73	178	1089	72
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	197	1019	222	173	757	117	221	1237	73	178	1089	72
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	197	1019	222	173	757	117	221	1237	73	178	1089	72
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	2.00	2.00	1.00	2.00	1.73	0.27	2.00	2.83	0.17	2.00	2.81	0.19
Final Sat.:	3200	3400	1600	3200	2872	428	3200	4733	267	3200	4702	298
Capacity Analysis Module:												
Vol/Sat:	0.06	0.30	0.14	0.05	0.26	0.27	0.07	0.26	0.27	0.06	0.23	0.24
Crit Moves:		****		****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #51 Main St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.801
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	52	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	89	1126	94	61	995	63	159	881	105	38	384	39
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	89	1126	94	61	995	63	159	881	105	38	384	39
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	89	1126	94	61	995	63	159	881	105	38	384	39
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	89	1126	94	61	995	63	159	881	105	38	384	39
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	89	1126	94	61	995	63	159	881	105	38	384	39
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.03	1.00
Lanes:	1.00	1.85	0.15	1.00	1.88	0.12	1.00	1.79	0.21	1.00	1.82	0.18
Final Sat.:	1600	3053	247	1600	3109	191	1600	2959	341	1600	3005	295
Capacity Analysis Module:												
Vol/Sat:	0.06	0.37	0.38	0.04	0.32	0.33	0.10	0.30	0.31	0.02	0.13	0.13
Crit Moves:			****	****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #52 Main St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.607
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	29	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	0	0 0	1 1	0	0 0	0 0	0	0 1	1 1	0
Volume Module:												
Base Vol:	54	1229	0	0	1083	53	0	0	0	65	656	85
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	54	1229	0	0	1083	53	0	0	0	65	656	85
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	54	1229	0	0	1083	53	0	0	0	65	656	85
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	54	1229	0	0	1083	53	0	0	0	65	656	85
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	54	1229	0	0	1083	53	0	0	0	65	656	85
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.03	1.00	1.00	1.06	1.00	1.00	1.03	1.00
Lanes:	1.00	2.00	0.00	0.00	1.91	0.09	0.00	0.00	0.00	0.24	2.44	0.32
Final Sat.:	1600	3400	0	0	3151	149	0	0	0	387	4007	506
Capacity Analysis Module:												
Vol/Sat:	0.03	0.36	0.00	0.00	0.34	0.36	0.00	0.00	0.00	0.04	0.16	0.17
Crit Moves:	****					****						****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #53 Main St and 4th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.537
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	25	Level Of Service:	A

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	- T	- R		L	- T	- R		L	- T	- R		L	- T	- R	
Control:	Permitted				Permitted				Permitted				Permitted			
Rights:	Include				Include				Include				Include			
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 1	1	0	0	0 1	1	0	0	0 0	1	0	0	0 0	1	0
Volume Module:																
Base Vol:	0	1055	50	0	1037	47	0		98	55	0		181	46		
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1055	50	0	1037	47	0		98	55	0		181	46		
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1055	50	0	1037	47	0		98	55	0		181	46		
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1055	50	0	1037	47	0		98	55	0		181	46		
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1055	50	0	1037	47	0		98	55	0		181	46		
Saturation Flow Module:																
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.00	1.91	0.09	0.00	1.91	0.09	0.00	0.64	0.36	0.00	0.80	0.20				
Final Sat.:	0	3155	145	0	3161	139	0		1025	575	0		1276	324		
Capacity Analysis Module:																
Vol/Sat:	0.00	0.33	0.35	0.00	0.33	0.34	0.00	0.10	0.10	0.00	0.14	0.14				
Crit Moves:			****	****				****				****				

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #54 Main St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.799
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	52	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	2	0 1	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	169	826	72	175	671	143	120	1214	90	114	1275	89
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	169	826	72	175	671	143	120	1214	90	114	1275	89
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	169	826	72	175	671	143	120	1214	90	114	1275	89
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	169	826	72	175	671	143	120	1214	90	114	1275	89
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	169	826	72	175	671	143	120	1214	90	114	1275	89
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	1.84	0.16	1.00	2.00	1.00	1.00	2.79	0.21	1.00	2.80	0.20
Final Sat.:	1600	3043	257	1600	3400	1600	1600	4669	331	1600	4687	313
Capacity Analysis Module:												
Vol/Sat:	0.11	0.27	0.28	0.11	0.20	0.09	0.08	0.26	0.27	0.07	0.27	0.28
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #55 Main St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.845
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	64	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Prot+Permit			Prot+Permit			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	0	1 0	1 0	1	0 1
Volume Module:												
Base Vol:	146	1147	71	134	908	66	78	345	42	85	480	216
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	146	1147	71	134	908	66	78	345	42	85	480	216
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	146	1147	71	134	908	66	78	345	42	85	480	216
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	146	1147	71	134	908	66	78	345	42	85	480	216
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	146	1147	71	134	908	66	78	345	42	85	480	216
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.00	1.00	1.00	1.06	1.00
Lanes:	1.00	1.88	0.12	1.00	1.86	0.14	1.00	0.89	0.11	1.00	1.00	1.00
Final Sat.:	1600	3113	187	1600	3083	217	1600	1426	174	1600	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.09	0.37	0.38	0.08	0.29	0.30	0.05	0.24	0.24	0.05	0.28	0.14
Crit Moves:		****	****				****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #56 Main St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.847
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	64	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	2	0 1	1 0	2	1 0
Volume Module:												
Base Vol:	99	1050	48	122	609	83	127	752	84	97	1296	134
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	99	1050	48	122	609	83	127	752	84	97	1296	134
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	99	1050	48	122	609	83	127	752	84	97	1296	134
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	99	1050	48	122	609	83	127	752	84	97	1296	134
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	99	1050	48	122	609	83	127	752	84	97	1296	134
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.05	1.00
Lanes:	1.00	1.91	0.09	1.00	1.76	0.24	1.00	2.00	1.00	1.00	2.72	0.28
Final Sat.:	1600	3160	140	1600	2916	384	1600	3400	1600	1600	4550	450
Capacity Analysis Module:												
Vol/Sat:	0.06	0.33	0.34	0.08	0.21	0.22	0.08	0.22	0.05	0.06	0.28	0.30
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #57 Main St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.777
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	48	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	488	1169	321	283	368	278	298	613	59	198	1570	456
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	488	1169	321	283	368	278	298	613	59	198	1570	456
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	488	1169	321	283	368	278	298	613	59	198	1570	456
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	488	1169	321	283	368	278	298	613	59	198	1570	456
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	488	1169	321	283	368	278	298	613	59	198	1570	456
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.15	0.23	0.20	0.09	0.07	0.17	0.09	0.12	0.04	0.06	0.31	0.28
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #58 Penn Wy and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.770
Loss Time (sec):	5	Average Delay (sec/veh):	20.7
Optimal Cycle:	48	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	0	0 2	0	0	0 0 0	0	0	2 1 0	1	0	3 0 0
Volume Module:												
Base Vol:	848	0	414	0	0	0	0	1590	513	123	1440	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	848	0	414	0	0	0	0	1590	513	123	1440	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	848	0	414	0	0	0	0	1590	513	123	1440	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	848	0	414	0	0	0	0	1590	513	123	1440	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	848	0	414	0	0	0	0	1590	513	123	1440	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.75	1.00	1.00	1.00	1.00	0.88	0.88	0.95	0.91	1.00
Lanes:	2.00	0.00	2.00	0.00	0.00	0.00	0.00	2.27	0.73	1.00	3.00	0.00
Final Sat.:	3502	0	2842	0	0	0	0	3777	1218	1805	5187	0
Capacity Analysis Module:												
Vol/Sat:	0.24	0.00	0.15	0.00	0.00	0.00	0.00	0.42	0.42	0.07	0.28	0.00
Crit Moves:	****							****		****		
Green/Cycle:	0.31	0.00	0.31	0.00	0.00	0.00	0.00	0.55	0.55	0.09	0.64	0.00
Volume/Cap:	0.77	0.00	0.46	0.00	0.00	0.00	0.00	0.77	0.77	0.77	0.44	0.00
Delay/Veh:	34.4	0.0	27.9	0.0	0.0	0.0	0.0	19.1	19.1	64.7	9.3	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	34.4	0.0	27.9	0.0	0.0	0.0	0.0	19.1	19.1	64.7	9.3	0.0
LOS by Move:	C	A	C	A	A	A	A	B	B	E	A	A
HCM2k85thQ:	21	0	9	0	0	0	0	29	29	9	12	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #59 I-5 NB Off Ramps/17th Street

Cycle (sec):	100	Critical Vol./Cap.(X):	0.589
Loss Time (sec):	5	Average Delay (sec/veh):	19.5
Optimal Cycle:	30	Level Of Service:	B

Street Name:	I-5 NB Off Ramps						17th Street					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Permitted		
Rights:	Include			Include			Ignore			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 1	0	0 1	1	0 0	0 1	1	0	3 0 1	0	0	2 1 0
Volume Module:												
Base Vol:	345	46	18	51	0	133	120	1099	722	0	1525	41
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	345	46	18	51	0	133	120	1099	722	0	1525	41
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Volume:	345	46	18	51	0	133	120	1099	0	0	1525	41
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	345	46	18	51	0	133	120	1099	0	0	1525	41
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
FinalVolume:	345	46	18	51	0	133	120	1099	0	0	1525	41
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.96	0.96	0.85	0.95	1.00	0.85	0.95	0.91	1.00	1.00	0.91	0.91
Lanes:	1.76	0.24	1.00	1.00	0.00	1.00	1.00	3.00	1.00	0.00	2.92	0.08
Final Sat.:	3212	428	1615	1805	0	1615	1805	5187	1900	0	5031	135
Capacity Analysis Module:												
Vol/Sat:	0.11	0.11	0.01	0.03	0.00	0.08	0.07	0.21	0.00	0.00	0.30	0.30
Crit Moves:		****				****	****				****	
Green/Cycle:	0.18	0.18	0.18	0.14	0.00	0.14	0.11	0.63	0.00	0.00	0.51	0.51
Volume/Cap:	0.59	0.59	0.06	0.20	0.00	0.59	0.59	0.34	0.00	0.00	0.59	0.59
Delay/Veh:	38.8	38.8	33.9	38.5	0.0	44.3	46.6	8.9	0.0	0.0	17.2	17.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	38.8	38.8	33.9	38.5	0.0	44.3	46.6	8.9	0.0	0.0	17.2	17.2
LOS by Move:	D	D	C	D	A	D	D	A	A	A	B	B
HCM2k85thQ:	10	10	1	2	0	7	7	9	0	0	19	19

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #60 Penn Wy and I-5 SB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.423
Loss Time (sec):	5	Average Delay (sec/veh):	22.0
Optimal Cycle:	22	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	2	0 1	2 0	2	0 0	0 0	0	0 0	1 0	0	0 2
Volume Module:												
Base Vol:	0	363	238	550	127	0	0	0	0	175	0	150
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	363	238	550	127	0	0	0	0	175	0	150
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	363	238	550	127	0	0	0	0	175	0	150
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	363	238	550	127	0	0	0	0	175	0	150
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	363	238	550	127	0	0	0	0	175	0	150
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.95	0.85	0.92	0.95	1.00	1.00	1.00	1.00	0.95	1.00	0.75
Lanes:	0.00	2.00	1.00	2.00	2.00	0.00	0.00	0.00	0.00	1.00	0.00	2.00
Final Sat.:	0	3610	1615	3502	3610	0	0	0	0	1805	0	2842
Capacity Analysis Module:												
Vol/Sat:	0.00	0.10	0.15	0.16	0.04	0.00	0.00	0.00	0.00	0.10	0.00	0.05
Crit Moves:			****	****						****		
Green/Cycle:	0.00	0.35	0.35	0.37	0.72	0.00	0.00	0.00	0.00	0.23	0.00	0.60
Volume/Cap:	0.00	0.29	0.42	0.42	0.05	0.00	0.00	0.00	0.00	0.42	0.00	0.09
Delay/Veh:	0.0	23.7	25.4	23.6	4.1	0.0	0.0	0.0	0.0	33.6	0.0	8.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	23.7	25.4	23.6	4.1	0.0	0.0	0.0	0.0	33.6	0.0	8.4
LOS by Move:	A	C	C	C	A	A	A	A	A	C	A	A
HCM2k85thQ:	0	7	9	10	1	0	0	0	0	8	0	2

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #61 Santiago St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.674
Loss Time (sec):	5	Average Delay (sec/veh):	17.1
Optimal Cycle:	0	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	1 0	0 1	0	1 0	0 1	0	0 1	0 0	1	0 0	1! 0	0
Volume Module:												
Base Vol:	97	299	30	13	187	48	266	76	269	14	19	18
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	97	299	30	13	187	48	266	76	269	14	19	18
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	97	299	30	13	187	48	266	76	269	14	19	18
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	97	299	30	13	187	48	266	76	269	14	19	18
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	97	299	30	13	187	48	266	76	269	14	19	18
Saturation Flow Module:												
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	0.91	0.09	1.00	0.80	0.20	0.78	0.22	1.00	0.27	0.38	0.35
Final Sat.:	481	478	48	458	401	103	395	113	597	122	166	157
Capacity Analysis Module:												
Vol/Sat:	0.20	0.63	0.63	0.03	0.47	0.47	0.67	0.67	0.45	0.11	0.11	0.11
Crit Moves:		****				****		****			****	
Delay/Veh:	11.7	19.4	19.4	10.4	15.1	15.1	22.2	22.2	13.1	11.2	11.2	11.2
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	11.7	19.4	19.4	10.4	15.1	15.1	22.2	22.2	13.1	11.2	11.2	11.2
LOS by Move:	B	C	C	B	C	C	C	C	B	B	B	B
ApproachDel:		17.6			14.8			18.2			11.2	
Delay Adj:		1.00			1.00			1.00			1.00	
ApprAdjDel:		17.6			14.8			18.2			11.2	
LOS by Appr:		C			B			C			B	
AllWayAvgQ:	0.2	1.5	1.5	0.0	0.8	0.8	1.8	1.8	0.7	0.1	0.1	0.1

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #62 Santiago St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.569
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	27	Level Of Service:	A

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	-	T	- R	L	-	T	- R	L	-	T	- R	L	-	T	- R
Control:	Permitted				Permitted				Protected				Protected			
Rights:	Include				Include				Include				Include			
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	1	0 1	1	0	1	0 1	1	0	1	1 0	1	0	2	0 1
Volume Module:																
Base Vol:	36	133	84		297	120	61		71	649	20		73	492	187	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	36	133	84		297	120	61		71	649	20		73	492	187	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	36	133	84		297	120	61		71	649	20		73	492	187	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	36	133	84		297	120	61		71	649	20		73	492	187	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	36	133	84		297	120	61		71	649	20		73	492	187	
Saturation Flow Module:																
Sat/Lane:	1600	1600	1600		1600	1600	1600		1600	1600	1600		1600	1600	1600	
Adjustment:	1.00	1.06	1.00		1.00	1.06	1.00		1.00	1.03	1.00		1.00	1.06	1.00	
Lanes:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.94	0.06		1.00	2.00	1.00	
Final Sat.:	1600	1700	1600		1600	1700	1600		1600	3204	96		1600	3400	1600	
Capacity Analysis Module:																
Vol/Sat:	0.02	0.08	0.05		0.19	0.07	0.04		0.04	0.20	0.21		0.05	0.14	0.12	
Crit Moves:		****				****					****	****				

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #64 Standard Ave and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.842
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	62	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0 1	0	0 0	1! 0	0	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	185	367	119	19	189	23	177	1218	129	91	1148	21
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	185	367	119	19	189	23	177	1218	129	91	1148	21
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	185	367	119	19	189	23	177	1218	129	91	1148	21
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	185	367	119	19	189	23	177	1218	129	91	1148	21
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	185	367	119	19	189	23	177	1218	129	91	1148	21
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.03	1.00	1.00	1.03	1.00
Lanes:	1.00	0.76	0.24	0.08	0.82	0.10	1.00	1.81	0.19	1.00	1.96	0.04
Final Sat.:	1600	1208	392	132	1309	159	1600	2994	306	1600	3243	57
Capacity Analysis Module:												
Vol/Sat:	0.12	0.30	0.30	0.01	0.14	0.14	0.11	0.41	0.42	0.06	0.35	0.37
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #65 Standard Ave and Mcfadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.682
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	35	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	169	667	82	137	300	52	53	351	60	117	712	206
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	169	667	82	137	300	52	53	351	60	117	712	206
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	169	667	82	137	300	52	53	351	60	117	712	206
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	169	667	82	137	300	52	53	351	60	117	712	206
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	169	667	82	137	300	52	53	351	60	117	712	206
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	1.78	0.22	1.00	1.70	0.30	1.00	1.71	0.29	1.00	1.55	0.45
Final Sat.:	1600	2950	350	1600	2827	473	1600	2833	467	1600	2582	718
Capacity Analysis Module:												
Vol/Sat:	0.11	0.23	0.23	0.09	0.11	0.11	0.03	0.12	0.13	0.07	0.28	0.29
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #66 Halladay St and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.677
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	35	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	0 1	0	0	0 0 0	0	1	0 1 0	0	1	1 0 0
Volume Module:												
Base Vol:	169	0	352	0	0	0	2	926	93	141	1121	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	169	0	352	0	0	0	2	926	93	141	1121	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	169	0	352	0	0	0	2	926	93	141	1121	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	169	0	352	0	0	0	2	926	93	141	1121	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	169	0	352	0	0	0	2	926	93	141	1121	0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.00	1.00	1.00	1.04	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.01	1.81	0.18	0.22	1.78	0.00
Final Sat.:	1600	0	1600	0	0	0	6	2902	291	358	2942	0
Capacity Analysis Module:												
Vol/Sat:	0.11	0.00	0.22	0.00	0.00	0.00	0.00	0.32	0.32	0.09	0.38	0.00
Crit Moves:			****					****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #67 Halladay St and Dyer Rd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.784
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	49	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0 1	0	1 0	0 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	41	180	208	114	15	32	150	1298	9	45	1451	116
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	41	180	208	114	15	32	150	1298	9	45	1451	116
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	41	180	208	114	15	32	150	1298	9	45	1451	116
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	41	180	208	114	15	32	150	1298	9	45	1451	116
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	41	180	208	114	15	32	150	1298	9	45	1451	116
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	0.46	0.54	1.00	0.32	0.68	1.00	2.98	0.02	1.00	2.78	0.22
Final Sat.:	1600	742	858	1600	511	1089	1600	4967	33	1600	4645	355
Capacity Analysis Module:												
Vol/Sat:	0.03	0.24	0.24	0.07	0.03	0.03	0.09	0.26	0.27	0.03	0.31	0.33
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #68 SR-55 SB Ramps and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.569
Loss Time (sec):	5	Average Delay (sec/veh):	17.3
Optimal Cycle:	28	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Ignore			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	0	0 0	2 0	0	0 2	0 0	2	1 1	0 0	3	0 1
Volume Module:												
Base Vol:	0	0	0	288	0	697	0	1167	1040	0	1532	602
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	288	0	697	0	1167	1040	0	1532	602
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Volume:	0	0	0	288	0	697	0	1167	0	0	1532	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	288	0	697	0	1167	0	0	1532	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
FinalVolume:	0	0	0	288	0	697	0	1167	0	0	1532	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.92	1.00	0.75	1.00	0.91	0.91	1.00	0.91	1.00
Lanes:	0.00	0.00	0.00	2.00	0.00	2.00	0.00	3.00	1.00	0.00	3.00	1.00
Final Sat.:	0	0	0	3502	0	2842	0	5187	1729	0	5187	1900
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.08	0.00	0.25	0.00	0.22	0.00	0.00	0.30	0.00
Crit Moves:						****	****				****	
Green/Cycle:	0.00	0.00	0.00	0.43	0.00	0.43	0.00	0.52	0.00	0.00	0.52	0.00
Volume/Cap:	0.00	0.00	0.00	0.19	0.00	0.57	0.00	0.43	0.00	0.00	0.57	0.00
Delay/Veh:	0.0	0.0	0.0	17.7	0.0	22.1	0.0	15.0	0.0	0.0	16.7	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	17.7	0.0	22.1	0.0	15.0	0.0	0.0	16.7	0.0
LOS by Move:	A	A	A	B	A	C	A	B	A	A	B	A
HCM2k85thQ:	0	0	0	4	0	15	0	12	0	0	18	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #69 SR-55 NB Ramps and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.497
Loss Time (sec):	5	Average Delay (sec/veh):	14.8
Optimal Cycle:	25	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Ignore			Include			Ignore			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	0	0 1	0 0	0	0 0	0 0	2	0 2	0 0	3	0 1
Volume Module:												
Base Vol:	641	0	426	0	0	0	0	711	731	0	1501	1081
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	641	0	426	0	0	0	0	711	731	0	1501	1081
User Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Volume:	641	0	0	0	0	0	0	711	0	0	1501	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	641	0	0	0	0	0	0	711	0	0	1501	0
PCE Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
FinalVolume:	641	0	0	0	0	0	0	711	0	0	1501	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.88	1.00	0.91	1.00
Lanes:	2.00	0.00	1.00	0.00	0.00	0.00	0.00	2.00	2.00	0.00	3.00	1.00
Final Sat.:	3502	0	1900	0	0	0	0	3610	3344	0	5187	1900
Capacity Analysis Module:												
Vol/Sat:	0.18	0.00	0.00	0.00	0.00	0.00	0.00	0.20	0.00	0.00	0.29	0.00
Crit Moves:	****						****				****	
Green/Cycle:	0.37	0.00	0.00	0.00	0.00	0.00	0.00	0.58	0.00	0.00	0.58	0.00
Volume/Cap:	0.50	0.00	0.00	0.00	0.00	0.00	0.00	0.34	0.00	0.00	0.50	0.00
Delay/Veh:	24.7	0.0	0.0	0.0	0.0	0.0	0.0	11.0	0.0	0.0	12.4	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	24.7	0.0	0.0	0.0	0.0	0.0	0.0	11.0	0.0	0.0	12.4	0.0
LOS by Move:	C	A	A	A	A	A	A	B	A	A	B	A
HCM2k85thQ:	12	0	0	0	0	0	0	9	0	0	15	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #70 SR-55 SB Ramps and Dyer Rd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.669
Loss Time (sec):	5	Average Delay (sec/veh):	25.7
Optimal Cycle:	36	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Ovl			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 1	0	0 2	0 1	1	0 1	1 0	3	0 1	2 0	3	0 1
Volume Module:												
Base Vol:	319	10	240	103	226	110	65	1523	207	556	991	64
Growth Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
Initial Bse:	319	10	240	103	226	110	65	1523	207	556	991	64
User Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
PHF Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
PHF Volume:	319	10	240	103	226	110	65	1523	207	556	991	64
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	319	10	240	103	226	110	65	1523	207	556	991	64
PCE Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
MLF Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
FinalVolume:	319	10	240	103	226	110	65	1523	207	556	991	64
Saturation Flow Module:												
Sat/Lane:	1900 1900		1900	1900 1900		1900	1900 1900		1900	1900 1900		1900
Adjustment:	0.95 0.95		0.75	0.94 0.94		0.85	0.95 0.91		0.85	0.92 0.91		0.85
Lanes:	1.94 0.06		2.00	0.63 1.37		1.00	1.00 3.00		1.00	2.00 3.00		1.00
Final Sat.:	3515	110	2842	1113 2443		1615	1805 5187		1615	3502 5187		1615
Capacity Analysis Module:												
Vol/Sat:	0.09 0.09		0.08	0.09 0.09		0.07	0.04 0.29		0.13	0.16 0.19		0.04
Crit Moves:	****			****			****			****		
Green/Cycle:	0.14 0.14		0.37	0.14 0.14		0.14	0.11 0.44		0.44	0.24 0.57		0.57
Volume/Cap:	0.67 0.67		0.23	0.67 0.67		0.49	0.34 0.67		0.29	0.67 0.34		0.07
Delay/Veh:	44.6 44.6		21.6	44.5 44.5		41.6	42.4 23.1		18.3	36.7 11.6		9.7
User DelAdj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
AdjDel/Veh:	44.6 44.6		21.6	44.5 44.5		41.6	42.4 23.1		18.3	36.7 11.6		9.7
LOS by Move:	D	D	C	D	D	D	D	C	B	D	B	A
HCM2k85thQ:	9	9	5	10	10	6	3	21	6	14	9	1

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #71 Glassell St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.620
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	30	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Ovl			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	1 0	1	1 0	1 1	0	1 0	1 0	1	1 0	1 1	0
Volume Module:												
Base Vol:	472	490	160	15	499	37	82	268	461	155	280	11
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	472	490	160	15	499	37	82	268	461	155	280	11
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	472	490	160	15	499	37	82	268	461	155	280	11
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	472	490	160	15	499	37	82	268	461	155	280	11
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	472	490	160	15	499	37	82	268	461	155	280	11
OvlAdjVol:			5						225			
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.03	1.00	1.00	1.06	1.00	1.00	1.03	1.00
Lanes:	2.00	1.00	1.00	1.00	1.86	0.14	1.00	1.00	1.00	1.00	1.92	0.08
Final Sat.:	3200	1700	1600	1600	3079	221	1600	1700	1600	1600	3179	121
Capacity Analysis Module:												
Vol/Sat:	0.15	0.29	0.10	0.01	0.16	0.17	0.05	0.16	0.29	0.10	0.09	0.09
OvlAdjV/S:			0.00						0.14			
Crit Moves:	****					****		****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #72 Glassell St and SR-22 WB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.766
Loss Time (sec):	5	Average Delay (sec/veh):	24.8
Optimal Cycle:	48	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	0	0 0	2 0	1	0 0	0 0	0	1 0	1! 0	1
Volume Module:												
Base Vol:	557	910	0	0	878	255	0	0	0	201	7	357
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	557	910	0	0	878	255	0	0	0	201	7	357
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	557	910	0	0	878	255	0	0	0	201	7	357
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	557	910	0	0	878	255	0	0	0	201	7	357
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	557	910	0	0	878	255	0	0	0	201	7	357
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.95	0.85	1.00	1.00	1.00	0.86	0.86	0.86
Lanes:	1.00	2.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	1.35	0.02	1.63
Final Sat.:	1805	3610	0	0	3610	1615	0	0	0	2201	40	2645
Capacity Analysis Module:												
Vol/Sat:	0.31	0.25	0.00	0.00	0.24	0.16	0.00	0.00	0.00	0.09	0.18	0.13
Crit Moves:	****				****						****	
Green/Cycle:	0.40	0.72	0.00	0.00	0.32	0.32	0.00	0.00	0.00	0.23	0.23	0.23
Volume/Cap:	0.77	0.35	0.00	0.00	0.77	0.50	0.00	0.00	0.00	0.40	0.77	0.59
Delay/Veh:	30.7	5.3	0.0	0.0	33.9	28.4	0.0	0.0	0.0	32.9	40.8	35.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	30.7	5.3	0.0	0.0	33.9	28.4	0.0	0.0	0.0	32.9	40.8	35.3
LOS by Move:	C	A	A	A	C	C	A	A	A	C	D	D
HCM2k85thQ:	24	9	0	0	21	10	0	0	0	6	15	10

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #73 Grand Ave / Glassell St and SR-22 EB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.812
Loss Time (sec):	5	Average Delay (sec/veh):	26.5
Optimal Cycle:	57	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	2	0 1	1 0	2	0 0	1 0	1!	0 1	0 0	0	0 0
Volume Module:												
Base Vol:	0	1202	307	381	681	0	229	4	503	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1202	307	381	681	0	229	4	503	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1202	307	381	681	0	229	4	503	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1202	307	381	681	0	229	4	503	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1202	307	381	681	0	229	4	503	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.95	0.85	0.95	0.95	1.00	0.86	0.86	0.86	1.00	1.00	1.00
Lanes:	0.00	2.00	1.00	1.00	2.00	0.00	1.31	0.01	1.68	0.00	0.00	0.00
Final Sat.:	0	3610	1615	1805	3610	0	2129	18	2731	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.33	0.19	0.21	0.19	0.00	0.11	0.23	0.18	0.00	0.00	0.00
Crit Moves:		****		****				****				
Green/Cycle:	0.00	0.41	0.41	0.26	0.67	0.00	0.28	0.28	0.28	0.00	0.00	0.00
Volume/Cap:	0.00	0.81	0.46	0.81	0.28	0.00	0.38	0.81	0.66	0.00	0.00	0.00
Delay/Veh:	0.0	29.6	22.0	45.1	6.8	0.0	29.2	39.2	33.2	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	29.6	22.0	45.1	6.8	0.0	29.2	39.2	33.2	0.0	0.0	0.0
LOS by Move:	A	C	C	D	A	A	C	D	C	A	A	A
HCM2k85thQ:	0	28	11	20	7	0	7	19	14	0	0	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #74 Grand Ave and Fairhaven Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.675
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	35	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 0	1	1 0	1 1	0	1 0	1 0	1
Volume Module:												
Base Vol:	46	1214	243	222	894	82	79	45	33	255	48	181
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	46	1214	243	222	894	82	79	45	33	255	48	181
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	46	1214	243	222	894	82	79	45	33	255	48	181
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	46	1214	243	222	894	82	79	45	33	255	48	181
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	46	1214	243	222	894	82	79	45	33	255	48	181
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	1.00	2.50	0.50	1.00	2.00	1.00	1.00	1.15	0.85	1.00	1.00	1.00
Final Sat.:	1600	4199	801	1600	3400	1600	1600	1946	1354	1600	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.29	0.30	0.14	0.26	0.05	0.05	0.02	0.02	0.16	0.03	0.11
Crit Moves:			****	****				****		****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #75 Grand Ave and Santa Clara Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.748
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	43	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			WideBypass			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	1	1 0	1 0	0	1 0	1 0	1	0 1
Volume Module:												
Base Vol:	88	1185	246	220	868	220	159	194	36	136	244	260
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	88	1185	246	220	868	220	159	194	36	136	244	260
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	88	1185	246	220	868	220	159	194	36	136	244	260
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	88	1185	246	220	868	220	159	194	36	136	244	260
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	88	1185	246	220	868	220	159	194	36	136	244	260
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.00	1.00	1.00	1.06	1.00
Lanes:	1.00	2.48	0.52	1.00	1.60	0.40	1.00	0.84	0.16	1.00	1.00	1.00
Final Sat.:	1600	4175	825	1600	2653	647	1600	1350	250	1600	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.06	0.28	0.30	0.14	0.33	0.34	0.10	0.14	0.14	0.09	0.14	0.16
Crit Moves:			****	****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #76 Grand Ave and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.828
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	59	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 0	1	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	246	877	221	270	582	128	346	886	149	300	1107	201
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	246	877	221	270	582	128	346	886	149	300	1107	201
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	246	877	221	270	582	128	346	886	149	300	1107	201
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	246	877	221	270	582	128	346	886	149	300	1107	201
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	246	877	221	270	582	128	346	886	149	300	1107	201
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.40	0.60	1.00	2.00	1.00	2.00	2.57	0.43	2.00	2.54	0.46
Final Sat.:	1600	4034	966	1600	3400	1600	3200	4309	691	3200	4262	738
Capacity Analysis Module:												
Vol/Sat:	0.15	0.22	0.23	0.17	0.17	0.08	0.11	0.21	0.22	0.09	0.26	0.27
Crit Moves:		****	****				****					****

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #77 Grand Ave and I-5 NB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.518
Loss Time (sec):	5	Average Delay (sec/veh):	10.0
Optimal Cycle:	26	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Ignore			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 2	0 1	1	0 3	0 0	0	0 0	0 0	2	0 0	0 1
Volume Module:												
Base Vol:	0	1297	1001	48	1164	0	0	0	0	143	0	172
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1297	1001	48	1164	0	0	0	0	143	0	172
User Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1297	0	48	1164	0	0	0	0	143	0	172
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1297	0	48	1164	0	0	0	0	143	0	172
PCE Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1297	0	48	1164	0	0	0	0	143	0	172
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.95	1.00	0.95	0.91	1.00	1.00	1.00	1.00	0.92	1.00	0.85
Lanes:	0.00	2.00	1.00	1.00	3.00	0.00	0.00	0.00	0.00	2.00	0.00	1.00
Final Sat.:	0	3610	1900	1805	5187	0	0	0	0	3502	0	1615
Capacity Analysis Module:												
Vol/Sat:	0.00	0.36	0.00	0.03	0.22	0.00	0.00	0.00	0.00	0.04	0.00	0.11
Crit Moves:		****		****								****
Green/Cycle:	0.00	0.69	0.00	0.05	0.74	0.00	0.00	0.00	0.00	0.21	0.00	0.21
Volume/Cap:	0.00	0.52	0.00	0.52	0.30	0.00	0.00	0.00	0.00	0.20	0.00	0.52
Delay/Veh:	0.0	7.5	0.0	51.3	4.3	0.0	0.0	0.0	0.0	33.0	0.0	36.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	7.5	0.0	51.3	4.3	0.0	0.0	0.0	0.0	33.0	0.0	36.8
LOS by Move:	A	A	A	D	A	A	A	A	A	C	A	D
HCM2k85thQ:	0	15	0	4	7	0	0	0	0	3	0	8

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #78 Grand Ave and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.605
Loss Time (sec):	5	Average Delay (sec/veh):	21.1
Optimal Cycle:	31	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	0 2	2 0	1	0 2	0 1	0	1 0
Volume Module:												
Base Vol:	167	1888	31	34	750	532	328	200	316	20	134	97
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	167	1888	31	34	750	532	328	200	316	20	134	97
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	167	1888	31	34	750	532	328	200	316	20	134	97
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	167	1888	31	34	750	532	328	200	316	20	134	97
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	167	1888	31	34	750	532	328	200	316	20	134	97
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	0.91	0.95	0.95	0.75	0.92	1.00	0.75	0.89	0.89	0.89
Lanes:	1.00	2.95	0.05	1.00	2.00	2.00	2.00	1.00	2.00	0.16	1.07	0.77
Final Sat.:	1805	5093	84	1805	3610	2842	3502	1900	2842	270	1808	1309
Capacity Analysis Module:												
Vol/Sat:	0.09	0.37	0.37	0.02	0.21	0.19	0.09	0.11	0.11	0.07	0.07	0.07
Crit Moves:		****		****					****	****		
Green/Cycle:	0.20	0.61	0.61	0.03	0.45	0.63	0.18	0.18	0.18	0.12	0.12	0.12
Volume/Cap:	0.47	0.61	0.61	0.61	0.47	0.30	0.51	0.57	0.61	0.61	0.61	0.61
Delay/Veh:	36.4	12.3	12.3	65.2	19.6	8.6	37.4	39.5	39.5	44.1	44.1	44.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	36.4	12.3	12.3	65.2	19.6	8.6	37.4	39.5	39.5	44.1	44.1	44.1
LOS by Move:	D	B	B	E	B	A	D	D	D	D	D	D
HCM2k85thQ:	8	20	20	3	13	7	8	10	9	8	8	8

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #79 Grand Ave and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.793
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	51	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	3 0	1	2 0	2 1	0	2 0	1 1	0
Volume Module:												
Base Vol:	165	1261	38	120	706	282	277	918	91	151	941	173
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	165	1261	38	120	706	282	277	918	91	151	941	173
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	165	1261	38	120	706	282	277	918	91	151	941	173
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	165	1261	38	120	706	282	277	918	91	151	941	173
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	165	1261	38	120	706	282	277	918	91	151	941	173
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.04	1.00
Lanes:	2.00	2.91	0.09	2.00	3.00	1.00	2.00	2.73	0.27	2.00	1.69	0.31
Final Sat.:	3200	4860	140	3200	5100	1600	3200	4567	433	3200	2803	497
Capacity Analysis Module:												
Vol/Sat:	0.05	0.26	0.27	0.04	0.14	0.18	0.09	0.20	0.21	0.05	0.34	0.35
Crit Moves:		****	****				****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #80 Grand Ave and Chestnut Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.615
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	30	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	140	1050	182	149	664	112	93	285	62	107	333	194
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	140	1050	182	149	664	112	93	285	62	107	333	194
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	140	1050	182	149	664	112	93	285	62	107	333	194
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	140	1050	182	149	664	112	93	285	62	107	333	194
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	140	1050	182	149	664	112	93	285	62	107	333	194
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	2.56	0.44	1.00	2.57	0.43	1.00	1.64	0.36	1.00	1.26	0.74
Final Sat.:	1600	4291	709	1600	4307	693	1600	2728	572	1600	2122	1178
Capacity Analysis Module:												
Vol/Sat:	0.09	0.24	0.26	0.09	0.15	0.16	0.06	0.10	0.11	0.07	0.16	0.16
Crit Moves:			****	****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #81 Grand Ave and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.759
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	45	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	202	1150	198	138	626	168	128	476	83	86	686	151
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	202	1150	198	138	626	168	128	476	83	86	686	151
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	202	1150	198	138	626	168	128	476	83	86	686	151
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	202	1150	198	138	626	168	128	476	83	86	686	151
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	202	1150	198	138	626	168	128	476	83	86	686	151
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	2.56	0.44	1.00	2.37	0.63	1.00	1.70	0.30	1.00	1.64	0.36
Final Sat.:	1600	4295	705	1600	3984	1016	1600	2825	475	1600	2723	577
Capacity Analysis Module:												
Vol/Sat:	0.13	0.27	0.28	0.09	0.16	0.17	0.08	0.17	0.17	0.05	0.25	0.26
Crit Moves:		****	****				****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #82 Grand Ave and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.909
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	92	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	242	1397	121	165	408	229	201	868	70	74	1333	173
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	242	1397	121	165	408	229	201	868	70	74	1333	173
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	242	1397	121	165	408	229	201	868	70	74	1333	173
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	242	1397	121	165	408	229	201	868	70	74	1333	173
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	242	1397	121	165	408	229	201	868	70	74	1333	173
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.76	0.24	1.00	2.00	1.00	1.00	2.78	0.22	1.00	2.66	0.34
Final Sat.:	1600	4617	383	1600	3400	1600	1600	4642	358	1600	4449	551
Capacity Analysis Module:												
Vol/Sat:	0.15	0.30	0.32	0.10	0.12	0.14	0.13	0.19	0.20	0.05	0.30	0.31
Crit Moves:		****	****				****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #83 Grand Ave and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.704
Loss Time (sec):	0	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	77	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0 2	1 0		1 0 2	1 0		2 0 3	0 1		2 0 2	0 1	
Volume Module:												
Base Vol:	242	873	193	146	471	217	342	656	142	151	964	316
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	242	873	193	146	471	217	342	656	142	151	964	316
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	242	873	193	146	471	217	342	656	142	151	964	316
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	242	873	193	146	471	217	342	656	142	151	964	316
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	242	873	193	146	471	217	342	656	142	151	964	316
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	2.46	0.54	1.00	2.05	0.95	2.00	3.00	1.00	2.00	2.00	1.00
Final Sat.:	1600	4131	869	1600	3486	1514	3200	5100	1600	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.15	0.21	0.22	0.09	0.14	0.14	0.11	0.13	0.09	0.05	0.28	0.20
Crit Moves:			****	****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #84 SR-55 NB Ramps and Dyer Rd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.376
Loss Time (sec):	5	Average Delay (sec/veh):	5.7
Optimal Cycle:	20	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Ignore			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	1! 0	1	0	0	0	0	0	3	0	1
Volume Module:												
Base Vol:	188	0	51	0	0	0	0	1339	681	0	1545	965
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	188	0	51	0	0	0	0	1339	681	0	1545	965
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Volume:	188	0	51	0	0	0	0	1339	0	0	1545	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	188	0	51	0	0	0	0	1339	0	0	1545	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
FinalVolume:	188	0	51	0	0	0	0	1339	0	0	1545	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.93	1.00	0.93	1.00	1.00	1.00	1.00	0.91	1.00	1.00	0.91	0.91
Lanes:	1.79	0.00	1.21	0.00	0.00	0.00	0.00	3.00	1.00	0.00	3.00	0.00
Final Sat.:	3161	0	2147	0	0	0	0	5187	1900	0	5187	0
Capacity Analysis Module:												
Vol/Sat:	0.06	0.00	0.02	0.00	0.00	0.00	0.00	0.26	0.00	0.00	0.30	0.00
Crit Moves:	****						****				****	
Green/Cycle:	0.16	0.00	0.16	0.00	0.00	0.00	0.00	0.79	0.00	0.00	0.79	0.00
Volume/Cap:	0.38	0.00	0.15	0.00	0.00	0.00	0.00	0.33	0.00	0.00	0.38	0.00
Delay/Veh:	38.1	0.0	36.3	0.0	0.0	0.0	0.0	3.0	0.0	0.0	3.1	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	38.1	0.0	36.3	0.0	0.0	0.0	0.0	3.0	0.0	0.0	3.1	0.0
LOS by Move:	D	A	D	A	A	A	A	A	A	A	A	A
HCM2k85thQ:	5	0	2	0	0	0	0	7	0	0	8	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #85 Cambridge St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.459
Loss Time (sec):	5	Average Delay (sec/veh):	12.5
Optimal Cycle:	0	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	1 0 1	0 0		0 0 1	0 1		1 0	0 0 1		0 0	0 0 0	
Volume Module:												
Base Vol:	191	266	0	0	196	181	219	0	144	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	191	266	0	0	196	181	219	0	144	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	191	266	0	0	196	181	219	0	144	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	191	266	0	0	196	181	219	0	144	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	191	266	0	0	196	181	219	0	144	0	0	0
Saturation Flow Module:												
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	0.00	0.00	1.00	1.00	1.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	535	579	0	0	563	631	500	0	595	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.36	0.46	xxxx	xxxx	0.35	0.29	0.44	xxxx	0.24	xxxx	xxxx	xxxx
Crit Moves:		****			****		****					
Delay/Veh:	12.7	13.6	0.0	0.0	12.0	10.3	14.7	0.0	10.2	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	12.7	13.6	0.0	0.0	12.0	10.3	14.7	0.0	10.2	0.0	0.0	0.0
LOS by Move:	B	B	*	*	B	B	B	*	B	*	*	*
ApproachDel:	13.3			11.2			12.9			xxxxxx		
Delay Adj:	1.00			1.00			1.00			xxxxx		
ApprAdjDel:	13.3			11.2			12.9			xxxxxx		
LOS by Appr:	B			B			B			*		
AllWayAvgQ:	0.5	0.8	0.0	0.0	0.5	0.4	0.7	0.0	0.3	0.0	0.0	0.0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #86 Cambridge St and Fairhaven Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.488
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	23	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	0 0	0	1 0	0 0	1	1 0	2 0	0	0 0	1 1	0
Volume Module:												
Base Vol:	0	0	0	219	0	76	141	246	0	0	273	340
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	219	0	76	141	246	0	0	273	340
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	219	0	76	141	246	0	0	273	340
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	219	0	76	141	246	0	0	273	340
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	219	0	76	141	246	0	0	273	340
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	0.00	0.00	0.00	1.00	0.00	1.00	1.00	2.00	0.00	0.00	1.00	1.00
Final Sat.:	0	0	0	1600	0	1600	1600	3400	0	0	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.14	0.00	0.05	0.09	0.07	0.00	0.00	0.16	0.21
Crit Moves:				****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #87 Mabury St and 1st Street

Cycle (sec):	100	Critical Vol./Cap.(X):	0.703
Loss Time (sec):	0	Average Delay (sec/veh):	26.1
Optimal Cycle:	77	Level Of Service:	C

Street Name:	Mabury						1st Street					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	0 1	1 0	1!	0 1	0 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	25	0	256	207	88	390	0	1512	24	40	549	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	25	0	256	207	88	390	0	1512	24	40	549	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	25	0	256	207	88	390	0	1512	24	40	549	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	25	0	256	207	88	390	0	1512	24	40	549	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	25	0	256	207	88	390	0	1512	24	40	549	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	0.85	0.90	0.90	0.90	1.00	0.91	0.91	0.95	0.91	0.91
Lanes:	1.00	0.00	1.00	1.27	0.23	1.50	0.00	2.95	0.05	1.00	3.00	0.00
Final Sat.:	1805	0	1615	2171	390	2576	0	5096	81	1805	5187	0
Capacity Analysis Module:												
Vol/Sat:	0.01	0.00	0.16	0.10	0.23	0.15	0.00	0.30	0.30	0.02	0.11	0.00
Crit Moves:			****		****			****		****		
Green/Cycle:	0.23	0.00	0.23	0.32	0.32	0.32	0.00	0.42	0.42	0.03	0.45	0.00
Volume/Cap:	0.06	0.00	0.70	0.30	0.70	0.47	0.00	0.70	0.70	0.70	0.23	0.00
Delay/Veh:	30.5	0.0	41.7	25.6	32.1	27.4	0.0	24.8	24.8	80.7	16.7	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	30.5	0.0	41.7	25.6	32.1	27.4	0.0	24.8	24.8	80.7	16.7	0.0
LOS by Move:	C	A	D	C	C	C	A	C	C	F	B	A
HCM2k85thQ:	1	0	13	6	17	10	0	22	22	4	6	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #88 Tustin St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.444
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	21	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 1	0	0 1
Volume Module:												
Base Vol:	46	1033	245	90	801	15	13	5	20	183	5	112
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	46	1033	245	90	801	15	13	5	20	183	5	112
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	46	1033	245	90	801	15	13	5	20	183	5	112
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	46	1033	245	90	801	15	13	5	20	183	5	112
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	46	1033	245	90	801	15	13	5	20	183	5	112
OvlAdjVol:												22
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	1.00	2.42	0.58	1.00	2.94	0.06	1.00	1.00	1.00	1.95	0.05	1.00
Final Sat.:	1600	4080	920	1600	4912	88	1600	1700	1600	3115	85	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.25	0.27	0.06	0.16	0.17	0.01	0.00	0.01	0.06	0.06	0.07
OvlAdjV/S:												0.01
Crit Moves:		****		****				****			****	

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #89 Tustin St and SR-22 WB On-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.446
Loss Time (sec):	5	Average Delay (sec/veh):	12.1
Optimal Cycle:	36	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 0	0	0 0	2	1 0	0 0	0 0	0 0	0 0	0 0	0 0
Volume Module:												
Base Vol:	532	1529	0	0	711	365	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	532	1529	0	0	711	365	0	0	0	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	532	1529	0	0	711	365	0	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	532	1529	0	0	711	365	0	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	532	1529	0	0	711	365	0	0	0	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.95	1.00	1.00	0.86	0.86	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	2.00	2.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00
Final Sat.:	3502	3610	0	0	3282	1641	0	0	0	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.15	0.42	0.00	0.00	0.22	0.22	0.00	0.00	0.00	0.00	0.00	0.00
Crit Moves:		****										
Green/Cycle:	0.63	0.95	0.00	0.00	0.32	0.32	0.00	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.24	0.45	0.00	0.00	0.67	0.69	0.00	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	8.2	0.3	0.0	0.0	30.5	31.0	0.0	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	8.2	0.3	0.0	0.0	30.5	31.0	0.0	0.0	0.0	0.0	0.0	0.0
LOS by Move:	A	A	A	A	C	C	A	A	A	A	A	A
HCM2k85thQ:	6	4	0	0	17	18	0	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #90 Tustin St and SR-22 EB Off-Ramp / Seba Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.719
Loss Time (sec):	5	Average Delay (sec/veh):	23.7
Optimal Cycle:	41	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 2	1 0	1	0 3	0 0	1	0 1!	0 1	0	0 1!	0 0
Volume Module:												
Base Vol:	0	1547	4	52	707	0	458	41	581	28	0	67
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1547	4	52	707	0	458	41	581	28	0	67
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1547	4	52	707	0	458	41	581	28	0	67
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1547	4	52	707	0	458	41	581	28	0	67
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1547	4	52	707	0	458	41	581	28	0	67
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	0.91	0.10	0.91	1.00	0.90	0.90	0.90	0.89	1.00	0.89
Lanes:	0.00	2.99	0.01	1.00	3.00	0.00	1.41	0.07	1.52	0.29	0.00	0.71
Final Sat.:	0	5174	13	182	5187	0	2408	125	2595	499	0	1195
Capacity Analysis Module:												
Vol/Sat:	0.00	0.30	0.30	0.29	0.14	0.00	0.19	0.33	0.22	0.06	0.00	0.06
Crit Moves:		****						****		****		
Green/Cycle:	0.00	0.42	0.42	0.42	0.42	0.00	0.46	0.46	0.46	0.08	0.00	0.08
Volume/Cap:	0.00	0.72	0.72	0.69	0.33	0.00	0.42	0.72	0.49	0.72	0.00	0.72
Delay/Veh:	0.0	25.5	25.5	46.8	19.8	0.0	18.4	23.7	19.2	62.4	0.0	62.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	25.5	25.5	46.8	19.8	0.0	18.4	23.7	19.2	62.4	0.0	62.4
LOS by Move:	A	C	C	D	B	A	B	C	B	E	A	E
HCM2k85thQ:	0	23	23	4	8	0	10	22	13	7	0	7

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #91 Tustin Ave and Fairhaven Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.614
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	30	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	1 0	1 1	0	1 0	1 0	1
Volume Module:												
Base Vol:	239	1275	113	327	862	99	80	180	147	94	208	196
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	239	1275	113	327	862	99	80	180	147	94	208	196
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	239	1275	113	327	862	99	80	180	147	94	208	196
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	239	1275	113	327	862	99	80	180	147	94	208	196
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	239	1275	113	327	862	99	80	180	147	94	208	196
OvlAdjVol:												32
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.76	0.24	2.00	2.69	0.31	1.00	1.10	0.90	1.00	1.00	1.00
Final Sat.:	3200	4609	391	3200	4506	494	1600	1861	1439	1600	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.07	0.28	0.29	0.10	0.19	0.20	0.05	0.10	0.10	0.06	0.12	0.12
OvlAdjV/S:												0.02
Crit Moves:		****	****				****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #92 Tustin Ave and Santa Clara Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.647
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	32	Level Of Service:	B

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	T	R		L	T	R		L	T	R		L	T	R	
Control:	Protected				Protected				Permitted				Permitted			
Rights:	Include				Include				Include				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Y+R:	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0	
Lanes:	1	0	2	1	0	2	0	2	1	0	1	0	1	1	0	
Volume Module:																
Base Vol:	163	789	121		183	1444	100		61	192	113		125	157	148	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	163	789	121		183	1444	100		61	192	113		125	157	148	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	163	789	121		183	1444	100		61	192	113		125	157	148	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	163	789	121		183	1444	100		61	192	113		125	157	148	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	163	789	121		183	1444	100		61	192	113		125	157	148	
Saturation Flow Module:																
Sat/Lane:	1600	1600	1600		1600	1600	1600		1600	1600	1600		1600	1600	1600	
Adjustment:	1.00	1.05	1.00		1.00	1.04	1.00		1.00	1.05	1.00		1.00	1.06	1.00	
Lanes:	1.00	2.60	0.40		2.00	2.81	0.19		1.00	1.26	0.74		1.00	1.03	0.97	
Final Sat.:	1600	4362	638		3200	4689	311		1600	2114	1186		1600	1747	1553	
Capacity Analysis Module:																
Vol/Sat:	0.10	0.18	0.19		0.06	0.31	0.32		0.04	0.09	0.10		0.08	0.09	0.10	
Crit Moves:	****						****				****	****				

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #93 Tustin Ave and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.717
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	39	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 0	1	2 0	2 1	0	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	216	926	476	346	371	37	444	872	118	229	753	419
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	216	926	476	346	371	37	444	872	118	229	753	419
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	216	926	476	346	371	37	444	872	118	229	753	419
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	216	926	476	346	371	37	444	872	118	229	753	419
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	216	926	476	346	371	37	444	872	118	229	753	419
OvlAdjVol:			362									246
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.00	1.00	2.00	2.73	0.27	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	3400	1600	3200	4565	435	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.07	0.27	0.30	0.11	0.08	0.08	0.14	0.17	0.07	0.07	0.15	0.26
OvlAdjV/S:			0.23									0.15
Crit Moves:	****			****			****			****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #94 Tustin Ave and 4th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.692
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	36	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	2 0	2	0 1	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	55	598	194	459	339	124	180	642	33	88	596	367
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	55	598	194	459	339	124	180	642	33	88	596	367
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	55	598	194	459	339	124	180	642	33	88	596	367
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	55	598	194	459	339	124	180	642	33	88	596	367
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	55	598	194	459	339	124	180	642	33	88	596	367
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	1.00	2.27	0.73	2.00	2.00	1.00	1.00	2.85	0.15	1.00	2.00	1.00
Final Sat.:	1600	3824	1176	3200	3400	1600	1600	4765	235	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.16	0.16	0.14	0.10	0.08	0.11	0.13	0.14	0.06	0.18	0.23
Crit Moves:	****			****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #95 SR-55 SB Ramps / Auto Mall Dr and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.583
Loss Time (sec):	5	Average Delay (sec/veh):	25.1
Optimal Cycle:	29	Level Of Service:	C

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	- T	- R		L	- T	- R		L	- T	- R		L	- T	- R	
Control:	Split Phase				Split Phase				Protected				Protected			
Rights:	Include				Include				Ignore				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Y+R:	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0	
Lanes:	1	1 0	0	2	1	0 1	0	1	1	0 3	0	1	2	0 2	1	0
Volume Module:																
Base Vol:	377	30	398		110	50	37		35	1258	354		385	1003	120	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	377	30	398		110	50	37		35	1258	354		385	1003	120	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	0.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	0.00		1.00	1.00	1.00	
PHF Volume:	377	30	398		110	50	37		35	1258	0		385	1003	120	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	377	30	398		110	50	37		35	1258	0		385	1003	120	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	0.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	0.00		1.00	1.00	1.00	
FinalVolume:	377	30	398		110	50	37		35	1258	0		385	1003	120	
Saturation Flow Module:																
Sat/Lane:	1900	1900	1900		1900	1900	1900		1900	1900	1900		1900	1900	1900	
Adjustment:	0.96	0.96	0.75		0.95	1.00	0.85		0.95	0.91	1.00		0.92	0.90	0.90	
Lanes:	1.85	0.15	2.00		1.00	1.00	1.00		1.00	3.00	1.00		2.00	2.68	0.32	
Final Sat.:	3365	268	2842		1805	1900	1615		1805	5187	1900		3502	4559	545	
Capacity Analysis Module:																
Vol/Sat:	0.11	0.11	0.14		0.06	0.03	0.02		0.02	0.24	0.00		0.11	0.22	0.22	
Crit Moves:			****		****					****			****			
Green/Cycle:	0.24	0.24	0.24		0.10	0.10	0.10		0.05	0.42	0.00		0.19	0.56	0.56	
Volume/Cap:	0.47	0.47	0.58		0.58	0.25	0.22		0.40	0.58	0.00		0.58	0.40	0.40	
Delay/Veh:	32.9	32.9	34.8		47.3	41.8	41.7		49.0	22.9	0.0		38.3	12.7	12.7	
User DelAdj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
AdjDel/Veh:	32.9	32.9	34.8		47.3	41.8	41.7		49.0	22.9	0.0		38.3	12.7	12.7	
LOS by Move:	C	C	C		D	D	D		D	C	A		D	B	B	
HCM2k85thQ:	9	9	11		6	3	2		2	17	0		10	11	11	

Note: Queue reported is the number of cars per lane.
****

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #96 SR-55 NB Ramps / Del Amo Ave and Newport Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	0.525
Loss Time (sec):	5	Average Delay (sec/veh):	27.4
Optimal Cycle:	26	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0 2	1 0	1	0 3	0 1	2	0 1	1 0	1	0 1	1 0
Volume Module:												
Base Vol:	583	174	34	8	97	373	104	36	21	23	348	16
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	583	174	34	8	97	373	104	36	21	23	348	16
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	583	174	34	8	97	373	104	36	21	23	348	16
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	583	174	34	8	97	373	104	36	21	23	348	16
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	583	174	34	8	97	373	104	36	21	23	348	16
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.89	0.89	0.95	0.91	0.85	0.92	0.90	0.90	0.95	0.94	0.94
Lanes:	2.00	2.51	0.49	1.00	3.00	1.00	2.00	1.26	0.74	1.00	1.91	0.09
Final Sat.:	3502	4235	828	1805	5187	1615	3502	2155	1257	1805	3427	158
Capacity Analysis Module:												
Vol/Sat:	0.17	0.04	0.04	0.00	0.02	0.23	0.03	0.02	0.02	0.01	0.10	0.10
Crit Moves:	****					****	****				****	
Green/Cycle:	0.32	0.63	0.63	0.07	0.38	0.44	0.06	0.14	0.14	0.11	0.19	0.19
Volume/Cap:	0.53	0.07	0.07	0.07	0.05	0.53	0.53	0.12	0.12	0.12	0.53	0.53
Delay/Veh:	28.5	7.1	7.1	43.8	19.4	21.1	48.5	37.6	37.6	40.6	37.0	37.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	28.5	7.1	7.1	43.8	19.4	21.1	48.5	37.6	37.6	40.6	37.0	37.0
LOS by Move:	C	A	A	D	B	C	D	D	D	D	D	D
HCM2k85thQ:	12	1	1	0	1	13	4	1	1	1	9	9

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #97 Red Hill Ave and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.667
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	34	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Ovl			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0 3	0 1	2	0 3	0 1	2	0 3	0 1	2	0 3	0 1
Volume Module:												
Base Vol:	159	1146	182	145	289	234	327	838	102	62	803	392
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	159	1146	182	145	289	234	327	838	102	62	803	392
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	159	1146	182	145	289	234	327	838	102	62	803	392
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	159	1146	182	145	289	234	327	838	102	62	803	392
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	159	1146	182	145	289	234	327	838	102	62	803	392
OvlAdjVol:						70			22			
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.05	0.22	0.11	0.05	0.06	0.15	0.10	0.16	0.06	0.02	0.16	0.25
OvlAdjV/S:						0.04			0.01			
Crit Moves:	****			****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #98 Red Hill Ave and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.588
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	28	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0 3	1 0	2	0 2	1 0	1	0 2	1 0	2	0 3	0 1
Volume Module:												
Base Vol:	382	1308	150	58	223	283	222	603	94	25	525	69
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	382	1308	150	58	223	283	222	603	94	25	525	69
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	382	1308	150	58	223	283	222	603	94	25	525	69
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	382	1308	150	58	223	283	222	603	94	25	525	69
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	382	1308	150	58	223	283	222	603	94	25	525	69
OvlAdjVol:												40
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	3.59	0.41	2.00	2.00	1.00	1.00	2.60	0.40	2.00	3.00	1.00
Final Sat.:	3200	6042	658	3200	3400	1600	1600	4353	647	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.12	0.22	0.23	0.02	0.07	0.18	0.14	0.14	0.15	0.01	0.10	0.04
OvlAdjV/S:												0.03
Crit Moves:	****					****	****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #99 Red Hill Ave and Dyer Rd / barranca Pkwy

Cycle (sec):	100	Critical Vol./Cap.(X):	0.563
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	27	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	4 0	1	2 0	4 0	1	2 0	3 1	0	2 0	4 0	1
Volume Module:												
Base Vol:	356	1339	345	185	313	147	203	753	99	127	966	282
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	356	1339	345	185	313	147	203	753	99	127	966	282
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	356	1339	345	185	313	147	203	753	99	127	966	282
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	356	1339	345	185	313	147	203	753	99	127	966	282
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	356	1339	345	185	313	147	203	753	99	127	966	282
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	4.00	1.00	2.00	4.00	1.00	2.00	3.54	0.46	2.00	4.00	1.00
Final Sat.:	3200	6800	1600	3200	6800	1600	3200	5956	744	3200	6800	1600
Capacity Analysis Module:												
Vol/Sat:	0.11	0.20	0.22	0.06	0.05	0.09	0.06	0.13	0.13	0.04	0.14	0.18
Crit Moves:			****	****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #100 Red Hill Ave and Alton Pkwy

Cycle (sec):	100	Critical Vol./Cap.(X):	0.671
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	34	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	2 0	1	2 0	1 0	1
Volume Module:												
Base Vol:	27	1499	217	124	528	4	132	324	166	433	74	267
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	27	1499	217	124	528	4	132	324	166	433	74	267
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	27	1499	217	124	528	4	132	324	166	433	74	267
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	27	1499	217	124	528	4	132	324	166	433	74	267
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	27	1499	217	124	528	4	132	324	166	433	74	267
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	2.00	1.00	2.00	1.00	1.00
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	3400	1600	3200	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.02	0.29	0.14	0.08	0.10	0.00	0.08	0.10	0.10	0.14	0.04	0.17
Crit Moves:		****			****			****				****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #101 Red Hill Ave and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.657
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	33	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ignore			Include			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0 2	1 0		2 0 3	0 1		2 0 3	0 1		1 0 3	0 1	
Volume Module:												
Base Vol:	125	809	10	286	594	1042	597	275	42	39	818	574
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	125	809	10	286	594	1042	597	275	42	39	818	574
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Volume:	125	809	10	286	594	0	597	275	42	39	818	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	125	809	10	286	594	0	597	275	42	39	818	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
FinalVolume:	125	809	10	286	594	0	597	275	42	39	818	0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.96	0.04	2.00	3.00	1.00	2.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	4941	59	3200	5100	1600	3200	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.04	0.16	0.17	0.09	0.12	0.00	0.19	0.05	0.03	0.02	0.16	0.00
Crit Moves:			****	****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #102 Red Hill Ave and Main St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.774
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	47	Level Of Service:	C

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	- T	- R		L	- T	- R		L	- T	- R		L	- T	- R	
Control:	Protected				Protected				Protected				Protected			
Rights:	Include				Include				Include				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Y+R:	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0	
Lanes:	2	0 2	0	1	2	0 2	0	1	2	0 2	1	0	2	0 2	1	0
Volume Module:																
Base Vol:	416	897	242		74	538	292		198	705	191		279	1589	89	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	416	897	242		74	538	292		198	705	191		279	1589	89	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	416	897	242		74	538	292		198	705	191		279	1589	89	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	416	897	242		74	538	292		198	705	191		279	1589	89	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	416	897	242		74	538	292		198	705	191		279	1589	89	
Saturation Flow Module:																
Sat/Lane:	1600	1600	1600		1600	1600	1600		1600	1600	1600		1600	1600	1600	
Adjustment:	1.00	1.06	1.00		1.00	1.06	1.00		1.00	1.05	1.00		1.00	1.04	1.00	
Lanes:	2.00	2.00	1.00		2.00	2.00	1.00		2.00	2.36	0.64		2.00	2.84	0.16	
Final Sat.:	3200	3400	1600		3200	3400	1600		3200	3977	1023		3200	4745	255	
Capacity Analysis Module:																
Vol/Sat:	0.13	0.26	0.15		0.02	0.16	0.18		0.06	0.18	0.19		0.09	0.33	0.35	
Crit Moves:	****						****		****						****	

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #103 I-5 SB Ramps and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.431
Loss Time (sec):	5	Average Delay (sec/veh):	18.9
Optimal Cycle:	22	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	0	2	0	0	2	0	3	0	2	1
Volume Module:												
Base Vol:	0	0	0	278	0	61	561	634	0	0	668	183
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	278	0	61	561	634	0	0	668	183
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	278	0	61	561	634	0	0	668	183
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	278	0	61	561	634	0	0	668	183
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	278	0	61	561	634	0	0	668	183
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.92	1.00	0.85	0.92	0.91	1.00	1.00	0.88	0.88
Lanes:	0.00	0.00	0.00	2.00	0.00	1.00	2.00	3.00	0.00	0.00	2.35	0.65
Final Sat.:	0	0	0	3502	0	1615	3502	5187	0	0	3941	1080
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.08	0.00	0.04	0.16	0.12	0.00	0.00	0.17	0.17
Crit Moves:				****			****				****	
Green/Cycle:	0.00	0.00	0.00	0.18	0.00	0.56	0.37	0.77	0.00	0.00	0.39	0.39
Volume/Cap:	0.00	0.00	0.00	0.43	0.00	0.07	0.43	0.16	0.00	0.00	0.43	0.43
Delay/Veh:	0.0	0.0	0.0	36.6	0.0	10.3	23.7	3.1	0.0	0.0	22.3	22.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

AdjDel/Veh:	0.0	0.0	0.0	36.6	0.0	10.3	23.7	3.1	0.0	0.0	22.3	22.3
LOS by Move:	A	A	A	D	A	B	C	A	A	A	C	C
HCM2k85thQ:	0	0	0	7	0	1	10	3	0	0	11	11

****

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #104 Tustin Ranch Rd and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.606
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	29	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	0	2	0	0	2	0	3	0	0	1
Volume Module:												
Base Vol:	0	0	0	653	0	45	54	1795	0	0	548	236
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	653	0	45	54	1795	0	0	548	236
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	653	0	45	54	1795	0	0	548	236
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	653	0	45	54	1795	0	0	548	236
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	653	0	45	54	1795	0	0	548	236
OvlAdjVol:												0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	0.00	0.00	0.00	2.00	0.00	1.00	2.00	3.00	0.00	0.00	3.00	1.00
Final Sat.:	0	0	0	3200	0	1600	3200	5100	0	0	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.20	0.00	0.03	0.02	0.35	0.00	0.00	0.11	0.15
OvlAdjV/S:												0.00
Crit Moves:				****				****		****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #105 Von Karman Ave and Barranca Pkwy

Cycle (sec):	100	Critical Vol./Cap.(X):	0.874
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	74	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0 1	1 0		2 0 2			2 0 3	0 1		2 0 4	0 1	
Volume Module:												
Base Vol:	224	1290	281	67	343	236	560	1023	143	204	646	220
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	224	1290	281	67	343	236	560	1023	143	204	646	220
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	224	1290	281	67	343	236	560	1023	143	204	646	220
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	224	1290	281	67	343	236	560	1023	143	204	646	220
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	224	1290	281	67	343	236	560	1023	143	204	646	220
OvlAdjVol:						0						
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	1.64	0.36	2.00	2.00	2.00	2.00	3.00	1.00	2.00	4.00	1.00
Final Sat.:	3200	2728	572	3200	3400	3200	3200	5100	1600	3200	6800	1600
Capacity Analysis Module:												
Vol/Sat:	0.07	0.47	0.49	0.02	0.10	0.07	0.17	0.20	0.09	0.06	0.10	0.14
OvlAdjV/S:						0.00						
Crit Moves:			****	****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #106 Red Hill Avenue and El Camino Real

Cycle (sec):	100	Critical Vol./Cap.(X):	0.624
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	31	Level Of Service:	B

Street Name:	Red Hill Avenue						El Camino Real					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0 3	0 1	1	0 2	1 0	1	0 1	0 1	1	0 0	1 0
Volume Module:												
Base Vol:	415	1315	242	20	728	96	70	153	201	199	329	38
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	415	1315	242	20	728	96	70	153	201	199	329	38
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	415	1315	242	20	728	96	70	153	201	199	329	38
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	415	1315	242	20	728	96	70	153	201	199	329	38
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	415	1315	242	20	728	96	70	153	201	199	329	38
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	2.00	3.00	1.00	1.00	2.65	0.35	1.00	1.00	1.00	1.00	0.90	0.10
Final Sat.:	3200	5100	1600	1600	4441	559	1600	1700	1600	1600	1434	166
Capacity Analysis Module:												
Vol/Sat:	0.13	0.26	0.15	0.01	0.16	0.17	0.04	0.09	0.13	0.12	0.23	0.23
Crit Moves:	****					****	****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #107 Red Hill Avenue and I-5 NB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.626
Loss Time (sec):	5	Average Delay (sec/veh):	23.3
Optimal Cycle:	32	Level Of Service:	C

Street Name:	Red Hill Avenue						I-5 NB Ramps					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	0	0 0	3 0	1	0 0	0 0	0	1 1	0 1	0
Volume Module:												
Base Vol:	272	1557	0	0	939	190	0	0	0	256	96	415
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	272	1557	0	0	939	190	0	0	0	256	96	415
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	272	1557	0	0	939	190	0	0	0	256	96	415
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	272	1557	0	0	939	190	0	0	0	256	96	415
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	272	1557	0	0	939	190	0	0	0	256	96	415
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	1.00	1.00	0.91	0.85	1.00	1.00	1.00	0.83	0.83	0.83
Lanes:	1.00	3.00	0.00	0.00	3.00	1.00	0.00	0.00	0.00	1.45	0.55	1.00
Final Sat.:	1805	5187	0	0	5187	1615	0	0	0	2292	860	1576
Capacity Analysis Module:												
Vol/Sat:	0.15	0.30	0.00	0.00	0.18	0.12	0.00	0.00	0.00	0.11	0.11	0.26
Crit Moves:	****				****							****
Green/Cycle:	0.24	0.53	0.00	0.00	0.29	0.29	0.00	0.00	0.00	0.42	0.42	0.42
Volume/Cap:	0.63	0.57	0.00	0.00	0.63	0.41	0.00	0.00	0.00	0.27	0.27	0.63
Delay/Veh:	36.8	16.1	0.0	0.0	31.7	29.2	0.0	0.0	0.0	19.0	19.0	23.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

AdjDel/Veh:	36.8	16.1	0.0	0.0	31.7	29.2	0.0	0.0	0.0	19.0	19.0	23.8
LOS by Move:	D	B	A	A	C	C	A	A	A	B	B	C
HCM2k85thQ:	13	18	0	0	15	8	0	0	0	6	6	17

****

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #108 Red Hill Avenue and I-5 SB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.708
Loss Time (sec):	5	Average Delay (sec/veh):	22.8
Optimal Cycle:	40	Level Of Service:	C

Street Name:	Red Hill Avenue						I-5 SB Ramps					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 4	0 1	1	0 3	0 0	0	1 0	0 1	0	0 0	0 0
Volume Module:												
Base Vol:	0	1521	483	352	846	0	309	3	277	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1521	483	352	846	0	309	3	277	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1521	483	352	846	0	309	3	277	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1521	483	352	846	0	309	3	277	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1521	483	352	846	0	309	3	277	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	0.85	0.95	0.91	1.00	0.92	0.92	0.85	1.00	1.00	1.00
Lanes:	0.00	4.00	1.00	1.00	3.00	0.00	0.99	0.01	1.00	0.00	0.00	0.00
Final Sat.:	0	6916	1615	1805	5187	0	1733	17	1615	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.22	0.30	0.20	0.16	0.00	0.18	0.18	0.17	0.00	0.00	0.00
Crit Moves:			****	****				****				
Green/Cycle:	0.00	0.42	0.42	0.28	0.70	0.00	0.25	0.25	0.25	0.00	0.00	0.00
Volume/Cap:	0.00	0.52	0.71	0.71	0.23	0.00	0.71	0.71	0.68	0.00	0.00	0.00
Delay/Veh:	0.0	21.5	27.2	37.3	5.5	0.0	39.3	39.3	38.4	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	21.5	27.2	37.3	5.5	0.0	39.3	39.3	38.4	0.0	0.0	0.0
LOS by Move:	A	C	C	D	A	A	D	D	D	A	A	A
HCM2k85thQ:	0	15	20	17	5	0	15	15	13	0	0	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #109 Red Hill Avenue and Nisson Road

Cycle (sec):	100	Critical Vol./Cap.(X):	0.702
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	37	Level Of Service:	C

Street Name:	Red Hill Avenue						Nisson Road					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 1	0	1 0	2 1	0	1 0	0 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	30	1583	32	168	865	87	247	90	40	48	83	141
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	30	1583	32	168	865	87	247	90	40	48	83	141
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	30	1583	32	168	865	87	247	90	40	48	83	141
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	30	1583	32	168	865	87	247	90	40	48	83	141
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	30	1583	32	168	865	87	247	90	40	48	83	141
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	3.92	0.08	1.00	2.73	0.27	1.00	0.69	0.31	1.00	0.37	0.63
Final Sat.:	1600	6573	127	1600	4561	439	1600	1108	492	1600	593	1007
Capacity Analysis Module:												
Vol/Sat:	0.02	0.24	0.25	0.11	0.19	0.20	0.15	0.08	0.08	0.03	0.14	0.14
Crit Moves:			****	****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #110 Red Hill Avenue and Walnut Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	0.785
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	49	Level Of Service:	C

Street Name:	Red Hill Avenue						Walnut Avenue					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	2	1	0	1	0	2	1	0	1	0	1	1	0	2	0	1	1	0
Volume Module:												
Base Vol:	287	1395	100	136	422	176	112	269	111	205	682	178
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	287	1395	100	136	422	176	112	269	111	205	682	178
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	287	1395	100	136	422	176	112	269	111	205	682	178
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	287	1395	100	136	422	176	112	269	111	205	682	178
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	287	1395	100	136	422	176	112	269	111	205	682	178
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	2.80	0.20	1.00	2.12	0.88	1.00	1.42	0.58	2.00	1.59	0.41
Final Sat.:	1600	4679	321	1600	3587	1413	1600	2365	935	3200	2638	662
Capacity Analysis Module:												
Vol/Sat:	0.18	0.30	0.31	0.09	0.12	0.12	0.07	0.11	0.12	0.06	0.26	0.27
Crit Moves:			****	****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #111 Red Hill Avenue and Valencia Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	0.549
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	26	Level Of Service:	A

Street Name:	Red Hill Avenue						Valencia Avenue					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3	1 0	1 0	2	1 0	1 0	1	0 1	2 0	1	0 1
Volume Module:												
Base Vol:	220	1637	270	109	438	49	70	189	83	118	171	113
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	220	1637	270	109	438	49	70	189	83	118	171	113
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	220	1637	270	109	438	49	70	189	83	118	171	113
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	220	1637	270	109	438	49	70	189	83	118	171	113
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	220	1637	270	109	438	49	70	189	83	118	171	113
OvlAdjVol:			237									4
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	3.43	0.57	1.00	2.70	0.30	1.00	1.00	1.00	2.00	1.00	1.00
Final Sat.:	1600	5794	906	1600	4517	483	1600	1700	1600	3200	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.14	0.28	0.30	0.07	0.10	0.10	0.04	0.11	0.05	0.04	0.10	0.07
OvlAdjV/S:			0.26									0.00
Crit Moves:	****			****				****		****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #112 Tustin Ranch Road and Warner Avenue North

Cycle (sec):	100	Critical Vol./Cap.(X):	0.757
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	44	Level Of Service:	C

Street Name:	Tustin Ranch Road						Warner Avenue North					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	3 0 1	2	0	3 0 0	0	0	0 0 0	3	0	0 0 2
Volume Module:												
Base Vol:	0	1623	956	351	516	0	0	0	0	479	0	433
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1623	956	351	516	0	0	0	0	479	0	433
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1623	956	351	516	0	0	0	0	479	0	433
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1623	956	351	516	0	0	0	0	479	0	433
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1623	956	351	516	0	0	0	0	479	0	433
OvlAdjVol:			796									82
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	0.00	3.00	1.00	2.00	3.00	0.00	0.00	0.00	0.00	3.00	0.00	2.00
Final Sat.:	0	5100	1600	3200	5100	0	0	0	0	4800	0	3200
Capacity Analysis Module:												
Vol/Sat:	0.00	0.32	0.60	0.11	0.10	0.00	0.00	0.00	0.00	0.10	0.00	0.14
OvlAdjV/S:			0.50									0.03
Crit Moves:			****	****						****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #113 Tustin Ranch Road and Walnut Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	1.630
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Street Name:	Tustin Ranch Road						Walnut Avenue					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Ovl			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0	3 0 1	2	0	3 0 1	2	0	1 1 0	2	0	2 0 1
Volume Module:												
Base Vol:	762	3319	349	454	1206	782	365	447	255	287	1646	1304
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	762	3319	349	454	1206	782	365	447	255	287	1646	1304
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	762	3319	349	454	1206	782	365	447	255	287	1646	1304
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	762	3319	349	454	1206	782	365	447	255	287	1646	1304
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	762	3319	349	454	1206	782	365	447	255	287	1646	1304
OvlAdjVol:			206			600						1077
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	1.27	0.73	2.00	2.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	2138	1162	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.24	0.65	0.22	0.14	0.24	0.49	0.11	0.21	0.22	0.09	0.48	0.82
OvlAdjV/S:			0.13			0.37						0.67
Crit Moves:	****			****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #114 SR-55 SB Ramps and Irvine Boulevard

Cycle (sec):	100	Critical Vol./Cap.(X):	0.794
Loss Time (sec):	5	Average Delay (sec/veh):	21.5
Optimal Cycle:	53	Level Of Service:	C

Street Name:	SR-55 SB Ramps						Irvine Boulevard					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	0	1	0	1! 0	1	0	0	2	1	0
Volume Module:												
Base Vol:	0	0	0	434	0	259	0	862	581	372	1069	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	434	0	259	0	862	581	372	1069	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	434	0	259	0	862	581	372	1069	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	434	0	259	0	862	581	372	1069	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	434	0	259	0	862	581	372	1069	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.74	1.00	0.73	1.00	0.86	0.86	0.95	0.91	1.00
Lanes:	0.00	0.00	0.00	1.62	0.00	1.38	0.00	2.00	1.00	1.00	3.00	0.00
Final Sat.:	0	0	0	2276	0	1912	0	3251	1625	1805	5187	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.19	0.00	0.14	0.00	0.27	0.36	0.21	0.21	0.00
Crit Moves:				****					****	****		
Green/Cycle:	0.00	0.00	0.00	0.24	0.00	0.48	0.00	0.45	0.45	0.26	0.71	0.00
Volume/Cap:	0.00	0.00	0.00	0.79	0.00	0.28	0.00	0.59	0.79	0.79	0.29	0.00
Delay/Veh:	0.0	0.0	0.0	40.8	0.0	15.7	0.0	20.9	26.0	43.6	5.3	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	40.8	0.0	15.7	0.0	20.9	26.0	43.6	5.3	0.0
LOS by Move:	A	A	A	D	A	B	A	C	C	D	A	A

```
HCM2k85thQ:     0    0     0    15    0     6     0   17    27    19    7     0
*******************************************************************************
Note: Queue reported is the number of cars per lane.
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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #115 SR-55 NB Ramps and Irvine Boulevard

Cycle (sec):	100	Critical Vol./Cap.(X):	0.809
Loss Time (sec):	5	Average Delay (sec/veh):	19.5
Optimal Cycle:	56	Level Of Service:	B

Street Name:	SR-55 NB Ramps						Irvine Boulevard					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	1! 0	1	0	0	0	0	1	0	3	0	0	0	0	2	1	0
Volume Module:																		
Base Vol:	328	16	298	0	0	0	229	1048	0	0	1132	716
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	328	16	298	0	0	0	229	1048	0	0	1132	716
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	328	16	298	0	0	0	229	1048	0	0	1132	716
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	328	16	298	0	0	0	229	1048	0	0	1132	716
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	328	16	298	0	0	0	229	1048	0	0	1132	716
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.86	0.86	0.86	1.00	1.00	1.00	0.95	0.91	1.00	1.00	0.86	0.86
Lanes:	1.50	0.05	1.45	0.00	0.00	0.00	1.00	3.00	0.00	0.00	2.00	1.00
Final Sat.:	2444	79	2370	0	0	0	1805	5187	0	0	3257	1629
Capacity Analysis Module:												
Vol/Sat:	0.13	0.20	0.13	0.00	0.00	0.00	0.13	0.20	0.00	0.00	0.35	0.44
Crit Moves:		****					****					****
Green/Cycle:	0.25	0.25	0.25	0.00	0.00	0.00	0.16	0.70	0.00	0.00	0.54	0.54
Volume/Cap:	0.54	0.81	0.50	0.00	0.00	0.00	0.81	0.29	0.00	0.00	0.64	0.81
Delay/Veh:	33.0	41.5	32.5	0.0	0.0	0.0	56.5	5.7	0.0	0.0	16.4	20.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	33.0	41.5	32.5	0.0	0.0	0.0	56.5	5.7	0.0	0.0	16.4	20.8
LOS by Move:	C	D	C	A	A	A	E	A	A	A	B	C
HCM2k85thQ:	10	18	9	0	0	0	14	7	0	0	21	31

Note: Queue reported is the number of cars per lane.

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APPENDIX D.3

ANALYSIS WORKSHEETS –
2045 NP (AM Peak Hour)

Santa Ana Circulation Element

Scenario Report

Scenario:	2045 NP AM Peak Hour
Command:	Default Command
Volume:	2045 NP AM
Geometry:	MPAH
Impact Fee:	Default Impact Fee
Trip Generation:	Default Trip Generation
Trip Distribution:	Default Trip Distribution
Paths:	Default Path
Routes:	Default Route
Configuration:	Default Configuration

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Santa Ana Circulation Element

Impact Analysis Report
Level Of Service

Intersection	Base LOS	Base Del/Veh	Base V/C	Future LOS	Future Del/Veh	Future V/C	Change in
# 1 Euclid St and 1st St	D	xxxxx	0.858	D	xxxxx	0.858	+ 0.000 V/C
# 2 Euclid St and McFadden Ave	E	xxxxx	0.975	E	xxxxx	0.975	+ 0.000 V/C
# 3 Euclid St and Edinger Ave	E	xxxxx	0.991	E	xxxxx	0.991	+ 0.000 V/C
# 4 Newhope St and Hazard Ave	C	xxxxx	0.777	C	xxxxx	0.777	+ 0.000 V/C
# 5 Newhope St and McFadden Ave	D	xxxxx	0.827	D	xxxxx	0.827	+ 0.000 V/C
# 6 Harbor Blvd and SR-22 WB Off-R	C	28.4	0.738	C	28.4	0.738	+ 0.000 D/V
# 7 Trask Avenue and SR-22 EB On-R	B	12.0	0.516	B	12.0	0.516	+ 0.000 D/V
# 8 Harbor Blvd and Westminster Av	E	xxxxx	0.985	E	xxxxx	0.985	+ 0.000 V/C
# 9 Harbor Blvd and 1st St	C	xxxxx	0.789	C	xxxxx	0.789	+ 0.000 V/C
# 10 Harbor Blvd and McFadden Ave	C	xxxxx	0.788	C	xxxxx	0.788	+ 0.000 V/C
# 11 Harbor Blvd and Edinger Ave	D	xxxxx	0.836	D	xxxxx	0.836	+ 0.000 V/C
# 12 Harbor Blvd and Warner Ave	C	xxxxx	0.789	C	xxxxx	0.789	+ 0.000 V/C
# 13 Harbor Blvd and Segerstrom Ave	E	xxxxx	0.965	E	xxxxx	0.965	+ 0.000 V/C
# 14 MacArthur Blvd and Hyland Ave	B	xxxxx	0.611	B	xxxxx	0.611	+ 0.000 V/C
# 15 MacArthur Blvd and Harbor Blvd	C	xxxxx	0.736	C	xxxxx	0.736	+ 0.000 V/C
# 16 Harbor Blvd and Sunflower Ave	C	xxxxx	0.704	C	xxxxx	0.704	+ 0.000 V/C
# 17 Harbor Blvd and I-405 NB Off-R	B	18.0	0.569	B	18.0	0.569	+ 0.000 D/V
# 18 Harbor Blvd and I-405 SB Off-R	B	15.0	0.612	B	15.0	0.612	+ 0.000 D/V
# 19 Fairview St and Civic Center D	C	xxxxx	0.741	C	xxxxx	0.741	+ 0.000 V/C
# 20 Fairview St and 1st St	D	xxxxx	0.859	D	xxxxx	0.859	+ 0.000 V/C
# 21 Fairview St and McFadden Ave	C	xxxxx	0.799	C	xxxxx	0.799	+ 0.000 V/C
# 22 Fairview St and Edinger Ave	D	xxxxx	0.861	D	xxxxx	0.861	+ 0.000 V/C
# 23 Fairview St and Warner Ave	D	xxxxx	0.816	D	xxxxx	0.816	+ 0.000 V/C
# 24 Fairview St and MacArthur Blvd	B	xxxxx	0.684	B	xxxxx	0.684	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Base Del/ Veh	Base V/ C	Future LOS	Future Del/ Veh	Future V/ C	Change in
# 25 Fairview Rd and Sunflower Ave	B	xxxxx	0.697	B	xxxxx	0.697	+ 0.000 V/C
# 26 Greenville St and Edinger Ave	C	xxxxx	0.774	C	xxxxx	0.774	+ 0.000 V/C
# 27 Greenville St and Segerstrom A	D	xxxxx	0.843	D	xxxxx	0.843	+ 0.000 V/C
# 28 Raitt St and McFadden Ave	C	xxxxx	0.778	C	xxxxx	0.778	+ 0.000 V/C
# 29 Raitt St and Edinger Ave	E	xxxxx	0.990	E	xxxxx	0.990	+ 0.000 V/C
# 30 Bear St and MacArthur Blvd	C	xxxxx	0.707	C	xxxxx	0.707	+ 0.000 V/C
# 31 Bristol St and 17th St	D	xxxxx	0.869	D	xxxxx	0.869	+ 0.000 V/C
# 32 Bristol St and Civic Center Dr	E	xxxxx	0.942	E	xxxxx	0.942	+ 0.000 V/C
# 33 Bristol St and Santa Ana Blvd	D	xxxxx	0.806	D	xxxxx	0.806	+ 0.000 V/C
# 34 Bristol St and 1st St	C	xxxxx	0.767	C	xxxxx	0.767	+ 0.000 V/C
# 35 Bristol St and McFadden Ave	E	xxxxx	0.956	E	xxxxx	0.956	+ 0.000 V/C
# 36 Bristol St and Warner Ave	E	xxxxx	0.915	E	xxxxx	0.915	+ 0.000 V/C
# 37 Bristol St and Segerstrom Ave	D	xxxxx	0.811	D	xxxxx	0.811	+ 0.000 V/C
# 38 Bristol St and Alton Ave	A	xxxxx	0.545	A	xxxxx	0.545	+ 0.000 V/C
# 39 Bristol St and MacArthur Blvd	B	xxxxx	0.697	B	xxxxx	0.697	+ 0.000 V/C
# 40 Bristol St and Sunflower Ave	B	xxxxx	0.649	B	xxxxx	0.649	+ 0.000 V/C
# 41 Bristol St and I-405 NB Ramps	C	20.8	0.557	C	20.8	0.557	+ 0.000 D/V
# 42 Bristol St and I-405 SB Ramps	C	23.9	0.696	C	23.9	0.696	+ 0.000 D/V
# 43 Flower St and Santa Ana Blvd	D	xxxxx	0.806	D	xxxxx	0.806	+ 0.000 V/C
# 44 Flower St and 1st St	E	xxxxx	0.926	E	xxxxx	0.926	+ 0.000 V/C
# 45 Flower St and McFadden Ave	E	xxxxx	0.900	E	xxxxx	0.900	+ 0.000 V/C
# 46 Flower St and Segerstrom Ave	C	xxxxx	0.784	C	xxxxx	0.784	+ 0.000 V/C
# 47 Flower St and MacArthur Blvd	B	xxxxx	0.675	B	xxxxx	0.675	+ 0.000 V/C
# 48 Main St and La Veta Ave	A	xxxxx	0.586	A	xxxxx	0.586	+ 0.000 V/C
# 49 Main St and Mainplace Dr / Mem	A	xxxxx	0.512	A	xxxxx	0.512	+ 0.000 V/C
# 50 Main St and 17th St	D	xxxxx	0.884	D	xxxxx	0.884	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Base Del/ Veh	Base V/ C	Future LOS	Future Del/ Veh	Future V/ C	Change in
# 51 Main St and Civic Center Dr	C	xxxxx	0.783	C	xxxxx	0.783	+ 0.000 V/C
# 52 Main St and Santa Ana Blvd	E	xxxxx	0.926	E	xxxxx	0.926	+ 0.000 V/C
# 53 Main St and 4th St	A	xxxxx	0.410	A	xxxxx	0.410	+ 0.000 V/C
# 54 Main St and 1st St	B	xxxxx	0.681	B	xxxxx	0.681	+ 0.000 V/C
# 55 Main St and McFadden Ave	D	xxxxx	0.894	D	xxxxx	0.894	+ 0.000 V/C
# 56 Main St and Edinger Ave	C	xxxxx	0.795	C	xxxxx	0.795	+ 0.000 V/C
# 57 Main St and MacArthur Blvd	B	xxxxx	0.680	B	xxxxx	0.680	+ 0.000 V/C
# 58 Penn Wy and 17th St	B	12.2	0.660	B	12.2	0.660	+ 0.000 D/V
# 59 I-5 NB Off Ramps/17th Street	C	30.0	0.710	C	30.0	0.710	+ 0.000 D/V
# 60 Penn Wy and I-5 SB Ramps	C	21.2	0.550	C	21.2	0.550	+ 0.000 D/V
# 61 Santiago St and Civic Center D	F	159.2	1.769	F	159.2	1.769	+ 0.000 V/C
# 62 Santiago St and Santa Ana Blvd	F	xxxxx	1.037	F	xxxxx	1.037	+ 0.000 V/C
# 63 Standard Ave and 4th St	E	xxxxx	0.939	E	xxxxx	0.939	+ 0.000 V/C
# 64 Standard Ave and 1st St	E	xxxxx	0.976	E	xxxxx	0.976	+ 0.000 V/C
# 65 Standard Ave and Mcfadden Ave	D	xxxxx	0.854	D	xxxxx	0.854	+ 0.000 V/C
# 66 Halladay St and Warner Ave	C	xxxxx	0.775	C	xxxxx	0.775	+ 0.000 V/C
# 67 Halladay St and Dyer Rd	C	xxxxx	0.774	C	xxxxx	0.774	+ 0.000 V/C
# 68 SR-55 SB Ramps and MacArthur B	B	19.9	0.608	B	19.9	0.608	+ 0.000 D/V
# 69 SR-55 NB Ramps and MacArthur B	B	19.3	0.712	B	19.3	0.712	+ 0.000 D/V
# 70 SR-55 SB Ramps and Dyer Rd	C	25.7	0.617	C	25.7	0.617	+ 0.000 D/V
# 71 Glassell St and La Veta Ave	D	xxxxx	0.800	D	xxxxx	0.800	+ 0.000 V/C
# 72 Glassell St and SR-22 WB Ramps	C	32.6	0.871	C	32.6	0.871	+ 0.000 D/V
# 73 Grand Ave / Glassell St and SR	C	30.6	0.860	C	30.6	0.860	+ 0.000 D/V
# 74 Grand Ave and Fairhaven Ave	B	xxxxx	0.611	B	xxxxx	0.611	+ 0.000 V/C
# 75 Grand Ave and Santa Clara Ave	D	xxxxx	0.843	D	xxxxx	0.843	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Del/ Veh	V/ C	Future LOS	Del/ Veh	V/ C	Change in
# 76 Grand Ave and 17th St	C	xxxxx	0.794	C	xxxxx	0.794	+ 0.000 V/C
# 77 Grand Ave and I-5 NB Ramps	B	13.2	0.740	B	13.2	0.740	+ 0.000 D/V
# 78 Grand Ave and Santa Ana Blvd	C	26.0	0.832	C	26.0	0.832	+ 0.000 D/V
# 79 Grand Ave and 1st St	C	xxxxx	0.742	C	xxxxx	0.742	+ 0.000 V/C
# 80 Grand Ave and Chestnut Ave	C	xxxxx	0.704	C	xxxxx	0.704	+ 0.000 V/C
# 81 Grand Ave and McFadden Ave	D	xxxxx	0.892	D	xxxxx	0.892	+ 0.000 V/C
# 82 Grand Ave and Edinger Ave	D	xxxxx	0.838	D	xxxxx	0.838	+ 0.000 V/C
# 83 Grand Ave and Warner Ave	B	xxxxx	0.613	B	xxxxx	0.613	+ 0.000 V/C
# 84 SR-55 NB Ramps and Dyer Rd	B	17.1	0.569	B	17.1	0.569	+ 0.000 D/V
# 85 Cambridge St and La Veta Ave	D	31.6	0.971	D	31.6	0.971	+ 0.000 V/C
# 86 Cambridge St and Fairhaven Ave	A	xxxxx	0.563	A	xxxxx	0.563	+ 0.000 V/C
# 87 Mabury St and 1st Street	C	27.4	0.714	C	27.4	0.714	+ 0.000 D/V
# 88 Tustin St and La Veta Ave	A	xxxxx	0.446	A	xxxxx	0.446	+ 0.000 V/C
# 89 Tustin St and SR-22 WB On-Ramp	B	12.2	0.635	B	12.2	0.635	+ 0.000 D/V
# 90 Tustin St and SR-22 EB Off-Ram	C	25.0	0.723	C	25.0	0.723	+ 0.000 D/V
# 91 Tustin Ave and Fairhaven Ave	F	xxxxx	1.195	F	xxxxx	1.195	+ 0.000 V/C
# 92 Tustin Ave and Santa Clara Ave	F	xxxxx	1.037	F	xxxxx	1.037	+ 0.000 V/C
# 93 Tustin Ave and 17th St	E	xxxxx	0.911	E	xxxxx	0.911	+ 0.000 V/C
# 94 Tustin Ave and 4th St	D	xxxxx	0.830	D	xxxxx	0.830	+ 0.000 V/C
# 95 SR-55 SB Ramps / Auto Mall Dr	C	25.2	0.672	C	25.2	0.672	+ 0.000 D/V
# 96 SR-55 NB Ramps / Del Amo Ave a	C	25.4	0.673	C	25.4	0.673	+ 0.000 D/V
# 97 Red Hill Ave and Edinger Ave	B	xxxxx	0.610	B	xxxxx	0.610	+ 0.000 V/C
# 98 Red Hill Ave and Warner Ave	A	xxxxx	0.560	A	xxxxx	0.560	+ 0.000 V/C
# 99 Red Hill Ave and Dyer Rd / bar	A	xxxxx	0.548	A	xxxxx	0.548	+ 0.000 V/C
#100 Red Hill Ave and Alton Pkwy	D	xxxxx	0.855	D	xxxxx	0.855	+ 0.000 V/C
#101 Red Hill Ave and MacArthur Blv	B	xxxxx	0.686	B	xxxxx	0.686	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Base Del/ Veh	Base V/ C	Future LOS	Future Del/ Veh	Future V/ C	Change in
#102 Red Hill Ave and Main St	C	xxxxx	0.722	C	xxxxx	0.722	+ 0.000 V/C
#103 I-5 SB Ramps and Santa Ana Blv	B	17.4	0.559	B	17.4	0.559	+ 0.000 D/V
#104 Tustin Ranch Rd and Warner Ave	A	xxxxx	0.520	A	xxxxx	0.520	+ 0.000 V/C
#105 Von Karman Ave and Barranca Pk	E	xxxxx	0.929	E	xxxxx	0.929	+ 0.000 V/C
#106 Red Hill Avenue and El Camino	B	xxxxx	0.650	B	xxxxx	0.650	+ 0.000 V/C
#107 Red Hill Avenue and I-5 NB Ram	C	20.8	0.627	C	20.8	0.627	+ 0.000 D/V
#108 Red Hill Avenue and I-5 SB Ram	C	23.7	0.799	C	23.7	0.799	+ 0.000 D/V
#109 Red Hill Avenue and Nisson Roa	B	xxxxx	0.651	B	xxxxx	0.651	+ 0.000 V/C
#110 Red Hill Avenue and Walnut Ave	B	xxxxx	0.636	B	xxxxx	0.636	+ 0.000 V/C
#111 Red Hill Avenue and Valencia A	B	xxxxx	0.652	B	xxxxx	0.652	+ 0.000 V/C
#112 Tustin Ranch Road and Warner A	A	xxxxx	0.449	A	xxxxx	0.449	+ 0.000 V/C
#113 Tustin Ranch Road and Walnut A	F	xxxxx	1.273	F	xxxxx	1.273	+ 0.000 V/C
#114 SR-55 SB Ramps and Irvine Boul	D	41.0	1.099	D	41.0	1.099	+ 0.000 D/V
#115 SR-55 NB Ramps and Irvine Boul	C	23.5	0.906	C	23.5	0.906	+ 0.000 D/V

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #1 Euclid St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.858
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	68	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			WideBypass			WideBypass		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	103	870	149	200	1785	101	252	937	177	190	606	194
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	103	870	149	200	1785	101	252	937	177	190	606	194
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	103	870	149	200	1785	101	252	937	177	190	606	194
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	103	870	149	200	1785	101	252	937	177	190	606	194
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	103	870	149	200	1785	101	252	937	177	190	606	194
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	1.00	2.56	0.44	1.00	2.84	0.16	1.00	2.52	0.48	1.00	2.27	0.73
Final Sat.:	1600	4298	702	1600	4743	257	1600	4237	763	1600	3836	1164
Capacity Analysis Module:												
Vol/Sat:	0.06	0.20	0.21	0.13	0.38	0.39	0.16	0.22	0.23	0.12	0.16	0.17
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #2 Euclid St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.975
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	172	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	162	1227	146	148	2062	171	255	521	323	151	335	127
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	162	1227	146	148	2062	171	255	521	323	151	335	127
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	162	1227	146	148	2062	171	255	521	323	151	335	127
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	162	1227	146	148	2062	171	255	521	323	151	335	127
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	162	1227	146	148	2062	171	255	521	323	151	335	127
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.04	1.00
Lanes:	1.00	2.68	0.32	1.00	2.77	0.23	1.00	1.23	0.77	1.00	1.45	0.55
Final Sat.:	1600	4490	510	1600	4632	368	1600	2075	1225	1600	2420	880
Capacity Analysis Module:												
Vol/Sat:	0.10	0.27	0.29	0.09	0.45	0.47	0.16	0.25	0.26	0.09	0.14	0.14
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #3 Euclid St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.991
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	1 0	2 0	1	1 0	2 0	1
Volume Module:												
Base Vol:	127	801	68	168	2442	224	160	700	428	125	761	139
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	127	801	68	168	2442	224	160	700	428	125	761	139
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	127	801	68	168	2442	224	160	700	428	125	761	139
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	127	801	68	168	2442	224	160	700	428	125	761	139
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	127	801	68	168	2442	224	160	700	428	125	761	139
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.77	0.23	2.00	2.75	0.25	1.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	3200	4624	376	3200	4597	403	1600	3400	1600	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.04	0.17	0.18	0.05	0.53	0.56	0.10	0.21	0.27	0.08	0.22	0.09
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #4 Newhope St and Hazard Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.777
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	48	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Prot+Permit			Prot+Permit			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	78	590	84	88	1302	119	108	276	204	119	397	159
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	78	590	84	88	1302	119	108	276	204	119	397	159
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	78	590	84	88	1302	119	108	276	204	119	397	159
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	78	590	84	88	1302	119	108	276	204	119	397	159
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	78	590	84	88	1302	119	108	276	204	119	397	159
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.03	1.00	1.00	1.05	1.00	1.00	1.04	1.00
Lanes:	1.00	1.75	0.25	1.00	1.83	0.17	1.00	1.15	0.85	1.00	1.43	0.57
Final Sat.:	1600	2901	399	1600	3032	268	1600	1940	1360	1600	2385	915
Capacity Analysis Module:												
Vol/Sat:	0.05	0.20	0.21	0.06	0.43	0.44	0.07	0.14	0.15	0.07	0.17	0.17
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #5 Newhope St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.827
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	58	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	77	342	120	122	1213	113	135	492	160	190	484	108
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	77	342	120	122	1213	113	135	492	160	190	484	108
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	77	342	120	122	1213	113	135	492	160	190	484	108
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	77	342	120	122	1213	113	135	492	160	190	484	108
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	77	342	120	122	1213	113	135	492	160	190	484	108
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	1.48	0.52	1.00	1.83	0.17	1.00	1.51	0.49	1.00	1.64	0.36
Final Sat.:	1600	2469	831	1600	3027	273	1600	2515	785	1600	2716	584
Capacity Analysis Module:												
Vol/Sat:	0.05	0.14	0.14	0.08	0.40	0.41	0.08	0.20	0.20	0.12	0.18	0.19
Crit Moves:	****					****		****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #6 Harbor Blvd and SR-22 WB Off-Ramp / Banner Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.738
Loss Time (sec):	5	Average Delay (sec/veh):	28.4
Optimal Cycle:	43	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	0	0 0	2	1 0	0 0	1! 0	0	1 1	0 0	1
Volume Module:												
Base Vol:	84	1534	0	0	1465	19	81	0	133	836	44	117
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	84	1534	0	0	1465	19	81	0	133	836	44	117
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	84	1534	0	0	1465	19	81	0	133	836	44	117
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	84	1534	0	0	1465	19	81	0	133	836	44	117
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	84	1534	0	0	1465	19	81	0	133	836	44	117
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	1.00	1.00	0.91	0.91	0.90	1.00	0.90	0.96	0.96	0.85
Lanes:	1.00	3.00	0.00	0.00	2.96	0.04	0.38	0.00	0.62	1.90	0.10	1.00
Final Sat.:	1805	5187	0	0	5110	66	646	0	1061	3448	181	1615
Capacity Analysis Module:												
Vol/Sat:	0.05	0.30	0.00	0.00	0.29	0.29	0.13	0.00	0.13	0.24	0.24	0.07
Crit Moves:	****				****		****				****	
Green/Cycle:	0.06	0.45	0.00	0.00	0.39	0.39	0.17	0.00	0.17	0.33	0.33	0.33
Volume/Cap:	0.74	0.65	0.00	0.00	0.74	0.74	0.74	0.00	0.74	0.74	0.74	0.22
Delay/Veh:	68.3	22.0	0.0	0.0	27.7	27.7	49.0	0.0	49.0	32.2	32.2	24.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	68.3	22.0	0.0	0.0	27.7	27.7	49.0	0.0	49.0	32.2	32.2	24.5
LOS by Move:	E	C	A	A	C	C	D	A	D	C	C	C
HCM2k85thQ:	7	21	0	0	23	23	12	0	12	20	20	4

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #7 Trask Avenue and SR-22 EB On-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.516
Loss Time (sec):	5	Average Delay (sec/veh):	12.0
Optimal Cycle:	26	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	0 0	0	0 0	0 0	0	2 0	1 0	0	0 0	1 1	0
Volume Module:												
Base Vol:	0	0	0	0	0	0	713	893	0	0	852	159
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	0	0	0	713	893	0	0	852	159
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	0	0	0	713	893	0	0	852	159
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	0	0	0	713	893	0	0	852	159
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	0	0	0	713	893	0	0	852	159
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	0.92	1.00	1.00	1.00	0.93	0.93
Lanes:	0.00	0.00	0.00	0.00	0.00	0.00	2.00	1.00	0.00	0.00	1.69	0.31
Final Sat.:	0	0	0	0	0	0	3502	1900	0	0	2969	554
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.00	0.00	0.00	0.20	0.47	0.00	0.00	0.29	0.29
Crit Moves:							****				****	
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.39	0.95	0.00	0.00	0.56	0.56
Volume/Cap:	0.00	0.00	0.00	0.00	0.00	0.00	0.52	0.49	0.00	0.00	0.52	0.52
Delay/Veh:	0.0	0.0	0.0	0.0	0.0	0.0	23.4	0.5	0.0	0.0	14.1	14.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	0.0	0.0	0.0	23.4	0.5	0.0	0.0	14.1	14.1
LOS by Move:	A	A	A	A	A	A	C	A	A	A	B	B
HCM2k85thQ:	0	0	0	0	0	0	13	5	0	0	16	16

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #8 Harbor Blvd and Westminster Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.985
Loss Time (sec):	0	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	2 1	0	1 0	3 0	1
Volume Module:												
Base Vol:	178	1173	251	258	2021	115	192	1010	36	415	1124	306
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	178	1173	251	258	2021	115	192	1010	36	415	1124	306
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	178	1173	251	258	2021	115	192	1010	36	415	1124	306
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	178	1173	251	258	2021	115	192	1010	36	415	1124	306
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	178	1173	251	258	2021	115	192	1010	36	415	1124	306
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	2.90	0.10	1.00	3.00	1.00
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	4835	165	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.11	0.23	0.16	0.16	0.40	0.07	0.12	0.21	0.22	0.26	0.22	0.19
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #9 Harbor Blvd and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.789
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	50	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	95	756	163	236	1989	129	138	919	178	196	497	116
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	95	756	163	236	1989	129	138	919	178	196	497	116
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	95	756	163	236	1989	129	138	919	178	196	497	116
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	95	756	163	236	1989	129	138	919	178	196	497	116
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	95	756	163	236	1989	129	138	919	178	196	497	116
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	2.00	2.51	0.49	2.00	2.43	0.57
Final Sat.:	1600	5100	1600	1600	5100	1600	3200	4221	779	3200	4092	908
Capacity Analysis Module:												
Vol/Sat:	0.06	0.15	0.10	0.15	0.39	0.08	0.04	0.22	0.23	0.06	0.12	0.13
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #10 Harbor Blvd and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.788
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	50	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	104	1059	101	190	2004	67	177	462	138	138	311	89
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	104	1059	101	190	2004	67	177	462	138	138	311	89
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	104	1059	101	190	2004	67	177	462	138	138	311	89
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	104	1059	101	190	2004	67	177	462	138	138	311	89
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	104	1059	101	190	2004	67	177	462	138	138	311	89
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	2.00	3.00	1.00	2.00	2.90	0.10	1.00	1.54	0.46	1.00	1.56	0.44
Final Sat.:	3200	5100	1600	3200	4845	155	1600	2564	736	1600	2588	712
Capacity Analysis Module:												
Vol/Sat:	0.03	0.21	0.06	0.06	0.41	0.43	0.11	0.18	0.19	0.09	0.12	0.13
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #11 Harbor Blvd and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.836
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	61	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	2 0	2 1	0	2 0	3 0	1
Volume Module:												
Base Vol:	103	772	146	204	2349	68	123	743	373	191	424	176
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	103	772	146	204	2349	68	123	743	373	191	424	176
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	103	772	146	204	2349	68	123	743	373	191	424	176
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	103	772	146	204	2349	68	123	743	373	191	424	176
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	103	772	146	204	2349	68	123	743	373	191	424	176
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	2.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	3400	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.15	0.09	0.06	0.46	0.04	0.04	0.22	0.23	0.06	0.08	0.11
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #12 Harbor Blvd and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.789
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	50	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0 2	1 0		2 0 3	0 1		2 0 3	0 1		2 0 3	0 1	
Volume Module:												
Base Vol:	109	665	169	429	1933	116	101	1440	379	138	723	122
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	109	665	169	429	1933	116	101	1440	379	138	723	122
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	109	665	169	429	1933	116	101	1440	379	138	723	122
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	109	665	169	429	1933	116	101	1440	379	138	723	122
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	109	665	169	429	1933	116	101	1440	379	138	723	122
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.39	0.61	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	4027	973	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.17	0.17	0.13	0.38	0.07	0.03	0.28	0.24	0.04	0.14	0.08
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #13 Harbor Blvd and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.965
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	152	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 0	2	0 1
Volume Module:												
Base Vol:	107	759	48	169	2274	85	82	689	278	140	434	98
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	107	759	48	169	2274	85	82	689	278	140	434	98
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	107	759	48	169	2274	85	82	689	278	140	434	98
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	107	759	48	169	2274	85	82	689	278	140	434	98
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	107	759	48	169	2274	85	82	689	278	140	434	98
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	2.82	0.18	1.00	2.89	0.11	1.00	1.43	0.57	1.00	2.00	1.00
Final Sat.:	3200	4714	286	1600	4827	173	1600	2380	920	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.16	0.17	0.11	0.47	0.49	0.05	0.29	0.30	0.09	0.13	0.06
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #14 MacArthur Blvd and Hyland Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.611
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	30	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	0 1	0	1 0	1 0	1	1 0	3 0	1	1 0	3 0	1
Volume Module:												
Base Vol:	86	4	24	8	2	19	7	1934	767	68	517	21
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	86	4	24	8	2	19	7	1934	767	68	517	21
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	86	4	24	8	2	19	7	1934	767	68	517	21
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	86	4	24	8	2	19	7	1934	767	68	517	21
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	86	4	24	8	2	19	7	1934	767	68	517	21
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	0.14	0.86	1.00	1.00	1.00	1.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	229	1371	1600	1700	1600	1600	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.02	0.02	0.01	0.00	0.01	0.00	0.38	0.48	0.04	0.10	0.01
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #15 MacArthur Blvd and Harbor Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.736
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	41	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	1 0	3 0	1	1 0	3 0	1
Volume Module:												
Base Vol:	131	920	94	295	1701	118	131	1183	357	128	427	127
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	131	920	94	295	1701	118	131	1183	357	128	427	127
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	131	920	94	295	1701	118	131	1183	357	128	427	127
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	131	920	94	295	1701	118	131	1183	357	128	427	127
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	131	920	94	295	1701	118	131	1183	357	128	427	127
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	1.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	1600	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.04	0.18	0.06	0.09	0.33	0.07	0.08	0.23	0.22	0.08	0.08	0.08
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #16 Harbor Blvd and Sunflower Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.704
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	38	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	1 0	1 1	0	1 1	0 1	0
Volume Module:												
Base Vol:	221	1128	209	233	2204	54	13	153	59	139	178	101
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	221	1128	209	233	2204	54	13	153	59	139	178	101
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	221	1128	209	233	2204	54	13	153	59	139	178	101
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	221	1128	209	233	2204	54	13	153	59	139	178	101
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	221	1128	209	233	2204	54	13	153	59	139	178	101
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	1.00	1.44	0.56	1.00	1.28	0.72
Final Sat.:	3200	5100	1600	3200	5100	1600	1600	2409	891	1601	2146	1153
Capacity Analysis Module:												
Vol/Sat:	0.07	0.22	0.13	0.07	0.43	0.03	0.01	0.06	0.07	0.09	0.08	0.09
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #17 Harbor Blvd and I-405 NB Off-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.569
Loss Time (sec):	5	Average Delay (sec/veh):	18.0
Optimal Cycle:	28	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 4	0 0	0	0 4	0 0	0	0 0	0 0	1	0 1!	0 1
Volume Module:												
Base Vol:	0	1915	0	0	1978	0	0	0	0	399	0	702
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1915	0	0	1978	0	0	0	0	399	0	702
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1915	0	0	1978	0	0	0	0	399	0	702
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1915	0	0	1978	0	0	0	0	399	0	702
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1915	0	0	1978	0	0	0	0	399	0	702
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	1.00	1.00	0.91	1.00	1.00	1.00	1.00	0.89	1.00	0.89
Lanes:	0.00	4.00	0.00	0.00	4.00	0.00	0.00	0.00	0.00	1.36	0.00	1.64
Final Sat.:	0	6916	0	0	6916	0	0	0	0	2298	0	2762
Capacity Analysis Module:												
Vol/Sat:	0.00	0.28	0.00	0.00	0.29	0.00	0.00	0.00	0.00	0.17	0.00	0.25
Crit Moves:	****				****							****
Green/Cycle:	0.00	0.50	0.00	0.00	0.50	0.00	0.00	0.00	0.00	0.45	0.00	0.45
Volume/Cap:	0.00	0.55	0.00	0.00	0.57	0.00	0.00	0.00	0.00	0.39	0.00	0.57
Delay/Veh:	0.0	17.3	0.0	0.0	17.5	0.0	0.0	0.0	0.0	18.6	0.0	20.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	17.3	0.0	0.0	17.5	0.0	0.0	0.0	0.0	18.6	0.0	20.9
LOS by Move:	A	B	A	A	B	A	A	A	A	B	A	C
HCM2k85thQ:	0	17	0	0	18	0	0	0	0	9	0	15

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #18 Harbor Blvd and I-405 SB Off-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.612
Loss Time (sec):	5	Average Delay (sec/veh):	15.0
Optimal Cycle:	31	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Ignore			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 3	0 1	0	0 4	0 0	1	0 1!	0 1	0	0 0	0 0
Volume Module:												
Base Vol:	0	1963	0	0	1253	0	475	0	529	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1963	0	0	1253	0	475	0	529	0	0	0
User Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1963	0	0	1253	0	475	0	529	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1963	0	0	1253	0	475	0	529	0	0	0
PCE Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1963	0	0	1253	0	475	0	529	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	1.00	1.00	0.91	1.00	0.90	1.00	0.90	1.00	1.00	1.00
Lanes:	0.00	3.00	1.00	0.00	4.00	0.00	1.47	0.00	1.53	0.00	0.00	0.00
Final Sat.:	0	5187	1900	0	6916	0	2519	0	2610	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.38	0.00	0.00	0.18	0.00	0.19	0.00	0.20	0.00	0.00	0.00
Crit Moves:		****		****					****			
Green/Cycle:	0.00	0.62	0.00	0.00	0.62	0.00	0.33	0.00	0.33	0.00	0.00	0.00
Volume/Cap:	0.00	0.61	0.00	0.00	0.29	0.00	0.57	0.00	0.61	0.00	0.00	0.00
Delay/Veh:	0.0	12.0	0.0	0.0	8.9	0.0	28.0	0.0	28.7	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	12.0	0.0	0.0	8.9	0.0	28.0	0.0	28.7	0.0	0.0	0.0
LOS by Move:	A	B	A	A	A	A	C	A	C	A	A	A
HCM2k85thQ:	0	21	0	0	8	0	13	0	15	0	0	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #19 Fairview St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.741
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	42	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	0 1	0	1 0	1 1	0	0 1
Volume Module:												
Base Vol:	7	1569	508	332	1915	8	7	25	25	100	3	52
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	7	1569	508	332	1915	8	7	25	25	100	3	52
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	7	1569	508	332	1915	8	7	25	25	100	3	52
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	7	1569	508	332	1915	8	7	25	25	100	3	52
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	7	1569	508	332	1915	8	7	25	25	100	3	52
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	2.27	0.73	1.00	2.99	0.01	0.24	0.88	0.88	1.94	0.06	1.00
Final Sat.:	1600	3826	1174	1600	4980	20	393	1404	1404	3107	93	1600
Capacity Analysis Module:												
Vol/Sat:	0.00	0.41	0.43	0.21	0.38	0.40	0.02	0.02	0.02	0.03	0.03	0.03
Crit Moves:			****	****				****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #20 Fairview St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.859
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	68	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	2 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	200	1538	289	245	1426	164	200	1153	179	120	539	181
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	200	1538	289	245	1426	164	200	1153	179	120	539	181
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	200	1538	289	245	1426	164	200	1153	179	120	539	181
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	200	1538	289	245	1426	164	200	1153	179	120	539	181
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	200	1538	289	245	1426	164	200	1153	179	120	539	181
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	1.00	3.00	1.00	1.00	2.69	0.31	1.00	2.60	0.40	1.00	2.25	0.75
Final Sat.:	1600	5100	1600	1600	4505	495	1600	4355	645	1600	3793	1207
Capacity Analysis Module:												
Vol/Sat:	0.13	0.30	0.18	0.15	0.32	0.33	0.13	0.26	0.28	0.08	0.14	0.15
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #21 Fairview St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.799
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	52	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	141	1510	202	154	1878	136	221	522	120	147	380	30
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	141	1510	202	154	1878	136	221	522	120	147	380	30
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	141	1510	202	154	1878	136	221	522	120	147	380	30
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	141	1510	202	154	1878	136	221	522	120	147	380	30
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	141	1510	202	154	1878	136	221	522	120	147	380	30
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.03	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	1.63	0.37	1.00	1.85	0.15
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	2702	598	1600	3066	234
Capacity Analysis Module:												
Vol/Sat:	0.09	0.30	0.13	0.10	0.37	0.09	0.14	0.19	0.20	0.09	0.12	0.13
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #22 Fairview St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.861
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	69	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	200	883	123	223	1558	132	361	1002	268	421	553	110
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	200	883	123	223	1558	132	361	1002	268	421	553	110
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	200	883	123	223	1558	132	361	1002	268	421	553	110
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	200	883	123	223	1558	132	361	1002	268	421	553	110
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	200	883	123	223	1558	132	361	1002	268	421	553	110
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	2.00	2.63	0.37	2.00	2.77	0.23	2.00	2.37	0.63	2.00	2.50	0.50
Final Sat.:	3200	4413	587	3200	4625	375	3200	3987	1013	3200	4204	796
Capacity Analysis Module:												
Vol/Sat:	0.06	0.20	0.21	0.07	0.34	0.35	0.11	0.25	0.26	0.13	0.13	0.14
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #23 Fairview St and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.816
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	55	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	222	981	129	194	1543	113	68	991	295	267	807	261
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	222	981	129	194	1543	113	68	991	295	267	807	261
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	222	981	129	194	1543	113	68	991	295	267	807	261
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	222	981	129	194	1543	113	68	991	295	267	807	261
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	222	981	129	194	1543	113	68	991	295	267	807	261
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	2.65	0.35	2.00	2.80	0.20	2.00	2.31	0.69	2.00	2.27	0.73
Final Sat.:	3200	4442	558	3200	4672	328	3200	3899	1101	3200	3827	1173
Capacity Analysis Module:												
Vol/Sat:	0.07	0.22	0.23	0.06	0.33	0.35	0.02	0.25	0.27	0.08	0.21	0.22
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #24 Fairview St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.684
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	36	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	261	811	94	290	1460	163	121	984	145	235	494	140
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	261	811	94	290	1460	163	121	984	145	235	494	140
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	261	811	94	290	1460	163	121	984	145	235	494	140
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	261	811	94	290	1460	163	121	984	145	235	494	140
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	261	811	94	290	1460	163	121	984	145	235	494	140
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.69	0.31	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	4501	499	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.08	0.18	0.19	0.09	0.29	0.10	0.04	0.19	0.09	0.07	0.10	0.09
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #25 Fairview Rd and Sunflower Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.697
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	37	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	2 0	1 1	0	2 0	2 0	1
Volume Module:												
Base Vol:	185	1071	163	191	1627	104	49	322	65	357	299	154
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	185	1071	163	191	1627	104	49	322	65	357	299	154
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	185	1071	163	191	1627	104	49	322	65	357	299	154
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	185	1071	163	191	1627	104	49	322	65	357	299	154
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	185	1071	163	191	1627	104	49	322	65	357	299	154
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	2.82	0.18	2.00	1.66	0.34	2.00	2.00	1.00
Final Sat.:	3200	5100	1600	3200	4712	288	3200	2763	537	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.06	0.21	0.10	0.06	0.35	0.36	0.02	0.12	0.12	0.11	0.09	0.10
Crit Moves:	****					****		****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #26 Greenville St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.774
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	47	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	0 1	0 0	0	0 0	0 0	2	1 0	1 0	3	0 0
Volume Module:												
Base Vol:	186	0	251	0	0	0	0	1821	444	153	813	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	186	0	251	0	0	0	0	1821	444	153	813	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	186	0	251	0	0	0	0	1821	444	153	813	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	186	0	251	0	0	0	0	1821	444	153	813	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	186	0	251	0	0	0	0	1821	444	153	813	0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	2.41	0.59	1.00	3.00	0.00
Final Sat.:	1600	0	1600	0	0	0	0	4059	941	1600	5100	0
Capacity Analysis Module:												
Vol/Sat:	0.12	0.00	0.16	0.00	0.00	0.00	0.00	0.45	0.47	0.10	0.16	0.00
Crit Moves:			****						****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #27 Greenville St and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.843
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	63	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	1 0	1 0	0	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	47	193	78	154	472	160	46	1035	165	190	768	88
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	47	193	78	154	472	160	46	1035	165	190	768	88
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	47	193	78	154	472	160	46	1035	165	190	768	88
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	47	193	78	154	472	160	46	1035	165	190	768	88
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	47	193	78	154	472	160	46	1035	165	190	768	88
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	0.71	0.29	1.00	0.75	0.25	1.00	2.59	0.41	1.00	2.69	0.31
Final Sat.:	1600	1139	461	1600	1195	405	1600	4340	660	1600	4507	493
Capacity Analysis Module:												
Vol/Sat:	0.03	0.17	0.17	0.10	0.39	0.40	0.03	0.24	0.25	0.12	0.17	0.18
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #28 Raitt St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.778
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	48	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	67	618	98	123	938	128	122	743	95	146	461	82
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	67	618	98	123	938	128	122	743	95	146	461	82
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	67	618	98	123	938	128	122	743	95	146	461	82
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	67	618	98	123	938	128	122	743	95	146	461	82
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	67	618	98	123	938	128	122	743	95	146	461	82
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	1.73	0.27	1.00	1.76	0.24	1.00	1.77	0.23	1.00	1.70	0.30
Final Sat.:	1600	2862	438	1600	2916	384	1600	2937	363	1600	2817	483
Capacity Analysis Module:												
Vol/Sat:	0.04	0.22	0.22	0.08	0.32	0.33	0.08	0.25	0.26	0.09	0.16	0.17
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #29 Raitt St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.990
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	197	524	189	163	770	91	324	1980	285	122	954	56
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	197	524	189	163	770	91	324	1980	285	122	954	56
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	197	524	189	163	770	91	324	1980	285	122	954	56
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	197	524	189	163	770	91	324	1980	285	122	954	56
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	197	524	189	163	770	91	324	1980	285	122	954	56
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.03	1.00	1.00	1.05	1.00	1.00	1.04	1.00
Lanes:	1.00	1.47	0.53	1.00	1.79	0.21	1.00	2.62	0.38	1.00	2.83	0.17
Final Sat.:	1600	2452	848	1600	2962	338	1600	4396	604	1600	4734	266
Capacity Analysis Module:												
Vol/Sat:	0.12	0.21	0.22	0.10	0.26	0.27	0.20	0.45	0.47	0.08	0.20	0.21
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #30 Bear St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.707
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	38	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	1	1 0	2 0	1	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	71	245	104	199	756	300	92	1581	108	62	1041	88
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	71	245	104	199	756	300	92	1581	108	62	1041	88
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	71	245	104	199	756	300	92	1581	108	62	1041	88
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	71	245	104	199	756	300	92	1581	108	62	1041	88
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	71	245	104	199	756	300	92	1581	108	62	1041	88
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.81	0.19	1.00	2.77	0.23
Final Sat.:	1600	3400	1600	1600	3400	1600	1600	4693	307	1600	4626	374
Capacity Analysis Module:												
Vol/Sat:	0.04	0.07	0.07	0.12	0.22	0.19	0.06	0.34	0.35	0.04	0.23	0.24
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #31 Bristol St and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.869
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	72	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	2 0	2 1	0	2 0	3 0	1
Volume Module:												
Base Vol:	251	1292	218	487	2001	263	324	963	156	368	845	280
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	251	1292	218	487	2001	263	324	963	156	368	845	280
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	251	1292	218	487	2001	263	324	963	156	368	845	280
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	251	1292	218	487	2001	263	324	963	156	368	845	280
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	251	1292	218	487	2001	263	324	963	156	368	845	280
OvlAdjVol:												36
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	2.58	0.42	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	4331	669	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.08	0.25	0.14	0.15	0.39	0.16	0.10	0.22	0.23	0.12	0.17	0.17
OvlAdjV/S:												0.02
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #32 Bristol St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.942
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	120	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	133	1178	104	403	1628	101	165	836	103	127	421	87
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	133	1178	104	403	1628	101	165	836	103	127	421	87
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	133	1178	104	403	1628	101	165	836	103	127	421	87
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	133	1178	104	403	1628	101	165	836	103	127	421	87
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	133	1178	104	403	1628	101	165	836	103	127	421	87
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	2.76	0.24	1.00	2.82	0.18	1.00	1.78	0.22	1.00	1.66	0.34
Final Sat.:	1600	4611	389	1600	4720	280	1600	2949	351	1600	2752	548
Capacity Analysis Module:												
Vol/Sat:	0.08	0.26	0.27	0.25	0.34	0.36	0.10	0.28	0.29	0.08	0.15	0.16
Crit Moves:			****	****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #33 Bristol St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.806
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	53	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	69	1502	209	153	1684	46	171	831	72	185	462	79
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	69	1502	209	153	1684	46	171	831	72	185	462	79
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	69	1502	209	153	1684	46	171	831	72	185	462	79
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	69	1502	209	153	1684	46	171	831	72	185	462	79
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	69	1502	209	153	1684	46	171	831	72	185	462	79
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.63	0.37	1.00	2.92	0.08	1.00	2.76	0.24	1.00	2.56	0.44
Final Sat.:	1600	4414	586	1600	4872	128	1600	4617	383	1600	4299	701
Capacity Analysis Module:												
Vol/Sat:	0.04	0.34	0.36	0.10	0.35	0.36	0.11	0.18	0.19	0.12	0.11	0.11
Crit Moves:		****	****						****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #34 Bristol St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.767
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	46	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	205	1430	181	315	1298	183	200	1231	124	90	678	99
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	205	1430	181	315	1298	183	200	1231	124	90	678	99
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	205	1430	181	315	1298	183	200	1231	124	90	678	99
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	205	1430	181	315	1298	183	200	1231	124	90	678	99
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	205	1430	181	315	1298	183	200	1231	124	90	678	99
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	1.00	2.73	0.27	1.00	2.62	0.38
Final Sat.:	3200	5100	1600	3200	5100	1600	1600	4561	439	1600	4388	612
Capacity Analysis Module:												
Vol/Sat:	0.06	0.28	0.11	0.10	0.25	0.11	0.13	0.27	0.28	0.06	0.15	0.16
Crit Moves:		****		****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #35 Bristol St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.956
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	139	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0 3	0 1	1	0 3	0 1	1	0 1	1 0	1	0 1	1 0
Volume Module:												
Base Vol:	125	1117	56	204	1986	205	309	531	261	262	729	84
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	125	1117	56	204	1986	205	309	531	261	262	729	84
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	125	1117	56	204	1986	205	309	531	261	262	729	84
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	125	1117	56	204	1986	205	309	531	261	262	729	84
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	125	1117	56	204	1986	205	309	531	261	262	729	84
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.03	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	1.34	0.66	1.00	1.79	0.21
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	2245	1055	1600	2969	331
Capacity Analysis Module:												
Vol/Sat:	0.08	0.22	0.04	0.13	0.39	0.13	0.19	0.24	0.25	0.16	0.25	0.25
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #36 Bristol St and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.915
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	96	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	1 0	3 0	1	1 0	3 0	1
Volume Module:												
Base Vol:	221	993	183	600	2033	230	165	919	229	231	1058	180
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	221	993	183	600	2033	230	165	919	229	231	1058	180
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	221	993	183	600	2033	230	165	919	229	231	1058	180
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	221	993	183	600	2033	230	165	919	229	231	1058	180
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	221	993	183	600	2033	230	165	919	229	231	1058	180
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.53	0.47	2.00	2.70	0.30	1.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	4253	747	3200	4512	488	1600	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.07	0.23	0.24	0.19	0.45	0.47	0.10	0.18	0.14	0.14	0.21	0.11
Crit Moves:	****					****		****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #37 Bristol St and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.811
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	54	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	70	778	146	283	1011	119	280	1354	125	134	607	78
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	70	778	146	283	1011	119	280	1354	125	134	607	78
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	70	778	146	283	1011	119	280	1354	125	134	607	78
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	70	778	146	283	1011	119	280	1354	125	134	607	78
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	70	778	146	283	1011	119	280	1354	125	134	607	78
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.53	0.47	1.00	2.68	0.32	1.00	2.75	0.25	1.00	2.66	0.34
Final Sat.:	1600	4242	758	1600	4495	505	1600	4594	406	1600	4453	547
Capacity Analysis Module:												
Vol/Sat:	0.04	0.18	0.19	0.18	0.22	0.24	0.17	0.29	0.31	0.08	0.14	0.14
Crit Moves:			****	****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #38 Bristol St and Alton Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.545
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	26	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	3 0	1	1 0	1 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	11	739	55	230	1591	36	164	101	211	41	87	31
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	11	739	55	230	1591	36	164	101	211	41	87	31
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	11	739	55	230	1591	36	164	101	211	41	87	31
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	11	739	55	230	1591	36	164	101	211	41	87	31
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	11	739	55	230	1591	36	164	101	211	41	87	31
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	1.00	2.79	0.21	1.00	3.00	1.00	1.00	1.00	1.00	1.00	0.74	0.26
Final Sat.:	1600	4668	332	1600	5100	1600	1600	1700	1600	1600	1180	420
Capacity Analysis Module:												
Vol/Sat:	0.01	0.16	0.17	0.14	0.31	0.02	0.10	0.06	0.13	0.03	0.07	0.07
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #39 Bristol St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.697
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	37	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	102	531	133	329	1434	150	166	1203	244	159	758	110
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	102	531	133	329	1434	150	166	1203	244	159	758	110
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	102	531	133	329	1434	150	166	1203	244	159	758	110
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	102	531	133	329	1434	150	166	1203	244	159	758	110
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	102	531	133	329	1434	150	166	1203	244	159	758	110
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	2.72	0.28	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	4545	455	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.10	0.08	0.10	0.32	0.33	0.05	0.24	0.15	0.05	0.15	0.07
Crit Moves:	****					****		****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #40 Bristol St and Sunflower Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.649
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	32	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 1	2 0	3	0 1	2 0	2	1 1	2 0	3	0 1
Volume Module:												
Base Vol:	111	546	161	242	1370	103	112	926	375	295	480	166
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	111	546	161	242	1370	103	112	926	375	295	480	166
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	111	546	161	242	1370	103	112	926	375	295	480	166
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	111	546	161	242	1370	103	112	926	375	295	480	166
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	111	546	161	242	1370	103	112	926	375	295	480	166
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	2.85	1.15	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	4755	1845	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.11	0.10	0.08	0.27	0.06	0.04	0.19	0.20	0.09	0.09	0.10
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #41 Bristol St and I-405 NB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.557
Loss Time (sec):	5	Average Delay (sec/veh):	20.8
Optimal Cycle:	28	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 4	0 1	0	0 4	1 0	0	0 0	0 2	1	1 1	0 2
Volume Module:												
Base Vol:	0	1638	210	0	1918	8	0	0	36	145	87	796
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1638	210	0	1918	8	0	0	36	145	87	796
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1638	210	0	1918	8	0	0	36	145	87	796
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1638	210	0	1918	8	0	0	36	145	87	796
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1638	210	0	1918	8	0	0	36	145	87	796
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	0.85	1.00	0.91	0.91	1.00	1.00	0.75	0.92	0.92	0.75
Lanes:	0.00	4.00	1.00	0.00	4.98	0.02	0.00	0.00	2.00	1.88	1.12	2.00
Final Sat.:	0	6916	1615	0	8600	36	0	0	2842	3283	1970	2842
Capacity Analysis Module:												
Vol/Sat:	0.00	0.24	0.13	0.00	0.22	0.22	0.00	0.00	0.01	0.04	0.04	0.28
Crit Moves:		****			****				****			****
Green/Cycle:	0.00	0.42	0.42	0.00	0.42	0.42	0.00	0.00	0.02	0.50	0.50	0.50
Volume/Cap:	0.00	0.56	0.31	0.00	0.52	0.52	0.00	0.00	0.56	0.09	0.09	0.56
Delay/Veh:	0.0	21.9	19.3	0.0	21.4	21.4	0.0	0.0	58.8	13.0	13.0	17.7
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	21.9	19.3	0.0	21.4	21.4	0.0	0.0	58.8	13.0	13.0	17.7
LOS by Move:	A	C	B	A	C	C	A	A	E	B	B	B
HCM2k85thQ:	0	16	7	0	15	15	0	0	2	2	2	15

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #42 Bristol St and I-405 SB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.696
Loss Time (sec):	5	Average Delay (sec/veh):	23.9
Optimal Cycle:	38	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Ignore			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	4 0	0	0 0	2 1	1	3 0	0 0	1	0 0	0 0	0
Volume Module:												
Base Vol:	125	1225	0	0	1030	886	696	0	636	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	125	1225	0	0	1030	886	696	0	636	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	125	1225	0	0	1030	0	696	0	636	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	125	1225	0	0	1030	0	696	0	636	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	125	1225	0	0	1030	0	696	0	636	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	1.00	1.00	0.91	0.91	0.92	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	4.00	0.00	0.00	3.00	1.00	3.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	6916	0	0	5187	1729	5253	0	1615	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.07	0.18	0.00	0.00	0.20	0.00	0.13	0.00	0.39	0.00	0.00	0.00
Crit Moves:	****				****				****			
Green/Cycle:	0.10	0.38	0.00	0.00	0.29	0.00	0.57	0.00	0.57	0.00	0.00	0.00
Volume/Cap:	0.70	0.46	0.00	0.00	0.70	0.00	0.23	0.00	0.70	0.00	0.00	0.00
Delay/Veh:	54.9	23.1	0.0	0.0	33.4	0.0	10.9	0.0	18.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	54.9	23.1	0.0	0.0	33.4	0.0	10.9	0.0	18.0	0.0	0.0	0.0
LOS by Move:	D	C	A	A	C	A	B	A	B	A	A	A
HCM2k85thQ:	8	12	0	0	17	0	6	0	22	0	0	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #43 Flower St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.806
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	53	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Prot+Permit			Prot+Permit			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	1	1 0	2 0	1	1 0	3 0	1	1 0	2 1	0
Volume Module:												
Base Vol:	100	936	136	214	723	97	211	1315	150	143	458	191
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	100	936	136	214	723	97	211	1315	150	143	458	191
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	100	936	136	214	723	97	211	1315	150	143	458	191
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	100	936	136	214	723	97	211	1315	150	143	458	191
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	100	936	136	214	723	97	211	1315	150	143	458	191
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	3.00	1.00	1.00	2.12	0.88
Final Sat.:	1600	3400	1600	1600	3400	1600	1600	5100	1600	1600	3587	1413
Capacity Analysis Module:												
Vol/Sat:	0.06	0.28	0.09	0.13	0.21	0.06	0.13	0.26	0.09	0.09	0.13	0.14
Crit Moves:		****			****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #44 Flower St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.926
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	105	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	1	1 0	1 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	139	602	159	334	1058	111	148	1355	54	209	861	123
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	139	602	159	334	1058	111	148	1355	54	209	861	123
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	139	602	159	334	1058	111	148	1355	54	209	861	123
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	139	602	159	334	1058	111	148	1355	54	209	861	123
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	139	602	159	334	1058	111	148	1355	54	209	861	123
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	2.00	1.00	1.00	1.81	0.19	1.00	2.89	0.11	1.00	2.62	0.38
Final Sat.:	1600	3400	1600	1600	2996	304	1600	4816	184	1600	4400	600
Capacity Analysis Module:												
Vol/Sat:	0.09	0.18	0.10	0.21	0.35	0.37	0.09	0.28	0.29	0.13	0.20	0.21
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #45 Flower St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.900
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	87	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 1	0 1	0	0 1	0 1	0	0 1	0 1	0	0 1	0 1	0
Volume Module:												
Base Vol:	70	511	44	118	655	95	112	1278	97	113	764	103
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	70	511	44	118	655	95	112	1278	97	113	764	103
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	70	511	44	118	655	95	112	1278	97	113	764	103
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	70	511	44	118	655	95	112	1278	97	113	764	103
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	70	511	44	118	655	95	112	1278	97	113	764	103
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.22	1.64	0.14	0.27	1.51	0.22	0.15	1.72	0.13	0.23	1.56	0.21
Final Sat.:	358	2616	225	435	2415	350	241	2750	209	369	2495	336
Capacity Analysis Module:												
Vol/Sat:	0.04	0.20	0.20	0.07	0.27	0.27	0.07	0.46	0.46	0.07	0.31	0.31
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #46 Flower St and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.784
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	49	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	86	451	100	98	544	120	223	1531	427	104	764	84
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	86	451	100	98	544	120	223	1531	427	104	764	84
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	86	451	100	98	544	120	223	1531	427	104	764	84
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	86	451	100	98	544	120	223	1531	427	104	764	84
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	86	451	100	98	544	120	223	1531	427	104	764	84
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	1.64	0.36	1.00	1.64	0.36	1.00	2.35	0.65	1.00	2.70	0.30
Final Sat.:	1600	2719	581	1600	2722	578	1600	3953	1047	1600	4525	475
Capacity Analysis Module:												
Vol/Sat:	0.05	0.17	0.17	0.06	0.20	0.21	0.14	0.39	0.41	0.07	0.17	0.18
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #47 Flower St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.675
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	35	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	26	170	88	167	361	270	151	1716	73	62	1049	67
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	26	170	88	167	361	270	151	1716	73	62	1049	67
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	26	170	88	167	361	270	151	1716	73	62	1049	67
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	26	170	88	167	361	270	151	1716	73	62	1049	67
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	26	170	88	167	361	270	151	1716	73	62	1049	67
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	1.32	0.68	1.00	1.14	0.86	1.00	2.88	0.12	1.00	2.82	0.18
Final Sat.:	1600	2209	1091	1600	1931	1369	1600	4804	196	1600	4712	288
Capacity Analysis Module:												
Vol/Sat:	0.02	0.08	0.08	0.10	0.19	0.20	0.09	0.36	0.37	0.04	0.22	0.23
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #48 Main St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.586
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	28	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	2 0	2 1	0	2 0	2 0	1
Volume Module:												
Base Vol:	105	724	375	198	1034	173	361	510	222	316	276	230
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	105	724	375	198	1034	173	361	510	222	316	276	230
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	105	724	375	198	1034	173	361	510	222	316	276	230
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	105	724	375	198	1034	173	361	510	222	316	276	230
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	105	724	375	198	1034	173	361	510	222	316	276	230
OvlAdjVol:			217									131
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	2.57	0.43	2.00	2.09	0.91	2.00	2.00	1.00
Final Sat.:	3200	5100	1600	3200	4312	688	3200	3544	1456	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.14	0.23	0.06	0.24	0.25	0.11	0.14	0.15	0.10	0.08	0.14
OvlAdjV/S:			0.14									0.08
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #49 Main St and Mainplace Dr / Memory Ln

Cycle (sec):	100	Critical Vol./Cap.(X):	0.512
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	24	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 0	2 0	2	1 0	2 0	2	1 0	2 0	2	0 1
Volume Module:												
Base Vol:	34	920	301	36	1386	290	185	291	20	132	151	10
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	34	920	301	36	1386	290	185	291	20	132	151	10
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	34	920	301	36	1386	290	185	291	20	132	151	10
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	34	920	301	36	1386	290	185	291	20	132	151	10
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	34	920	301	36	1386	290	185	291	20	132	151	10
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	2.26	0.74	2.00	2.48	0.52	2.00	2.81	0.19	2.00	2.00	1.00
Final Sat.:	3200	3817	1183	3200	4169	831	3200	4691	309	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.01	0.24	0.25	0.01	0.33	0.35	0.06	0.06	0.06	0.04	0.04	0.01
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #50 Main St and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.884
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	78	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 0	1	2 0	2 0	1	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	133	788	169	237	1322	85	127	1336	67	356	1425	21
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	133	788	169	237	1322	85	127	1336	67	356	1425	21
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	133	788	169	237	1322	85	127	1336	67	356	1425	21
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	133	788	169	237	1322	85	127	1336	67	356	1425	21
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	133	788	169	237	1322	85	127	1336	67	356	1425	21
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	2.00	2.00	1.00	2.00	2.00	1.00	2.00	2.86	0.14	2.00	2.96	0.04
Final Sat.:	3200	3400	1600	3200	3400	1600	3200	4771	229	3200	4930	70
Capacity Analysis Module:												
Vol/Sat:	0.04	0.23	0.11	0.07	0.39	0.05	0.04	0.28	0.29	0.11	0.29	0.30
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #51 Main St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.783
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	49	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	142	925	81	55	970	148	120	727	132	43	566	34
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	142	925	81	55	970	148	120	727	132	43	566	34
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	142	925	81	55	970	148	120	727	132	43	566	34
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	142	925	81	55	970	148	120	727	132	43	566	34
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	142	925	81	55	970	148	120	727	132	43	566	34
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.03	1.00
Lanes:	1.00	1.84	0.16	1.00	1.74	0.26	1.00	1.69	0.31	1.00	1.89	0.11
Final Sat.:	1600	3042	258	1600	2876	424	1600	2808	492	1600	3119	181
Capacity Analysis Module:												
Vol/Sat:	0.09	0.30	0.31	0.03	0.34	0.35	0.08	0.26	0.27	0.03	0.18	0.19
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #52 Main St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.926
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	105	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	0	0 0	1 1	0	0 0	0 0	0	0 1	0 1	0
Volume Module:												
Base Vol:	66	1050	0	0	1135	100	0	0	0	90	1250	96
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	66	1050	0	0	1135	100	0	0	0	90	1250	96
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	66	1050	0	0	1135	100	0	0	0	90	1250	96
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	66	1050	0	0	1135	100	0	0	0	90	1250	96
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	66	1050	0	0	1135	100	0	0	0	90	1250	96
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.03	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	1.00	2.00	0.00	0.00	1.84	0.16	0.00	0.00	0.00	0.13	1.74	0.13
Final Sat.:	1600	3400	0	0	3041	259	0	0	0	201	2786	214
Capacity Analysis Module:												
Vol/Sat:	0.04	0.31	0.00	0.00	0.37	0.39	0.00	0.00	0.00	0.06	0.45	0.45
Crit Moves:	****					****					****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #53 Main St and 4th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.410
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	20	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	1	1 0	0 0	1	1 0	0 0	1	1 0	0 0	1	1 0
Volume Module:												
Base Vol:	0	939	22	0	961	24	0	70	11	0	140	34
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	939	22	0	961	24	0	70	11	0	140	34
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	939	22	0	961	24	0	70	11	0	140	34
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	939	22	0	961	24	0	70	11	0	140	34
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	939	22	0	961	24	0	70	11	0	140	34
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	0.00	1.95	0.05	0.00	1.95	0.05	0.00	1.73	0.27	0.00	1.61	0.39
Final Sat.:	0	3227	73	0	3222	78	0	2865	435	0	2675	625
Capacity Analysis Module:												
Vol/Sat:	0.00	0.29	0.30	0.00	0.30	0.31	0.00	0.02	0.03	0.00	0.05	0.05
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #54 Main St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.681
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	35	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	2	0 1	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	103	532	54	77	772	57	106	1241	131	86	1072	60
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	103	532	54	77	772	57	106	1241	131	86	1072	60
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	103	532	54	77	772	57	106	1241	131	86	1072	60
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	103	532	54	77	772	57	106	1241	131	86	1072	60
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	103	532	54	77	772	57	106	1241	131	86	1072	60
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.04	1.00
Lanes:	1.00	1.82	0.18	1.00	2.00	1.00	1.00	2.71	0.29	1.00	2.84	0.16
Final Sat.:	1600	3005	295	1600	3400	1600	1600	4542	458	1600	4746	254
Capacity Analysis Module:												
Vol/Sat:	0.06	0.18	0.18	0.05	0.23	0.04	0.07	0.27	0.29	0.05	0.23	0.24
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #55 Main St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.894
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	84	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Prot+Permit			Prot+Permit			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	93	874	44	162	1259	25	86	815	89	164	438	126
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	93	874	44	162	1259	25	86	815	89	164	438	126
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	93	874	44	162	1259	25	86	815	89	164	438	126
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	93	874	44	162	1259	25	86	815	89	164	438	126
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	93	874	44	162	1259	25	86	815	89	164	438	126
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.04	1.00
Lanes:	1.00	1.90	0.10	1.00	1.96	0.04	1.00	1.80	0.20	1.00	1.55	0.45
Final Sat.:	1600	3147	153	1600	3238	62	1600	2985	315	1600	2585	715
Capacity Analysis Module:												
Vol/Sat:	0.06	0.28	0.29	0.10	0.39	0.40	0.05	0.27	0.28	0.10	0.17	0.18
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #56 Main St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.795
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	51	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	99	561	61	211	975	51	96	1248	168	108	630	77
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	99	561	61	211	975	51	96	1248	168	108	630	77
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	99	561	61	211	975	51	96	1248	168	108	630	77
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	99	561	61	211	975	51	96	1248	168	108	630	77
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	99	561	61	211	975	51	96	1248	168	108	630	77
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	1.80	0.20	1.00	1.90	0.10	1.00	2.64	0.36	1.00	2.67	0.33
Final Sat.:	1600	2986	314	1600	3141	159	1600	4431	569	1600	4477	523
Capacity Analysis Module:												
Vol/Sat:	0.06	0.19	0.19	0.13	0.31	0.32	0.06	0.28	0.29	0.07	0.14	0.15
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #57 Main St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.680
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	35	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	51	295	250	539	709	160	230	1336	242	138	411	206
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	51	295	250	539	709	160	230	1336	242	138	411	206
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	51	295	250	539	709	160	230	1336	242	138	411	206
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	51	295	250	539	709	160	230	1336	242	138	411	206
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	51	295	250	539	709	160	230	1336	242	138	411	206
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.02	0.06	0.16	0.17	0.14	0.10	0.07	0.26	0.15	0.04	0.08	0.13
Crit Moves:			****	****				****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #58 Penn Wy and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.660
Loss Time (sec):	5	Average Delay (sec/veh):	12.2
Optimal Cycle:	35	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	0	0 2	0	0	0 0 0	0	0	2 1 0	1	0	3 0 0
Volume Module:												
Base Vol:	52	0	260	0	0	0	0	1341	684	214	1722	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	52	0	260	0	0	0	0	1341	684	214	1722	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	52	0	260	0	0	0	0	1341	684	214	1722	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	52	0	260	0	0	0	0	1341	684	214	1722	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	52	0	260	0	0	0	0	1341	684	214	1722	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.75	1.00	1.00	1.00	1.00	0.86	0.86	0.95	0.91	1.00
Lanes:	2.00	0.00	2.00	0.00	0.00	0.00	0.00	2.00	1.00	1.00	3.00	0.00
Final Sat.:	3502	0	2842	0	0	0	0	3282	1641	1805	5187	0
Capacity Analysis Module:												
Vol/Sat:	0.01	0.00	0.09	0.00	0.00	0.00	0.00	0.41	0.42	0.12	0.33	0.00
Crit Moves:			****						****	****		
Green/Cycle:	0.14	0.00	0.14	0.00	0.00	0.00	0.00	0.63	0.63	0.18	0.81	0.00
Volume/Cap:	0.11	0.00	0.66	0.00	0.00	0.00	0.00	0.65	0.66	0.66	0.41	0.00
Delay/Veh:	37.8	0.0	44.9	0.0	0.0	0.0	0.0	11.9	12.2	43.1	2.7	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	37.8	0.0	44.9	0.0	0.0	0.0	0.0	11.9	12.2	43.1	2.7	0.0
LOS by Move:	D	A	D	A	A	A	A	B	B	D	A	A
HCM2k85thQ:	1	0	9	0	0	0	0	22	22	11	9	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #59 I-5 NB Off Ramps/17th Street

Cycle (sec):	100	Critical Vol./Cap.(X):	0.710
Loss Time (sec):	5	Average Delay (sec/veh):	30.0
Optimal Cycle:	40	Level Of Service:	C

Street Name:	I-5 NB Off Ramps						17th Street					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Permitted		
Rights:	Include			Include			Ignore			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 1	0	0 1	1	0 0	0 1	1	0	3 0 1	0	0	2 1 0
Volume Module:												
Base Vol:	720	33	21	54	0	279	89	932	337	0	1245	20
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	720	33	21	54	0	279	89	932	337	0	1245	20
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Volume:	720	33	21	54	0	279	89	932	0	0	1245	20
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	720	33	21	54	0	279	89	932	0	0	1245	20
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
FinalVolume:	720	33	21	54	0	279	89	932	0	0	1245	20
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.95	1.00	0.85	0.95	0.91	1.00	1.00	0.91	0.91
Lanes:	1.91	0.09	1.00	1.00	0.00	1.00	1.00	3.00	1.00	0.00	2.95	0.05
Final Sat.:	3466	159	1615	1805	0	1615	1805	5187	1900	0	5095	82
Capacity Analysis Module:												
Vol/Sat:	0.21	0.21	0.01	0.03	0.00	0.17	0.05	0.18	0.00	0.00	0.24	0.24
Crit Moves:	****					****	****				****	
Green/Cycle:	0.29	0.29	0.29	0.24	0.00	0.24	0.07	0.41	0.00	0.00	0.34	0.34
Volume/Cap:	0.71	0.71	0.04	0.12	0.00	0.71	0.71	0.43	0.00	0.00	0.71	0.71
Delay/Veh:	33.8	33.8	25.4	29.6	0.0	40.5	62.7	21.1	0.0	0.0	29.8	29.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	33.8	33.8	25.4	29.6	0.0	40.5	62.7	21.1	0.0	0.0	29.8	29.8
LOS by Move:	C	C	C	C	A	D	E	C	A	A	C	C
HCM2k85thQ:	17	17	1	2	0	14	7	12	0	0	20	20

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #60 Penn Wy and I-5 SB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.550
Loss Time (sec):	5	Average Delay (sec/veh):	21.2
Optimal Cycle:	27	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	2	0 1	2 0	2	0 0	0 0	0	0 0	1 0	0	0 2
Volume Module:												
Base Vol:	0	260	230	951	196	0	0	0	0	196	0	110
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	260	230	951	196	0	0	0	0	196	0	110
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	260	230	951	196	0	0	0	0	196	0	110
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	260	230	951	196	0	0	0	0	196	0	110
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	260	230	951	196	0	0	0	0	196	0	110
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.95	0.85	0.92	0.95	1.00	1.00	1.00	1.00	0.95	1.00	0.75
Lanes:	0.00	2.00	1.00	2.00	2.00	0.00	0.00	0.00	0.00	1.00	0.00	2.00
Final Sat.:	0	3610	1615	3502	3610	0	0	0	0	1805	0	2842
Capacity Analysis Module:												
Vol/Sat:	0.00	0.07	0.14	0.27	0.05	0.00	0.00	0.00	0.00	0.11	0.00	0.04
Crit Moves:			****	****						****		
Green/Cycle:	0.00	0.26	0.26	0.49	0.75	0.00	0.00	0.00	0.00	0.20	0.00	0.69
Volume/Cap:	0.00	0.28	0.55	0.55	0.07	0.00	0.00	0.00	0.00	0.55	0.00	0.06
Delay/Veh:	0.0	29.8	33.6	18.0	3.2	0.0	0.0	0.0	0.0	38.0	0.0	5.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	29.8	33.6	18.0	3.2	0.0	0.0	0.0	0.0	38.0	0.0	5.0
LOS by Move:	A	C	C	B	A	A	A	A	A	D	A	A
HCM2k85thQ:	0	5	10	16	1	0	0	0	0	9	0	1

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #61 Santiago St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	1.769
Loss Time (sec):	5	Average Delay (sec/veh):	159.2
Optimal Cycle:	0	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	1 0	0 1	0	1 0	0 1	0	0 1	0 0	1	0 0	1! 0	0
Volume Module:												
Base Vol:	487	338	47	10	590	144	182	69	369	108	99	20
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	487	338	47	10	590	144	182	69	369	108	99	20
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	487	338	47	10	590	144	182	69	369	108	99	20
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	487	338	47	10	590	144	182	69	369	108	99	20
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	487	338	47	10	590	144	182	69	369	108	99	20
Saturation Flow Module:												
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	0.88	0.12	1.00	0.80	0.20	0.73	0.27	1.00	0.47	0.44	0.09
Final Sat.:	402	377	52	381	333	81	289	110	453	177	162	33
Capacity Analysis Module:												
Vol/Sat:	1.21	0.90	0.90	0.03	1.77	1.77	0.63	0.63	0.82	0.61	0.61	0.61
Crit Moves:	****				****				****	****		
Delay/Veh:	144.0	50.3	50.3	12.3	377	376.5	25.7	25.7	36.9	26.3	26.3	26.3
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	144.0	50.3	50.3	12.3	377	376.5	25.7	25.7	36.9	26.3	26.3	26.3
LOS by Move:	F	F	F	B	F	F	D	D	E	D	D	D
ApproachDel:	102.6			371.6			32.3			26.3		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	102.6			371.6			32.3			26.3		
LOS by Appr:	F			F			D			D		
AllWayAvgQ:	14.8	4.7	4.7	0.0	42.1	42.1	1.5	1.5	3.3	1.4	1.4	1.4

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #62 Santiago St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	1.037
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	2	0 1
Volume Module:												
Base Vol:	188	611	303	409	972	253	118	732	99	297	699	258
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	188	611	303	409	972	253	118	732	99	297	699	258
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	188	611	303	409	972	253	118	732	99	297	699	258
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	188	611	303	409	972	253	118	732	99	297	699	258
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	188	611	303	409	972	253	118	732	99	297	699	258
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	1.00	1.34	0.66	1.00	1.59	0.41	1.00	1.76	0.24	1.00	2.00	1.00
Final Sat.:	1600	2239	1061	1600	2639	661	1600	2919	381	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.12	0.27	0.29	0.26	0.37	0.38	0.07	0.25	0.26	0.19	0.21	0.16
Crit Moves:			****	****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #63 Standard Ave and 4th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.939
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	117	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	1	1 0	2 0	1	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	191	473	248	431	604	332	298	624	219	250	550	340
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	191	473	248	431	604	332	298	624	219	250	550	340
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	191	473	248	431	604	332	298	624	219	250	550	340
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	191	473	248	431	604	332	298	624	219	250	550	340
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	191	473	248	431	604	332	298	624	219	250	550	340
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	1.48	0.52	1.00	1.24	0.76
Final Sat.:	1600	3400	1600	1600	3400	1600	1600	2469	831	1600	2078	1222
Capacity Analysis Module:												
Vol/Sat:	0.12	0.14	0.16	0.27	0.18	0.21	0.19	0.25	0.26	0.16	0.26	0.28
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #64 Standard Ave and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.976
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	176	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	45	631	312	123	901	41	161	1959	121	194	1282	120
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	45	631	312	123	901	41	161	1959	121	194	1282	120
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	45	631	312	123	901	41	161	1959	121	194	1282	120
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	45	631	312	123	901	41	161	1959	121	194	1282	120
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	45	631	312	123	901	41	161	1959	121	194	1282	120
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	1.34	0.66	1.00	1.91	0.09	1.00	2.83	0.17	1.00	2.74	0.26
Final Sat.:	1600	2241	1059	1600	3161	139	1600	4721	279	1600	4589	411
Capacity Analysis Module:												
Vol/Sat:	0.03	0.28	0.29	0.08	0.29	0.29	0.10	0.41	0.43	0.12	0.28	0.29
Crit Moves:		****	****						****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #65 Standard Ave and Mcfadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.854
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	66	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	94	360	162	226	547	38	34	1178	142	150	397	89
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	94	360	162	226	547	38	34	1178	142	150	397	89
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	94	360	162	226	547	38	34	1178	142	150	397	89
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	94	360	162	226	547	38	34	1178	142	150	397	89
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	94	360	162	226	547	38	34	1178	142	150	397	89
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	1.38	0.62	1.00	1.87	0.13	1.00	1.78	0.22	1.00	1.63	0.37
Final Sat.:	1600	2307	993	1600	3092	208	1600	2956	344	1600	2714	586
Capacity Analysis Module:												
Vol/Sat:	0.06	0.16	0.16	0.14	0.18	0.18	0.02	0.40	0.41	0.09	0.15	0.15
Crit Moves:		****		****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #66 Halladay St and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.775
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	47	Level Of Service:	C

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	- T	- R		L	- T	- R		L	- T	- R		L	- T	- R	
Control:	Permitted				Permitted				Permitted				Permitted			
Rights:	Include				Include				Include				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Y+R:	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0	
Lanes:	1	0 0	0	1	0	0 0	0	0	0	0 0	2	1	0	0 1	2	0 0
Volume Module:																
Base Vol:	95	0	220		0	0	0		0	1833	207		260	986	0	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	95	0	220		0	0	0		0	1833	207		260	986	0	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	95	0	220		0	0	0		0	1833	207		260	986	0	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	95	0	220		0	0	0		0	1833	207		260	986	0	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	95	0	220		0	0	0		0	1833	207		260	986	0	
Saturation Flow Module:																
Sat/Lane:	1600	1600	1600		1600	1600	1600		1600	1600	1600		1600	1600	1600	
Adjustment:	1.00	1.06	1.00		1.00	1.06	1.00		1.00	1.05	1.00		1.00	1.05	1.00	
Lanes:	1.00	0.00	1.00		0.00	0.00	0.00		0.00	2.70	0.30		0.63	2.37	0.00	
Final Sat.:	1600	0	1600		0	0	0		0	4513	487		1002	3998	0	
Capacity Analysis Module:																
Vol/Sat:	0.06	0.00	0.14		0.00	0.00	0.00		0.00	0.41	0.43		0.16	0.25	0.00	
Crit Moves:			****								****		****			

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #67 Halladay St and Dyer Rd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.774
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	47	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0 1	0	1 0	0 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	15	24	65	450	91	96	46	1537	11	104	566	91
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	15	24	65	450	91	96	46	1537	11	104	566	91
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	15	24	65	450	91	96	46	1537	11	104	566	91
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	15	24	65	450	91	96	46	1537	11	104	566	91
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	15	24	65	450	91	96	46	1537	11	104	566	91
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	0.27	0.73	1.00	0.49	0.51	1.00	2.98	0.02	1.00	2.58	0.42
Final Sat.:	1600	431	1169	1600	779	821	1600	4966	34	1600	4335	665
Capacity Analysis Module:												
Vol/Sat:	0.01	0.06	0.06	0.28	0.12	0.12	0.03	0.31	0.32	0.07	0.13	0.14
Crit Moves:		****		****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #68 SR-55 SB Ramps and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.608
Loss Time (sec):	5	Average Delay (sec/veh):	19.9
Optimal Cycle:	31	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Ignore			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	0	2	0	0	0	2	0	0	2	1	1	0	0	3	0	1
Volume Module:																		
Base Vol:	0	0	0	998	0	903	0	1349	961	0	1187	135
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	998	0	903	0	1349	961	0	1187	135
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Volume:	0	0	0	998	0	903	0	1349	0	0	1187	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	998	0	903	0	1349	0	0	1187	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
FinalVolume:	0	0	0	998	0	903	0	1349	0	0	1187	0
Saturation Flow Module:																		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.92	1.00	0.75	1.00	0.91	0.91	1.00	0.91	1.00
Lanes:	0.00	0.00	0.00	2.00	0.00	2.00	0.00	3.00	1.00	0.00	3.00	1.00
Final Sat.:	0	0	0	3502	0	2842	0	5187	1729	0	5187	1900
Capacity Analysis Module:																		
Vol/Sat:	0.00	0.00	0.00	0.29	0.00	0.32	0.00	0.26	0.00	0.00	0.23	0.00
Crit Moves:						****		****		****		
Green/Cycle:	0.00	0.00	0.00	0.52	0.00	0.52	0.00	0.43	0.00	0.00	0.43	0.00
Volume/Cap:	0.00	0.00	0.00	0.55	0.00	0.61	0.00	0.61	0.00	0.00	0.54	0.00
Delay/Veh:	0.0	0.0	0.0	16.3	0.0	17.5	0.0	22.6	0.0	0.0	21.5	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	16.3	0.0	17.5	0.0	22.6	0.0	0.0	21.5	0.0
LOS by Move:	A	A	A	B	A	B	A	C	A	A	C	A
HCM2k85thQ:	0	0	0	16	0	17	0	18	0	0	15	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #69 SR-55 NB Ramps and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.712
Loss Time (sec):	5	Average Delay (sec/veh):	19.3
Optimal Cycle:	40	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Ignore			Include			Ignore			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	0	0 1	0 0	0	0 0	0 0	2	0 2	0 0	3	0 1
Volume Module:												
Base Vol:	946	0	1017	0	0	0	0	1468	726	0	517	244
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	946	0	1017	0	0	0	0	1468	726	0	517	244
User Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Volume:	946	0	0	0	0	0	0	1468	0	0	517	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	946	0	0	0	0	0	0	1468	0	0	517	0
PCE Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
FinalVolume:	946	0	0	0	0	0	0	1468	0	0	517	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.88	1.00	0.91	1.00
Lanes:	2.00	0.00	1.00	0.00	0.00	0.00	0.00	2.00	2.00	0.00	3.00	1.00
Final Sat.:	3502	0	1900	0	0	0	0	3610	3344	0	5187	1900
Capacity Analysis Module:												
Vol/Sat:	0.27	0.00	0.00	0.00	0.00	0.00	0.00	0.41	0.00	0.00	0.10	0.00
Crit Moves:	****							****		****		
Green/Cycle:	0.38	0.00	0.00	0.00	0.00	0.00	0.00	0.57	0.00	0.00	0.57	0.00
Volume/Cap:	0.71	0.00	0.00	0.00	0.00	0.00	0.00	0.71	0.00	0.00	0.17	0.00
Delay/Veh:	28.2	0.0	0.0	0.0	0.0	0.0	0.0	16.7	0.0	0.0	10.3	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	28.2	0.0	0.0	0.0	0.0	0.0	0.0	16.7	0.0	0.0	10.3	0.0
LOS by Move:	C	A	A	A	A	A	A	B	A	A	B	A
HCM2k85thQ:	21	0	0	0	0	0	0	26	0	0	4	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #70 SR-55 SB Ramps and Dyer Rd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.617
Loss Time (sec):	5	Average Delay (sec/veh):	25.7
Optimal Cycle:	31	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Ovl			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 1	0	0 2	0 1	1	0 1	1 0	3	0 1	2 0	3	0 1
Volume Module:												
Base Vol:	286	34	654	50	242	60	37	1422	303	348	552	92
Growth Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
Initial Bse:	286	34	654	50	242	60	37	1422	303	348	552	92
User Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
PHF Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
PHF Volume:	286	34	654	50	242	60	37	1422	303	348	552	92
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	286	34	654	50	242	60	37	1422	303	348	552	92
PCE Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
MLF Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
FinalVolume:	286	34	654	50	242	60	37	1422	303	348	552	92
Saturation Flow Module:												
Sat/Lane:	1900 1900		1900	1900 1900		1900	1900 1900		1900	1900 1900		1900
Adjustment:	0.96 0.96		0.75	0.94 0.94		0.85	0.95 0.91		0.85	0.92 0.91		0.85
Lanes:	1.79 0.21		2.00	0.34 1.66		1.00	1.00 3.00		1.00	2.00 3.00		1.00
Final Sat.:	3250	386	2842	613 2968		1615	1805 5187		1615	3502 5187		1615
Capacity Analysis Module:												
Vol/Sat:	0.09 0.09		0.23	0.08 0.08		0.04	0.02 0.27		0.19	0.10 0.11		0.06
Crit Moves:			****		****			****			****	
Green/Cycle:	0.21 0.21		0.37	0.13 0.13		0.13	0.10 0.44		0.44	0.16 0.51		0.51
Volume/Cap:	0.42 0.42		0.62	0.62 0.62		0.28	0.21 0.62		0.42	0.62 0.21		0.11
Delay/Veh:	34.4 34.4		26.6	43.4 43.4		39.8	42.1 21.8		19.4	41.1 13.6		12.9
User DelAdj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
AdjDel/Veh:	34.4 34.4		26.6	43.4 43.4		39.8	42.1 21.8		19.4	41.1 13.6		12.9
LOS by Move:	C	C	C	D	D	D	D	C	B	D	B	B
HCM2k85thQ:	7	7	15	8	8	3	2	19	10	9	5	2

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #71 Glassell St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.800
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	52	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Ovl			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	1 0	1	1 0	1 1	0	1 0	1 0	1	1 0	1 1	0
Volume Module:												
Base Vol:	263	412	125	14	295	65	57	293	563	367	387	9
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	263	412	125	14	295	65	57	293	563	367	387	9
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	263	412	125	14	295	65	57	293	563	367	387	9
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	263	412	125	14	295	65	57	293	563	367	387	9
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	263	412	125	14	295	65	57	293	563	367	387	9
OvlAdjVol:			0						431			
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.03	1.00
Lanes:	2.00	1.00	1.00	1.00	1.64	0.36	1.00	1.00	1.00	1.00	1.95	0.05
Final Sat.:	3200	1700	1600	1600	2722	578	1600	1700	1600	1600	3227	73
Capacity Analysis Module:												
Vol/Sat:	0.08	0.24	0.08	0.01	0.11	0.11	0.04	0.17	0.35	0.23	0.12	0.12
OvlAdjV/S:			0.00						0.27			
Crit Moves:		****		****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #72 Glassell St and SR-22 WB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.871
Loss Time (sec):	5	Average Delay (sec/veh):	32.6
Optimal Cycle:	75	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	0	0 0	2 0	1	0 0	0 0	0	1 0	1! 0	1
Volume Module:												
Base Vol:	511	574	0	0	1017	473	0	0	0	323	6	483
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	511	574	0	0	1017	473	0	0	0	323	6	483
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	511	574	0	0	1017	473	0	0	0	323	6	483
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	511	574	0	0	1017	473	0	0	0	323	6	483
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	511	574	0	0	1017	473	0	0	0	323	6	483
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.95	0.85	1.00	1.00	1.00	0.86	0.86	0.86
Lanes:	1.00	2.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	1.39	0.01	1.60
Final Sat.:	1805	3610	0	0	3610	1615	0	0	0	2270	24	2588
Capacity Analysis Module:												
Vol/Sat:	0.28	0.16	0.00	0.00	0.28	0.29	0.00	0.00	0.00	0.14	0.25	0.19
Crit Moves:	****					****					****	
Green/Cycle:	0.33	0.66	0.00	0.00	0.34	0.34	0.00	0.00	0.00	0.29	0.29	0.29
Volume/Cap:	0.87	0.24	0.00	0.00	0.84	0.87	0.00	0.00	0.00	0.49	0.87	0.65
Delay/Veh:	45.1	6.9	0.0	0.0	35.9	45.4	0.0	0.0	0.0	29.7	42.8	32.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	45.1	6.9	0.0	0.0	35.9	45.4	0.0	0.0	0.0	29.7	42.8	32.3
LOS by Move:	D	A	A	A	D	D	A	A	A	C	D	C
HCM2k85thQ:	26	6	0	0	26	25	0	0	0	10	22	14

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #73 Grand Ave / Glassell St and SR-22 EB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.860
Loss Time (sec):	5	Average Delay (sec/veh):	30.6
Optimal Cycle:	71	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	2	0 1	1 0	2	0 0	1 0	1!	0 1	0 0	0	0 0
Volume Module:												
Base Vol:	0	918	245	373	977	0	270	0	1058	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	918	245	373	977	0	270	0	1058	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	918	245	373	977	0	270	0	1058	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	918	245	373	977	0	270	0	1058	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	918	245	373	977	0	270	0	1058	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.95	0.85	0.95	0.95	1.00	0.87	1.00	0.87	1.00	1.00	1.00
Lanes:	0.00	2.00	1.00	1.00	2.00	0.00	1.20	0.00	1.80	0.00	0.00	0.00
Final Sat.:	0	3610	1615	1805	3610	0	1992	0	2974	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.25	0.15	0.21	0.27	0.00	0.14	0.00	0.36	0.00	0.00	0.00
Crit Moves:		****		****					****			
Green/Cycle:	0.00	0.30	0.30	0.24	0.54	0.00	0.41	0.00	0.41	0.00	0.00	0.00
Volume/Cap:	0.00	0.86	0.51	0.86	0.50	0.00	0.33	0.00	0.86	0.00	0.00	0.00
Delay/Veh:	0.0	40.4	30.2	52.1	15.0	0.0	19.9	0.0	31.8	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	40.4	30.2	52.1	15.0	0.0	19.9	0.0	31.8	0.0	0.0	0.0
LOS by Move:	A	D	C	D	B	A	B	A	C	A	A	A
HCM2k85thQ:	0	25	10	21	15	0	7	0	28	0	0	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #74 Grand Ave and Fairhaven Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.611
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	30	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 0	1	0 1
Volume Module:												
Base Vol:	21	840	121	207	1718	38	99	60	59	213	24	192
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	21	840	121	207	1718	38	99	60	59	213	24	192
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	21	840	121	207	1718	38	99	60	59	213	24	192
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	21	840	121	207	1718	38	99	60	59	213	24	192
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	21	840	121	207	1718	38	99	60	59	213	24	192
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	2.62	0.38	1.00	2.94	0.06	1.00	1.01	0.99	1.00	1.00	1.00
Final Sat.:	1600	4396	604	1600	4896	104	1600	1713	1587	1600	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.01	0.19	0.20	0.13	0.35	0.37	0.06	0.04	0.04	0.13	0.01	0.12
Crit Moves:	****					****	****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #75 Grand Ave and Santa Clara Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.843
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	63	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			WideBypass			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0 2	1	0	1 0 2	1	0	1 0 0	1	0	1 0 1	0	1
Volume Module:												
Base Vol:	75	528	136	345	1687	380	143	233	48	200	385	352
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	75	528	136	345	1687	380	143	233	48	200	385	352
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	75	528	136	345	1687	380	143	233	48	200	385	352
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	75	528	136	345	1687	380	143	233	48	200	385	352
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	75	528	136	345	1687	380	143	233	48	200	385	352
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.00	1.00	1.00	1.06	1.00
Lanes:	1.00	2.39	0.61	1.00	2.45	0.55	1.00	0.83	0.17	1.00	1.00	1.00
Final Sat.:	1600	4017	983	1600	4118	882	1600	1327	273	1600	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.05	0.13	0.14	0.22	0.41	0.43	0.09	0.18	0.18	0.13	0.23	0.22
Crit Moves:	****					****	****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #76 Grand Ave and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.794
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	51	Level Of Service:	C

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	T	R		L	T	R		L	T	R		L	T	R	
Control:	Protected				Protected				Protected				Protected			
Rights:	Include				Include				Include				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Y+R:	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0	
Lanes:	1	0	2	1	0	1	0	2	1	0	2	0	1	0	2	0
Volume Module:																
Base Vol:	186	543	373		340	1426	280		198	739	94		342	894	98	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	186	543	373		340	1426	280		198	739	94		342	894	98	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	186	543	373		340	1426	280		198	739	94		342	894	98	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	186	543	373		340	1426	280		198	739	94		342	894	98	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	186	543	373		340	1426	280		198	739	94		342	894	98	
Saturation Flow Module:																
Sat/Lane:	1600	1600	1600		1600	1600	1600		1600	1600	1600		1600	1600	1600	
Adjustment:	1.00	1.06	1.00		1.00	1.05	1.00		1.00	1.05	1.00		1.00	1.05	1.00	
Lanes:	1.00	2.00	1.00		1.00	2.51	0.49		2.00	2.66	0.34		2.00	2.70	0.30	
Final Sat.:	1600	3400	1600		1600	4212	788		3200	4458	542		3200	4526	474	
Capacity Analysis Module:																
Vol/Sat:	0.12	0.16	0.23		0.21	0.34	0.36		0.06	0.17	0.17		0.11	0.20	0.21	
Crit Moves:	****						****			****			****			

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #77 Grand Ave and I-5 NB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.740
Loss Time (sec):	5	Average Delay (sec/veh):	13.2
Optimal Cycle:	44	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Ignore			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	2 0	1	1 0	3 0	0	0 0	0 0	0 0	2 0	0 0	1
Volume Module:												
Base Vol:	0	1056	610	90	2893	0	0	0	0	507	0	140
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1056	610	90	2893	0	0	0	0	507	0	140
User Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1056	0	90	2893	0	0	0	0	507	0	140
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1056	0	90	2893	0	0	0	0	507	0	140
PCE Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1056	0	90	2893	0	0	0	0	507	0	140
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.95	1.00	0.95	0.91	1.00	1.00	1.00	1.00	0.92	1.00	0.85
Lanes:	0.00	2.00	1.00	1.00	3.00	0.00	0.00	0.00	0.00	2.00	0.00	1.00
Final Sat.:	0	3610	1900	1805	5187	0	0	0	0	3502	0	1615
Capacity Analysis Module:												
Vol/Sat:	0.00	0.29	0.00	0.05	0.56	0.00	0.00	0.00	0.00	0.14	0.00	0.09
Crit Moves:	****				****					****		
Green/Cycle:	0.00	0.64	0.00	0.11	0.75	0.00	0.00	0.00	0.00	0.20	0.00	0.20
Volume/Cap:	0.00	0.45	0.00	0.45	0.74	0.00	0.00	0.00	0.00	0.74	0.00	0.44
Delay/Veh:	0.0	9.1	0.0	43.3	7.6	0.0	0.0	0.0	0.0	42.1	0.0	36.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	9.1	0.0	43.3	7.6	0.0	0.0	0.0	0.0	42.1	0.0	36.4
LOS by Move:	A	A	A	D	A	A	A	A	A	D	A	D
HCM2k85thQ:	0	13	0	5	28	0	0	0	0	14	0	7

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #78 Grand Ave and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.832
Loss Time (sec):	5	Average Delay (sec/veh):	26.0
Optimal Cycle:	62	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	0 2	2 0	1	0 2	0 1	0	1 0
Volume Module:												
Base Vol:	49	694	35	202	1670	1473	268	221	436	6	475	43
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	49	694	35	202	1670	1473	268	221	436	6	475	43
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	49	694	35	202	1670	1473	268	221	436	6	475	43
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	49	694	35	202	1670	1473	268	221	436	6	475	43
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	49	694	35	202	1670	1473	268	221	436	6	475	43
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.90	0.90	0.95	0.95	0.75	0.92	1.00	0.75	0.94	0.94	0.94
Lanes:	1.00	2.86	0.14	1.00	2.00	2.00	2.00	1.00	2.00	0.02	1.82	0.16
Final Sat.:	1805	4903	247	1805	3610	2842	3502	1900	2842	41	3230	292
Capacity Analysis Module:												
Vol/Sat:	0.03	0.14	0.14	0.11	0.46	0.52	0.08	0.12	0.15	0.15	0.15	0.15
Crit Moves:	****				****				****		****	
Green/Cycle:	0.03	0.33	0.33	0.26	0.56	0.74	0.18	0.18	0.18	0.18	0.18	0.18
Volume/Cap:	0.83	0.43	0.43	0.43	0.83	0.70	0.42	0.63	0.83	0.83	0.83	0.83
Delay/Veh:	109.2	26.4	26.4	31.5	21.4	8.1	36.5	41.3	50.1	48.9	48.9	48.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	109.2	26.4	26.4	31.5	21.4	8.1	36.5	41.3	50.1	48.9	48.9	48.9
LOS by Move:	F	C	C	C	C	A	D	D	D	D	D	D
HCM2k85thQ:	5	10	10	8	35	22	6	11	15	16	16	16

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #79 Grand Ave and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.742
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	42	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	3 0	1	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	208	732	34	99	1432	174	436	1041	295	216	759	87
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	208	732	34	99	1432	174	436	1041	295	216	759	87
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	208	732	34	99	1432	174	436	1041	295	216	759	87
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	208	732	34	99	1432	174	436	1041	295	216	759	87
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	208	732	34	99	1432	174	436	1041	295	216	759	87
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	2.00	2.87	0.13	2.00	3.00	1.00	2.00	2.34	0.66	2.00	2.69	0.31
Final Sat.:	3200	4787	213	3200	5100	1600	3200	3940	1060	3200	4506	494
Capacity Analysis Module:												
Vol/Sat:	0.07	0.15	0.16	0.03	0.28	0.11	0.14	0.26	0.28	0.07	0.17	0.18
Crit Moves:	****				****				****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #80 Grand Ave and Chestnut Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.704
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	38	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	53	737	163	161	1937	53	34	239	63	130	384	239
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	53	737	163	161	1937	53	34	239	63	130	384	239
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	53	737	163	161	1937	53	34	239	63	130	384	239
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	53	737	163	161	1937	53	34	239	63	130	384	239
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	53	737	163	161	1937	53	34	239	63	130	384	239
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	2.46	0.54	1.00	2.92	0.08	1.00	1.58	0.42	1.00	1.23	0.77
Final Sat.:	1600	4131	869	1600	4872	128	1600	2632	668	1600	2072	1228
Capacity Analysis Module:												
Vol/Sat:	0.03	0.18	0.19	0.10	0.40	0.41	0.02	0.09	0.09	0.08	0.19	0.19
Crit Moves:	****					****	****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #81 Grand Ave and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.892
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	82	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	181	764	114	249	1672	163	205	663	201	123	440	111
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	181	764	114	249	1672	163	205	663	201	123	440	111
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	181	764	114	249	1672	163	205	663	201	123	440	111
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	181	764	114	249	1672	163	205	663	201	123	440	111
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	181	764	114	249	1672	163	205	663	201	123	440	111
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	2.61	0.39	1.00	2.73	0.27	1.00	1.53	0.47	1.00	1.60	0.40
Final Sat.:	1600	4377	623	1600	4574	426	1600	2556	744	1600	2655	645
Capacity Analysis Module:												
Vol/Sat:	0.11	0.17	0.18	0.16	0.37	0.38	0.13	0.26	0.27	0.08	0.17	0.17
Crit Moves:	****					****			****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #82 Grand Ave and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.838
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	61	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	47	472	92	243	1244	243	347	1447	139	189	724	158
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	47	472	92	243	1244	243	347	1447	139	189	724	158
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	47	472	92	243	1244	243	347	1447	139	189	724	158
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	47	472	92	243	1244	243	347	1447	139	189	724	158
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	47	472	92	243	1244	243	347	1447	139	189	724	158
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.51	0.49	1.00	2.51	0.49	1.00	2.74	0.26	1.00	2.46	0.54
Final Sat.:	1600	4217	783	1600	4216	784	1600	4579	421	1600	4140	860
Capacity Analysis Module:												
Vol/Sat:	0.03	0.11	0.12	0.15	0.30	0.31	0.22	0.32	0.33	0.12	0.17	0.18
Crit Moves:	****					****			****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #83 Grand Ave and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.613
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	30	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	2 0	3 0	1	2 0	2 1	0
Volume Module:												
Base Vol:	142	429	184	183	528	178	333	1188	467	92	406	137
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	142	429	184	183	528	178	333	1188	467	92	406	137
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	142	429	184	183	528	178	333	1188	467	92	406	137
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	142	429	184	183	528	178	333	1188	467	92	406	137
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	142	429	184	183	528	178	333	1188	467	92	406	137
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	2.10	0.90	1.00	2.24	0.76	2.00	3.00	1.00	2.00	2.24	0.76
Final Sat.:	1600	3559	1441	1600	3790	1210	3200	5100	1600	3200	3789	1211
Capacity Analysis Module:												
Vol/Sat:	0.09	0.12	0.13	0.11	0.14	0.15	0.10	0.23	0.29	0.03	0.11	0.11
Crit Moves:		****	****						****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #84 SR-55 NB Ramps and Dyer Rd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.569
Loss Time (sec):	5	Average Delay (sec/veh):	17.1
Optimal Cycle:	28	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Ignore			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	1! 0	1	0	0	0	0	0	3	0	1	0	0	2	1	0
Volume Module:												
Base Vol:	610	0	499	0	0	0	0	1618	647	0	599	361
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	610	0	499	0	0	0	0	1618	647	0	599	361
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Volume:	610	0	499	0	0	0	0	1618	0	0	599	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	610	0	499	0	0	0	0	1618	0	0	599	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
FinalVolume:	610	0	499	0	0	0	0	1618	0	0	599	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.91	1.00	0.91	1.00	1.00	1.00	1.00	0.91	1.00	1.00	0.91	0.91
Lanes:	1.55	0.00	1.45	0.00	0.00	0.00	0.00	3.00	1.00	0.00	3.00	0.00
Final Sat.:	2674	0	2501	0	0	0	0	5187	1900	0	5187	0
Capacity Analysis Module:												
Vol/Sat:	0.23	0.00	0.20	0.00	0.00	0.00	0.00	0.31	0.00	0.00	0.12	0.00
Crit Moves:	****							****		****		
Green/Cycle:	0.40	0.00	0.40	0.00	0.00	0.00	0.00	0.55	0.00	0.00	0.55	0.00
Volume/Cap:	0.57	0.00	0.50	0.00	0.00	0.00	0.00	0.57	0.00	0.00	0.21	0.00
Delay/Veh:	23.6	0.0	22.6	0.0	0.0	0.0	0.0	15.1	0.0	0.0	11.6	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	23.6	0.0	22.6	0.0	0.0	0.0	0.0	15.1	0.0	0.0	11.6	0.0
LOS by Move:	C	A	C	A	A	A	A	B	A	A	B	A
HCM2k85thQ:	15	0	12	0	0	0	0	18	0	0	5	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #85 Cambridge St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.971
Loss Time (sec):	5	Average Delay (sec/veh):	31.6
Optimal Cycle:	0	Level Of Service:	D

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	T	R		L	T	R		L	T	R		L	T	R	
Control:	Stop Sign				Stop Sign				Stop Sign				Stop Sign			
Rights:	Include				Include				Include				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Lanes:	1	0	1	0	0	0	0	1	0	1	0	0	0	1	0	0
Volume Module:																
Base Vol:	185	235	0		0	480	245		273	0	436		0	0	0	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	185	235	0		0	480	245		273	0	436		0	0	0	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	185	235	0		0	480	245		273	0	436		0	0	0	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	185	235	0		0	480	245		273	0	436		0	0	0	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	185	235	0		0	480	245		273	0	436		0	0	0	
Saturation Flow Module:																
Adjustment:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Lanes:	1.00	1.00	0.00		0.00	1.00	1.00		1.00	0.00	1.00		0.00	0.00	0.00	
Final Sat.:	434	463	0		0	494	537		458	0	539		0	0	0	
Capacity Analysis Module:																
Vol/Sat:	0.43	0.51	xxxx		xxxx	0.97	0.46		0.60	xxxx	0.81		xxxx	xxxx	xxxx	
Crit Moves:		****				****					****					
Delay/Veh:	16.6	17.7	0.0		0.0	59.7	14.5		21.1	0.0	30.8		0.0	0.0	0.0	
Delay Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
AdjDel/Veh:	16.6	17.7	0.0		0.0	59.7	14.5		21.1	0.0	30.8		0.0	0.0	0.0	
LOS by Move:	C	C	*		*	F	B		C	*	D		*	*	*	
ApproachDel:	17.2				44.4				27.1				xxxxxx			
Delay Adj:	1.00				1.00				1.00				xxxxx			
ApprAdjDel:	17.2				44.4				27.1				xxxxxx			
LOS by Appr:	C				E				D				*			
AllWayAvgQ:	0.7	0.9	0.0		0.0	6.8	0.8		1.4	0.0	3.2		0.0	0.0	0.0	

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #86 Cambridge St and Fairhaven Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.563
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	27	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	0	0 0	1 0	0	0 1	1 0	2	0 0	0 0	1	1 0
Volume Module:												
Base Vol:	0	0	0	567	0	183	80	216	0	0	195	152
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	567	0	183	80	216	0	0	195	152
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	567	0	183	80	216	0	0	195	152
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	567	0	183	80	216	0	0	195	152
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	567	0	183	80	216	0	0	195	152
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	0.00	0.00	0.00	1.00	0.00	1.00	1.00	2.00	0.00	0.00	1.12	0.88
Final Sat.:	0	0	0	1600	0	1600	1600	3400	0	0	1898	1402
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.35	0.00	0.11	0.05	0.06	0.00	0.00	0.10	0.11
Crit Moves:				****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #87 Mabury St and 1st Street

Cycle (sec):	100	Critical Vol./Cap.(X):	0.714
Loss Time (sec):	5	Average Delay (sec/veh):	27.4
Optimal Cycle:	40	Level Of Service:	C

Street Name:	Mabury								1st Street			
Approach:	North Bound				South Bound				East Bound			
Movement:	L	T	R		L	T	R		L	T	R	
Control:	Split Phase				Split Phase				Protected			
Rights:	Include				Include				Include			
Min. Green:	0	0	0		0	0	0		0	0	0	
Y+R:	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0	
Lanes:	1	0	0	1	1	0	1!	0	1	0	0	2
Volume Module:												
Base Vol:	6	0	157		248	147	554		0	1230	13	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	6	0	157		248	147	554		0	1230	13	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	6	0	157		248	147	554		0	1230	13	
Reduct Vol:	0	0	0		0	0	0		0	0	0	
Reduced Vol:	6	0	157		248	147	554		0	1230	13	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	6	0	157		248	147	554		0	1230	13	
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900		1900	1900	1900		1900	1900	1900	
Adjustment:	0.95	1.00	0.85		0.90	0.90	0.90		1.00	0.91	0.91	
Lanes:	1.00	0.00	1.00		1.23	0.27	1.50		0.00	2.97	0.03	
Final Sat.:	1805	0	1615		2097	459	2575		0	5128	54	
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.10		0.12	0.32	0.22		0.00	0.24	0.24	
Crit Moves:			****			****				****		
Green/Cycle:	0.14	0.00	0.14		0.45	0.45	0.45		0.00	0.34	0.34	
Volume/Cap:	0.02	0.00	0.71		0.26	0.71	0.48		0.00	0.71	0.71	
Delay/Veh:	37.5	0.0	51.9		17.3	24.2	19.6		0.0	30.4	30.4	
User DelAdj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
AdjDel/Veh:	37.5	0.0	51.9		17.3	24.2	19.6		0.0	30.4	30.4	
LOS by Move:	D	A	D		B	C	B		A	C	C	
HCM2k85thQ:	0	0	10		6	22	12		0	20	20	

Approach:	West Bound											
Movement:	L	T	R									
Control:	Protected											
Rights:	Include											
Min. Green:	0	0	0									
Y+R:	4.0	4.0	4.0									
Lanes:	1	0	2	1	0							
Volume Module:												
Base Vol:	38	579	0									
Growth Adj:	1.00	1.00	1.00									
Initial Bse:	38	579	0									
User Adj:	1.00	1.00	1.00									
PHF Adj:	1.00	1.00	1.00									
PHF Volume:	38	579	0									
Reduct Vol:	0	0	0									
Reduced Vol:	38	579	0									
PCE Adj:	1.00	1.00	1.00									
MLF Adj:	1.00	1.00	1.00									
FinalVolume:	38	579	0									
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900									
Adjustment:	0.95	0.91	0.91									
Lanes:	1.00	3.00	0.00									
Final Sat.:	1805	5187	0									
Capacity Analysis Module:												
Vol/Sat:	0.02	0.11	0.00									
Crit Moves:		****										
Green/Cycle:	0.03	0.37	0.00									
Volume/Cap:	0.71	0.31	0.00									
Delay/Veh:	84.9	22.8	0.0									
User DelAdj:	1.00	1.00	1.00									
AdjDel/Veh:	84.9	22.8	0.0									
LOS by Move:	F	C	A									
HCM2k85thQ:	4	7	0									

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #88 Tustin St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.446
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	22	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 1	0	0 1
Volume Module:												
Base Vol:	7	301	87	67	1307	4	9	11	35	307	1	47
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	7	301	87	67	1307	4	9	11	35	307	1	47
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	7	301	87	67	1307	4	9	11	35	307	1	47
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	7	301	87	67	1307	4	9	11	35	307	1	47
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	7	301	87	67	1307	4	9	11	35	307	1	47
OvlAdjVol:												0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	1.00	2.33	0.67	1.00	2.99	0.01	1.00	1.00	1.00	1.99	0.01	1.00
Final Sat.:	1600	3924	1076	1600	4985	15	1600	1700	1600	3190	10	1600
Capacity Analysis Module:												
Vol/Sat:	0.00	0.08	0.08	0.04	0.26	0.27	0.01	0.01	0.02	0.10	0.10	0.03
OvlAdjV/S:												0.00
Crit Moves:	****					****			****		****	

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #89 Tustin St and SR-22 WB On-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.635
Loss Time (sec):	5	Average Delay (sec/veh):	12.2
Optimal Cycle:	33	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0 2	0 0	0	0 2	1 0	0	0 0	0 0	0	0 0	0 0
Volume Module:												
Base Vol:	614	571	0	0	1171	699	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	614	571	0	0	1171	699	0	0	0	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	614	571	0	0	1171	699	0	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	614	571	0	0	1171	699	0	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	614	571	0	0	1171	699	0	0	0	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.95	1.00	1.00	0.86	0.86	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	2.00	2.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00
Final Sat.:	3502	3610	0	0	3264	1632	0	0	0	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.18	0.16	0.00	0.00	0.36	0.43	0.00	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****					****						
Green/Cycle:	0.28	0.95	0.00	0.00	0.67	0.67	0.00	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.64	0.17	0.00	0.00	0.53	0.64	0.00	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	33.2	0.2	0.0	0.0	8.4	9.8	0.0	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	33.2	0.2	0.0	0.0	8.4	9.8	0.0	0.0	0.0	0.0	0.0	0.0
LOS by Move:	C	A	A	A	A	A	A	A	A	A	A	A
HCM2k85thQ:	14	1	0	0	15	21	0	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #90 Tustin St and SR-22 EB Off-Ramp / Seba Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.723
Loss Time (sec):	5	Average Delay (sec/veh):	25.0
Optimal Cycle:	42	Level Of Service:	C

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	- T	- R		L	- T	- R		L	- T	- R		L	- T	- R	
Control:	Permitted				Permitted				Split Phase				Split Phase			
Rights:	Include				Include				Include				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Y+R:	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0	
Lanes:	0	0 2	1 0		1	0 3	0 0		1	0 1!	0 1		0	0 1!	0 0	
Volume Module:																
Base Vol:	0	953	5		17	1049	0		275	27	1182		6	0	43	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	0	953	5		17	1049	0		275	27	1182		6	0	43	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	0	953	5		17	1049	0		275	27	1182		6	0	43	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	0	953	5		17	1049	0		275	27	1182		6	0	43	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	0	953	5		17	1049	0		275	27	1182		6	0	43	
Saturation Flow Module:																
Sat/Lane:	1900	1900	1900		1900	1900	1900		1900	1900	1900		1900	1900	1900	
Adjustment:	1.00	0.91	0.91		0.14	0.91	1.00		0.87	0.87	0.87		0.88	1.00	0.88	
Lanes:	0.00	2.98	0.02		1.00	3.00	0.00		1.18	0.04	1.78		0.12	0.00	0.88	
Final Sat.:	0	5155	27		272	5187	0		1961	59	2956		204	0	1460	
Capacity Analysis Module:																
Vol/Sat:	0.00	0.18	0.18		0.06	0.20	0.00		0.14	0.46	0.40		0.03	0.00	0.03	
Crit Moves:						****				****			****			
Green/Cycle:	0.00	0.28	0.28		0.28	0.28	0.00		0.63	0.63	0.63		0.04	0.00	0.04	
Volume/Cap:	0.00	0.66	0.66		0.22	0.72	0.00		0.22	0.72	0.63		0.72	0.00	0.72	
Delay/Veh:	0.0	33.0	33.0		29.2	34.4	0.0		8.0	13.9	12.0		79.1	0.0	79.1	
User DelAdj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
AdjDel/Veh:	0.0	33.0	33.0		29.2	34.4	0.0		8.0	13.9	12.0		79.1	0.0	79.1	
LOS by Move:	A	C	C		C	C	A		A	B	B		E	A	E	
HCM2k85thQ:	0	16	16		1	18	0		5	25	19		5	0	5	

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #91 Tustin Ave and Fairhaven Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	1.195
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	- T	- R		L	- T	- R		L	- T	- R		L	- T	- R	
Control:	Protected				Protected				Protected				Protected			
Rights:	Include				Include				Include				Ovl			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Y+R:	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0	
Lanes:	2	0 2	1	0	2	0 2	1	0	1	0 1	1	0	1	0 1	0	1
Volume Module:																
Base Vol:	101	586	103		280	2411	48		84	317	546		416	460	601	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	101	586	103		280	2411	48		84	317	546		416	460	601	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	101	586	103		280	2411	48		84	317	546		416	460	601	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	101	586	103		280	2411	48		84	317	546		416	460	601	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	101	586	103		280	2411	48		84	317	546		416	460	601	
OvlAdjVol:															461	
Saturation Flow Module:																
Sat/Lane:	1600	1600	1600		1600	1600	1600		1600	1600	1600		1600	1600	1600	
Adjustment:	1.00	1.05	1.00		1.00	1.04	1.00		1.00	1.06	1.00		1.00	1.06	1.00	
Lanes:	2.00	2.55	0.45		2.00	2.94	0.06		1.00	1.00	1.00		1.00	1.00	1.00	
Final Sat.:	3200	4282	718		3200	4906	94		1600	1700	1600		1600	1700	1600	
Capacity Analysis Module:																
Vol/Sat:	0.03	0.14	0.14		0.09	0.49	0.51		0.05	0.19	0.34		0.26	0.27	0.38	
OvlAdjV/S:															0.29	
Crit Moves:	****						****				****	****				

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #92 Tustin Ave and Santa Clara Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	1.037
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	2 0	2	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	83	453	84	151	3094	98	124	314	229	160	332	166
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	83	453	84	151	3094	98	124	314	229	160	332	166
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	83	453	84	151	3094	98	124	314	229	160	332	166
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	83	453	84	151	3094	98	124	314	229	160	332	166
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	83	453	84	151	3094	98	124	314	229	160	332	166
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.53	0.47	2.00	2.91	0.09	1.00	1.16	0.84	1.00	1.33	0.67
Final Sat.:	1600	4249	751	3200	4853	147	1600	1950	1350	1600	2233	1067
Capacity Analysis Module:												
Vol/Sat:	0.05	0.11	0.11	0.05	0.64	0.67	0.08	0.16	0.17	0.10	0.15	0.16
Crit Moves:	****					****			****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #93 Tustin Ave and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.911
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	93	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 0	1	2 0	2 1	0	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	127	226	269	716	2067	23	230	908	155	664	1047	256
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	127	226	269	716	2067	23	230	908	155	664	1047	256
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	127	226	269	716	2067	23	230	908	155	664	1047	256
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	127	226	269	716	2067	23	230	908	155	664	1047	256
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	127	226	269	716	2067	23	230	908	155	664	1047	256
OvlAdjVol:			0									0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.00	1.00	2.00	2.97	0.03	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	3400	1600	3200	4947	53	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.04	0.07	0.17	0.22	0.42	0.44	0.07	0.18	0.10	0.21	0.21	0.16
OvlAdjV/S:			0.00									0.00
Crit Moves:	****					****		****		****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #94 Tustin Ave and 4th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.830
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	59	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	2 0	2	0 1	1 0	2	1 1	1 0	2	1 0
Volume Module:												
Base Vol:	80	408	365	601	765	237	126	748	31	125	704	457
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	80	408	365	601	765	237	126	748	31	125	704	457
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	80	408	365	601	765	237	126	748	31	125	704	457
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	80	408	365	601	765	237	126	748	31	125	704	457
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	80	408	365	601	765	237	126	748	31	125	704	457
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	2.00	1.00	2.00	2.00	1.00	1.00	3.00	1.00	1.00	2.00	1.00
Final Sat.:	1600	3400	1600	3200	3400	1600	1600	5100	1600	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.05	0.12	0.23	0.19	0.23	0.15	0.08	0.15	0.02	0.08	0.21	0.29
Crit Moves:		****	****				****					****

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #95 SR-55 SB Ramps / Auto Mall Dr and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.672
Loss Time (sec):	5	Average Delay (sec/veh):	25.2
Optimal Cycle:	36	Level Of Service:	C

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	- T	- R		L	- T	- R		L	- T	- R		L	- T	- R	
Control:	Split Phase				Split Phase				Protected				Protected			
Rights:	Include				Include				Ignore				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Y+R:	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0	
Lanes:	1	1 0	0	2	1	0 1	0	1	1	0 3	0	1	2	0 2	1	0
Volume Module:																
Base Vol:	456	68	540		50	30	8		36	1426	527		510	1202	157	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	456	68	540		50	30	8		36	1426	527		510	1202	157	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	0.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	0.00		1.00	1.00	1.00	
PHF Volume:	456	68	540		50	30	8		36	1426	0		510	1202	157	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	456	68	540		50	30	8		36	1426	0		510	1202	157	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	0.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	0.00		1.00	1.00	1.00	
FinalVolume:	456	68	540		50	30	8		36	1426	0		510	1202	157	
Saturation Flow Module:																
Sat/Lane:	1900	1900	1900		1900	1900	1900		1900	1900	1900		1900	1900	1900	
Adjustment:	0.96	0.96	0.75		0.95	1.00	0.85		0.95	0.91	1.00		0.92	0.89	0.89	
Lanes:	1.74	0.26	2.00		1.00	1.00	1.00		1.00	3.00	1.00		2.00	2.65	0.35	
Final Sat.:	3168	472	2842		1805	1900	1615		1805	5187	1900		3502	4510	589	
Capacity Analysis Module:																
Vol/Sat:	0.14	0.14	0.19		0.03	0.02	0.00		0.02	0.27	0.00		0.15	0.27	0.27	
Crit Moves:			****		****					****			****			
Green/Cycle:	0.28	0.28	0.28		0.04	0.04	0.04		0.04	0.41	0.00		0.22	0.58	0.58	
Volume/Cap:	0.51	0.51	0.67		0.67	0.38	0.12		0.46	0.67	0.00		0.67	0.46	0.46	
Delay/Veh:	30.5	30.5	34.0		68.7	49.8	47.0		50.8	24.9	0.0		38.3	12.0	12.0	
User DelAdj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
AdjDel/Veh:	30.5	30.5	34.0		68.7	49.8	47.0		50.8	24.9	0.0		38.3	12.0	12.0	
LOS by Move:	C	C	C		E	D	D		D	C	A		D	B	B	
HCM2k85thQ:	11	11	14		4	2	1		3	20	0		13	13	13	

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Note: Queue reported is the number of cars per lane.
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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #96 SR-55 NB Ramps / Del Amo Ave and Newport Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	0.673
Loss Time (sec):	5	Average Delay (sec/veh):	25.4
Optimal Cycle:	36	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0 2		1 0	1 0 3		0 1	2 0 1		1 0	1 0 1		1 0
Volume Module:												
Base Vol:	392	104	66	21	512	750	269	163	155	8	223	4
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	392	104	66	21	512	750	269	163	155	8	223	4
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	392	104	66	21	512	750	269	163	155	8	223	4
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	392	104	66	21	512	750	269	163	155	8	223	4
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	392	104	66	21	512	750	269	163	155	8	223	4
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.86	0.86	0.95	0.91	0.85	0.92	0.88	0.88	0.95	0.95	0.95
Lanes:	2.00	2.00	1.00	1.00	3.00	1.00	2.00	1.03	0.97	1.00	1.96	0.04
Final Sat.:	3502	3257	1629	1805	5187	1615	3502	1715	1631	1805	3536	63
Capacity Analysis Module:												
Vol/Sat:	0.11	0.03	0.04	0.01	0.10	0.46	0.08	0.10	0.10	0.00	0.06	0.06
Crit Moves:	****					****	****				****	
Green/Cycle:	0.17	0.58	0.58	0.17	0.58	0.69	0.11	0.20	0.20	0.01	0.09	0.09
Volume/Cap:	0.67	0.06	0.07	0.07	0.17	0.67	0.67	0.48	0.48	0.48	0.67	0.67
Delay/Veh:	42.2	9.3	9.4	35.3	10.0	10.6	47.0	36.0	36.0	69.3	49.1	49.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	42.2	9.3	9.4	35.3	10.0	10.6	47.0	36.0	36.0	69.3	49.1	49.1
LOS by Move:	D	A	A	D	B	B	D	D	D	E	D	D
HCM2k85thQ:	11	1	2	1	4	21	8	8	8	1	8	8

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #97 Red Hill Ave and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.610
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	30	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Ovl			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0 3	0 1	2	0 3	0 1	2	0 3	0 1	2	0 3	0 1
Volume Module:												
Base Vol:	101	374	106	195	828	388	397	1276	271	255	1234	173
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	101	374	106	195	828	388	397	1276	271	255	1234	173
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	101	374	106	195	828	388	397	1276	271	255	1234	173
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	101	374	106	195	828	388	397	1276	271	255	1234	173
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	101	374	106	195	828	388	397	1276	271	255	1234	173
OvlAdjVol:						189			221			
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.07	0.07	0.06	0.16	0.24	0.12	0.25	0.17	0.08	0.24	0.11
OvlAdjV/S:						0.12			0.14			
Crit Moves:	****				****		****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #98 Red Hill Ave and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.560
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	27	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0 3	1 0	2	0 2	1 0	1	0 2	1 0	2	0 3	0 1
Volume Module:												
Base Vol:	68	414	56	20	935	188	273	464	342	120	427	76
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	68	414	56	20	935	188	273	464	342	120	427	76
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	68	414	56	20	935	188	273	464	342	120	427	76
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	68	414	56	20	935	188	273	464	342	120	427	76
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	68	414	56	20	935	188	273	464	342	120	427	76
OvlAdjVol:												66
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.52	0.48	2.00	2.50	0.50	1.00	2.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5937	763	3200	4196	804	1600	3400	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.02	0.07	0.07	0.01	0.22	0.23	0.17	0.14	0.21	0.04	0.08	0.05
OvlAdjV/S:												0.04
Crit Moves:	****					****	****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #99 Red Hill Ave and Dyer Rd / barranca Pkwy

Cycle (sec):	100	Critical Vol./Cap.(X):	0.548
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	26	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	4 0	1	2 0	4 0	1	2 0	3 1	0	2 0	4 0	1
Volume Module:												
Base Vol:	152	379	125	163	719	94	124	748	296	511	571	111
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	152	379	125	163	719	94	124	748	296	511	571	111
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	152	379	125	163	719	94	124	748	296	511	571	111
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	152	379	125	163	719	94	124	748	296	511	571	111
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	152	379	125	163	719	94	124	748	296	511	571	111
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	4.00	1.00	2.00	4.00	1.00	2.00	3.00	1.00	2.00	4.00	1.00
Final Sat.:	3200	6800	1600	3200	6800	1600	3200	5100	1600	3200	6800	1600
Capacity Analysis Module:												
Vol/Sat:	0.05	0.06	0.08	0.05	0.11	0.06	0.04	0.15	0.19	0.16	0.08	0.07
Crit Moves:	****				****				****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #100 Red Hill Ave and Alton Pkwy

Cycle (sec):	100	Critical Vol./Cap.(X):	0.855
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	67	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	2 0	1	2 0	1 0	1
Volume Module:												
Base Vol:	225	663	415	212	962	154	158	490	490	342	290	163
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	225	663	415	212	962	154	158	490	490	342	290	163
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	225	663	415	212	962	154	158	490	490	342	290	163
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	225	663	415	212	962	154	158	490	490	342	290	163
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	225	663	415	212	962	154	158	490	490	342	290	163
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	2.00	1.00	2.00	1.00	1.00
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	3400	1600	3200	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.14	0.13	0.26	0.13	0.19	0.10	0.10	0.14	0.31	0.11	0.17	0.10
Crit Moves:			****	****					****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #101 Red Hill Ave and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.686
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	36	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ignore			Include			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0 2	1 0		2 0 3	0 1		2 0 3	0 1		1 0 3	0 1	
Volume Module:												
Base Vol:	83	742	13	186	481	441	1153	596	98	32	308	789
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	83	742	13	186	481	441	1153	596	98	32	308	789
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Volume:	83	742	13	186	481	0	1153	596	98	32	308	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	83	742	13	186	481	0	1153	596	98	32	308	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
FinalVolume:	83	742	13	186	481	0	1153	596	98	32	308	0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.95	0.05	2.00	3.00	1.00	2.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	4917	83	3200	5100	1600	3200	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.15	0.16	0.06	0.09	0.00	0.36	0.12	0.06	0.02	0.06	0.00
Crit Moves:			****	****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #102 Red Hill Ave and Main St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.722
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	40	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 0	1	2 0	2 0	1	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	201	691	449	56	261	90	139	1309	254	154	309	73
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	201	691	449	56	261	90	139	1309	254	154	309	73
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	201	691	449	56	261	90	139	1309	254	154	309	73
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	201	691	449	56	261	90	139	1309	254	154	309	73
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	201	691	449	56	261	90	139	1309	254	154	309	73
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	2.00	2.00	1.00	2.00	2.00	1.00	2.00	2.51	0.49	2.00	2.43	0.57
Final Sat.:	3200	3400	1600	3200	3400	1600	3200	4220	780	3200	4083	917
Capacity Analysis Module:												
Vol/Sat:	0.06	0.20	0.28	0.02	0.08	0.06	0.04	0.31	0.33	0.05	0.08	0.08
Crit Moves:			****	****					****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #103 I-5 SB Ramps and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.559
Loss Time (sec):	5	Average Delay (sec/veh):	17.4
Optimal Cycle:	28	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	0	2	0	0	2	0	3	0	2	1
Volume Module:												
Base Vol:	0	0	0	382	0	38	332	569	0	0	1267	370
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	382	0	38	332	569	0	0	1267	370
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	382	0	38	332	569	0	0	1267	370
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	382	0	38	332	569	0	0	1267	370
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	382	0	38	332	569	0	0	1267	370
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.92	1.00	0.85	0.92	0.91	1.00	1.00	0.88	0.88
Lanes:	0.00	0.00	0.00	2.00	0.00	1.00	2.00	3.00	0.00	0.00	2.32	0.68
Final Sat.:	0	0	0	3502	0	1615	3502	5187	0	0	3878	1133
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.11	0.00	0.02	0.09	0.11	0.00	0.00	0.33	0.33
Crit Moves:				****			****				****	
Green/Cycle:	0.00	0.00	0.00	0.20	0.00	0.37	0.17	0.75	0.00	0.00	0.58	0.58
Volume/Cap:	0.00	0.00	0.00	0.56	0.00	0.06	0.56	0.15	0.00	0.00	0.56	0.56
Delay/Veh:	0.0	0.0	0.0	37.4	0.0	20.7	39.3	3.4	0.0	0.0	13.0	13.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

AdjDel/Veh:	0.0	0.0	0.0	37.4	0.0	20.7	39.3	3.4	0.0	0.0	13.0	13.0
LOS by Move:	A	A	A	D	A	C	D	A	A	A	B	B
HCM2k85thQ:	0	0	0	9	0	1	9	3	0	0	17	17

****

Note: Queue reported is the number of cars per lane.

****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #104 Tustin Ranch Rd and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.520
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	25	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	0 0	0	2 0	0 0	1	2 0	3 0	0	0 0	3 0	1
Volume Module:												
Base Vol:	0	0	0	141	0	18	27	357	0	0	2130	490
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	141	0	18	27	357	0	0	2130	490
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	141	0	18	27	357	0	0	2130	490
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	141	0	18	27	357	0	0	2130	490
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	141	0	18	27	357	0	0	2130	490
OvlAdjVol:												420
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	0.00	0.00	0.00	2.00	0.00	1.00	2.00	3.00	0.00	0.00	3.00	1.00
Final Sat.:	0	0	0	3200	0	1600	3200	5100	0	0	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.04	0.00	0.01	0.01	0.07	0.00	0.00	0.42	0.31
OvlAdjV/S:												0.26
Crit Moves:				****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #105 Von Karman Ave and Barranca Pkwy

Cycle (sec):	100	Critical Vol./Cap.(X):	0.929
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	108	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0 1	1 0		2 0 2		0 2	2 0 3		0 1	2 0 4		0 1
Volume Module:												
Base Vol:	401	533	319	104	1021	446	219	823	275	902	1482	46
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	401	533	319	104	1021	446	219	823	275	902	1482	46
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	401	533	319	104	1021	446	219	823	275	902	1482	46
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	401	533	319	104	1021	446	219	823	275	902	1482	46
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	401	533	319	104	1021	446	219	823	275	902	1482	46
OvlAdjVol:						227						
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	1.25	0.75	2.00	2.00	2.00	2.00	3.00	1.00	2.00	4.00	1.00
Final Sat.:	3200	2102	1198	3200	3400	3200	3200	5100	1600	3200	6800	1600
Capacity Analysis Module:												
Vol/Sat:	0.13	0.25	0.27	0.03	0.30	0.14	0.07	0.16	0.17	0.28	0.22	0.03
OvlAdjV/S:						0.07						
Crit Moves:	****			****					****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #106 Red Hill Avenue and El Camino Real

Cycle (sec):	100	Critical Vol./Cap.(X):	0.650
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	33	Level Of Service:	B

Street Name:	Red Hill Avenue						El Camino Real					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	1 0	2 1	0	1 0	1 0	1	1 0	0 1	0
Volume Module:												
Base Vol:	230	589	190	19	1372	84	48	108	114	246	202	15
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	230	589	190	19	1372	84	48	108	114	246	202	15
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	230	589	190	19	1372	84	48	108	114	246	202	15
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	230	589	190	19	1372	84	48	108	114	246	202	15
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	230	589	190	19	1372	84	48	108	114	246	202	15
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	2.00	3.00	1.00	1.00	2.83	0.17	1.00	1.00	1.00	1.00	0.93	0.07
Final Sat.:	3200	5100	1600	1600	4723	277	1600	1700	1600	1600	1489	111
Capacity Analysis Module:												
Vol/Sat:	0.07	0.12	0.12	0.01	0.29	0.30	0.03	0.06	0.07	0.15	0.14	0.14
Crit Moves:	****					****			****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #107 Red Hill Avenue and I-5 NB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.627
Loss Time (sec):	5	Average Delay (sec/veh):	20.8
Optimal Cycle:	32	Level Of Service:	C

Street Name:	Red Hill Avenue						I-5 NB Ramps					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	3	0	0	3	0	1	0	0	0	2	0	0	0	1
Volume Module:												
Base Vol:	254	843	0	0	1171	440	0	0	0	258	0	295
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	254	843	0	0	1171	440	0	0	0	258	0	295
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	254	843	0	0	1171	440	0	0	0	258	0	295
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	254	843	0	0	1171	440	0	0	0	258	0	295
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	254	843	0	0	1171	440	0	0	0	258	0	295
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	1.00	1.00	0.91	0.85	1.00	1.00	1.00	0.60	1.00	0.85
Lanes:	1.00	3.00	0.00	0.00	3.00	1.00	0.00	0.00	0.00	2.00	0.00	1.00
Final Sat.:	1805	5187	0	0	5187	1615	0	0	0	2267	0	1615
Capacity Analysis Module:												
Vol/Sat:	0.14	0.16	0.00	0.00	0.23	0.27	0.00	0.00	0.00	0.11	0.00	0.18
Crit Moves:	****					****						****
Green/Cycle:	0.22	0.66	0.00	0.00	0.43	0.43	0.00	0.00	0.00	0.29	0.00	0.29
Volume/Cap:	0.63	0.25	0.00	0.00	0.52	0.63	0.00	0.00	0.00	0.39	0.00	0.63
Delay/Veh:	38.1	7.0	0.0	0.0	20.9	23.8	0.0	0.0	0.0	28.7	0.0	33.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

AdjDel/Veh:	38.1	7.0	0.0	0.0	20.9	23.8	0.0	0.0	0.0	28.7	0.0	33.4
LOS by Move:	D	A	A	A	C	C	A	A	A	C	A	C
HCM2k85thQ:	12	6	0	0	15	17	0	0	0	6	0	13

****

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #108 Red Hill Avenue and I-5 SB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.799
Loss Time (sec):	5	Average Delay (sec/veh):	23.7
Optimal Cycle:	54	Level Of Service:	C

Street Name:	Red Hill Avenue						I-5 SB Ramps					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	4 0	1	1 0	3 0	0	0 1	0 0	1	0 0	0 0	0
Volume Module:												
Base Vol:	0	1191	527	371	1064	0	200	5	367	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1191	527	371	1064	0	200	5	367	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1191	527	371	1064	0	200	5	367	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1191	527	371	1064	0	200	5	367	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1191	527	371	1064	0	200	5	367	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	0.85	0.95	0.91	1.00	0.95	0.95	0.85	1.00	1.00	1.00
Lanes:	0.00	4.00	1.00	1.00	3.00	0.00	0.98	0.02	1.00	0.00	0.00	0.00
Final Sat.:	0	6916	1615	1805	5187	0	1757	44	1615	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.17	0.33	0.21	0.21	0.00	0.11	0.11	0.23	0.00	0.00	0.00
Crit Moves:			****	****					****			
Green/Cycle:	0.00	0.41	0.41	0.26	0.67	0.00	0.28	0.28	0.28	0.00	0.00	0.00
Volume/Cap:	0.00	0.42	0.80	0.80	0.31	0.00	0.40	0.40	0.80	0.00	0.00	0.00
Delay/Veh:	0.0	21.2	32.8	44.2	7.1	0.0	29.4	29.4	42.7	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	21.2	32.8	44.2	7.1	0.0	29.4	29.4	42.7	0.0	0.0	0.0
LOS by Move:	A	C	C	D	A	A	C	C	D	A	A	A
HCM2k85thQ:	0	11	24	19	8	0	8	8	19	0	0	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #109 Red Hill Avenue and Nisson Road

Cycle (sec):	100	Critical Vol./Cap.(X):	0.651
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	33	Level Of Service:	B

Street Name:	Red Hill Avenue						Nisson Road					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 1	0	1 0	2 1	0	1 0	0 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	16	1244	23	162	1222	128	242	59	48	49	26	215
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	16	1244	23	162	1222	128	242	59	48	49	26	215
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	16	1244	23	162	1222	128	242	59	48	49	26	215
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	16	1244	23	162	1222	128	242	59	48	49	26	215
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	16	1244	23	162	1222	128	242	59	48	49	26	215
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	3.93	0.07	1.00	2.72	0.28	1.00	0.55	0.45	1.00	0.11	0.89
Final Sat.:	1600	6584	116	1600	4545	455	1600	882	718	1600	173	1427
Capacity Analysis Module:												
Vol/Sat:	0.01	0.19	0.20	0.10	0.27	0.28	0.15	0.07	0.07	0.03	0.15	0.15
Crit Moves:			****	****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #110 Red Hill Avenue and Walnut Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	0.636
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	31	Level Of Service:	B

Street Name:	Red Hill Avenue						Walnut Avenue					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	2	1	0	1	0	2	1	0	1	0	1	1	0	2	0	1	1	0
Volume Module:												
Base Vol:	129	502	95	113	1135	127	104	258	254	263	348	166
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	129	502	95	113	1135	127	104	258	254	263	348	166
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	129	502	95	113	1135	127	104	258	254	263	348	166
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	129	502	95	113	1135	127	104	258	254	263	348	166
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	129	502	95	113	1135	127	104	258	254	263	348	166
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.05	1.00
Lanes:	1.00	2.52	0.48	1.00	2.70	0.30	1.00	1.01	0.99	2.00	1.35	0.65
Final Sat.:	1600	4236	764	1600	4517	483	1600	1713	1588	3200	2267	1033
Capacity Analysis Module:												
Vol/Sat:	0.08	0.12	0.12	0.07	0.25	0.26	0.07	0.15	0.16	0.08	0.15	0.16
Crit Moves:	****					****			****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #111 Red Hill Avenue and Valencia Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	0.652
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	33	Level Of Service:	B

Street Name:	Red Hill Avenue						Valencia Avenue					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 1	0	1 0	2 1	0	1 0	1 0	1	2 0	1 0	1
Volume Module:												
Base Vol:	98	412	133	126	1082	72	49	343	133	315	433	153
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	98	412	133	126	1082	72	49	343	133	315	433	153
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	98	412	133	126	1082	72	49	343	133	315	433	153
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	98	412	133	126	1082	72	49	343	133	315	433	153
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	98	412	133	126	1082	72	49	343	133	315	433	153
OvlAdjVol:			0									27
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	3.02	0.98	1.00	2.81	0.19	1.00	1.00	1.00	2.00	1.00	1.00
Final Sat.:	1600	5138	1562	1600	4701	299	1600	1700	1600	3200	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.06	0.08	0.09	0.08	0.23	0.24	0.03	0.20	0.08	0.10	0.25	0.10
OvlAdjV/S:			0.00									0.02
Crit Moves:	****					****		****		****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #112 Tustin Ranch Road and Warner Avenue North

Cycle (sec):	100	Critical Vol./Cap.(X):	0.449
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	22	Level Of Service:	A

Street Name:	Tustin Ranch Road						Warner Avenue North					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	3 0 1	2	0	3 0 0	0	0	0 0 0	3	0	0 0 2
Volume Module:												
Base Vol:	0	321	311	403	1140	0	0	0	0	844	0	249
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	321	311	403	1140	0	0	0	0	844	0	249
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	321	311	403	1140	0	0	0	0	844	0	249
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	321	311	403	1140	0	0	0	0	844	0	249
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	321	311	403	1140	0	0	0	0	844	0	249
OvlAdjVol:			30									0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	0.00	3.00	1.00	2.00	3.00	0.00	0.00	0.00	0.00	3.00	0.00	2.00
Final Sat.:	0	5100	1600	3200	5100	0	0	0	0	4800	0	3200
Capacity Analysis Module:												
Vol/Sat:	0.00	0.06	0.19	0.13	0.22	0.00	0.00	0.00	0.00	0.18	0.00	0.08
OvlAdjV/S:			0.02									0.00
Crit Moves:	****			****					****			

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #113 Tustin Ranch Road and Walnut Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	1.273
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Street Name:	Tustin Ranch Road						Walnut Avenue					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Ovl			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0	3 0 1	2	0	3 0 1	2	0	1 1 0	2	0	2 0 1
Volume Module:												
Base Vol:	92	567	283	1076	3008	401	834	1089	658	187	276	233
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	92	567	283	1076	3008	401	834	1089	658	187	276	233
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	92	567	283	1076	3008	401	834	1089	658	187	276	233
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	92	567	283	1076	3008	401	834	1089	658	187	276	233
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	92	567	283	1076	3008	401	834	1089	658	187	276	233
OvlAdjVol:			190			0						0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	1.25	0.75	2.00	2.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	2095	1205	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.11	0.18	0.34	0.59	0.25	0.26	0.52	0.55	0.06	0.08	0.15
OvlAdjV/S:			0.12			0.00						0.00
Crit Moves:	****			****					****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #114 SR-55 SB Ramps and Irvine Boulevard

Cycle (sec):	100	Critical Vol./Cap.(X):	1.099
Loss Time (sec):	5	Average Delay (sec/veh):	41.0
Optimal Cycle:	180	Level Of Service:	D

Street Name:	SR-55 SB Ramps						Irvine Boulevard					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	0	1	0	1! 0	1	0	0	2	1	0	1	0	3	0	0
Volume Module:												
Base Vol:	0	0	0	65	0	71	0	647	1161	496	1240	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	65	0	71	0	647	1161	496	1240	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	65	0	71	0	647	1161	496	1240	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	65	0	71	0	647	1161	496	1240	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	65	0	71	0	647	1161	496	1240	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.88	1.00	0.77	1.00	0.82	0.82	0.95	0.91	1.00
Lanes:	0.00	0.00	0.00	1.44	0.00	1.56	0.00	2.00	1.00	1.00	3.00	0.00
Final Sat.:	0	0	0	2417	0	2277	0	3126	1563	1805	5187	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.03	0.00	0.03	0.00	0.21	0.74	0.27	0.24	0.00
Crit Moves:				****					****	****		
Green/Cycle:	0.00	0.00	0.00	0.02	0.00	0.05	0.00	0.68	0.68	0.25	0.93	0.00
Volume/Cap:	0.00	0.00	0.00	0.90	0.00	0.65	0.00	0.31	1.10	1.10	0.26	0.00
Delay/Veh:	0.0	0.0	0.0	94.5	0.0	54.0	0.0	6.6	70.6	109.4	0.4	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	94.5	0.0	54.0	0.0	6.6	70.6	109.4	0.4	0.0
LOS by Move:	A	A	A	F	A	D	A	A	E	F	A	A

```
HCM2k85thQ:     0    0     0     5    0     4     0    7    77    36    2     0
********************************************************************************
Note: Queue reported is the number of cars per lane.
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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #115 SR-55 NB Ramps and Irvine Boulevard

Cycle (sec):	100	Critical Vol./Cap.(X):	0.906
Loss Time (sec):	5	Average Delay (sec/veh):	23.5
Optimal Cycle:	93	Level Of Service:	C

Street Name:	SR-55 NB Ramps						Irvine Boulevard					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	1! 0	1	0	0	0	0	1	0	3	0	0	0	0	2	1	0
Volume Module:												
Base Vol:	440	0	423	0	0	0	222	526	0	0	1241	863
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	440	0	423	0	0	0	222	526	0	0	1241	863
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	440	0	423	0	0	0	222	526	0	0	1241	863
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	440	0	423	0	0	0	222	526	0	0	1241	863
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	440	0	423	0	0	0	222	526	0	0	1241	863
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.75	1.00	0.74	1.00	1.00	1.00	0.95	0.91	1.00	1.00	0.85	0.85
Lanes:	1.51	0.00	1.49	0.00	0.00	0.00	1.00	3.00	0.00	0.00	2.00	1.00
Final Sat.:	2135	0	2097	0	0	0	1805	5187	0	0	3247	1624
Capacity Analysis Module:												
Vol/Sat:	0.21	0.00	0.20	0.00	0.00	0.00	0.12	0.10	0.00	0.00	0.38	0.53
Crit Moves:	****						****					****
Green/Cycle:	0.23	0.00	0.45	0.00	0.00	0.00	0.14	0.72	0.00	0.00	0.59	0.59
Volume/Cap:	0.91	0.00	0.44	0.00	0.00	0.00	0.91	0.14	0.00	0.00	0.65	0.91
Delay/Veh:	49.8	0.0	18.8	0.0	0.0	0.0	76.1	4.3	0.0	0.0	14.3	23.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	49.8	0.0	18.8	0.0	0.0	0.0	76.1	4.3	0.0	0.0	14.3	23.8
LOS by Move:	D	A	B	A	A	A	E	A	A	A	B	C
HCM2k85thQ:	18	0	10	0	0	0	15	3	0	0	21	42

Note: Queue reported is the number of cars per lane.

APPENDIX D.4

ANALYSIS WORKSHEETS –
2045 NP (PM Peak Hour)

Santa Ana Circulation Element

Scenario Report

Scenario:	2045 NP PM Peak Hour
Command:	Default Command
Volume:	2045 NP PM
Geometry:	MPAH
Impact Fee:	Default Impact Fee
Trip Generation:	Default Trip Generation
Trip Distribution:	Default Trip Distribution
Paths:	Default Path
Routes:	Default Route
Configuration:	Default Configuration

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Santa Ana Circulation Element

Impact Analysis Report
Level Of Service

Intersection	Base LOS	Base Del/ Veh	Base V/ C	Future LOS	Future Del/ Veh	Future V/ C	Change in
# 1 Euclid St and 1st St	D	xxxxx	0.857	D	xxxxx	0.857	+ 0.000 V/C
# 2 Euclid St and McFadden Ave	D	xxxxx	0.896	D	xxxxx	0.896	+ 0.000 V/C
# 3 Euclid St and Edinger Ave	D	xxxxx	0.888	D	xxxxx	0.888	+ 0.000 V/C
# 4 Newhope St and Hazard Ave	C	xxxxx	0.721	C	xxxxx	0.721	+ 0.000 V/C
# 5 Newhope St and McFadden Ave	C	xxxxx	0.777	C	xxxxx	0.777	+ 0.000 V/C
# 6 Harbor Blvd and SR-22 WB Off-R	C	24.2	0.802	C	24.2	0.802	+ 0.000 D/V
# 7 Trask Avenue and SR-22 EB On-R	B	13.6	0.533	B	13.6	0.533	+ 0.000 D/V
# 8 Harbor Blvd and Westminster Av	D	xxxxx	0.882	D	xxxxx	0.882	+ 0.000 V/C
# 9 Harbor Blvd and 1st St	C	xxxxx	0.778	C	xxxxx	0.778	+ 0.000 V/C
# 10 Harbor Blvd and McFadden Ave	D	xxxxx	0.812	D	xxxxx	0.812	+ 0.000 V/C
# 11 Harbor Blvd and Edinger Ave	C	xxxxx	0.725	C	xxxxx	0.725	+ 0.000 V/C
# 12 Harbor Blvd and Warner Ave	F	xxxxx	1.536	F	xxxxx	1.536	+ 0.000 V/C
# 13 Harbor Blvd and Segerstrom Ave	E	xxxxx	0.921	E	xxxxx	0.921	+ 0.000 V/C
# 14 MacArthur Blvd and Hyland Ave	E	xxxxx	0.917	E	xxxxx	0.917	+ 0.000 V/C
# 15 MacArthur Blvd and Harbor Blvd	C	xxxxx	0.783	C	xxxxx	0.783	+ 0.000 V/C
# 16 Harbor Blvd and Sunflower Ave	C	xxxxx	0.717	C	xxxxx	0.717	+ 0.000 V/C
# 17 Harbor Blvd and I-405 NB Off-R	B	18.3	0.699	B	18.3	0.699	+ 0.000 D/V
# 18 Harbor Blvd and I-405 SB Off-R	B	17.9	0.667	B	17.9	0.667	+ 0.000 D/V
# 19 Fairview St and Civic Center D	C	xxxxx	0.757	C	xxxxx	0.757	+ 0.000 V/C
# 20 Fairview St and 1st St	D	xxxxx	0.898	D	xxxxx	0.898	+ 0.000 V/C
# 21 Fairview St and McFadden Ave	C	xxxxx	0.776	C	xxxxx	0.776	+ 0.000 V/C
# 22 Fairview St and Edinger Ave	E	xxxxx	0.916	E	xxxxx	0.916	+ 0.000 V/C
# 23 Fairview St and Warner Ave	C	xxxxx	0.796	C	xxxxx	0.796	+ 0.000 V/C
# 24 Fairview St and MacArthur Blvd	C	xxxxx	0.768	C	xxxxx	0.768	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Base Del/ Veh	Base V/ C	Future LOS	Future Del/ Veh	Future V/ C	Change in
# 25 Fairview Rd and Sunflower Ave	C	xxxxx	0.715	C	xxxxx	0.715	+ 0.000 V/C
# 26 Greenville St and Edinger Ave	A	xxxxx	0.595	A	xxxxx	0.595	+ 0.000 V/C
# 27 Greenville St and Segerstrom A	C	xxxxx	0.777	C	xxxxx	0.777	+ 0.000 V/C
# 28 Raitt St and McFadden Ave	C	xxxxx	0.758	C	xxxxx	0.758	+ 0.000 V/C
# 29 Raitt St and Edinger Ave	E	xxxxx	0.986	E	xxxxx	0.986	+ 0.000 V/C
# 30 Bear St and MacArthur Blvd	D	xxxxx	0.851	D	xxxxx	0.851	+ 0.000 V/C
# 31 Bristol St and 17th St	D	xxxxx	0.823	D	xxxxx	0.823	+ 0.000 V/C
# 32 Bristol St and Civic Center Dr	E	xxxxx	0.911	E	xxxxx	0.911	+ 0.000 V/C
# 33 Bristol St and Santa Ana Blvd	D	xxxxx	0.853	D	xxxxx	0.853	+ 0.000 V/C
# 34 Bristol St and 1st St	D	xxxxx	0.826	D	xxxxx	0.826	+ 0.000 V/C
# 35 Bristol St and McFadden Ave	F	xxxxx	1.028	F	xxxxx	1.028	+ 0.000 V/C
# 36 Bristol St and Warner Ave	D	xxxxx	0.847	D	xxxxx	0.847	+ 0.000 V/C
# 37 Bristol St and Segerstrom Ave	D	xxxxx	0.870	D	xxxxx	0.870	+ 0.000 V/C
# 38 Bristol St and Alton Ave	B	xxxxx	0.633	B	xxxxx	0.633	+ 0.000 V/C
# 39 Bristol St and MacArthur Blvd	C	xxxxx	0.765	C	xxxxx	0.765	+ 0.000 V/C
# 40 Bristol St and Sunflower Ave	B	xxxxx	0.692	B	xxxxx	0.692	+ 0.000 V/C
# 41 Bristol St and I-405 NB Ramps	C	28.5	0.825	C	28.5	0.825	+ 0.000 D/V
# 42 Bristol St and I-405 SB Ramps	B	18.3	0.617	B	18.3	0.617	+ 0.000 D/V
# 43 Flower St and Santa Ana Blvd	D	xxxxx	0.857	D	xxxxx	0.857	+ 0.000 V/C
# 44 Flower St and 1st St	D	xxxxx	0.877	D	xxxxx	0.877	+ 0.000 V/C
# 45 Flower St and McFadden Ave	F	xxxxx	1.037	F	xxxxx	1.037	+ 0.000 V/C
# 46 Flower St and Segerstrom Ave	D	xxxxx	0.866	D	xxxxx	0.866	+ 0.000 V/C
# 47 Flower St and MacArthur Blvd	E	xxxxx	0.900	E	xxxxx	0.900	+ 0.000 V/C
# 48 Main St and La Veta Ave	B	xxxxx	0.696	B	xxxxx	0.696	+ 0.000 V/C
# 49 Main St and Mainplace Dr / Mem	A	xxxxx	0.556	A	xxxxx	0.556	+ 0.000 V/C
# 50 Main St and 17th St	C	xxxxx	0.760	C	xxxxx	0.760	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Base Del/ Veh	Base V/ C	Future LOS	Future Del/ Veh	Future V/ C	Change in
# 51 Main St and Civic Center Dr	E	xxxxx	0.965	E	xxxxx	0.965	+ 0.000 V/C
# 52 Main St and Santa Ana Blvd	C	xxxxx	0.792	C	xxxxx	0.792	+ 0.000 V/C
# 53 Main St and 4th St	A	xxxxx	0.447	A	xxxxx	0.447	+ 0.000 V/C
# 54 Main St and 1st St	C	xxxxx	0.744	C	xxxxx	0.744	+ 0.000 V/C
# 55 Main St and McFadden Ave	E	xxxxx	0.925	E	xxxxx	0.925	+ 0.000 V/C
# 56 Main St and Edinger Ave	D	xxxxx	0.867	D	xxxxx	0.867	+ 0.000 V/C
# 57 Main St and MacArthur Blvd	C	xxxxx	0.749	C	xxxxx	0.749	+ 0.000 V/C
# 58 Penn Wy and 17th St	C	25.5	0.872	C	25.5	0.872	+ 0.000 D/V
# 59 I-5 NB Off Ramps/17th Street	B	19.0	0.587	B	19.0	0.587	+ 0.000 D/V
# 60 Penn Wy and I-5 SB Ramps	C	23.0	0.573	C	23.0	0.573	+ 0.000 D/V
# 61 Santiago St and Civic Center D	F	110.0	1.369	F	110.0	1.369	+ 0.000 V/C
# 62 Santiago St and Santa Ana Blvd	F	xxxxx	1.017	F	xxxxx	1.017	+ 0.000 V/C
# 63 Standard Ave and 4th St	D	xxxxx	0.893	D	xxxxx	0.893	+ 0.000 V/C
# 64 Standard Ave and 1st St	E	xxxxx	0.968	E	xxxxx	0.968	+ 0.000 V/C
# 65 Standard Ave and Mcfadden Ave	D	xxxxx	0.828	D	xxxxx	0.828	+ 0.000 V/C
# 66 Halladay St and Warner Ave	E	xxxxx	0.975	E	xxxxx	0.975	+ 0.000 V/C
# 67 Halladay St and Dyer Rd	F	xxxxx	1.057	F	xxxxx	1.057	+ 0.000 V/C
# 68 SR-55 SB Ramps and MacArthur B	B	17.8	0.567	B	17.8	0.567	+ 0.000 D/V
# 69 SR-55 NB Ramps and MacArthur B	B	14.6	0.484	B	14.6	0.484	+ 0.000 D/V
# 70 SR-55 SB Ramps and Dyer Rd	C	26.5	0.663	C	26.5	0.663	+ 0.000 D/V
# 71 Glassell St and La Veta Ave	C	xxxxx	0.744	C	xxxxx	0.744	+ 0.000 V/C
# 72 Glassell St and SR-22 WB Ramps	C	30.5	0.910	C	30.5	0.910	+ 0.000 D/V
# 73 Grand Ave / Glassell St and SR	C	31.6	0.931	C	31.6	0.931	+ 0.000 D/V
# 74 Grand Ave and Fairhaven Ave	C	xxxxx	0.717	C	xxxxx	0.717	+ 0.000 V/C
# 75 Grand Ave and Santa Clara Ave	D	xxxxx	0.808	D	xxxxx	0.808	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Base Del/ Veh	Base V/ C	Future LOS	Future Del/ Veh	Future V/ C	Change in
# 76 Grand Ave and 17th St	D	xxxxx	0.856	D	xxxxx	0.856	+ 0.000 V/C
# 77 Grand Ave and I-5 NB Ramps	B	10.1	0.696	B	10.1	0.696	+ 0.000 D/V
# 78 Grand Ave and Santa Ana Blvd	C	25.3	0.721	C	25.3	0.721	+ 0.000 D/V
# 79 Grand Ave and 1st St	C	xxxxx	0.731	C	xxxxx	0.731	+ 0.000 V/C
# 80 Grand Ave and Chestnut Ave	B	xxxxx	0.688	B	xxxxx	0.688	+ 0.000 V/C
# 81 Grand Ave and McFadden Ave	D	xxxxx	0.821	D	xxxxx	0.821	+ 0.000 V/C
# 82 Grand Ave and Edinger Ave	F	xxxxx	1.004	F	xxxxx	1.004	+ 0.000 V/C
# 83 Grand Ave and Warner Ave	D	xxxxx	0.827	D	xxxxx	0.827	+ 0.000 V/C
# 84 SR-55 NB Ramps and Dyer Rd	A	5.7	0.362	A	5.7	0.362	+ 0.000 D/V
# 85 Cambridge St and La Veta Ave	C	19.7	0.779	C	19.7	0.779	+ 0.000 V/C
# 86 Cambridge St and Fairhaven Ave	A	xxxxx	0.454	A	xxxxx	0.454	+ 0.000 V/C
# 87 Mabury St and 1st Street	C	27.1	0.674	C	27.1	0.674	+ 0.000 D/V
# 88 Tustin St and La Veta Ave	A	xxxxx	0.381	A	xxxxx	0.381	+ 0.000 V/C
# 89 Tustin St and SR-22 WB On-Ramp	B	11.1	0.391	B	11.1	0.391	+ 0.000 D/V
# 90 Tustin St and SR-22 EB Off-Ram	C	23.1	0.679	C	23.1	0.679	+ 0.000 D/V
# 91 Tustin Ave and Fairhaven Ave	B	xxxxx	0.685	B	xxxxx	0.685	+ 0.000 V/C
# 92 Tustin Ave and Santa Clara Ave	C	xxxxx	0.709	C	xxxxx	0.709	+ 0.000 V/C
# 93 Tustin Ave and 17th St	C	xxxxx	0.797	C	xxxxx	0.797	+ 0.000 V/C
# 94 Tustin Ave and 4th St	C	xxxxx	0.708	C	xxxxx	0.708	+ 0.000 V/C
# 95 SR-55 SB Ramps / Auto Mall Dr	C	25.5	0.702	C	25.5	0.702	+ 0.000 D/V
# 96 SR-55 NB Ramps / Del Amo Ave a	D	39.1	0.895	D	39.1	0.895	+ 0.000 D/V
# 97 Red Hill Ave and Edinger Ave	D	xxxxx	0.853	D	xxxxx	0.853	+ 0.000 V/C
# 98 Red Hill Ave and Warner Ave	D	xxxxx	0.875	D	xxxxx	0.875	+ 0.000 V/C
# 99 Red Hill Ave and Dyer Rd / bar	A	xxxxx	0.537	A	xxxxx	0.537	+ 0.000 V/C
#100 Red Hill Ave and Alton Pkwy	F	xxxxx	1.184	F	xxxxx	1.184	+ 0.000 V/C
#101 Red Hill Ave and MacArthur Blv	C	xxxxx	0.702	C	xxxxx	0.702	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Del/ Veh	V/ C	Future LOS	Del/ Veh	V/ C	Change in
#102 Red Hill Ave and Main St	D	xxxxx	0.813	D	xxxxx	0.813	+ 0.000 V/C
#103 I-5 SB Ramps and Santa Ana Blv	B	18.4	0.487	B	18.4	0.487	+ 0.000 D/V
#104 Tustin Ranch Rd and Warner Ave	B	xxxxx	0.645	B	xxxxx	0.645	+ 0.000 V/C
#105 Von Karman Ave and Barranca Pk	F	xxxxx	1.228	F	xxxxx	1.228	+ 0.000 V/C
#106 Red Hill Avenue and El Camino	C	xxxxx	0.779	C	xxxxx	0.779	+ 0.000 V/C
#107 Red Hill Avenue and I-5 NB Ram	C	23.7	0.634	C	23.7	0.634	+ 0.000 D/V
#108 Red Hill Avenue and I-5 SB Ram	C	22.2	0.695	C	22.2	0.695	+ 0.000 D/V
#109 Red Hill Avenue and Nisson Roa	B	xxxxx	0.693	B	xxxxx	0.693	+ 0.000 V/C
#110 Red Hill Avenue and Walnut Ave	C	xxxxx	0.739	C	xxxxx	0.739	+ 0.000 V/C
#111 Red Hill Avenue and Valencia A	C	xxxxx	0.738	C	xxxxx	0.738	+ 0.000 V/C
#112 Tustin Ranch Road and Warner A	D	xxxxx	0.885	D	xxxxx	0.885	+ 0.000 V/C
#113 Tustin Ranch Road and Walnut A	F	xxxxx	1.715	F	xxxxx	1.715	+ 0.000 V/C
#114 SR-55 SB Ramps and Irvine Boul	C	25.6	0.915	C	25.6	0.915	+ 0.000 D/V
#115 SR-55 NB Ramps and Irvine Boul	C	22.9	0.932	C	22.9	0.932	+ 0.000 D/V

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #1 Euclid St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.857
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	67	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			WideBypass			WideBypass		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	169	1569	126	152	866	164	212	678	106	209	877	209
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	169	1569	126	152	866	164	212	678	106	209	877	209
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	169	1569	126	152	866	164	212	678	106	209	877	209
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	169	1569	126	152	866	164	212	678	106	209	877	209
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	169	1569	126	152	866	164	212	678	106	209	877	209
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.78	0.22	1.00	2.52	0.48	1.00	2.59	0.41	1.00	2.42	0.58
Final Sat.:	1600	4643	357	1600	4236	764	1600	4351	649	1600	4076	924
Capacity Analysis Module:												
Vol/Sat:	0.11	0.34	0.35	0.10	0.20	0.21	0.13	0.16	0.16	0.13	0.22	0.23
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #2 Euclid St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.896
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	84	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	242	1973	127	96	1179	143	207	475	210	114	479	221
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	242	1973	127	96	1179	143	207	475	210	114	479	221
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	242	1973	127	96	1179	143	207	475	210	114	479	221
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	242	1973	127	96	1179	143	207	475	210	114	479	221
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	242	1973	127	96	1179	143	207	475	210	114	479	221
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.82	0.18	1.00	2.68	0.32	1.00	1.39	0.61	1.00	1.37	0.63
Final Sat.:	1600	4710	290	1600	4481	519	1600	2319	981	1600	2290	1010
Capacity Analysis Module:												
Vol/Sat:	0.15	0.42	0.44	0.06	0.26	0.28	0.13	0.20	0.21	0.07	0.21	0.22
Crit Moves:		****	****				****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #3 Euclid St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.888
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	80	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	1 0	2 0	1	1 0	2 0	1
Volume Module:												
Base Vol:	539	1610	115	136	897	228	141	544	163	128	1181	191
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	539	1610	115	136	897	228	141	544	163	128	1181	191
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	539	1610	115	136	897	228	141	544	163	128	1181	191
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	539	1610	115	136	897	228	141	544	163	128	1181	191
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	539	1610	115	136	897	228	141	544	163	128	1181	191
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.80	0.20	2.00	2.39	0.61	1.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	3200	4680	320	3200	4027	973	1600	3400	1600	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.17	0.34	0.36	0.04	0.22	0.23	0.09	0.16	0.10	0.08	0.35	0.12
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #4 Newhope St and Hazard Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.721
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	40	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Prot+Permit			Prot+Permit			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	105	1159	45	46	982	117	75	148	91	88	556	160
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	105	1159	45	46	982	117	75	148	91	88	556	160
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	105	1159	45	46	982	117	75	148	91	88	556	160
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	105	1159	45	46	982	117	75	148	91	88	556	160
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	105	1159	45	46	982	117	75	148	91	88	556	160
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.05	1.00	1.00	1.04	1.00
Lanes:	1.00	1.93	0.07	1.00	1.79	0.21	1.00	1.24	0.76	1.00	1.55	0.45
Final Sat.:	1600	3180	120	1600	2959	341	1600	2082	1218	1600	2585	715
Capacity Analysis Module:												
Vol/Sat:	0.07	0.36	0.38	0.03	0.33	0.34	0.05	0.07	0.07	0.06	0.22	0.22
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #5 Newhope St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.777
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	48	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	188	1009	177	140	625	121	61	372	54	104	574	164
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	188	1009	177	140	625	121	61	372	54	104	574	164
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	188	1009	177	140	625	121	61	372	54	104	574	164
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	188	1009	177	140	625	121	61	372	54	104	574	164
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	188	1009	177	140	625	121	61	372	54	104	574	164
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	1.70	0.30	1.00	1.68	0.32	1.00	1.75	0.25	1.00	1.56	0.44
Final Sat.:	1600	2822	478	1600	2781	519	1600	2894	406	1600	2589	711
Capacity Analysis Module:												
Vol/Sat:	0.12	0.36	0.37	0.09	0.22	0.23	0.04	0.13	0.13	0.07	0.22	0.23
Crit Moves:		****	****				****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #6 Harbor Blvd and SR-22 WB Off-Ramp / Banner Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.802
Loss Time (sec):	5	Average Delay (sec/veh):	24.2
Optimal Cycle:	55	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	0	0 0	2 1	0	0 0	1! 0	0	1 1	0 0	1
Volume Module:												
Base Vol:	109	2457	0	0	1801	58	102	0	86	576	78	165
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	109	2457	0	0	1801	58	102	0	86	576	78	165
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	109	2457	0	0	1801	58	102	0	86	576	78	165
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	109	2457	0	0	1801	58	102	0	86	576	78	165
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	109	2457	0	0	1801	58	102	0	86	576	78	165
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	1.00	1.00	0.91	0.91	0.91	1.00	0.91	0.96	0.96	0.85
Lanes:	1.00	3.00	0.00	0.00	2.91	0.09	0.54	0.00	0.46	1.76	0.24	1.00
Final Sat.:	1805	5187	0	0	5000	161	942	0	794	3206	434	1615
Capacity Analysis Module:												
Vol/Sat:	0.06	0.47	0.00	0.00	0.36	0.36	0.11	0.00	0.11	0.18	0.18	0.10
Crit Moves:		****		****			****			****		
Green/Cycle:	0.08	0.59	0.00	0.00	0.51	0.51	0.14	0.00	0.14	0.22	0.22	0.22
Volume/Cap:	0.71	0.80	0.00	0.00	0.71	0.71	0.80	0.00	0.80	0.80	0.80	0.46
Delay/Veh:	59.1	17.5	0.0	0.0	20.0	20.0	59.6	0.0	59.6	42.4	42.4	34.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	59.1	17.5	0.0	0.0	20.0	20.0	59.6	0.0	59.6	42.4	42.4	34.4
LOS by Move:	E	B	A	A	C	C	E	A	E	D	D	C
HCM2k85thQ:	7	33	0	0	25	25	12	0	12	18	18	7

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #7 Trask Avenue and SR-22 EB On-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.533
Loss Time (sec):	5	Average Delay (sec/veh):	13.6
Optimal Cycle:	26	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	0 0	0	0 0	0 0	0	2 0	1 0	0	0 0	1 1	0
Volume Module:												
Base Vol:	0	0	0	0	0	0	927	695	0	0	786	74
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	0	0	0	927	695	0	0	786	74
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	0	0	0	927	695	0	0	786	74
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	0	0	0	927	695	0	0	786	74
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	0	0	0	927	695	0	0	786	74
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	0.92	1.00	1.00	1.00	0.94	0.94
Lanes:	0.00	0.00	0.00	0.00	0.00	0.00	2.00	1.00	0.00	0.00	1.83	0.17
Final Sat.:	0	0	0	0	0	0	3502	1900	0	0	3256	307
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.00	0.00	0.00	0.26	0.37	0.00	0.00	0.24	0.24
Crit Moves:							****				****	
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.50	0.95	0.00	0.00	0.45	0.45
Volume/Cap:	0.00	0.00	0.00	0.00	0.00	0.00	0.53	0.39	0.00	0.00	0.53	0.53
Delay/Veh:	0.0	0.0	0.0	0.0	0.0	0.0	17.5	0.3	0.0	0.0	20.1	20.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	0.0	0.0	0.0	17.5	0.3	0.0	0.0	20.1	20.1
LOS by Move:	A	A	A	A	A	A	B	A	A	A	C	C
HCM2k85thQ:	0	0	0	0	0	0	15	3	0	0	15	15

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #8 Harbor Blvd and Westminster Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.882
Loss Time (sec):	0	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	2 1	0	1 0	3 0	1
Volume Module:												
Base Vol:	237	1178	279	254	1752	202	215	768	37	357	1238	281
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	237	1178	279	254	1752	202	215	768	37	357	1238	281
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	237	1178	279	254	1752	202	215	768	37	357	1238	281
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	237	1178	279	254	1752	202	215	768	37	357	1238	281
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	237	1178	279	254	1752	202	215	768	37	357	1238	281
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	2.86	0.14	1.00	3.00	1.00
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	4779	221	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.15	0.23	0.17	0.16	0.34	0.13	0.13	0.16	0.17	0.22	0.24	0.18
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #9 Harbor Blvd and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.778
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	48	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	290	1497	166	245	1073	145	179	730	138	199	929	151
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	290	1497	166	245	1073	145	179	730	138	199	929	151
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	290	1497	166	245	1073	145	179	730	138	199	929	151
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	290	1497	166	245	1073	145	179	730	138	199	929	151
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	290	1497	166	245	1073	145	179	730	138	199	929	151
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	2.00	2.52	0.48	2.00	2.58	0.42
Final Sat.:	1600	5100	1600	1600	5100	1600	3200	4237	763	3200	4329	671
Capacity Analysis Module:												
Vol/Sat:	0.18	0.29	0.10	0.15	0.21	0.09	0.06	0.17	0.18	0.06	0.21	0.23
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #10 Harbor Blvd and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.812
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	55	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	169	1737	127	256	1094	99	241	585	73	198	474	137
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	169	1737	127	256	1094	99	241	585	73	198	474	137
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	169	1737	127	256	1094	99	241	585	73	198	474	137
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	169	1737	127	256	1094	99	241	585	73	198	474	137
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	169	1737	127	256	1094	99	241	585	73	198	474	137
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	2.00	3.00	1.00	2.00	2.75	0.25	1.00	1.78	0.22	1.00	1.55	0.45
Final Sat.:	3200	5100	1600	3200	4602	398	1600	2945	355	1600	2582	718
Capacity Analysis Module:												
Vol/Sat:	0.05	0.34	0.08	0.08	0.24	0.25	0.15	0.20	0.21	0.12	0.18	0.19
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #11 Harbor Blvd and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.725
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	40	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	2 0	2 1	0	2 0	3 0	1
Volume Module:												
Base Vol:	254	1597	193	290	843	77	109	460	81	196	1138	379
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	254	1597	193	290	843	77	109	460	81	196	1138	379
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	254	1597	193	290	843	77	109	460	81	196	1138	379
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	254	1597	193	290	843	77	109	460	81	196	1138	379
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	254	1597	193	290	843	77	109	460	81	196	1138	379
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	2.55	0.45	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	4281	719	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.08	0.31	0.12	0.09	0.17	0.05	0.03	0.11	0.11	0.06	0.22	0.24
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #12 Harbor Blvd and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	1.536
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	762	3656	275	262	1457	174	217	1539	352	249	2589	827
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	762	3656	275	262	1457	174	217	1539	352	249	2589	827
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	762	3656	275	262	1457	174	217	1539	352	249	2589	827
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	762	3656	275	262	1457	174	217	1539	352	249	2589	827
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	762	3656	275	262	1457	174	217	1539	352	249	2589	827
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.79	0.21	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	4664	336	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.24	0.78	0.82	0.08	0.29	0.11	0.07	0.30	0.22	0.08	0.51	0.52
Crit Moves:		****	****				****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #13 Harbor Blvd and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.921
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	100	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	1 0	2 1	0	1 0	1 1	0	1 0	2 0	1
Volume Module:												
Base Vol:	273	1717	55	87	986	87	118	585	192	117	1269	461
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	273	1717	55	87	986	87	118	585	192	117	1269	461
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	273	1717	55	87	986	87	118	585	192	117	1269	461
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	273	1717	55	87	986	87	118	585	192	117	1269	461
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	273	1717	55	87	986	87	118	585	192	117	1269	461
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	2.91	0.09	1.00	2.76	0.24	1.00	1.51	0.49	1.00	2.00	1.00
Final Sat.:	3200	4851	149	1600	4611	389	1600	2509	791	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.09	0.35	0.37	0.05	0.21	0.22	0.07	0.23	0.24	0.07	0.37	0.29
Crit Moves:			****	****			****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #14 MacArthur Blvd and Hyland Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.917
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	98	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	0 1	0	1 0	1 0	1	1 0	3 0	1	1 0	3 0	1
Volume Module:												
Base Vol:	1201	7	70	12	4	14	36	891	160	25	2350	12
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	1201	7	70	12	4	14	36	891	160	25	2350	12
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	1201	7	70	12	4	14	36	891	160	25	2350	12
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	1201	7	70	12	4	14	36	891	160	25	2350	12
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	1201	7	70	12	4	14	36	891	160	25	2350	12
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	0.09	0.91	1.00	1.00	1.00	1.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	145	1455	1600	1700	1600	1600	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.38	0.05	0.05	0.01	0.00	0.01	0.02	0.17	0.10	0.02	0.46	0.01
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #15 MacArthur Blvd and Harbor Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.783
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	49	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	1 0	3 0	1	1 0	3 0	1
Volume Module:												
Base Vol:	558	1441	83	204	1028	169	140	489	181	81	1373	258
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	558	1441	83	204	1028	169	140	489	181	81	1373	258
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	558	1441	83	204	1028	169	140	489	181	81	1373	258
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	558	1441	83	204	1028	169	140	489	181	81	1373	258
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	558	1441	83	204	1028	169	140	489	181	81	1373	258
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	1.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	1600	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.17	0.28	0.05	0.06	0.20	0.11	0.09	0.10	0.11	0.05	0.27	0.16
Crit Moves:	****				****		****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #16 Harbor Blvd and Sunflower Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.717
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	39	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	1 0	1 1	0	1 1	0 1	0
Volume Module:												
Base Vol:	154	1532	228	114	1417	43	81	220	229	301	632	236
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	154	1532	228	114	1417	43	81	220	229	301	632	236
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	154	1532	228	114	1417	43	81	220	229	301	632	236
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	154	1532	228	114	1417	43	81	220	229	301	632	236
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	154	1532	228	114	1417	43	81	220	229	301	632	236
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	1.00	1.00	1.00	1.00	1.39	0.61
Final Sat.:	3200	5100	1600	3200	5100	1600	1600	1700	1600	1600	2322	974
Capacity Analysis Module:												
Vol/Sat:	0.05	0.30	0.14	0.04	0.28	0.03	0.05	0.13	0.14	0.19	0.27	0.24
Crit Moves:		****			****				****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #17 Harbor Blvd and I-405 NB Off-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.699
Loss Time (sec):	5	Average Delay (sec/veh):	18.3
Optimal Cycle:	39	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 4	0 0	0	0 4	0 0	0	0 0	0 0	1	0 1!	0 1
Volume Module:												
Base Vol:	0	1529	0	0	2570	0	0	0	0	496	0	801
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1529	0	0	2570	0	0	0	0	496	0	801
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1529	0	0	2570	0	0	0	0	496	0	801
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1529	0	0	2570	0	0	0	0	496	0	801
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1529	0	0	2570	0	0	0	0	496	0	801
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	1.00	1.00	0.91	1.00	1.00	1.00	1.00	0.89	1.00	0.89
Lanes:	0.00	4.00	0.00	0.00	4.00	0.00	0.00	0.00	0.00	1.38	0.00	1.62
Final Sat.:	0	6916	0	0	6916	0	0	0	0	2337	0	2735
Capacity Analysis Module:												
Vol/Sat:	0.00	0.22	0.00	0.00	0.37	0.00	0.00	0.00	0.00	0.21	0.00	0.29
Crit Moves:	****				****							****
Green/Cycle:	0.00	0.53	0.00	0.00	0.53	0.00	0.00	0.00	0.00	0.42	0.00	0.42
Volume/Cap:	0.00	0.42	0.00	0.00	0.70	0.00	0.00	0.00	0.00	0.51	0.00	0.70
Delay/Veh:	0.0	14.2	0.0	0.0	18.1	0.0	0.0	0.0	0.0	21.6	0.0	25.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	14.2	0.0	0.0	18.1	0.0	0.0	0.0	0.0	21.6	0.0	25.1
LOS by Move:	A	B	A	A	B	A	A	A	A	C	A	C
HCM2k85thQ:	0	12	0	0	25	0	0	0	0	13	0	20

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #18 Harbor Blvd and I-405 SB Off-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.667
Loss Time (sec):	5	Average Delay (sec/veh):	17.9
Optimal Cycle:	35	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Ignore			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 3	0 1	0	0 4	0 0	1	0 1!	0 1	0	0 0	0 0
Volume Module:												
Base Vol:	0	1881	0	0	2281	0	177	0	815	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1881	0	0	2281	0	177	0	815	0	0	0
User Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1881	0	0	2281	0	177	0	815	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1881	0	0	2281	0	177	0	815	0	0	0
PCE Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1881	0	0	2281	0	177	0	815	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	1.00	1.00	0.91	1.00	0.87	1.00	0.87	1.00	1.00	1.00
Lanes:	0.00	3.00	1.00	0.00	4.00	0.00	1.18	0.00	1.82	0.00	0.00	0.00
Final Sat.:	0	5187	1900	0	6916	0	1946	0	3008	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.36	0.00	0.00	0.33	0.00	0.09	0.00	0.27	0.00	0.00	0.00
Crit Moves:		****			****				****			
Green/Cycle:	0.00	0.54	0.00	0.00	0.54	0.00	0.41	0.00	0.41	0.00	0.00	0.00
Volume/Cap:	0.00	0.67	0.00	0.00	0.61	0.00	0.22	0.00	0.67	0.00	0.00	0.00
Delay/Veh:	0.0	17.0	0.0	0.0	15.8	0.0	19.4	0.0	25.3	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	17.0	0.0	0.0	15.8	0.0	19.4	0.0	25.3	0.0	0.0	0.0
LOS by Move:	A	B	A	A	B	A	B	A	C	A	A	A
HCM2k85thQ:	0	23	0	0	20	0	5	0	18	0	0	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #19 Fairview St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.757
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	44	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	0 1	0	1 0	1 1	0	0 1
Volume Module:												
Base Vol:	6	1690	475	192	1784	0	2	6	4	424	0	157
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	6	1690	475	192	1784	0	2	6	4	424	0	157
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	6	1690	475	192	1784	0	2	6	4	424	0	157
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	6	1690	475	192	1784	0	2	6	4	424	0	157
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	6	1690	475	192	1784	0	2	6	4	424	0	157
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.00	1.00	1.00	1.06	1.00
Lanes:	1.00	2.34	0.66	1.00	3.00	0.00	0.33	1.00	0.67	2.00	0.00	1.00
Final Sat.:	1600	3947	1053	1600	5000	0	533	1600	1067	3200	0	1600
Capacity Analysis Module:												
Vol/Sat:	0.00	0.43	0.45	0.12	0.36	0.00	0.00	0.00	0.00	0.13	0.00	0.10
Crit Moves:			****	****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #20 Fairview St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.898
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	85	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	2 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	169	1241	112	236	1468	291	193	781	136	140	1060	164
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	169	1241	112	236	1468	291	193	781	136	140	1060	164
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	169	1241	112	236	1468	291	193	781	136	140	1060	164
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	169	1241	112	236	1468	291	193	781	136	140	1060	164
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	169	1241	112	236	1468	291	193	781	136	140	1060	164
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	3.00	1.00	1.00	2.50	0.50	1.00	2.56	0.44	1.00	2.60	0.40
Final Sat.:	1600	5100	1600	1600	4206	794	1600	4288	712	1600	4357	643
Capacity Analysis Module:												
Vol/Sat:	0.11	0.24	0.07	0.15	0.35	0.37	0.12	0.18	0.19	0.09	0.24	0.26
Crit Moves:	****					****	****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #21 Fairview St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.776
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	47	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	164	1507	131	191	1553	242	158	427	106	158	545	134
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	164	1507	131	191	1553	242	158	427	106	158	545	134
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	164	1507	131	191	1553	242	158	427	106	158	545	134
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	164	1507	131	191	1553	242	158	427	106	158	545	134
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	164	1507	131	191	1553	242	158	427	106	158	545	134
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	1.60	0.40	1.00	1.61	0.39
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	2664	636	1600	2668	632
Capacity Analysis Module:												
Vol/Sat:	0.10	0.30	0.08	0.12	0.30	0.15	0.10	0.16	0.17	0.10	0.20	0.21
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #22 Fairview St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.916
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	97	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	184	1759	108	204	635	156	205	628	168	650	1375	302
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	184	1759	108	204	635	156	205	628	168	650	1375	302
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	184	1759	108	204	635	156	205	628	168	650	1375	302
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	184	1759	108	204	635	156	205	628	168	650	1375	302
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	184	1759	108	204	635	156	205	628	168	650	1375	302
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	2.00	2.83	0.17	2.00	2.41	0.59	2.00	2.37	0.63	2.00	2.46	0.54
Final Sat.:	3200	4722	278	3200	4053	947	3200	3987	1013	3200	4136	864
Capacity Analysis Module:												
Vol/Sat:	0.06	0.37	0.39	0.06	0.16	0.16	0.06	0.16	0.17	0.20	0.33	0.35
Crit Moves:		****	****				****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #23 Fairview St and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.796
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	51	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	213	1695	158	182	921	93	168	815	126	168	989	214
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	213	1695	158	182	921	93	168	815	126	168	989	214
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	213	1695	158	182	921	93	168	815	126	168	989	214
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	213	1695	158	182	921	93	168	815	126	168	989	214
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	213	1695	158	182	921	93	168	815	126	168	989	214
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	2.00	2.74	0.26	2.00	2.72	0.28	2.00	2.60	0.40	2.00	2.47	0.53
Final Sat.:	3200	4591	409	3200	4560	440	3200	4357	643	3200	4146	854
Capacity Analysis Module:												
Vol/Sat:	0.07	0.37	0.39	0.06	0.20	0.21	0.05	0.19	0.20	0.05	0.24	0.25
Crit Moves:		****	****				****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #24 Fairview St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.768
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	46	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0 2	1 0		2 0 3	0 1		2 0 3	0 1		2 0 3	0 1	
Volume Module:												
Base Vol:	225	1550	84	171	955	88	240	627	211	167	1272	223
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	225	1550	84	171	955	88	240	627	211	167	1272	223
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	225	1550	84	171	955	88	240	627	211	167	1272	223
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	225	1550	84	171	955	88	240	627	211	167	1272	223
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	225	1550	84	171	955	88	240	627	211	167	1272	223
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.85	0.15	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	4753	247	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.07	0.33	0.34	0.05	0.19	0.06	0.08	0.12	0.13	0.05	0.25	0.14
Crit Moves:			****	****			****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #25 Fairview Rd and Sunflower Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.715
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	39	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	2 0	1 1	0	2 0	2 0	1
Volume Module:												
Base Vol:	192	1725	375	140	1162	81	262	555	123	227	569	148
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	192	1725	375	140	1162	81	262	555	123	227	569	148
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	192	1725	375	140	1162	81	262	555	123	227	569	148
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	192	1725	375	140	1162	81	262	555	123	227	569	148
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	192	1725	375	140	1162	81	262	555	123	227	569	148
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	2.80	0.20	2.00	1.64	0.36	2.00	2.00	1.00
Final Sat.:	3200	5100	1600	3200	4687	313	3200	2719	581	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.06	0.34	0.23	0.04	0.25	0.26	0.08	0.20	0.21	0.07	0.17	0.09
Crit Moves:		****		****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #26 Greenville St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.595
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	29	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	0 1	0 0	0	0 0	0 0	2	1 0	1 0	3	0 0
Volume Module:												
Base Vol:	293	0	195	0	0	0	0	1061	216	153	1811	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	293	0	195	0	0	0	0	1061	216	153	1811	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	293	0	195	0	0	0	0	1061	216	153	1811	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	293	0	195	0	0	0	0	1061	216	153	1811	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	293	0	195	0	0	0	0	1061	216	153	1811	0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	2.49	0.51	1.00	3.00	0.00
Final Sat.:	1600	0	1600	0	0	0	0	4188	812	1600	5100	0
Capacity Analysis Module:												
Vol/Sat:	0.18	0.00	0.12	0.00	0.00	0.00	0.00	0.25	0.27	0.10	0.36	0.00
Crit Moves:	****								****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #27 Greenville St and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.777
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	48	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0 1	0	1 0	0 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	50	414	31	52	171	52	127	967	55	37	1414	202
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	50	414	31	52	171	52	127	967	55	37	1414	202
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	50	414	31	52	171	52	127	967	55	37	1414	202
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	50	414	31	52	171	52	127	967	55	37	1414	202
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	50	414	31	52	171	52	127	967	55	37	1414	202
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	0.93	0.07	1.00	0.77	0.23	1.00	2.84	0.16	1.00	2.62	0.38
Final Sat.:	1600	1489	111	1600	1227	373	1600	4742	258	1600	4400	600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.28	0.28	0.03	0.14	0.14	0.08	0.20	0.21	0.02	0.32	0.34
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #28 Raitt St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.758
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	45	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	61	872	136	51	533	86	108	640	56	98	827	114
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	61	872	136	51	533	86	108	640	56	98	827	114
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	61	872	136	51	533	86	108	640	56	98	827	114
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	61	872	136	51	533	86	108	640	56	98	827	114
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	61	872	136	51	533	86	108	640	56	98	827	114
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.03	1.00	1.00	1.04	1.00
Lanes:	1.00	1.73	0.27	1.00	1.72	0.28	1.00	1.84	0.16	1.00	1.76	0.24
Final Sat.:	1600	2868	432	1600	2855	445	1600	3043	257	1600	2912	388
Capacity Analysis Module:												
Vol/Sat:	0.04	0.30	0.32	0.03	0.19	0.19	0.07	0.21	0.22	0.06	0.28	0.29
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #29 Raitt St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.986
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	171	736	206	114	396	99	125	862	123	192	2171	194
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	171	736	206	114	396	99	125	862	123	192	2171	194
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	171	736	206	114	396	99	125	862	123	192	2171	194
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	171	736	206	114	396	99	125	862	123	192	2171	194
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	171	736	206	114	396	99	125	862	123	192	2171	194
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	1.56	0.44	1.00	1.60	0.40	1.00	2.63	0.37	1.00	2.75	0.25
Final Sat.:	1600	2600	700	1600	2660	640	1600	4401	599	1600	4606	394
Capacity Analysis Module:												
Vol/Sat:	0.11	0.28	0.29	0.07	0.15	0.15	0.08	0.20	0.21	0.12	0.47	0.49
Crit Moves:		****	****				****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #30 Bear St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.851
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	65	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	1	1 0	2 0	1	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	273	1314	343	115	278	84	80	753	78	67	1211	193
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	273	1314	343	115	278	84	80	753	78	67	1211	193
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	273	1314	343	115	278	84	80	753	78	67	1211	193
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	273	1314	343	115	278	84	80	753	78	67	1211	193
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	273	1314	343	115	278	84	80	753	78	67	1211	193
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.72	0.28	1.00	2.59	0.41
Final Sat.:	1600	3400	1600	1600	3400	1600	1600	4549	451	1600	4340	660
Capacity Analysis Module:												
Vol/Sat:	0.17	0.39	0.21	0.07	0.08	0.05	0.05	0.17	0.17	0.04	0.28	0.29
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #31 Bristol St and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.823
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	57	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	2 0	2 1	0	2 0	3 0	1
Volume Module:												
Base Vol:	375	1823	303	337	1794	305	238	530	96	311	1170	396
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	375	1823	303	337	1794	305	238	530	96	311	1170	396
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	375	1823	303	337	1794	305	238	530	96	311	1170	396
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	375	1823	303	337	1794	305	238	530	96	311	1170	396
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	375	1823	303	337	1794	305	238	530	96	311	1170	396
OvlAdjVol:												228
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	2.54	0.46	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	4264	736	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.12	0.36	0.19	0.11	0.35	0.19	0.07	0.12	0.13	0.10	0.23	0.25
OvlAdjV/S:												0.14
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #32 Bristol St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.911
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	93	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	222	1550	81	159	1458	149	195	549	65	184	789	89
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	222	1550	81	159	1458	149	195	549	65	184	789	89
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	222	1550	81	159	1458	149	195	549	65	184	789	89
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	222	1550	81	159	1458	149	195	549	65	184	789	89
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	222	1550	81	159	1458	149	195	549	65	184	789	89
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.03	1.00	1.00	1.03	1.00
Lanes:	1.00	2.85	0.15	1.00	2.72	0.28	1.00	1.79	0.21	1.00	1.80	0.20
Final Sat.:	1600	4762	238	1600	4555	445	1600	2961	339	1600	2976	324
Capacity Analysis Module:												
Vol/Sat:	0.14	0.33	0.34	0.10	0.32	0.33	0.12	0.19	0.19	0.12	0.27	0.27
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #33 Bristol St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.853
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	66	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	84	1702	97	117	1558	59	155	428	100	392	624	168
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	84	1702	97	117	1558	59	155	428	100	392	624	168
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	84	1702	97	117	1558	59	155	428	100	392	624	168
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	84	1702	97	117	1558	59	155	428	100	392	624	168
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	84	1702	97	117	1558	59	155	428	100	392	624	168
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.84	0.16	1.00	2.89	0.11	1.00	2.43	0.57	1.00	2.36	0.64
Final Sat.:	1600	4741	259	1600	4825	175	1600	4091	909	1600	3982	1018
Capacity Analysis Module:												
Vol/Sat:	0.05	0.36	0.37	0.07	0.32	0.34	0.10	0.10	0.11	0.25	0.16	0.16
Crit Moves:			****	****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #34 Bristol St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.826
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	58	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	392	1693	250	223	1209	273	160	900	115	196	1204	114
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	392	1693	250	223	1209	273	160	900	115	196	1204	114
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	392	1693	250	223	1209	273	160	900	115	196	1204	114
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	392	1693	250	223	1209	273	160	900	115	196	1204	114
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	392	1693	250	223	1209	273	160	900	115	196	1204	114
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	1.00	2.66	0.34	1.00	2.74	0.26
Final Sat.:	3200	5100	1600	3200	5100	1600	1600	4456	544	1600	4585	415
Capacity Analysis Module:												
Vol/Sat:	0.12	0.33	0.16	0.07	0.24	0.17	0.10	0.20	0.21	0.12	0.26	0.27
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #35 Bristol St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	1.028
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	224	1649	90	111	1554	358	245	478	95	209	1088	127
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	224	1649	90	111	1554	358	245	478	95	209	1088	127
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	224	1649	90	111	1554	358	245	478	95	209	1088	127
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	224	1649	90	111	1554	358	245	478	95	209	1088	127
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	224	1649	90	111	1554	358	245	478	95	209	1088	127
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.03	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	1.67	0.33	1.00	1.79	0.21
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	2769	531	1600	2966	334
Capacity Analysis Module:												
Vol/Sat:	0.14	0.32	0.06	0.07	0.30	0.22	0.15	0.17	0.18	0.13	0.37	0.38
Crit Moves:	****			****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #36 Bristol St and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.847
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	64	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	1 0	3 0	1	1 0	3 0	1
Volume Module:												
Base Vol:	316	1182	216	338	1128	219	176	791	140	251	1481	434
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	316	1182	216	338	1128	219	176	791	140	251	1481	434
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	316	1182	216	338	1128	219	176	791	140	251	1481	434
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	316	1182	216	338	1128	219	176	791	140	251	1481	434
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	316	1182	216	338	1128	219	176	791	140	251	1481	434
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.54	0.46	2.00	2.51	0.49	1.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	4258	742	3200	4220	780	1600	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.10	0.28	0.29	0.11	0.27	0.28	0.11	0.16	0.09	0.16	0.29	0.27
Crit Moves:		****	****				****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

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Intersection #37 Bristol St and Segerstrom Ave

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Cycle (sec):	100	Critical Vol./Cap.(X):	0.870	
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx	
Optimal Cycle:	72	Level Of Service:	D	

****

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	182	1347	237	101	927	194	224	803	64	145	1313	66
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	182	1347	237	101	927	194	224	803	64	145	1313	66
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	182	1347	237	101	927	194	224	803	64	145	1313	66
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	182	1347	237	101	927	194	224	803	64	145	1313	66
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	182	1347	237	101	927	194	224	803	64	145	1313	66
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	2.55	0.45	1.00	2.48	0.52	1.00	2.78	0.22	1.00	2.86	0.14
Final Sat.:	1600	4282	718	1600	4169	831	1600	4646	354	1600	4770	230
Capacity Analysis Module:												
Vol/Sat:	0.11	0.31	0.33	0.06	0.22	0.23	0.14	0.17	0.18	0.09	0.28	0.29
Crit Moves:			****	****			****					****

****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #38 Bristol St and Alton Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.633
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	31	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	3 0	1	1 0	1 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	43	1681	158	116	870	54	25	35	65	91	150	29
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	43	1681	158	116	870	54	25	35	65	91	150	29
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	43	1681	158	116	870	54	25	35	65	91	150	29
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	43	1681	158	116	870	54	25	35	65	91	150	29
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	43	1681	158	116	870	54	25	35	65	91	150	29
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	1.00	2.74	0.26	1.00	3.00	1.00	1.00	1.00	1.00	1.00	0.84	0.16
Final Sat.:	1600	4588	412	1600	5100	1600	1600	1700	1600	1600	1341	259
Capacity Analysis Module:												
Vol/Sat:	0.03	0.37	0.38	0.07	0.17	0.03	0.02	0.02	0.04	0.06	0.11	0.11
Crit Moves:			****	****			****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #39 Bristol St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.765
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	46	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	321	1369	249	233	858	135	335	746	158	236	1370	259
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	321	1369	249	233	858	135	335	746	158	236	1370	259
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	321	1369	249	233	858	135	335	746	158	236	1370	259
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	321	1369	249	233	858	135	335	746	158	236	1370	259
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	321	1369	249	233	858	135	335	746	158	236	1370	259
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	2.59	0.41	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	4347	653	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.10	0.27	0.16	0.07	0.20	0.21	0.10	0.15	0.10	0.07	0.27	0.16
Crit Moves:		****			****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #40 Bristol St and Sunflower Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.692
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	36	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	1	2 0	3 0	1	2 0	2 1	1	2 0	3 0	1
Volume Module:												
Base Vol:	651	1376	222	215	804	230	301	588	228	247	952	303
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	651	1376	222	215	804	230	301	588	228	247	952	303
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	651	1376	222	215	804	230	301	588	228	247	952	303
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	651	1376	222	215	804	230	301	588	228	247	952	303
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	651	1376	222	215	804	230	301	588	228	247	952	303
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	2.88	1.12	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	4812	1788	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.20	0.27	0.14	0.07	0.16	0.14	0.09	0.12	0.13	0.08	0.19	0.19
Crit Moves:	****				****		****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #41 Bristol St and I-405 NB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.825
Loss Time (sec):	5	Average Delay (sec/veh):	28.5
Optimal Cycle:	60	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	4 0 1	0	0	4 1 0	0	0	0 0 2	1	1	1 0 2
Volume Module:												
Base Vol:	0	2094	183	0	2375	22	0	0	204	350	300	1162
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	2094	183	0	2375	22	0	0	204	350	300	1162
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	2094	183	0	2375	22	0	0	204	350	300	1162
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	2094	183	0	2375	22	0	0	204	350	300	1162
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	2094	183	0	2375	22	0	0	204	350	300	1162
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	0.85	1.00	0.91	0.91	1.00	1.00	0.75	0.93	0.93	0.75
Lanes:	0.00	4.00	1.00	0.00	4.95	0.05	0.00	0.00	2.00	1.62	1.38	2.00
Final Sat.:	0	6916	1615	0	8557	79	0	0	2842	2840	2434	2842
Capacity Analysis Module:												
Vol/Sat:	0.00	0.30	0.11	0.00	0.28	0.28	0.00	0.00	0.07	0.12	0.12	0.41
Crit Moves:		****		****					****			****
Green/Cycle:	0.00	0.37	0.37	0.00	0.37	0.37	0.00	0.00	0.09	0.50	0.50	0.50
Volume/Cap:	0.00	0.82	0.31	0.00	0.76	0.76	0.00	0.00	0.82	0.25	0.25	0.82
Delay/Veh:	0.0	31.0	22.9	0.0	28.8	28.8	0.0	0.0	64.6	14.5	14.5	25.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	31.0	22.9	0.0	28.8	28.8	0.0	0.0	64.6	14.5	14.5	25.6
LOS by Move:	A	C	C	A	C	C	A	A	E	B	B	C
HCM2k85thQ:	0	27	6	0	23	23	0	0	9	6	6	28

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #42 Bristol St and I-405 SB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.617
Loss Time (sec):	5	Average Delay (sec/veh):	18.3
Optimal Cycle:	31	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Ignore			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	4 0	0	0 0	2 1	1	3 0	0 0	1	0 0	0 0	0
Volume Module:												
Base Vol:	128	1570	0	0	1611	942	740	0	331	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	128	1570	0	0	1611	942	740	0	331	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	128	1570	0	0	1611	0	740	0	331	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	128	1570	0	0	1611	0	740	0	331	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	128	1570	0	0	1611	0	740	0	331	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	1.00	1.00	0.91	0.91	0.92	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	4.00	0.00	0.00	3.00	1.00	3.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	6916	0	0	5187	1729	5253	0	1615	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.07	0.23	0.00	0.00	0.31	0.00	0.14	0.00	0.20	0.00	0.00	0.00
Crit Moves:	****				****				****			
Green/Cycle:	0.11	0.62	0.00	0.00	0.50	0.00	0.33	0.00	0.33	0.00	0.00	0.00
Volume/Cap:	0.62	0.37	0.00	0.00	0.62	0.00	0.42	0.00	0.62	0.00	0.00	0.00
Delay/Veh:	47.7	9.5	0.0	0.0	18.4	0.0	26.1	0.0	30.2	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	47.7	9.5	0.0	0.0	18.4	0.0	26.1	0.0	30.2	0.0	0.0	0.0
LOS by Move:	D	A	A	A	B	A	C	A	C	A	A	A
HCM2k85thQ:	7	10	0	0	20	0	10	0	14	0	0	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #43 Flower St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.857
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	68	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Prot+Permit			Prot+Permit			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	1	1 0	2 0	1	1 0	3 0	1	1 0	2 1	0
Volume Module:												
Base Vol:	198	1206	99	87	442	40	357	1029	168	186	649	192
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	198	1206	99	87	442	40	357	1029	168	186	649	192
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	198	1206	99	87	442	40	357	1029	168	186	649	192
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	198	1206	99	87	442	40	357	1029	168	186	649	192
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	198	1206	99	87	442	40	357	1029	168	186	649	192
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	3.00	1.00	1.00	2.32	0.68
Final Sat.:	1600	3400	1600	1600	3400	1600	1600	5100	1600	1600	3904	1096
Capacity Analysis Module:												
Vol/Sat:	0.12	0.35	0.06	0.05	0.13	0.03	0.22	0.20	0.11	0.12	0.17	0.18
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #44 Flower St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.877
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	75	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	1	1 0	1 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	224	1063	253	200	549	159	102	895	74	210	1415	147
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	224	1063	253	200	549	159	102	895	74	210	1415	147
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	224	1063	253	200	549	159	102	895	74	210	1415	147
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	224	1063	253	200	549	159	102	895	74	210	1415	147
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	224	1063	253	200	549	159	102	895	74	210	1415	147
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.00	1.00	1.00	1.55	0.45	1.00	2.77	0.23	1.00	2.72	0.28
Final Sat.:	1600	3400	1600	1600	2581	719	1600	4633	367	1600	4548	452
Capacity Analysis Module:												
Vol/Sat:	0.14	0.31	0.16	0.13	0.21	0.22	0.06	0.19	0.20	0.13	0.31	0.33
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #45 Flower St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	1.037
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 1	0 1	0	0 1	0 1	0	0 1	0 1	0	0 1	0 1	0
Volume Module:												
Base Vol:	91	1234	76	46	352	57	124	928	37	93	1159	166
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	91	1234	76	46	352	57	124	928	37	93	1159	166
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	91	1234	76	46	352	57	124	928	37	93	1159	166
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	91	1234	76	46	352	57	124	928	37	93	1159	166
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	91	1234	76	46	352	57	124	928	37	93	1159	166
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	0.13	1.76	0.11	0.20	1.55	0.25	0.23	1.70	0.07	0.13	1.64	0.23
Final Sat.:	208	2819	174	324	2476	401	364	2727	109	210	2616	375
Capacity Analysis Module:												
Vol/Sat:	0.06	0.44	0.44	0.03	0.14	0.14	0.08	0.34	0.34	0.06	0.44	0.44
Crit Moves:		****			****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #46 Flower St and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.866
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	71	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	122	859	82	84	469	135	203	1011	107	92	1545	99
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	122	859	82	84	469	135	203	1011	107	92	1545	99
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	122	859	82	84	469	135	203	1011	107	92	1545	99
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	122	859	82	84	469	135	203	1011	107	92	1545	99
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	122	859	82	84	469	135	203	1011	107	92	1545	99
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.04	1.00
Lanes:	1.00	1.83	0.17	1.00	1.55	0.45	1.00	2.71	0.29	1.00	2.82	0.18
Final Sat.:	1600	3021	279	1600	2585	715	1600	4541	459	1600	4711	289
Capacity Analysis Module:												
Vol/Sat:	0.08	0.28	0.29	0.05	0.18	0.19	0.13	0.22	0.23	0.06	0.33	0.34
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #47 Flower St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.900
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	87	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	130	621	72	93	224	215	199	947	60	62	1989	177
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	130	621	72	93	224	215	199	947	60	62	1989	177
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	130	621	72	93	224	215	199	947	60	62	1989	177
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	130	621	72	93	224	215	199	947	60	62	1989	177
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	130	621	72	93	224	215	199	947	60	62	1989	177
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	1.79	0.21	1.00	1.02	0.98	1.00	2.82	0.18	1.00	2.75	0.25
Final Sat.:	1600	2968	332	1600	1733	1567	1600	4714	286	1600	4608	392
Capacity Analysis Module:												
Vol/Sat:	0.08	0.21	0.22	0.06	0.13	0.14	0.12	0.20	0.21	0.04	0.43	0.45
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #48 Main St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.696
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	37	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	2 0	2 1	0	2 0	2 0	1
Volume Module:												
Base Vol:	347	1059	496	164	1031	266	327	466	209	282	563	226
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	347	1059	496	164	1031	266	327	466	209	282	563	226
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	347	1059	496	164	1031	266	327	466	209	282	563	226
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	347	1059	496	164	1031	266	327	466	209	282	563	226
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	347	1059	496	164	1031	266	327	466	209	282	563	226
OvlAdjVol:			355									144
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	2.38	0.62	2.00	2.07	0.93	2.00	2.00	1.00
Final Sat.:	3200	5100	1600	3200	4016	984	3200	3514	1486	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.11	0.21	0.31	0.05	0.26	0.27	0.10	0.13	0.14	0.09	0.17	0.14
OvlAdjV/S:			0.22									0.09
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #49 Main St and Mainplace Dr / Memory Ln

Cycle (sec):	100	Critical Vol./Cap.(X):	0.556
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	26	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 0	2 0	2	1 0	2 0	2	1 0	2 0	2	0 1
Volume Module:												
Base Vol:	113	1186	225	52	1083	143	391	433	116	278	227	110
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	113	1186	225	52	1083	143	391	433	116	278	227	110
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	113	1186	225	52	1083	143	391	433	116	278	227	110
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	113	1186	225	52	1083	143	391	433	116	278	227	110
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	113	1186	225	52	1083	143	391	433	116	278	227	110
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	2.52	0.48	2.00	2.65	0.35	2.00	2.37	0.63	2.00	2.00	1.00
Final Sat.:	3200	4235	765	3200	4440	560	3200	3986	1014	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.04	0.28	0.29	0.02	0.24	0.26	0.12	0.11	0.11	0.09	0.07	0.07
Crit Moves:			****	****				****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #50 Main St and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.760
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	45	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 0	1	2 0	2 0	1	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	197	1019	222	222	969	150	224	1256	74	199	1220	81
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	197	1019	222	222	969	150	224	1256	74	199	1220	81
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	197	1019	222	222	969	150	224	1256	74	199	1220	81
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	197	1019	222	222	969	150	224	1256	74	199	1220	81
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	197	1019	222	222	969	150	224	1256	74	199	1220	81
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	2.00	2.00	1.00	2.00	2.00	1.00	2.00	2.83	0.17	2.00	2.81	0.19
Final Sat.:	3200	3400	1600	3200	3400	1600	3200	4733	267	3200	4701	299
Capacity Analysis Module:												
Vol/Sat:	0.06	0.30	0.14	0.07	0.28	0.09	0.07	0.27	0.28	0.06	0.26	0.27
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #51 Main St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.965
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	152	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	92	1169	98	52	853	54	239	1322	158	38	384	39
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	92	1169	98	52	853	54	239	1322	158	38	384	39
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	92	1169	98	52	853	54	239	1322	158	38	384	39
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	92	1169	98	52	853	54	239	1322	158	38	384	39
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	92	1169	98	52	853	54	239	1322	158	38	384	39
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.03	1.00
Lanes:	1.00	1.85	0.15	1.00	1.88	0.12	1.00	1.79	0.21	1.00	1.82	0.18
Final Sat.:	1600	3052	248	1600	3109	191	1600	2958	342	1600	3005	295
Capacity Analysis Module:												
Vol/Sat:	0.06	0.38	0.40	0.03	0.27	0.28	0.15	0.45	0.46	0.02	0.13	0.13
Crit Moves:			****	****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #52 Main St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.792
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	50	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	0	0 0	1 1	0	0 0	0 0	0	0 1	0 1	0
Volume Module:												
Base Vol:	51	1152	0	0	1083	53	0	0	0	92	924	120
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	51	1152	0	0	1083	53	0	0	0	92	924	120
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	51	1152	0	0	1083	53	0	0	0	92	924	120
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	51	1152	0	0	1083	53	0	0	0	92	924	120
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	51	1152	0	0	1083	53	0	0	0	92	924	120
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.03	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	1.00	2.00	0.00	0.00	1.91	0.09	0.00	0.00	0.00	0.16	1.63	0.21
Final Sat.:	1600	3400	0	0	3151	149	0	0	0	259	2603	338
Capacity Analysis Module:												
Vol/Sat:	0.03	0.34	0.00	0.00	0.34	0.36	0.00	0.00	0.00	0.06	0.36	0.35
Crit Moves:	****					****					****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #53 Main St and 4th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.447
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	22	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	1	1 0	0 0	1	1 0	0 0	1	1 0	0 0	1	1 0
Volume Module:												
Base Vol:	0	950	45	0	864	39	0	70	39	0	229	58
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	950	45	0	864	39	0	70	39	0	229	58
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	950	45	0	864	39	0	70	39	0	229	58
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	950	45	0	864	39	0	70	39	0	229	58
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	950	45	0	864	39	0	70	39	0	229	58
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.05	1.00	1.00	1.04	1.00
Lanes:	0.00	1.91	0.09	0.00	1.91	0.09	0.00	1.28	0.72	0.00	1.60	0.40
Final Sat.:	0	3155	145	0	3162	138	0	2155	1145	0	2653	647
Capacity Analysis Module:												
Vol/Sat:	0.00	0.30	0.31	0.00	0.27	0.28	0.00	0.03	0.03	0.00	0.09	0.09
Crit Moves:		****	****				****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #54 Main St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.744
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	43	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	2	0 1	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	152	745	65	146	559	119	116	1172	87	111	1244	87
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	152	745	65	146	559	119	116	1172	87	111	1244	87
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	152	745	65	146	559	119	116	1172	87	111	1244	87
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	152	745	65	146	559	119	116	1172	87	111	1244	87
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	152	745	65	146	559	119	116	1172	87	111	1244	87
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	1.84	0.16	1.00	2.00	1.00	1.00	2.79	0.21	1.00	2.80	0.20
Final Sat.:	1600	3043	257	1600	3400	1600	1600	4668	332	1600	4686	314
Capacity Analysis Module:												
Vol/Sat:	0.10	0.24	0.25	0.09	0.16	0.07	0.07	0.25	0.26	0.07	0.27	0.28
Crit Moves:		****	****				****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #55 Main St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.925
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	104	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Prot+Permit			Prot+Permit			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	132	1037	64	123	834	61	182	805	98	133	752	338
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	132	1037	64	123	834	61	182	805	98	133	752	338
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	132	1037	64	123	834	61	182	805	98	133	752	338
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	132	1037	64	123	834	61	182	805	98	133	752	338
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	132	1037	64	123	834	61	182	805	98	133	752	338
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	1.88	0.12	1.00	1.86	0.14	1.00	1.78	0.22	1.00	1.38	0.62
Final Sat.:	1600	3114	186	1600	3082	218	1600	2953	347	1600	2308	992
Capacity Analysis Module:												
Vol/Sat:	0.08	0.33	0.34	0.08	0.27	0.28	0.11	0.27	0.28	0.08	0.33	0.34
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #56 Main St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.867
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	71	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	101	1069	49	129	646	88	118	698	78	102	1364	141
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	101	1069	49	129	646	88	118	698	78	102	1364	141
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	101	1069	49	129	646	88	118	698	78	102	1364	141
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	101	1069	49	129	646	88	118	698	78	102	1364	141
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	101	1069	49	129	646	88	118	698	78	102	1364	141
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	1.91	0.09	1.00	1.76	0.24	1.00	2.70	0.30	1.00	2.72	0.28
Final Sat.:	1600	3160	140	1600	2916	384	1600	4518	482	1600	4550	450
Capacity Analysis Module:												
Vol/Sat:	0.06	0.34	0.35	0.08	0.22	0.23	0.07	0.15	0.16	0.06	0.30	0.31
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #57 Main St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.749
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	43	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	446	1067	293	265	345	260	313	643	62	193	1529	444
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	446	1067	293	265	345	260	313	643	62	193	1529	444
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	446	1067	293	265	345	260	313	643	62	193	1529	444
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	446	1067	293	265	345	260	313	643	62	193	1529	444
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	446	1067	293	265	345	260	313	643	62	193	1529	444
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.14	0.21	0.18	0.08	0.07	0.16	0.10	0.13	0.04	0.06	0.30	0.28
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #58 Penn Wy and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.872
Loss Time (sec):	5	Average Delay (sec/veh):	25.5
Optimal Cycle:	76	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	0	0 2	0 0	0	0 0	0 0	2	1 0	1 0	3	0 0
Volume Module:												
Base Vol:	1113	0	543	0	0	0	0	1672	539	123	1440	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	1113	0	543	0	0	0	0	1672	539	123	1440	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	1113	0	543	0	0	0	0	1672	539	123	1440	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	1113	0	543	0	0	0	0	1672	539	123	1440	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	1113	0	543	0	0	0	0	1672	539	123	1440	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.75	1.00	1.00	1.00	1.00	0.88	0.88	0.95	0.91	1.00
Lanes:	2.00	0.00	2.00	0.00	0.00	0.00	0.00	2.27	0.73	1.00	3.00	0.00
Final Sat.:	3502	0	2842	0	0	0	0	3777	1218	1805	5187	0
Capacity Analysis Module:												
Vol/Sat:	0.32	0.00	0.19	0.00	0.00	0.00	0.00	0.44	0.44	0.07	0.28	0.00
Crit Moves:	****							****		****		
Green/Cycle:	0.36	0.00	0.36	0.00	0.00	0.00	0.00	0.51	0.51	0.08	0.59	0.00
Volume/Cap:	0.87	0.00	0.52	0.00	0.00	0.00	0.00	0.87	0.87	0.87	0.47	0.00
Delay/Veh:	36.5	0.0	25.5	0.0	0.0	0.0	0.0	25.4	25.4	86.2	12.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	36.5	0.0	25.5	0.0	0.0	0.0	0.0	25.4	25.4	86.2	12.0	0.0
LOS by Move:	D	A	C	A	A	A	A	C	C	F	B	A
HCM2k85thQ:	28	0	12	0	0	0	0	36	36	10	14	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #59 I-5 NB Off Ramps/17th Street

Cycle (sec):	100	Critical Vol./Cap.(X):	0.587
Loss Time (sec):	5	Average Delay (sec/veh):	19.0
Optimal Cycle:	29	Level Of Service:	B

Street Name:	I-5 NB Off Ramps						17th Street					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Permitted		
Rights:	Include			Include			Ignore			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 1	0	0 1	1	0 0	0 1	1	0 3	0 1	0	0 2	1 0
Volume Module:												
Base Vol:	330	44	17	50	0	130	114	1047	688	0	1564	42
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	330	44	17	50	0	130	114	1047	688	0	1564	42
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Volume:	330	44	17	50	0	130	114	1047	0	0	1564	42
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	330	44	17	50	0	130	114	1047	0	0	1564	42
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
FinalVolume:	330	44	17	50	0	130	114	1047	0	0	1564	42
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.96	0.96	0.85	0.95	1.00	0.85	0.95	0.91	1.00	1.00	0.91	0.91
Lanes:	1.76	0.24	1.00	1.00	0.00	1.00	1.00	3.00	1.00	0.00	2.92	0.08
Final Sat.:	3212	428	1615	1805	0	1615	1805	5187	1900	0	5031	135
Capacity Analysis Module:												
Vol/Sat:	0.10	0.10	0.01	0.03	0.00	0.08	0.06	0.20	0.00	0.00	0.31	0.31
Crit Moves:		****				****	****				****	
Green/Cycle:	0.18	0.18	0.18	0.14	0.00	0.14	0.11	0.64	0.00	0.00	0.53	0.53
Volume/Cap:	0.59	0.59	0.06	0.20	0.00	0.59	0.59	0.32	0.00	0.00	0.59	0.59
Delay/Veh:	39.3	39.3	34.5	38.7	0.0	44.5	47.1	8.3	0.0	0.0	16.4	16.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	39.3	39.3	34.5	38.7	0.0	44.5	47.1	8.3	0.0	0.0	16.4	16.4
LOS by Move:	D	D	C	D	A	D	D	A	A	A	B	B
HCM2k85thQ:	9	9	1	2	0	7	7	8	0	0	19	19

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #60 Penn Wy and I-5 SB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.573
Loss Time (sec):	5	Average Delay (sec/veh):	23.0
Optimal Cycle:	29	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	2	0 1	2 0	2	0 0	0 0	0	0 0	1 0	0	0 2
Volume Module:												
Base Vol:	0	631	414	592	137	0	0	0	0	214	0	183
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	631	414	592	137	0	0	0	0	214	0	183
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	631	414	592	137	0	0	0	0	214	0	183
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	631	414	592	137	0	0	0	0	214	0	183
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	631	414	592	137	0	0	0	0	214	0	183
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.95	0.85	0.92	0.95	1.00	1.00	1.00	1.00	0.95	1.00	0.75
Lanes:	0.00	2.00	1.00	2.00	2.00	0.00	0.00	0.00	0.00	1.00	0.00	2.00
Final Sat.:	0	3610	1615	3502	3610	0	0	0	0	1805	0	2842
Capacity Analysis Module:												
Vol/Sat:	0.00	0.17	0.26	0.17	0.04	0.00	0.00	0.00	0.00	0.12	0.00	0.06
Crit Moves:			****	****						****		
Green/Cycle:	0.00	0.45	0.45	0.30	0.74	0.00	0.00	0.00	0.00	0.21	0.00	0.50
Volume/Cap:	0.00	0.39	0.57	0.57	0.05	0.00	0.00	0.00	0.00	0.57	0.00	0.13
Delay/Veh:	0.0	18.6	21.6	30.7	3.4	0.0	0.0	0.0	0.0	37.8	0.0	13.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	18.6	21.6	30.7	3.4	0.0	0.0	0.0	0.0	37.8	0.0	13.3
LOS by Move:	A	B	C	C	A	A	A	A	A	D	A	B
HCM2k85thQ:	0	10	15	13	1	0	0	0	0	10	0	3

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #61 Santiago St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	1.369
Loss Time (sec):	5	Average Delay (sec/veh):	110.0
Optimal Cycle:	0	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	1 0	0 1	0	1 0	0 1	0	0 1	0 0	1	0 0	1! 0	0
Volume Module:												
Base Vol:	194	598	60	32	456	117	266	76	269	14	19	18
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	194	598	60	32	456	117	266	76	269	14	19	18
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	194	598	60	32	456	117	266	76	269	14	19	18
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	194	598	60	32	456	117	266	76	269	14	19	18
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	194	598	60	32	456	117	266	76	269	14	19	18
Saturation Flow Module:												
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	0.91	0.09	1.00	0.80	0.20	0.78	0.22	1.00	0.27	0.38	0.35
Final Sat.:	446	437	44	428	375	96	342	98	507	104	141	134
Capacity Analysis Module:												
Vol/Sat:	0.44	1.37	1.37	0.07	1.22	1.22	0.78	0.78	0.53	0.13	0.13	0.13
Crit Moves:		****			****			****			****	
Delay/Veh:	16.8	201	200.6	11.6	140	140.1	34.1	34.1	17.5	13.7	13.7	13.7
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	16.8	201	200.6	11.6	140	140.1	34.1	34.1	17.5	13.7	13.7	13.7
LOS by Move:	C	F	F	B	F	F	D	D	C	B	B	B
ApproachDel:	158.8			133.3			26.8			13.7		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	158.8			133.3			26.8			13.7		
LOS by Appr:	F			F			D			B		
AllWayAvgQ:	0.7	25.4	25.4	0.1	17.0	17.0	2.8	2.8	1.1	0.1	0.1	0.1

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #62 Santiago St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	1.017
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	2	0 1
Volume Module:												
Base Vol:	135	833	236	378	639	103	164	673	87	255	546	347
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	135	833	236	378	639	103	164	673	87	255	546	347
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	135	833	236	378	639	103	164	673	87	255	546	347
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	135	833	236	378	639	103	164	673	87	255	546	347
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	135	833	236	378	639	103	164	673	87	255	546	347
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	1.00	1.56	0.44	1.00	1.72	0.28	1.00	1.77	0.23	1.00	2.00	1.00
Final Sat.:	1600	2594	706	1600	2856	444	1600	2934	366	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.08	0.32	0.33	0.24	0.22	0.23	0.10	0.23	0.24	0.16	0.16	0.22
Crit Moves:			****	****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #63 Standard Ave and 4th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.893
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	83	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	1	1 0	2 0	1	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	251	543	214	254	428	297	302	477	201	233	648	351
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	251	543	214	254	428	297	302	477	201	233	648	351
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	251	543	214	254	428	297	302	477	201	233	648	351
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	251	543	214	254	428	297	302	477	201	233	648	351
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	251	543	214	254	428	297	302	477	201	233	648	351
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	1.41	0.59	1.00	1.30	0.70
Final Sat.:	1600	3400	1600	1600	3400	1600	1600	2351	949	1600	2176	1124
Capacity Analysis Module:												
Vol/Sat:	0.16	0.16	0.13	0.16	0.13	0.19	0.19	0.20	0.21	0.15	0.30	0.31
Crit Moves:	****					****	****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #64 Standard Ave and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.968
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	159	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	109	724	259	142	679	47	168	1279	65	264	1888	116
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	109	724	259	142	679	47	168	1279	65	264	1888	116
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	109	724	259	142	679	47	168	1279	65	264	1888	116
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	109	724	259	142	679	47	168	1279	65	264	1888	116
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	109	724	259	142	679	47	168	1279	65	264	1888	116
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	1.47	0.53	1.00	1.87	0.13	1.00	2.85	0.15	1.00	2.83	0.17
Final Sat.:	1600	2457	843	1600	3093	207	1600	4768	232	1600	4722	278
Capacity Analysis Module:												
Vol/Sat:	0.07	0.29	0.31	0.09	0.22	0.23	0.11	0.27	0.28	0.17	0.40	0.42
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #65 Standard Ave and Mcfadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.828
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	59	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	194	765	94	237	520	90	95	632	108	123	749	217
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	194	765	94	237	520	90	95	632	108	123	749	217
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	194	765	94	237	520	90	95	632	108	123	749	217
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	194	765	94	237	520	90	95	632	108	123	749	217
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	194	765	94	237	520	90	95	632	108	123	749	217
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	1.78	0.22	1.00	1.70	0.30	1.00	1.71	0.29	1.00	1.55	0.45
Final Sat.:	1600	2950	350	1600	2828	472	1600	2833	467	1600	2581	719
Capacity Analysis Module:												
Vol/Sat:	0.12	0.26	0.27	0.15	0.18	0.19	0.06	0.22	0.23	0.08	0.29	0.30
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #66 Halladay St and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.975
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	173	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	0 1	0	0	0 0 0	0	1	1 1 0	0	1	2 0 0
Volume Module:												
Base Vol:	338	0	704	0	0	0	2	868	87	272	2159	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	338	0	704	0	0	0	2	868	87	272	2159	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	338	0	704	0	0	0	2	868	87	272	2159	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	338	0	704	0	0	0	2	868	87	272	2159	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	338	0	704	0	0	0	2	868	87	272	2159	0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.02	1.00	1.00	1.05	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.01	2.72	0.27	0.34	2.66	0.00
Final Sat.:	1600	0	1600	0	0	0	10	4454	436	537	4463	0
Capacity Analysis Module:												
Vol/Sat:	0.21	0.00	0.44	0.00	0.00	0.00	0.00	0.19	0.20	0.17	0.48	0.00
Crit Moves:			****				****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #67 Halladay St and Dyer Rd

Cycle (sec):	100	Critical Vol./Cap.(X):	1.057
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	1 0	1 0	0	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	61	266	307	296	39	83	147	1271	9	51	1655	132
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	61	266	307	296	39	83	147	1271	9	51	1655	132
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	61	266	307	296	39	83	147	1271	9	51	1655	132
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	61	266	307	296	39	83	147	1271	9	51	1655	132
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	61	266	307	296	39	83	147	1271	9	51	1655	132
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	0.46	0.54	1.00	0.32	0.68	1.00	2.98	0.02	1.00	2.78	0.22
Final Sat.:	1600	743	857	1600	511	1089	1600	4966	34	1600	4645	355
Capacity Analysis Module:												
Vol/Sat:	0.04	0.36	0.36	0.19	0.08	0.08	0.09	0.26	0.27	0.03	0.36	0.37
Crit Moves:		****	****				****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #68 SR-55 SB Ramps and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.567
Loss Time (sec):	5	Average Delay (sec/veh):	17.8
Optimal Cycle:	28	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Ignore			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	0	2	0	0	0	2	0	0	2	1	1	0	0	3	0	1
Volume Module:												
Base Vol:	0	0	0	299	0	724	0	1187	1058	0	1474	579
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	299	0	724	0	1187	1058	0	1474	579
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Volume:	0	0	0	299	0	724	0	1187	0	0	1474	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	299	0	724	0	1187	0	0	1474	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
FinalVolume:	0	0	0	299	0	724	0	1187	0	0	1474	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.92	1.00	0.75	1.00	0.91	0.91	1.00	0.91	1.00
Lanes:	0.00	0.00	0.00	2.00	0.00	2.00	0.00	3.00	1.00	0.00	3.00	1.00
Final Sat.:	0	0	0	3502	0	2842	0	5187	1729	0	5187	1900
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.09	0.00	0.25	0.00	0.23	0.00	0.00	0.28	0.00
Crit Moves:						****	****				****	
Green/Cycle:	0.00	0.00	0.00	0.45	0.00	0.45	0.00	0.50	0.00	0.00	0.50	0.00
Volume/Cap:	0.00	0.00	0.00	0.19	0.00	0.57	0.00	0.46	0.00	0.00	0.57	0.00
Delay/Veh:	0.0	0.0	0.0	16.7	0.0	21.0	0.0	16.3	0.0	0.0	17.7	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	16.7	0.0	21.0	0.0	16.3	0.0	0.0	17.7	0.0
LOS by Move:	A	A	A	B	A	C	A	B	A	A	B	A
HCM2k85thQ:	0	0	0	4	0	15	0	13	0	0	18	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #69 SR-55 NB Ramps and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.484
Loss Time (sec):	5	Average Delay (sec/veh):	14.6
Optimal Cycle:	24	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Ignore			Include			Ignore			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	0	0 1	0 0	0	0 0	0 0	2	0 2	0 0	3	0 1
Volume Module:												
Base Vol:	620	0	412	0	0	0	0	711	731	0	1467	1056
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	620	0	412	0	0	0	0	711	731	0	1467	1056
User Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Volume:	620	0	0	0	0	0	0	711	0	0	1467	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	620	0	0	0	0	0	0	711	0	0	1467	0
PCE Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
FinalVolume:	620	0	0	0	0	0	0	711	0	0	1467	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.88	1.00	0.91	1.00
Lanes:	2.00	0.00	1.00	0.00	0.00	0.00	0.00	2.00	2.00	0.00	3.00	1.00
Final Sat.:	3502	0	1900	0	0	0	0	3610	3344	0	5187	1900
Capacity Analysis Module:												
Vol/Sat:	0.18	0.00	0.00	0.00	0.00	0.00	0.00	0.20	0.00	0.00	0.28	0.00
Crit Moves:	****						****				****	
Green/Cycle:	0.37	0.00	0.00	0.00	0.00	0.00	0.00	0.58	0.00	0.00	0.58	0.00
Volume/Cap:	0.48	0.00	0.00	0.00	0.00	0.00	0.00	0.34	0.00	0.00	0.48	0.00
Delay/Veh:	24.7	0.0	0.0	0.0	0.0	0.0	0.0	10.9	0.0	0.0	12.2	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	24.7	0.0	0.0	0.0	0.0	0.0	0.0	10.9	0.0	0.0	12.2	0.0
LOS by Move:	C	A	A	A	A	A	A	B	A	A	B	A
HCM2k85thQ:	12	0	0	0	0	0	0	9	0	0	15	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #70 SR-55 SB Ramps and Dyer Rd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.663
Loss Time (sec):	5	Average Delay (sec/veh):	26.5
Optimal Cycle:	35	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Ovl			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 1	0	0 2	0 1	1	0 1	1 0	3	0 1	2 0	3	0 1
Volume Module:												
Base Vol:	386	12	291	103	226	110	59	1379	187	566	1008	65
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	386	12	291	103	226	110	59	1379	187	566	1008	65
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	386	12	291	103	226	110	59	1379	187	566	1008	65
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	386	12	291	103	226	110	59	1379	187	566	1008	65
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	386	12	291	103	226	110	59	1379	187	566	1008	65
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.75	0.94	0.94	0.85	0.95	0.91	0.85	0.92	0.91	0.85
Lanes:	1.94	0.06	2.00	0.63	1.37	1.00	1.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3516	109	2842	1113	2443	1615	1805	5187	1615	3502	5187	1615
Capacity Analysis Module:												
Vol/Sat:	0.11	0.11	0.10	0.09	0.09	0.07	0.03	0.27	0.12	0.16	0.19	0.04
Crit Moves:		****			****			****			****	
Green/Cycle:	0.17	0.17	0.41	0.14	0.14	0.14	0.09	0.40	0.40	0.24	0.55	0.55
Volume/Cap:	0.66	0.66	0.25	0.66	0.66	0.49	0.35	0.66	0.29	0.66	0.35	0.07
Delay/Veh:	41.9	41.9	19.5	44.1	44.1	41.4	43.8	25.2	20.5	36.1	12.5	10.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	41.9	41.9	19.5	44.1	44.1	41.4	43.8	25.2	20.5	36.1	12.5	10.5
LOS by Move:	D	D	B	D	D	D	D	C	C	D	B	B
HCM2k85thQ:	11	11	5	10	10	6	3	20	6	14	10	1

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #71 Glassell St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.744
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	43	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Ovl			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0 1	0 1		1 0 1	1 0		1 0 1	0 1		1 0 1	1 0	
Volume Module:												
Base Vol:	573	595	194	14	471	35	106	346	594	211	382	15
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	573	595	194	14	471	35	106	346	594	211	382	15
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	573	595	194	14	471	35	106	346	594	211	382	15
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	573	595	194	14	471	35	106	346	594	211	382	15
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	573	595	194	14	471	35	106	346	594	211	382	15
OvlAdjVol:			0						308			
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.03	1.00	1.00	1.06	1.00	1.00	1.03	1.00
Lanes:	2.00	1.00	1.00	1.00	1.86	0.14	1.00	1.00	1.00	1.00	1.92	0.08
Final Sat.:	3200	1700	1600	1600	3079	221	1600	1700	1600	1600	3179	121
Capacity Analysis Module:												
Vol/Sat:	0.18	0.35	0.12	0.01	0.15	0.16	0.07	0.20	0.37	0.13	0.12	0.12
OvlAdjV/S:			0.00						0.19			
Crit Moves:	****			****				****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #72 Glassell St and SR-22 WB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.910
Loss Time (sec):	5	Average Delay (sec/veh):	30.5
Optimal Cycle:	96	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	0	0 0	2 0	1	0 0	0 0	0	1 0	1! 0	1
Volume Module:												
Base Vol:	660	1079	0	0	1007	293	0	0	0	251	9	446
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	660	1079	0	0	1007	293	0	0	0	251	9	446
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	660	1079	0	0	1007	293	0	0	0	251	9	446
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	660	1079	0	0	1007	293	0	0	0	251	9	446
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	660	1079	0	0	1007	293	0	0	0	251	9	446
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.95	0.85	1.00	1.00	1.00	0.86	0.86	0.86
Lanes:	1.00	2.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	1.35	0.03	1.62
Final Sat.:	1805	3610	0	0	3610	1615	0	0	0	2200	41	2644
Capacity Analysis Module:												
Vol/Sat:	0.37	0.30	0.00	0.00	0.28	0.18	0.00	0.00	0.00	0.11	0.22	0.17
Crit Moves:	****				****						****	
Green/Cycle:	0.40	0.71	0.00	0.00	0.31	0.31	0.00	0.00	0.00	0.24	0.24	0.24
Volume/Cap:	0.91	0.42	0.00	0.00	0.91	0.59	0.00	0.00	0.00	0.47	0.91	0.70
Delay/Veh:	43.7	6.2	0.0	0.0	44.3	31.3	0.0	0.0	0.0	32.7	51.5	36.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	43.7	6.2	0.0	0.0	44.3	31.3	0.0	0.0	0.0	32.7	51.5	36.8
LOS by Move:	D	A	A	A	D	C	A	A	A	C	D	D
HCM2k85thQ:	33	11	0	0	29	13	0	0	0	8	21	14

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #73 Grand Ave / Glassell St and SR-22 EB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.931
Loss Time (sec):	5	Average Delay (sec/veh):	31.6
Optimal Cycle:	113	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 2	0 1	1	0 2	0 0	1	0 1!	0 1	0	0 0	0 0
Volume Module:												
Base Vol:	0	1450	370	461	824	0	229	4	503	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1450	370	461	824	0	229	4	503	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1450	370	461	824	0	229	4	503	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1450	370	461	824	0	229	4	503	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1450	370	461	824	0	229	4	503	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.95	0.85	0.95	0.95	1.00	0.86	0.86	0.86	1.00	1.00	1.00
Lanes:	0.00	2.00	1.00	1.00	2.00	0.00	1.31	0.01	1.68	0.00	0.00	0.00
Final Sat.:	0	3610	1615	1805	3610	0	2129	18	2731	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.40	0.23	0.26	0.23	0.00	0.11	0.23	0.18	0.00	0.00	0.00
Crit Moves:		****		****				****				
Green/Cycle:	0.00	0.43	0.43	0.27	0.71	0.00	0.24	0.24	0.24	0.00	0.00	0.00
Volume/Cap:	0.00	0.93	0.53	0.93	0.32	0.00	0.44	0.93	0.75	0.00	0.00	0.00
Delay/Veh:	0.0	37.4	21.8	59.8	5.7	0.0	32.2	54.5	38.4	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	37.4	21.8	59.8	5.7	0.0	32.2	54.5	38.4	0.0	0.0	0.0
LOS by Move:	A	D	C	E	A	A	C	D	D	A	A	A
HCM2k85thQ:	0	38	13	27	8	0	8	23	15	0	0	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #74 Grand Ave and Fairhaven Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.717
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	39	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	1 0	1 1	0	1 0	1 0	1
Volume Module:												
Base Vol:	54	1420	284	242	974	89	79	45	33	221	42	157
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	54	1420	284	242	974	89	79	45	33	221	42	157
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	54	1420	284	242	974	89	79	45	33	221	42	157
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	54	1420	284	242	974	89	79	45	33	221	42	157
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	54	1420	284	242	974	89	79	45	33	221	42	157
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	1.00	2.50	0.50	1.00	2.75	0.25	1.00	1.15	0.85	1.00	1.00	1.00
Final Sat.:	1600	4200	800	1600	4598	402	1600	1946	1354	1600	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.34	0.36	0.15	0.21	0.22	0.05	0.02	0.02	0.14	0.02	0.10
Crit Moves:			****	****				****		****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #75 Grand Ave and Santa Clara Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.808
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	54	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			WideBypass			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	1 0	0 1	0	1 0	1 0	1
Volume Module:												
Base Vol:	103	1386	288	240	946	240	114	139	26	157	282	300
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	103	1386	288	240	946	240	114	139	26	157	282	300
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	103	1386	288	240	946	240	114	139	26	157	282	300
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	103	1386	288	240	946	240	114	139	26	157	282	300
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	103	1386	288	240	946	240	114	139	26	157	282	300
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.00	1.00	1.00	1.06	1.00
Lanes:	1.00	2.48	0.52	1.00	2.39	0.61	1.00	0.84	0.16	1.00	1.00	1.00
Final Sat.:	1600	4174	826	1600	4029	971	1600	1348	252	1600	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.06	0.33	0.35	0.15	0.23	0.25	0.07	0.10	0.10	0.10	0.17	0.19
Crit Moves:			****	****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #76 Grand Ave and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.856
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	67	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	288	1026	259	294	634	140	321	823	138	279	1031	187
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	288	1026	259	294	634	140	321	823	138	279	1031	187
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	288	1026	259	294	634	140	321	823	138	279	1031	187
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	288	1026	259	294	634	140	321	823	138	279	1031	187
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	288	1026	259	294	634	140	321	823	138	279	1031	187
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.40	0.60	1.00	2.46	0.54	2.00	2.57	0.43	2.00	2.54	0.46
Final Sat.:	1600	4033	967	1600	4132	868	3200	4311	689	3200	4263	737
Capacity Analysis Module:												
Vol/Sat:	0.18	0.25	0.27	0.18	0.15	0.16	0.10	0.19	0.20	0.09	0.24	0.25
Crit Moves:			****	****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #77 Grand Ave and I-5 NB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.696
Loss Time (sec):	5	Average Delay (sec/veh):	10.1
Optimal Cycle:	38	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Ignore			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	2 0	1	1 0	3 0	0	0 0	0 0	0 0	2 0	0 0	1
Volume Module:												
Base Vol:	0	1855	1432	62	1501	0	0	0	0	151	0	182
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1855	1432	62	1501	0	0	0	0	151	0	182
User Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1855	0	62	1501	0	0	0	0	151	0	182
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1855	0	62	1501	0	0	0	0	151	0	182
PCE Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1855	0	62	1501	0	0	0	0	151	0	182
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.95	1.00	0.95	0.91	1.00	1.00	1.00	1.00	0.92	1.00	0.85
Lanes:	0.00	2.00	1.00	1.00	3.00	0.00	0.00	0.00	0.00	2.00	0.00	1.00
Final Sat.:	0	3610	1900	1805	5187	0	0	0	0	3502	0	1615
Capacity Analysis Module:												
Vol/Sat:	0.00	0.51	0.00	0.03	0.29	0.00	0.00	0.00	0.00	0.04	0.00	0.11
Crit Moves:		****		****								****
Green/Cycle:	0.00	0.74	0.00	0.05	0.79	0.00	0.00	0.00	0.00	0.16	0.00	0.16
Volume/Cap:	0.00	0.70	0.00	0.70	0.37	0.00	0.00	0.00	0.00	0.27	0.00	0.70
Delay/Veh:	0.0	7.8	0.0	68.1	3.2	0.0	0.0	0.0	0.0	36.9	0.0	47.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	7.8	0.0	68.1	3.2	0.0	0.0	0.0	0.0	36.9	0.0	47.5
LOS by Move:	A	A	A	E	A	A	A	A	A	D	A	D
HCM2k85thQ:	0	25	0	5	8	0	0	0	0	4	0	10

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #78 Grand Ave and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.721
Loss Time (sec):	5	Average Delay (sec/veh):	25.3
Optimal Cycle:	41	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	0 2	2 0	1	0 2	0 1	0	1 0
Volume Module:												
Base Vol:	185	2091	34	38	839	595	377	230	363	34	228	165
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	185	2091	34	38	839	595	377	230	363	34	228	165
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	185	2091	34	38	839	595	377	230	363	34	228	165
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	185	2091	34	38	839	595	377	230	363	34	228	165
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	185	2091	34	38	839	595	377	230	363	34	228	165
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	0.91	0.95	0.95	0.75	0.92	1.00	0.75	0.89	0.89	0.89
Lanes:	1.00	2.95	0.05	1.00	2.00	2.00	2.00	1.00	2.00	0.16	1.07	0.77
Final Sat.:	1805	5094	83	1805	3610	2842	3502	1900	2842	270	1809	1309
Capacity Analysis Module:												
Vol/Sat:	0.10	0.41	0.41	0.02	0.23	0.21	0.11	0.12	0.13	0.13	0.13	0.13
Crit Moves:		****		****					****	****		
Green/Cycle:	0.18	0.57	0.57	0.03	0.42	0.59	0.18	0.18	0.18	0.17	0.17	0.17
Volume/Cap:	0.56	0.72	0.72	0.72	0.56	0.35	0.61	0.68	0.72	0.72	0.72	0.72
Delay/Veh:	39.3	16.6	16.6	86.7	22.8	10.6	39.7	44.2	43.9	43.3	43.3	43.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	39.3	16.6	16.6	86.7	22.8	10.6	39.7	44.2	43.9	43.3	43.3	43.3
LOS by Move:	D	B	B	F	C	B	D	D	D	D	D	D
HCM2k85thQ:	9	27	27	4	16	8	10	12	12	12	12	12

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #79 Grand Ave and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.731
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	41	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	3 0	1	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	188	1435	43	113	667	266	326	1080	107	154	958	176
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	188	1435	43	113	667	266	326	1080	107	154	958	176
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	188	1435	43	113	667	266	326	1080	107	154	958	176
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	188	1435	43	113	667	266	326	1080	107	154	958	176
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	188	1435	43	113	667	266	326	1080	107	154	958	176
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	2.00	2.91	0.09	2.00	3.00	1.00	2.00	2.73	0.27	2.00	2.53	0.47
Final Sat.:	3200	4860	140	3200	5100	1600	3200	4567	433	3200	4255	745
Capacity Analysis Module:												
Vol/Sat:	0.06	0.30	0.31	0.04	0.13	0.17	0.10	0.24	0.25	0.05	0.23	0.24
Crit Moves:		****	****				****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #80 Grand Ave and Chestnut Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.688
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	36	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0 2	1 0	1	0 2	1 0	1	0 1	1 0	1	0 1	1 0
Volume Module:												
Base Vol:	152	1142	198	161	716	121	70	214	47	139	433	252
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	152	1142	198	161	716	121	70	214	47	139	433	252
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	152	1142	198	161	716	121	70	214	47	139	433	252
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	152	1142	198	161	716	121	70	214	47	139	433	252
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	152	1142	198	161	716	121	70	214	47	139	433	252
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	2.56	0.44	1.00	2.57	0.43	1.00	1.64	0.36	1.00	1.26	0.74
Final Sat.:	1600	4291	709	1600	4306	694	1600	2724	576	1600	2123	1177
Capacity Analysis Module:												
Vol/Sat:	0.10	0.27	0.28	0.10	0.17	0.17	0.04	0.08	0.08	0.09	0.20	0.21
Crit Moves:		****	****				****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #81 Grand Ave and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.821
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	57	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	222	1264	218	153	696	187	122	454	79	96	762	168
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	222	1264	218	153	696	187	122	454	79	96	762	168
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	222	1264	218	153	696	187	122	454	79	96	762	168
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	222	1264	218	153	696	187	122	454	79	96	762	168
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	222	1264	218	153	696	187	122	454	79	96	762	168
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	2.56	0.44	1.00	2.36	0.64	1.00	1.70	0.30	1.00	1.64	0.36
Final Sat.:	1600	4294	706	1600	3983	1017	1600	2826	474	1600	2722	578
Capacity Analysis Module:												
Vol/Sat:	0.14	0.29	0.31	0.10	0.17	0.18	0.08	0.16	0.17	0.06	0.28	0.29
Crit Moves:		****	****				****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #82 Grand Ave and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	1.004
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	244	1411	122	203	501	281	182	787	63	93	1673	217
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	244	1411	122	203	501	281	182	787	63	93	1673	217
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	244	1411	122	203	501	281	182	787	63	93	1673	217
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	244	1411	122	203	501	281	182	787	63	93	1673	217
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	244	1411	122	203	501	281	182	787	63	93	1673	217
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.76	0.24	1.00	2.00	1.00	1.00	2.78	0.22	1.00	2.66	0.34
Final Sat.:	1600	4618	382	1600	3400	1600	1600	4644	356	1600	4449	551
Capacity Analysis Module:												
Vol/Sat:	0.15	0.31	0.32	0.13	0.15	0.18	0.11	0.17	0.18	0.06	0.38	0.39
Crit Moves:		****	****				****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #83 Grand Ave and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.827
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	58	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	2 0	3 0	1	2 0	2 1	0
Volume Module:												
Base Vol:	183	660	146	140	452	208	306	587	127	241	1539	504
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	183	660	146	140	452	208	306	587	127	241	1539	504
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	183	660	146	140	452	208	306	587	127	241	1539	504
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	183	660	146	140	452	208	306	587	127	241	1539	504
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	183	660	146	140	452	208	306	587	127	241	1539	504
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	2.46	0.54	1.00	2.05	0.95	2.00	3.00	1.00	2.00	2.26	0.74
Final Sat.:	1600	4131	869	1600	3487	1513	3200	5100	1600	3200	3816	1184
Capacity Analysis Module:												
Vol/Sat:	0.11	0.16	0.17	0.09	0.13	0.14	0.10	0.12	0.08	0.08	0.40	0.43
Crit Moves:		****	****				****					****

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #84 SR-55 NB Ramps and Dyer Rd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.362
Loss Time (sec):	5	Average Delay (sec/veh):	5.7
Optimal Cycle:	20	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Ignore			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	1! 0	1	0	0	0	0	0	3	0	1	0	0	2	1	0
Volume Module:												
Base Vol:	181	0	49	0	0	0	0	1183	602	0	1488	929
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	181	0	49	0	0	0	0	1183	602	0	1488	929
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Volume:	181	0	49	0	0	0	0	1183	0	0	1488	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	181	0	49	0	0	0	0	1183	0	0	1488	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
FinalVolume:	181	0	49	0	0	0	0	1183	0	0	1488	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.93	1.00	0.93	1.00	1.00	1.00	1.00	0.91	1.00	1.00	0.91	0.91
Lanes:	1.79	0.00	1.21	0.00	0.00	0.00	0.00	3.00	1.00	0.00	3.00	0.00
Final Sat.:	3162	0	2146	0	0	0	0	5187	1900	0	5187	0
Capacity Analysis Module:												
Vol/Sat:	0.06	0.00	0.02	0.00	0.00	0.00	0.00	0.23	0.00	0.00	0.29	0.00
Crit Moves:	****						****				****	
Green/Cycle:	0.16	0.00	0.16	0.00	0.00	0.00	0.00	0.79	0.00	0.00	0.79	0.00
Volume/Cap:	0.36	0.00	0.14	0.00	0.00	0.00	0.00	0.29	0.00	0.00	0.36	0.00
Delay/Veh:	38.0	0.0	36.3	0.0	0.0	0.0	0.0	2.8	0.0	0.0	3.1	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	38.0	0.0	36.3	0.0	0.0	0.0	0.0	2.8	0.0	0.0	3.1	0.0
LOS by Move:	D	A	D	A	A	A	A	A	A	A	A	A
HCM2k85thQ:	5	0	2	0	0	0	0	6	0	0	8	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #85 Cambridge St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.779
Loss Time (sec):	5	Average Delay (sec/veh):	19.7
Optimal Cycle:	0	Level Of Service:	C

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	T	R		L	T	R		L	T	R		L	T	R	
Control:	Stop Sign				Stop Sign				Stop Sign				Stop Sign			
Rights:	Include				Include				Include				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Lanes:	1	0	1	0	0	0	0	1	0	1	1	0	0	0	1	0
Volume Module:																
Base Vol:	235	327	0		0	206	191		375	0	247		0	0	0	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	235	327	0		0	206	191		375	0	247		0	0	0	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	235	327	0		0	206	191		375	0	247		0	0	0	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	235	327	0		0	206	191		375	0	247		0	0	0	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	235	327	0		0	206	191		375	0	247		0	0	0	
Saturation Flow Module:																
Adjustment:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Lanes:	1.00	1.00	0.00		0.00	1.00	1.00		1.00	0.00	1.00		0.00	0.00	0.00	
Final Sat.:	474	510	0		0	485	534		482	0	567		0	0	0	
Capacity Analysis Module:																
Vol/Sat:	0.50	0.64	xxxx		xxxx	0.42	0.36		0.78	xxxx	0.44		xxxx	xxxx	xxxx	
Crit Moves:		****				****			****							
Delay/Veh:	17.0	20.9	0.0		0.0	14.9	12.6		30.8	0.0	13.4		0.0	0.0	0.0	
Delay Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
AdjDel/Veh:	17.0	20.9	0.0		0.0	14.9	12.6		30.8	0.0	13.4		0.0	0.0	0.0	
LOS by Move:	C	C	*		*	B	B		D	*	B		*	*	*	
ApproachDel:	19.3				13.8				23.9				xxxxxx			
Delay Adj:	1.00				1.00				1.00				xxxxx			
ApprAdjDel:	19.3				13.8				23.9				xxxxxx			
LOS by Appr:	C				B				C				*			
AllWayAvgQ:	0.9	1.6	0.0		0.0	0.7	0.5		2.8	0.0	0.7		0.0	0.0	0.0	

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #86 Cambridge St and Fairhaven Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.454
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	22	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	0	0 0	1 0	0	0 1	1 0	2	0 0	0 0	1	1 0
Volume Module:												
Base Vol:	0	0	0	219	0	76	132	230	0	0	237	295
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	219	0	76	132	230	0	0	237	295
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	219	0	76	132	230	0	0	237	295
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	219	0	76	132	230	0	0	237	295
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	219	0	76	132	230	0	0	237	295
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	0.00	0.00	0.00	1.00	0.00	1.00	1.00	2.00	0.00	0.00	1.00	1.00
Final Sat.:	0	0	0	1600	0	1600	1600	3400	0	0	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.14	0.00	0.05	0.08	0.07	0.00	0.00	0.14	0.18
Crit Moves:				****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #87 Mabury St and 1st Street

Cycle (sec):	100	Critical Vol./Cap.(X):	0.674
Loss Time (sec):	5	Average Delay (sec/veh):	27.1
Optimal Cycle:	36	Level Of Service:	C

Street Name:	Mabury						1st Street					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	0 1	1 0		1! 0 1	0 0		2 1 0	1 0		2 1 0
Volume Module:												
Base Vol:	23	0	233	195	83	367	0	1334	21	40	549	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	23	0	233	195	83	367	0	1334	21	40	549	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	23	0	233	195	83	367	0	1334	21	40	549	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	23	0	233	195	83	367	0	1334	21	40	549	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	23	0	233	195	83	367	0	1334	21	40	549	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	0.85	0.90	0.90	0.90	1.00	0.91	0.91	0.95	0.91	0.91
Lanes:	1.00	0.00	1.00	1.27	0.23	1.50	0.00	2.95	0.05	1.00	3.00	0.00
Final Sat.:	1805	0	1615	2171	390	2576	0	5096	80	1805	5187	0
Capacity Analysis Module:												
Vol/Sat:	0.01	0.00	0.14	0.09	0.21	0.14	0.00	0.26	0.26	0.02	0.11	0.00
Crit Moves:			****		****			****		****		
Green/Cycle:	0.21	0.00	0.21	0.32	0.32	0.32	0.00	0.39	0.39	0.03	0.42	0.00
Volume/Cap:	0.06	0.00	0.67	0.28	0.67	0.45	0.00	0.67	0.67	0.67	0.25	0.00
Delay/Veh:	31.4	0.0	41.3	25.8	31.7	27.6	0.0	26.3	26.3	74.4	18.8	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	31.4	0.0	41.3	25.8	31.7	27.6	0.0	26.3	26.3	74.4	18.8	0.0
LOS by Move:	C	A	D	C	C	C	A	C	C	E	B	A
HCM2k85thQ:	1	0	12	6	16	10	0	20	20	4	6	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #88 Tustin St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.381
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	19	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 1	0	0 1
Volume Module:												
Base Vol:	38	863	205	79	701	13	13	5	20	146	4	90
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	38	863	205	79	701	13	13	5	20	146	4	90
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	38	863	205	79	701	13	13	5	20	146	4	90
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	38	863	205	79	701	13	13	5	20	146	4	90
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	38	863	205	79	701	13	13	5	20	146	4	90
OvlAdjVol:												11
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	1.00	2.42	0.58	1.00	2.95	0.05	1.00	1.00	1.00	1.95	0.05	1.00
Final Sat.:	1600	4079	921	1600	4913	87	1600	1700	1600	3115	85	1600
Capacity Analysis Module:												
Vol/Sat:	0.02	0.21	0.22	0.05	0.14	0.15	0.01	0.00	0.01	0.05	0.05	0.06
OvlAdjV/S:												0.01
Crit Moves:		****	****					****		****		

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #89 Tustin St and SR-22 WB On-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.391
Loss Time (sec):	5	Average Delay (sec/veh):	11.1
Optimal Cycle:	29	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0 2	0 0	0	0 2	1 0	0	0 0	0 0	0	0 0	0 0
Volume Module:												
Base Vol:	466	1340	0	0	573	294	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	466	1340	0	0	573	294	0	0	0	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	466	1340	0	0	573	294	0	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	466	1340	0	0	573	294	0	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	466	1340	0	0	573	294	0	0	0	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.95	1.00	1.00	0.86	0.86	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	2.00	2.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00
Final Sat.:	3502	3610	0	0	3282	1641	0	0	0	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.13	0.37	0.00	0.00	0.17	0.18	0.00	0.00	0.00	0.00	0.00	0.00
Crit Moves:		****										
Green/Cycle:	0.65	0.95	0.00	0.00	0.30	0.30	0.00	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.21	0.39	0.00	0.00	0.57	0.59	0.00	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	7.3	0.3	0.0	0.0	29.9	30.2	0.0	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	7.3	0.3	0.0	0.0	29.9	30.2	0.0	0.0	0.0	0.0	0.0	0.0
LOS by Move:	A	A	A	A	C	C	A	A	A	A	A	A
HCM2k85thQ:	5	3	0	0	13	14	0	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #90 Tustin St and SR-22 EB Off-Ramp / Seba Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.679
Loss Time (sec):	5	Average Delay (sec/veh):	23.1
Optimal Cycle:	37	Level Of Service:	C

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	- T	- R		L	- T	- R		L	- T	- R		L	- T	- R	
Control:	Permitted				Permitted				Split Phase				Split Phase			
Rights:	Include				Include				Include				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Y+R:	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0	
Lanes:	0	0 2	1	0	1	0 3	0	0	1	0	1!	0 1	0	0	1!	0 0
Volume Module:																
Base Vol:	0	1416	4		40	550	0		440	39	558		28	0	67	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	0	1416	4		40	550	0		440	39	558		28	0	67	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	0	1416	4		40	550	0		440	39	558		28	0	67	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	0	1416	4		40	550	0		440	39	558		28	0	67	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	0	1416	4		40	550	0		440	39	558		28	0	67	
Saturation Flow Module:																
Sat/Lane:	1900	1900	1900		1900	1900	1900		1900	1900	1900		1900	1900	1900	
Adjustment:	1.00	0.91	0.91		0.10	0.91	1.00		0.90	0.90	0.90		0.89	1.00	0.89	
Lanes:	0.00	2.99	0.01		1.00	3.00	0.00		1.41	0.07	1.52		0.29	0.00	0.71	
Final Sat.:	0	5172	15		188	5187	0		2408	124	2596		499	0	1195	
Capacity Analysis Module:																
Vol/Sat:	0.00	0.27	0.27		0.21	0.11	0.00		0.18	0.31	0.21		0.06	0.00	0.06	
Crit Moves:		****								****			****			
Green/Cycle:	0.00	0.40	0.40		0.40	0.40	0.00		0.46	0.46	0.46		0.08	0.00	0.08	
Volume/Cap:	0.00	0.68	0.68		0.53	0.26	0.00		0.39	0.68	0.46		0.68	0.00	0.68	
Delay/Veh:	0.0	25.4	25.4		29.3	20.0	0.0		17.7	22.2	18.5		57.2	0.0	57.2	
User DelAdj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
AdjDel/Veh:	0.0	25.4	25.4		29.3	20.0	0.0		17.7	22.2	18.5		57.2	0.0	57.2	
LOS by Move:	A	C	C		C	B	A		B	C	B		E	A	E	
HCM2k85thQ:	0	21	21		3	6	0		10	20	12		7	0	7	

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #91 Tustin Ave and Fairhaven Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.685
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	36	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 0	2 0	2	1 0	1 0	1	1 0	1 0	1	0 1
Volume Module:												
Base Vol:	280	1492	132	356	940	108	89	200	163	100	220	208
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	280	1492	132	356	940	108	89	200	163	100	220	208
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	280	1492	132	356	940	108	89	200	163	100	220	208
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	280	1492	132	356	940	108	89	200	163	100	220	208
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	280	1492	132	356	940	108	89	200	163	100	220	208
OvlAdjVol:												30
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.76	0.24	2.00	2.69	0.31	1.00	1.10	0.90	1.00	1.00	1.00
Final Sat.:	3200	4610	390	3200	4505	495	1600	1863	1437	1600	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.09	0.32	0.34	0.11	0.21	0.22	0.06	0.11	0.11	0.06	0.13	0.13
OvlAdjV/S:												0.02
Crit Moves:		****	****				****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #92 Tustin Ave and Santa Clara Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.709
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	38	Level Of Service:	C

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	T	R		L	T	R		L	T	R		L	T	R	
Control:	Protected				Protected				Permitted				Permitted			
Rights:	Include				Include				Include				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Y+R:	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0	
Lanes:	1	0	2	1	0	2	0	2	1	0	1	0	1	1	0	
Volume Module:																
Base Vol:	191	923	142		199	1574	109		68	213	125		133	166	157	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	191	923	142		199	1574	109		68	213	125		133	166	157	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	191	923	142		199	1574	109		68	213	125		133	166	157	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	191	923	142		199	1574	109		68	213	125		133	166	157	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	191	923	142		199	1574	109		68	213	125		133	166	157	
Saturation Flow Module:																
Sat/Lane:	1600	1600	1600		1600	1600	1600		1600	1600	1600		1600	1600	1600	
Adjustment:	1.00	1.05	1.00		1.00	1.04	1.00		1.00	1.05	1.00		1.00	1.06	1.00	
Lanes:	1.00	2.60	0.40		2.00	2.81	0.19		1.00	1.26	0.74		1.00	1.03	0.97	
Final Sat.:	1600	4360	640		3200	4689	311		1600	2117	1183		1600	1745	1555	
Capacity Analysis Module:																
Vol/Sat:	0.12	0.21	0.22		0.06	0.34	0.35		0.04	0.10	0.11		0.08	0.10	0.10	
Crit Moves:	****						****				****		****			

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #93 Tustin Ave and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.797
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	51	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 0	1	2 0	2 1	0	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	253	1083	557	377	404	40	493	968	131	243	798	444
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	253	1083	557	377	404	40	493	968	131	243	798	444
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	253	1083	557	377	404	40	493	968	131	243	798	444
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	253	1083	557	377	404	40	493	968	131	243	798	444
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	253	1083	557	377	404	40	493	968	131	243	798	444
OvlAdjVol:			436									256
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.00	1.00	2.00	2.73	0.27	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	3400	1600	3200	4568	432	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.08	0.32	0.35	0.12	0.09	0.09	0.15	0.19	0.08	0.08	0.16	0.28
OvlAdjV/S:			0.27									0.16
Crit Moves:	****			****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #94 Tustin Ave and 4th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.708
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	38	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	2 0	2	0 1	1 0	2	1 1	1 0	2	1 0
Volume Module:												
Base Vol:	28	299	97	459	339	124	208	743	38	118	798	491
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	28	299	97	459	339	124	208	743	38	118	798	491
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	28	299	97	459	339	124	208	743	38	118	798	491
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	28	299	97	459	339	124	208	743	38	118	798	491
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	28	299	97	459	339	124	208	743	38	118	798	491
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	2.27	0.73	2.00	2.00	1.00	1.00	3.00	1.00	1.00	2.00	1.00
Final Sat.:	1600	3824	1176	3200	3400	1600	1600	5100	1600	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.02	0.08	0.08	0.14	0.10	0.08	0.13	0.15	0.02	0.07	0.23	0.31
Crit Moves:		****			****			****				****

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #95 SR-55 SB Ramps / Auto Mall Dr and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.702
Loss Time (sec):	5	Average Delay (sec/veh):	25.5
Optimal Cycle:	39	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Ignore			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 1	0	0 2	1	0 1	0 1	1	0 3	0 1	2	0 2	1 0
Volume Module:												
Base Vol:	401	32	423	110	50	37	47	1677	472	468	1220	146
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	401	32	423	110	50	37	47	1677	472	468	1220	146
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Volume:	401	32	423	110	50	37	47	1677	0	468	1220	146
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	401	32	423	110	50	37	47	1677	0	468	1220	146
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
FinalVolume:	401	32	423	110	50	37	47	1677	0	468	1220	146
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.96	0.96	0.75	0.95	1.00	0.85	0.95	0.91	1.00	0.92	0.90	0.90
Lanes:	1.85	0.15	2.00	1.00	1.00	1.00	1.00	3.00	1.00	2.00	2.68	0.32
Final Sat.:	3364	268	2842	1805	1900	1615	1805	5187	1900	3502	4558	546
Capacity Analysis Module:												
Vol/Sat:	0.12	0.12	0.15	0.06	0.03	0.02	0.03	0.32	0.00	0.13	0.27	0.27
Crit Moves:			****	****				****		****		
Green/Cycle:	0.21	0.21	0.21	0.09	0.09	0.09	0.06	0.46	0.00	0.19	0.59	0.59
Volume/Cap:	0.56	0.56	0.70	0.70	0.30	0.26	0.45	0.70	0.00	0.70	0.45	0.45
Delay/Veh:	36.2	36.2	40.2	57.7	43.9	43.7	48.7	22.4	0.0	41.2	11.4	11.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	36.2	36.2	40.2	57.7	43.9	43.7	48.7	22.4	0.0	41.2	11.4	11.4
LOS by Move:	D	D	D	E	D	D	D	C	A	D	B	B
HCM2k85thQ:	10	10	13	7	3	2	3	23	0	13	13	13

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Note: Queue reported is the number of cars per lane.
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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #96 SR-55 NB Ramps / Del Amo Ave and Newport Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	0.895
Loss Time (sec):	5	Average Delay (sec/veh):	39.1
Optimal Cycle:	87	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0 2	1 0		1 0 3	0 1		2 0 1	1 0		1 0 1	1 0	
Volume Module:												
Base Vol:	879	262	51	17	209	803	156	54	32	23	348	16
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	879	262	51	17	209	803	156	54	32	23	348	16
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	879	262	51	17	209	803	156	54	32	23	348	16
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	879	262	51	17	209	803	156	54	32	23	348	16
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	879	262	51	17	209	803	156	54	32	23	348	16
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.89	0.89	0.95	0.91	0.85	0.92	0.90	0.90	0.95	0.94	0.94
Lanes:	2.00	2.51	0.49	1.00	3.00	1.00	2.00	1.26	0.74	1.00	1.91	0.09
Final Sat.:	3502	4238	825	1805	5187	1615	3502	2140	1268	1805	3427	158
Capacity Analysis Module:												
Vol/Sat:	0.25	0.06	0.06	0.01	0.04	0.50	0.04	0.03	0.03	0.01	0.10	0.10
Crit Moves:	****					****	****				****	
Green/Cycle:	0.28	0.68	0.68	0.10	0.51	0.56	0.05	0.11	0.11	0.05	0.11	0.11
Volume/Cap:	0.89	0.09	0.09	0.09	0.08	0.89	0.89	0.23	0.23	0.23	0.89	0.89
Delay/Veh:	45.1	5.4	5.4	40.7	12.7	31.0	86.9	41.1	41.1	46.5	65.2	65.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	45.1	5.4	5.4	40.7	12.7	31.0	86.9	41.1	41.1	46.5	65.2	65.2
LOS by Move:	D	A	A	D	B	C	F	D	D	D	E	E
HCM2k85thQ:	25	2	2	1	2	36	8	2	2	1	14	14

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #97 Red Hill Ave and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.853
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	66	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Ovl			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0 3	0 1	2	0 3	0 1	2	0 3	0 1	2	0 3	0 1
Volume Module:												
Base Vol:	204	1469	233	105	209	169	410	1051	128	90	1161	567
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	204	1469	233	105	209	169	410	1051	128	90	1161	567
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	204	1469	233	105	209	169	410	1051	128	90	1161	567
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	204	1469	233	105	209	169	410	1051	128	90	1161	567
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	204	1469	233	105	209	169	410	1051	128	90	1161	567
OvlAdjVol:						0			26			
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.06	0.29	0.15	0.03	0.04	0.11	0.13	0.21	0.08	0.03	0.23	0.35
OvlAdjV/S:						0.00			0.02			
Crit Moves:	****			****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #98 Red Hill Ave and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.875
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	74	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0 3	1 0	2	0 2	1 0	1	0 2	1 0	2	0 3	0 1
Volume Module:												
Base Vol:	374	1281	147	79	304	385	246	668	104	76	1598	210
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	374	1281	147	79	304	385	246	668	104	76	1598	210
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	374	1281	147	79	304	385	246	668	104	76	1598	210
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	374	1281	147	79	304	385	246	668	104	76	1598	210
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	374	1281	147	79	304	385	246	668	104	76	1598	210
OvlAdjVol:												171
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	3.59	0.41	2.00	2.00	1.00	1.00	2.60	0.40	2.00	3.00	1.00
Final Sat.:	3200	6041	659	3200	3400	1600	1600	4353	647	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.12	0.21	0.22	0.02	0.09	0.24	0.15	0.15	0.16	0.02	0.31	0.13
OvlAdjV/S:												0.11
Crit Moves:	****					****	****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #99 Red Hill Ave and Dyer Rd / barranca Pkwy

Cycle (sec):	100	Critical Vol./Cap.(X):	0.537
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	25	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	4 0	1	2 0	4 0	1	2 0	3 1	0	2 0	4 0	1
Volume Module:												
Base Vol:	338	1270	327	241	408	192	172	637	84	110	839	245
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	338	1270	327	241	408	192	172	637	84	110	839	245
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	338	1270	327	241	408	192	172	637	84	110	839	245
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	338	1270	327	241	408	192	172	637	84	110	839	245
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	338	1270	327	241	408	192	172	637	84	110	839	245
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	4.00	1.00	2.00	4.00	1.00	2.00	3.53	0.47	2.00	4.00	1.00
Final Sat.:	3200	6800	1600	3200	6800	1600	3200	5954	746	3200	6800	1600
Capacity Analysis Module:												
Vol/Sat:	0.11	0.19	0.20	0.08	0.06	0.12	0.05	0.11	0.11	0.03	0.12	0.15
Crit Moves:			****	****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #100 Red Hill Ave and Alton Pkwy

Cycle (sec):	100	Critical Vol./Cap.(X):	1.184
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	2 0	1	2 0	1 0	1
Volume Module:												
Base Vol:	26	1458	211	122	519	4	434	1065	545	1299	222	801
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	26	1458	211	122	519	4	434	1065	545	1299	222	801
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	26	1458	211	122	519	4	434	1065	545	1299	222	801
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	26	1458	211	122	519	4	434	1065	545	1299	222	801
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	26	1458	211	122	519	4	434	1065	545	1299	222	801
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	2.00	1.00	2.00	1.00	1.00
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	3400	1600	3200	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.02	0.29	0.13	0.08	0.10	0.00	0.27	0.31	0.34	0.41	0.13	0.50
Crit Moves:		****			****			****				****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #101 Red Hill Ave and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.702	
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx	
Optimal Cycle:	37	Level Of Service:	C	

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ignore			Include			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0 2	1 0		2 0 3	0 1		2 0 3	0 1		1 0 3	0 1	
Volume Module:												
Base Vol:	175	1133	14	315	654	1148	499	230	35	39	810	568
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	175	1133	14	315	654	1148	499	230	35	39	810	568
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Volume:	175	1133	14	315	654	0	499	230	35	39	810	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	175	1133	14	315	654	0	499	230	35	39	810	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
FinalVolume:	175	1133	14	315	654	0	499	230	35	39	810	0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.96	0.04	2.00	3.00	1.00	2.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	4941	59	3200	5100	1600	3200	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.05	0.23	0.24	0.10	0.13	0.00	0.16	0.05	0.02	0.02	0.16	0.00
Crit Moves:			****	****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #102 Red Hill Ave and Main St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.813
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	55	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 0	1	2 0	2 0	1	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	436	941	254	88	640	348	200	714	193	277	1575	88
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	436	941	254	88	640	348	200	714	193	277	1575	88
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	436	941	254	88	640	348	200	714	193	277	1575	88
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	436	941	254	88	640	348	200	714	193	277	1575	88
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	436	941	254	88	640	348	200	714	193	277	1575	88
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.04	1.00
Lanes:	2.00	2.00	1.00	2.00	2.00	1.00	2.00	2.36	0.64	2.00	2.84	0.16
Final Sat.:	3200	3400	1600	3200	3400	1600	3200	3979	1021	3200	4746	254
Capacity Analysis Module:												
Vol/Sat:	0.14	0.28	0.16	0.03	0.19	0.22	0.06	0.18	0.19	0.09	0.33	0.35
Crit Moves:	****					****	****					****

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #103 I-5 SB Ramps and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.487
Loss Time (sec):	5	Average Delay (sec/veh):	18.4
Optimal Cycle:	24	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	0	2	0	0	2	0	3	0	2	1
Volume Module:												
Base Vol:	0	0	0	304	0	67	765	865	0	621		170
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	304	0	67	765	865	0	621		170
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	304	0	67	765	865	0	621		170
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	304	0	67	765	865	0	621		170
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	304	0	67	765	865	0	621		170
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.92	1.00	0.85	0.92	0.91	1.00	1.00	0.88	0.88
Lanes:	0.00	0.00	0.00	2.00	0.00	1.00	2.00	3.00	0.00	0.00	2.36	0.64
Final Sat.:	0	0	0	3502	0	1615	3502	5187	0	0	3942	1079
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.09	0.00	0.04	0.22	0.17	0.00	0.00	0.16	0.16
Crit Moves:				****			****				****	
Green/Cycle:	0.00	0.00	0.00	0.18	0.00	0.63	0.45	0.77	0.00	0.00	0.32	0.32
Volume/Cap:	0.00	0.00	0.00	0.49	0.00	0.07	0.49	0.22	0.00	0.00	0.49	0.49
Delay/Veh:	0.0	0.0	0.0	37.6	0.0	7.3	19.7	3.2	0.0	0.0	27.4	27.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

AdjDel/Veh:	0.0	0.0	0.0	37.6	0.0	7.3	19.7	3.2	0.0	0.0	27.4	27.4
LOS by Move:	A	A	A	D	A	A	B	A	A	A	C	C
HCM2k85thQ:	0	0	0	8	0	1	13	4	0	0	11	11

****

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #104 Tustin Ranch Rd and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.645
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	32	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	0 0	0	2 0	0 0	1	2 0	3 0	0	0 0	3 0	1
Volume Module:												
Base Vol:	0	0	0	553	0	38	65	2154	0	0	856	369
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	553	0	38	65	2154	0	0	856	369
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	553	0	38	65	2154	0	0	856	369
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	553	0	38	65	2154	0	0	856	369
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	553	0	38	65	2154	0	0	856	369
OvlAdjVol:												92
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	0.00	0.00	0.00	2.00	0.00	1.00	2.00	3.00	0.00	0.00	3.00	1.00
Final Sat.:	0	0	0	3200	0	1600	3200	5100	0	0	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.17	0.00	0.02	0.02	0.42	0.00	0.00	0.17	0.23
OvlAdjV/S:												0.06
Crit Moves:				****				****		****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #105 Von Karman Ave and Barranca Pkwy

Cycle (sec):	100	Critical Vol./Cap.(X):	1.228	
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx	
Optimal Cycle:	180	Level Of Service:	F	

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0 1	1 0		2 0 2		0 2	2 0 3		0 1	2 0 4		0 1
Volume Module:												
Base Vol:	346	1992	434	80	407	280	765	1397	195	232	734	250
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	346	1992	434	80	407	280	765	1397	195	232	734	250
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	346	1992	434	80	407	280	765	1397	195	232	734	250
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	346	1992	434	80	407	280	765	1397	195	232	734	250
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	346	1992	434	80	407	280	765	1397	195	232	734	250
OvlAdjVol:						0						
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	1.64	0.36	2.00	2.00	2.00	2.00	3.00	1.00	2.00	4.00	1.00
Final Sat.:	3200	2728	572	3200	3400	3200	3200	5100	1600	3200	6800	1600
Capacity Analysis Module:												
Vol/Sat:	0.11	0.73	0.76	0.03	0.12	0.09	0.24	0.27	0.12	0.07	0.11	0.16
OvlAdjV/S:						0.00						
Crit Moves:		****	****				****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #106 Red Hill Avenue and El Camino Real

Cycle (sec):	100	Critical Vol./Cap.(X):	0.779
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	48	Level Of Service:	C

Street Name:	Red Hill Avenue						El Camino Real					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0	3	0	1	1	0	2	1	0	1	0	1	0	1	0	1	0
Volume Module:												
Base Vol:	421	1334	246	20	728	96	70	153	201	332	548	63
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	421	1334	246	20	728	96	70	153	201	332	548	63
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	421	1334	246	20	728	96	70	153	201	332	548	63
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	421	1334	246	20	728	96	70	153	201	332	548	63
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	421	1334	246	20	728	96	70	153	201	332	548	63
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	2.00	3.00	1.00	1.00	2.65	0.35	1.00	1.00	1.00	1.00	0.90	0.10
Final Sat.:	3200	5100	1600	1600	4441	559	1600	1700	1600	1600	1435	165
Capacity Analysis Module:												
Vol/Sat:	0.13	0.26	0.15	0.01	0.16	0.17	0.04	0.09	0.13	0.21	0.38	0.38
Crit Moves:	****					****	****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #107 Red Hill Avenue and I-5 NB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.634
Loss Time (sec):	5	Average Delay (sec/veh):	23.7
Optimal Cycle:	33	Level Of Service:	C

Street Name:	Red Hill Avenue						I-5 NB Ramps					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	3	0	0	0	3	0	1	0	0	0	0	0	1	1	0	1	0
Volume Module:												
Base Vol:	277	1585	0	0	921	186	0	0	0	264	99	428
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	277	1585	0	0	921	186	0	0	0	264	99	428
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	277	1585	0	0	921	186	0	0	0	264	99	428
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	277	1585	0	0	921	186	0	0	0	264	99	428
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	277	1585	0	0	921	186	0	0	0	264	99	428
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	1.00	1.00	0.91	0.85	1.00	1.00	1.00	0.83	0.83	0.83
Lanes:	1.00	3.00	0.00	0.00	3.00	1.00	0.00	0.00	0.00	1.45	0.55	1.00
Final Sat.:	1805	5187	0	0	5187	1615	0	0	0	2292	860	1576
Capacity Analysis Module:												
Vol/Sat:	0.15	0.31	0.00	0.00	0.18	0.12	0.00	0.00	0.00	0.12	0.12	0.27
Crit Moves:	****				****							****
Green/Cycle:	0.24	0.52	0.00	0.00	0.28	0.28	0.00	0.00	0.00	0.43	0.43	0.43
Volume/Cap:	0.63	0.59	0.00	0.00	0.63	0.41	0.00	0.00	0.00	0.27	0.27	0.63
Delay/Veh:	37.0	16.8	0.0	0.0	32.5	29.9	0.0	0.0	0.0	18.5	18.5	23.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

AdjDel/Veh:	37.0	16.8	0.0	0.0	32.5	29.9	0.0	0.0	0.0	18.5	18.5	23.5
LOS by Move:	D	B	A	A	C	C	A	A	A	B	B	C
HCM2k85thQ:	13	19	0	0	15	8	0	0	0	6	6	17

****

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #108 Red Hill Avenue and I-5 SB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.695
Loss Time (sec):	5	Average Delay (sec/veh):	22.2
Optimal Cycle:	38	Level Of Service:	C

Street Name:	Red Hill Avenue						I-5 SB Ramps					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	4 0	1	1 0	3 0	0	0 1	0 0	1	0 0	0 0	0
Volume Module:												
Base Vol:	0	1551	492	334	802	0	296	3	265	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1551	492	334	802	0	296	3	265	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1551	492	334	802	0	296	3	265	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1551	492	334	802	0	296	3	265	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1551	492	334	802	0	296	3	265	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	0.85	0.95	0.91	1.00	0.92	0.92	0.85	1.00	1.00	1.00
Lanes:	0.00	4.00	1.00	1.00	3.00	0.00	0.99	0.01	1.00	0.00	0.00	0.00
Final Sat.:	0	6916	1615	1805	5187	0	1732	18	1615	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.22	0.30	0.19	0.15	0.00	0.17	0.17	0.16	0.00	0.00	0.00
Crit Moves:			****	****				****				
Green/Cycle:	0.00	0.44	0.44	0.27	0.70	0.00	0.25	0.25	0.25	0.00	0.00	0.00
Volume/Cap:	0.00	0.51	0.70	0.70	0.22	0.00	0.70	0.70	0.67	0.00	0.00	0.00
Delay/Veh:	0.0	20.5	25.7	37.5	5.2	0.0	39.2	39.2	38.4	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	20.5	25.7	37.5	5.2	0.0	39.2	39.2	38.4	0.0	0.0	0.0
LOS by Move:	A	C	C	D	A	A	D	D	D	A	A	A
HCM2k85thQ:	0	15	20	16	5	0	15	15	13	0	0	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #109 Red Hill Avenue and Nisson Road

Cycle (sec):	100	Critical Vol./Cap.(X):	0.693
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	36	Level Of Service:	B

Street Name:	Red Hill Avenue						Nisson Road					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 1	0	1 0	2 1	0	1 0	0 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	31	1614	33	146	750	75	247	90	40	48	83	141
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	31	1614	33	146	750	75	247	90	40	48	83	141
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	31	1614	33	146	750	75	247	90	40	48	83	141
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	31	1614	33	146	750	75	247	90	40	48	83	141
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	31	1614	33	146	750	75	247	90	40	48	83	141
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	3.92	0.08	1.00	2.73	0.27	1.00	0.69	0.31	1.00	0.37	0.63
Final Sat.:	1600	6572	128	1600	4564	436	1600	1108	492	1600	593	1007
Capacity Analysis Module:												
Vol/Sat:	0.02	0.25	0.26	0.09	0.16	0.17	0.15	0.08	0.08	0.03	0.14	0.14
Crit Moves:			****	****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #110 Red Hill Avenue and Walnut Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	0.739
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	42	Level Of Service:	C

Street Name:	Red Hill Avenue						Walnut Avenue					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	2	1	0	1	0	2	1	0	1	0	1	1	0	2	0	1	1	0
Volume Module:												
Base Vol:	273	1325	95	128	396	165	88	211	87	196	654	171
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	273	1325	95	128	396	165	88	211	87	196	654	171
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	273	1325	95	128	396	165	88	211	87	196	654	171
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	273	1325	95	128	396	165	88	211	87	196	654	171
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	273	1325	95	128	396	165	88	211	87	196	654	171
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	2.80	0.20	1.00	2.12	0.88	1.00	1.42	0.58	2.00	1.59	0.41
Final Sat.:	1600	4679	321	1600	3588	1412	1600	2366	934	3200	2637	663
Capacity Analysis Module:												
Vol/Sat:	0.17	0.28	0.30	0.08	0.11	0.12	0.06	0.09	0.09	0.06	0.25	0.26
Crit Moves:			****	****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #111 Red Hill Avenue and Valencia Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	0.738
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	42	Level Of Service:	C

Street Name:	Red Hill Avenue						Valencia Avenue					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3	1 0	1 0	2	1 0	1 0	1	0 1	2 0	1	0 1
Volume Module:												
Base Vol:	241	1791	295	109	438	49	100	270	119	291	422	279
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	241	1791	295	109	438	49	100	270	119	291	422	279
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	241	1791	295	109	438	49	100	270	119	291	422	279
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	241	1791	295	109	438	49	100	270	119	291	422	279
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	241	1791	295	109	438	49	100	270	119	291	422	279
OvlAdjVol:			213									170
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	3.43	0.57	1.00	2.70	0.30	1.00	1.00	1.00	2.00	1.00	1.00
Final Sat.:	1600	5795	905	1600	4517	483	1600	1700	1600	3200	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.15	0.31	0.33	0.07	0.10	0.10	0.06	0.16	0.07	0.09	0.25	0.17
OvlAdjV/S:			0.23									0.11
Crit Moves:	****			****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #112 Tustin Ranch Road and Warner Avenue North

Cycle (sec):	100	Critical Vol./Cap.(X):	0.885
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	79	Level Of Service:	D

Street Name:	Tustin Ranch Road						Warner Avenue North					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	3 0 1	2	0	3 0 0	0	0	0 0 0	3	0	0 0 2
Volume Module:												
Base Vol:	0	1916	1128	415	610	0	0	0	0	682	0	616
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1916	1128	415	610	0	0	0	0	682	0	616
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1916	1128	415	610	0	0	0	0	682	0	616
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1916	1128	415	610	0	0	0	0	682	0	616
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1916	1128	415	610	0	0	0	0	682	0	616
OvlAdjVol:			901									201
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	0.00	3.00	1.00	2.00	3.00	0.00	0.00	0.00	0.00	3.00	0.00	2.00
Final Sat.:	0	5100	1600	3200	5100	0	0	0	0	4800	0	3200
Capacity Analysis Module:												
Vol/Sat:	0.00	0.38	0.71	0.13	0.12	0.00	0.00	0.00	0.00	0.14	0.00	0.19
OvlAdjV/S:			0.56									0.06
Crit Moves:			****	****						****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #113 Tustin Ranch Road and Walnut Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	1.715
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Street Name:	Tustin Ranch Road						Walnut Avenue					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Ovl			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0	3	0	1	2	0	3	0	1	2	0	1	1	0	2	0	2	0	1
Volume Module:																				
Base Vol:	842	3666	385	550	1460	947	329	402	230	297	1703	1349
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	842	3666	385	550	1460	947	329	402	230	297	1703	1349
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	842	3666	385	550	1460	947	329	402	230	297	1703	1349
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	842	3666	385	550	1460	947	329	402	230	297	1703	1349
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	842	3666	385	550	1460	947	329	402	230	297	1703	1349
OvlAdjVol:			237			783						1074
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	1.27	0.73	2.00	2.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	2135	1165	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.26	0.72	0.24	0.17	0.29	0.59	0.10	0.19	0.20	0.09	0.50	0.84
OvlAdjV/S:			0.15			0.49						0.67
Crit Moves:	****			****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #114 SR-55 SB Ramps and Irvine Boulevard

Cycle (sec):	100	Critical Vol./Cap.(X):	0.915
Loss Time (sec):	5	Average Delay (sec/veh):	25.6
Optimal Cycle:	100	Level Of Service:	C

Street Name:	SR-55 SB Ramps						Irvine Boulevard					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	0	1	0	1! 0	1	0	2	1	0	1
Volume Module:												
Base Vol:	0	0	0	467	0	279	0	987	665	461	1324	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	467	0	279	0	987	665	461	1324	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	467	0	279	0	987	665	461	1324	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	467	0	279	0	987	665	461	1324	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	467	0	279	0	987	665	461	1324	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.74	1.00	0.73	1.00	0.86	0.86	0.95	0.91	1.00
Lanes:	0.00	0.00	0.00	1.62	0.00	1.38	0.00	2.00	1.00	1.00	3.00	0.00
Final Sat.:	0	0	0	2278	0	1915	0	3251	1625	1805	5187	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.20	0.00	0.15	0.00	0.30	0.41	0.26	0.26	0.00
Crit Moves:				****					****	****		
Green/Cycle:	0.00	0.00	0.00	0.22	0.00	0.45	0.00	0.45	0.45	0.28	0.73	0.00
Volume/Cap:	0.00	0.00	0.00	0.92	0.00	0.33	0.00	0.68	0.92	0.92	0.35	0.00
Delay/Veh:	0.0	0.0	0.0	52.8	0.0	17.9	0.0	22.7	33.6	56.2	5.1	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	52.8	0.0	17.9	0.0	22.7	33.6	56.2	5.1	0.0
LOS by Move:	A	A	A	D	A	B	A	C	C	E	A	A

```
HCM2k85thQ:     0    0     0    19    0     6     0   21    36    26    9     0
*******************************************************************************
Note: Queue reported is the number of cars per lane.
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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #115 SR-55 NB Ramps and Irvine Boulevard

Cycle (sec):	100	Critical Vol./Cap.(X):	0.932
Loss Time (sec):	5	Average Delay (sec/veh):	22.9
Optimal Cycle:	113	Level Of Service:	C

Street Name:	SR-55 NB Ramps						Irvine Boulevard					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	1! 0	1	0	0	0	0	1	0	3	0
Volume Module:												
Base Vol:	367	18	333	0	0	0	254	1163	0	0	1336	845
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	367	18	333	0	0	0	254	1163	0	0	1336	845
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	367	18	333	0	0	0	254	1163	0	0	1336	845
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	367	18	333	0	0	0	254	1163	0	0	1336	845
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	367	18	333	0	0	0	254	1163	0	0	1336	845
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.86	0.86	0.86	1.00	1.00	1.00	0.95	0.91	1.00	1.00	0.86	0.86
Lanes:	1.50	0.05	1.45	0.00	0.00	0.00	1.00	3.00	0.00	0.00	2.00	1.00
Final Sat.:	2444	80	2369	0	0	0	1805	5187	0	0	3257	1629
Capacity Analysis Module:												
Vol/Sat:	0.15	0.23	0.14	0.00	0.00	0.00	0.14	0.22	0.00	0.00	0.41	0.52
Crit Moves:	****						****					****
Green/Cycle:	0.24	0.24	0.24	0.00	0.00	0.00	0.15	0.71	0.00	0.00	0.56	0.56
Volume/Cap:	0.62	0.93	0.58	0.00	0.00	0.00	0.93	0.32	0.00	0.00	0.74	0.93
Delay/Veh:	34.8	55.0	34.1	0.0	0.0	0.0	78.6	5.6	0.0	0.0	17.6	27.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	34.8	55.0	34.1	0.0	0.0	0.0	78.6	5.6	0.0	0.0	17.6	27.9
LOS by Move:	C	D	C	A	A	A	E	A	A	A	B	C
HCM2k85thQ:	12	23	11	0	0	0	17	8	0	0	26	44

Note: Queue reported is the number of cars per lane.

APPENDIX D.5

ANALYSIS WORKSHEETS –
2045 WP (AM Peak Hour)

Santa Ana Circulation Element

Scenario Report

Scenario:	2045 WP AM Peak Hour
Command:	Default Command
Volume:	2045 AM
Geometry:	SACE
Impact Fee:	Default Impact Fee
Trip Generation:	Default Trip Generation
Trip Distribution:	Default Trip Distribution
Paths:	Default Path
Routes:	Default Route
Configuration:	Default Configuration

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Santa Ana Circulation Element

Impact Analysis Report
Level Of Service

Intersection	Base LOS	Base Del/ Veh	Base V/ C	Future LOS	Future Del/ Veh	Future V/ C	Change in
# 1 Euclid St and 1st St	C	xxxxx	0.789	C	xxxxx	0.789	+ 0.000 V/C
# 2 Euclid St and McFadden Ave	E	xxxxx	0.906	E	xxxxx	0.906	+ 0.000 V/C
# 3 Euclid St and Edinger Ave	F	xxxxx	1.008	F	xxxxx	1.008	+ 0.000 V/C
# 4 Newhope St and Hazard Ave	A	xxxxx	0.600	A	xxxxx	0.600	+ 0.000 V/C
# 5 Newhope St and McFadden Ave	C	xxxxx	0.728	C	xxxxx	0.728	+ 0.000 V/C
# 6 Harbor Blvd and SR-22 WB Off-R	C	28.5	0.715	C	28.5	0.715	+ 0.000 D/V
# 7 Trask Avenue and SR-22 EB On-R	B	11.0	0.389	B	11.0	0.389	+ 0.000 D/V
# 8 Harbor Blvd and Westminster Av	E	xxxxx	0.936	E	xxxxx	0.936	+ 0.000 V/C
# 9 Harbor Blvd and 1st St	C	xxxxx	0.749	C	xxxxx	0.749	+ 0.000 V/C
# 10 Harbor Blvd and McFadden Ave	C	xxxxx	0.729	C	xxxxx	0.729	+ 0.000 V/C
# 11 Harbor Blvd and Edinger Ave	D	xxxxx	0.857	D	xxxxx	0.857	+ 0.000 V/C
# 12 Harbor Blvd and Warner Ave	C	xxxxx	0.800	C	xxxxx	0.800	+ 0.000 V/C
# 13 Harbor Blvd and Segerstrom Ave	F	xxxxx	1.007	F	xxxxx	1.007	+ 0.000 V/C
# 14 MacArthur Blvd and Hyland Ave	B	xxxxx	0.615	B	xxxxx	0.615	+ 0.000 V/C
# 15 MacArthur Blvd and Harbor Blvd	C	xxxxx	0.747	C	xxxxx	0.747	+ 0.000 V/C
# 16 Harbor Blvd and Sunflower Ave	C	xxxxx	0.705	C	xxxxx	0.705	+ 0.000 V/C
# 17 Harbor Blvd and I-405 NB Off-R	B	17.9	0.573	B	17.9	0.573	+ 0.000 D/V
# 18 Harbor Blvd and I-405 SB Off-R	B	15.0	0.618	B	15.0	0.618	+ 0.000 D/V
# 19 Fairview St and Civic Center D	B	xxxxx	0.676	B	xxxxx	0.676	+ 0.000 V/C
# 20 Fairview St and 1st St	D	xxxxx	0.887	D	xxxxx	0.887	+ 0.000 V/C
# 21 Fairview St and McFadden Ave	D	xxxxx	0.800	D	xxxxx	0.800	+ 0.000 V/C
# 22 Fairview St and Edinger Ave	D	xxxxx	0.882	D	xxxxx	0.882	+ 0.000 V/C
# 23 Fairview St and Warner Ave	D	xxxxx	0.847	D	xxxxx	0.847	+ 0.000 V/C
# 24 Fairview St and MacArthur Blvd	B	xxxxx	0.690	B	xxxxx	0.690	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Base Del/ Veh	Base V/ C	Future LOS	Future Del/ Veh	Future V/ C	Change in
# 25 Fairview Rd and Sunflower Ave	B	xxxxx	0.687	B	xxxxx	0.687	+ 0.000 V/C
# 26 Greenville St and Edinger Ave	C	xxxxx	0.742	C	xxxxx	0.742	+ 0.000 V/C
# 27 Greenville St and Segerstrom A	D	xxxxx	0.878	D	xxxxx	0.878	+ 0.000 V/C
# 28 Raitt St and McFadden Ave	C	xxxxx	0.704	C	xxxxx	0.704	+ 0.000 V/C
# 29 Raitt St and Edinger Ave	E	xxxxx	0.926	E	xxxxx	0.926	+ 0.000 V/C
# 30 Bear St and MacArthur Blvd	C	xxxxx	0.756	C	xxxxx	0.756	+ 0.000 V/C
# 31 Bristol St and 17th St	D	xxxxx	0.853	D	xxxxx	0.853	+ 0.000 V/C
# 32 Bristol St and Civic Center Dr	E	xxxxx	0.931	E	xxxxx	0.931	+ 0.000 V/C
# 33 Bristol St and Santa Ana Blvd	C	xxxxx	0.721	C	xxxxx	0.721	+ 0.000 V/C
# 34 Bristol St and 1st St	E	xxxxx	0.902	E	xxxxx	0.902	+ 0.000 V/C
# 35 Bristol St and McFadden Ave	E	xxxxx	0.947	E	xxxxx	0.947	+ 0.000 V/C
# 36 Bristol St and Warner Ave	E	xxxxx	0.956	E	xxxxx	0.956	+ 0.000 V/C
# 37 Bristol St and Segerstrom Ave	D	xxxxx	0.854	D	xxxxx	0.854	+ 0.000 V/C
# 38 Bristol St and Alton Ave	A	xxxxx	0.553	A	xxxxx	0.553	+ 0.000 V/C
# 39 Bristol St and MacArthur Blvd	C	xxxxx	0.759	C	xxxxx	0.759	+ 0.000 V/C
# 40 Bristol St and Sunflower Ave	B	xxxxx	0.682	B	xxxxx	0.682	+ 0.000 V/C
# 41 Bristol St and I-405 NB Ramps	C	20.9	0.568	C	20.9	0.568	+ 0.000 D/V
# 42 Bristol St and I-405 SB Ramps	C	23.6	0.697	C	23.6	0.697	+ 0.000 D/V
# 43 Flower St and Santa Ana Blvd	A	xxxxx	0.586	A	xxxxx	0.586	+ 0.000 V/C
# 44 Flower St and 1st St	E	xxxxx	0.926	E	xxxxx	0.926	+ 0.000 V/C
# 45 Flower St and McFadden Ave	D	xxxxx	0.807	D	xxxxx	0.807	+ 0.000 V/C
# 46 Flower St and Segerstrom Ave	D	xxxxx	0.818	D	xxxxx	0.818	+ 0.000 V/C
# 47 Flower St and MacArthur Blvd	C	xxxxx	0.710	C	xxxxx	0.710	+ 0.000 V/C
# 48 Main St and La Veta Ave	A	xxxxx	0.536	A	xxxxx	0.536	+ 0.000 V/C
# 49 Main St and Mainplace Dr / Mem	A	xxxxx	0.456	A	xxxxx	0.456	+ 0.000 V/C
# 50 Main St and 17th St	D	xxxxx	0.845	D	xxxxx	0.845	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Base Del/ Veh	Base V/ C	Future LOS	Future Del/ Veh	Future V/ C	Change in
# 51 Main St and Civic Center Dr	D	xxxxx	0.848	D	xxxxx	0.848	+ 0.000 V/C
# 52 Main St and Santa Ana Blvd	C	xxxxx	0.791	C	xxxxx	0.791	+ 0.000 V/C
# 53 Main St and 4th St	A	xxxxx	0.441	A	xxxxx	0.441	+ 0.000 V/C
# 54 Main St and 1st St	C	xxxxx	0.754	C	xxxxx	0.754	+ 0.000 V/C
# 55 Main St and McFadden Ave	E	xxxxx	0.916	E	xxxxx	0.916	+ 0.000 V/C
# 56 Main St and Edinger Ave	D	xxxxx	0.876	D	xxxxx	0.876	+ 0.000 V/C
# 57 Main St and MacArthur Blvd	C	xxxxx	0.721	C	xxxxx	0.721	+ 0.000 V/C
# 58 Penn Wy and 17th St	B	12.1	0.648	B	12.1	0.648	+ 0.000 D/V
# 59 I-5 NB Off Ramps/17th Street	C	31.4	0.792	C	31.4	0.792	+ 0.000 D/V
# 60 Penn Wy and I-5 SB Ramps	B	19.5	0.462	B	19.5	0.462	+ 0.000 D/V
# 61 Santiago St and Civic Center D	F	62.6	1.147	F	62.6	1.147	+ 0.000 V/C
# 62 Santiago St and Santa Ana Blvd	F	xxxxx	1.444	F	xxxxx	1.444	+ 0.000 V/C
# 63 Standard Ave and 4th St	F	xxxxx	1.340	F	xxxxx	1.340	+ 0.000 V/C
# 64 Standard Ave and 1st St	F	xxxxx	1.515	F	xxxxx	1.515	+ 0.000 V/C
# 65 Standard Ave and Mcfadden Ave	D	xxxxx	0.843	D	xxxxx	0.843	+ 0.000 V/C
# 66 Halladay St and Warner Ave	B	xxxxx	0.692	B	xxxxx	0.692	+ 0.000 V/C
# 67 Halladay St and Dyer Rd	B	xxxxx	0.668	B	xxxxx	0.668	+ 0.000 V/C
# 68 SR-55 SB Ramps and MacArthur B	C	20.2	0.623	C	20.2	0.623	+ 0.000 D/V
# 69 SR-55 NB Ramps and MacArthur B	B	19.5	0.728	B	19.5	0.728	+ 0.000 D/V
# 70 SR-55 SB Ramps and Dyer Rd	C	25.9	0.610	C	25.9	0.610	+ 0.000 D/V
# 71 Glassell St and La Veta Ave	D	xxxxx	0.826	D	xxxxx	0.826	+ 0.000 V/C
# 72 Glassell St and SR-22 WB Ramps	C	30.1	0.841	C	30.1	0.841	+ 0.000 D/V
# 73 Grand Ave / Glassell St and SR	C	29.6	0.842	C	29.6	0.842	+ 0.000 D/V
# 74 Grand Ave and Fairhaven Ave	A	xxxxx	0.569	A	xxxxx	0.569	+ 0.000 V/C
# 75 Grand Ave and Santa Clara Ave	C	xxxxx	0.798	C	xxxxx	0.798	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Base Del/ Veh	Base V/ C	Future LOS	Future Del/ Veh	Future V/ C	Change in
# 76 Grand Ave and 17th St	D	xxxxx	0.856	D	xxxxx	0.856	+ 0.000 V/C
# 77 Grand Ave and I-5 NB Ramps	B	15.3	0.845	B	15.3	0.845	+ 0.000 D/V
# 78 Grand Ave and Santa Ana Blvd	C	26.3	0.881	C	26.3	0.881	+ 0.000 D/V
# 79 Grand Ave and 1st St	D	xxxxx	0.827	D	xxxxx	0.827	+ 0.000 V/C
# 80 Grand Ave and Chestnut Ave	E	xxxxx	0.935	E	xxxxx	0.935	+ 0.000 V/C
# 81 Grand Ave and McFadden Ave	F	xxxxx	1.011	F	xxxxx	1.011	+ 0.000 V/C
# 82 Grand Ave and Edinger Ave	D	xxxxx	0.897	D	xxxxx	0.897	+ 0.000 V/C
# 83 Grand Ave and Warner Ave	B	xxxxx	0.664	B	xxxxx	0.664	+ 0.000 V/C
# 84 SR-55 NB Ramps and Dyer Rd	B	17.3	0.565	B	17.3	0.565	+ 0.000 D/V
# 85 Cambridge St and La Veta Ave	C	23.4	0.900	C	23.4	0.900	+ 0.000 V/C
# 86 Cambridge St and Fairhaven Ave	B	xxxxx	0.624	B	xxxxx	0.624	+ 0.000 V/C
# 87 Mabury St and 1st Street	C	27.4	0.755	C	27.4	0.755	+ 0.000 D/V
# 88 Tustin St and La Veta Ave	A	xxxxx	0.362	A	xxxxx	0.362	+ 0.000 V/C
# 89 Tustin St and SR-22 WB On-Ramp	B	13.2	0.575	B	13.2	0.575	+ 0.000 D/V
# 90 Tustin St and SR-22 EB Off-Ram	C	23.8	0.718	C	23.8	0.718	+ 0.000 D/V
# 91 Tustin Ave and Fairhaven Ave	F	xxxxx	1.242	F	xxxxx	1.242	+ 0.000 V/C
# 92 Tustin Ave and Santa Clara Ave	F	xxxxx	1.097	F	xxxxx	1.097	+ 0.000 V/C
# 93 Tustin Ave and 17th St	D	xxxxx	0.805	D	xxxxx	0.805	+ 0.000 V/C
# 94 Tustin Ave and 4th St	C	xxxxx	0.791	C	xxxxx	0.791	+ 0.000 V/C
# 95 SR-55 SB Ramps / Auto Mall Dr	C	25.1	0.653	C	25.1	0.653	+ 0.000 D/V
# 96 SR-55 NB Ramps / Del Amo Ave a	C	25.6	0.665	C	25.6	0.665	+ 0.000 D/V
# 97 Red Hill Ave and Edinger Ave	B	xxxxx	0.619	B	xxxxx	0.619	+ 0.000 V/C
# 98 Red Hill Ave and Warner Ave	A	xxxxx	0.580	A	xxxxx	0.580	+ 0.000 V/C
# 99 Red Hill Ave and Dyer Rd / bar	A	xxxxx	0.577	A	xxxxx	0.577	+ 0.000 V/C
#100 Red Hill Ave and Alton Pkwy	E	xxxxx	0.930	E	xxxxx	0.930	+ 0.000 V/C
#101 Red Hill Ave and MacArthur Blv	C	xxxxx	0.709	C	xxxxx	0.709	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Del/ Veh	V/ C	Future LOS	Del/ Veh	V/ C	Change in
#102 Red Hill Ave and Main St	C	xxxxx	0.733	C	xxxxx	0.733	+ 0.000 V/C
#103 I-5 SB Ramps and Santa Ana Blv	B	17.8	0.480	B	17.8	0.480	+ 0.000 D/V
#104 Tustin Ranch Rd and Warner Ave	A	xxxxx	0.530	A	xxxxx	0.530	+ 0.000 V/C
#105 Von Karman Ave and Barranca Pk	E	xxxxx	0.930	E	xxxxx	0.930	+ 0.000 V/C
#106 Red Hill Avenue and El Camino	B	xxxxx	0.640	B	xxxxx	0.640	+ 0.000 V/C
#107 Red Hill Avenue and I-5 NB Ram	C	20.1	0.631	C	20.1	0.631	+ 0.000 D/V
#108 Red Hill Avenue and I-5 SB Ram	C	24.7	0.824	C	24.7	0.824	+ 0.000 D/V
#109 Red Hill Avenue and Nisson Roa	B	xxxxx	0.655	B	xxxxx	0.655	+ 0.000 V/C
#110 Red Hill Avenue and Walnut Ave	B	xxxxx	0.643	B	xxxxx	0.643	+ 0.000 V/C
#111 Red Hill Avenue and Valencia A	B	xxxxx	0.677	B	xxxxx	0.677	+ 0.000 V/C
#112 Tustin Ranch Road and Warner A	A	xxxxx	0.459	A	xxxxx	0.459	+ 0.000 V/C
#113 Tustin Ranch Road and Walnut A	F	xxxxx	1.313	F	xxxxx	1.313	+ 0.000 V/C
#114 SR-55 SB Ramps and Irvine Boul	E	55.4	1.176	E	55.4	1.176	+ 0.000 D/V
#115 SR-55 NB Ramps and Irvine Boul	C	20.3	0.824	C	20.3	0.824	+ 0.000 D/V

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #1 Euclid St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.789
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	50	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			WideBypass			WideBypass		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	100	845	145	194	1725	97	194	721	136	190	606	194
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	100	845	145	194	1725	97	194	721	136	190	606	194
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	100	845	145	194	1725	97	194	721	136	190	606	194
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	100	845	145	194	1725	97	194	721	136	190	606	194
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	100	845	145	194	1725	97	194	721	136	190	606	194
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	1.00	2.56	0.44	1.00	2.84	0.16	1.00	2.52	0.48	1.00	2.27	0.73
Final Sat.:	1600	4297	703	1600	4744	256	1600	4238	762	1600	3836	1164
Capacity Analysis Module:												
Vol/Sat:	0.06	0.20	0.21	0.12	0.36	0.38	0.12	0.17	0.18	0.12	0.16	0.17
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #2 Euclid St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.906
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	90	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	156	1185	141	146	2031	168	227	463	287	106	234	89
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	156	1185	141	146	2031	168	227	463	287	106	234	89
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	156	1185	141	146	2031	168	227	463	287	106	234	89
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	156	1185	141	146	2031	168	227	463	287	106	234	89
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	156	1185	141	146	2031	168	227	463	287	106	234	89
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.04	1.00
Lanes:	1.00	2.68	0.32	1.00	2.77	0.23	1.00	1.23	0.77	1.00	1.45	0.55
Final Sat.:	1600	4490	510	1600	4633	367	1600	2075	1225	1600	2418	882
Capacity Analysis Module:												
Vol/Sat:	0.10	0.26	0.28	0.09	0.44	0.46	0.14	0.22	0.23	0.07	0.10	0.10
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #3 Euclid St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	1.008
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	1 0	2 0	1	1 0	2 0	1
Volume Module:												
Base Vol:	127	801	68	180	2613	240	136	595	364	133	812	148
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	127	801	68	180	2613	240	136	595	364	133	812	148
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	127	801	68	180	2613	240	136	595	364	133	812	148
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	127	801	68	180	2613	240	136	595	364	133	812	148
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	127	801	68	180	2613	240	136	595	364	133	812	148
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.77	0.23	2.00	2.75	0.25	1.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	3200	4624	376	3200	4596	404	1600	3400	1600	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.04	0.17	0.18	0.06	0.57	0.59	0.09	0.17	0.23	0.08	0.24	0.09
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #4 Newhope St and Hazard Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.600
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	29	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Prot+Permit			Prot+Permit			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	0 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	94	708	101	86	1262	116	17	44	33	17	57	23
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	94	708	101	86	1262	116	17	44	33	17	57	23
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	94	708	101	86	1262	116	17	44	33	17	57	23
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	94	708	101	86	1262	116	17	44	33	17	57	23
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	94	708	101	86	1262	116	17	44	33	17	57	23
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.03	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.75	0.25	1.00	1.83	0.17	1.00	0.57	0.43	1.00	0.71	0.29
Final Sat.:	1600	2900	400	1600	3031	269	1600	914	686	1600	1140	460
Capacity Analysis Module:												
Vol/Sat:	0.06	0.24	0.25	0.05	0.42	0.43	0.01	0.05	0.05	0.01	0.05	0.05
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #5 Newhope St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.728
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	40	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	77	342	120	116	1153	107	106	386	125	122	311	69
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	77	342	120	116	1153	107	106	386	125	122	311	69
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	77	342	120	116	1153	107	106	386	125	122	311	69
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	77	342	120	116	1153	107	106	386	125	122	311	69
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	77	342	120	116	1153	107	106	386	125	122	311	69
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	1.48	0.52	1.00	1.83	0.17	1.00	1.51	0.49	1.00	1.64	0.36
Final Sat.:	1600	2469	831	1600	3028	272	1600	2517	783	1600	2719	581
Capacity Analysis Module:												
Vol/Sat:	0.05	0.14	0.14	0.07	0.38	0.39	0.07	0.15	0.16	0.08	0.11	0.12
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #6 Harbor Blvd and SR-22 WB Off-Ramp / Banner Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.715
Loss Time (sec):	5	Average Delay (sec/veh):	28.5
Optimal Cycle:	40	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	0	0 0	2 1	0	0 0	1! 0	0	1 1	0 0	1
Volume Module:												
Base Vol:	83	1508	0	0	1354	18	81	0	133	836	44	117
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	83	1508	0	0	1354	18	81	0	133	836	44	117
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	83	1508	0	0	1354	18	81	0	133	836	44	117
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	83	1508	0	0	1354	18	81	0	133	836	44	117
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	83	1508	0	0	1354	18	81	0	133	836	44	117
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	1.00	1.00	0.91	0.91	0.90	1.00	0.90	0.96	0.96	0.85
Lanes:	1.00	3.00	0.00	0.00	2.96	0.04	0.38	0.00	0.62	1.90	0.10	1.00
Final Sat.:	1805	5187	0	0	5109	68	646	0	1061	3448	181	1615
Capacity Analysis Module:												
Vol/Sat:	0.05	0.29	0.00	0.00	0.27	0.27	0.13	0.00	0.13	0.24	0.24	0.07
Crit Moves:	****				****		****				****	
Green/Cycle:	0.06	0.44	0.00	0.00	0.37	0.37	0.18	0.00	0.18	0.34	0.34	0.34
Volume/Cap:	0.71	0.67	0.00	0.00	0.71	0.71	0.71	0.00	0.71	0.71	0.71	0.21
Delay/Veh:	64.8	23.3	0.0	0.0	28.2	28.2	46.8	0.0	46.8	30.8	30.8	23.7
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	64.8	23.3	0.0	0.0	28.2	28.2	46.8	0.0	46.8	30.8	30.8	23.7
LOS by Move:	E	C	A	A	C	C	D	A	D	C	C	C
HCM2k85thQ:	6	21	0	0	21	21	12	0	12	19	19	4

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #7 Trask Avenue and SR-22 EB On-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.389
Loss Time (sec):	5	Average Delay (sec/veh):	11.0
Optimal Cycle:	21	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	0 0	0	0 0	0 0	0	2 0	1 0	0	0 0	1 1	0
Volume Module:												
Base Vol:	0	0	0	0	0	0	558	699	0	0	624	117
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	0	0	0	558	699	0	0	624	117
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	0	0	0	558	699	0	0	624	117
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	0	0	0	558	699	0	0	624	117
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	0	0	0	558	699	0	0	624	117
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	0.92	1.00	1.00	1.00	0.93	0.93
Lanes:	0.00	0.00	0.00	0.00	0.00	0.00	2.00	1.00	0.00	0.00	1.68	0.32
Final Sat.:	0	0	0	0	0	0	3502	1900	0	0	2967	556
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.00	0.00	0.00	0.16	0.37	0.00	0.00	0.21	0.21
Crit Moves:							****				****	
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.41	0.95	0.00	0.00	0.54	0.54
Volume/Cap:	0.00	0.00	0.00	0.00	0.00	0.00	0.39	0.39	0.00	0.00	0.39	0.39
Delay/Veh:	0.0	0.0	0.0	0.0	0.0	0.0	20.9	0.3	0.0	0.0	13.5	13.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	0.0	0.0	0.0	20.9	0.3	0.0	0.0	13.5	13.5
LOS by Move:	A	A	A	A	A	A	C	A	A	A	B	B
HCM2k85thQ:	0	0	0	0	0	0	10	3	0	0	11	11

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #8 Harbor Blvd and Westminster Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.936
Loss Time (sec):	0	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	2 1	0	1 0	3 0	1
Volume Module:												
Base Vol:	159	1045	224	251	1970	113	173	910	32	406	1100	299
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	159	1045	224	251	1970	113	173	910	32	406	1100	299
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	159	1045	224	251	1970	113	173	910	32	406	1100	299
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	159	1045	224	251	1970	113	173	910	32	406	1100	299
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	159	1045	224	251	1970	113	173	910	32	406	1100	299
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	2.90	0.10	1.00	3.00	1.00
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	4837	163	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.10	0.20	0.14	0.16	0.39	0.07	0.11	0.19	0.20	0.25	0.22	0.19
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #9 Harbor Blvd and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.749
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	43	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	102	816	176	223	1876	122	118	788	153	229	580	135
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	102	816	176	223	1876	122	118	788	153	229	580	135
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	102	816	176	223	1876	122	118	788	153	229	580	135
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	102	816	176	223	1876	122	118	788	153	229	580	135
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	102	816	176	223	1876	122	118	788	153	229	580	135
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	2.00	2.51	0.49	2.00	2.43	0.57
Final Sat.:	1600	5100	1600	1600	5100	1600	3200	4220	780	3200	4094	906
Capacity Analysis Module:												
Vol/Sat:	0.06	0.16	0.11	0.14	0.37	0.08	0.04	0.19	0.20	0.07	0.14	0.15
Crit Moves:	****				****				****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #10 Harbor Blvd and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.729
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	40	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	1 0	1 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	101	1026	98	179	1891	63	136	353	106	85	193	55
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	101	1026	98	179	1891	63	136	353	106	85	193	55
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	101	1026	98	179	1891	63	136	353	106	85	193	55
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	101	1026	98	179	1891	63	136	353	106	85	193	55
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	101	1026	98	179	1891	63	136	353	106	85	193	55
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.00	1.00
Lanes:	2.00	3.00	1.00	2.00	2.90	0.10	1.00	1.54	0.46	1.00	0.78	0.22
Final Sat.:	3200	5100	1600	3200	4845	155	1600	2561	739	1600	1245	355
Capacity Analysis Module:												
Vol/Sat:	0.03	0.20	0.06	0.06	0.39	0.41	0.09	0.14	0.14	0.05	0.16	0.15
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #11 Harbor Blvd and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.857
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	67	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	2 0	2 1	0	2 0	3 0	1
Volume Module:												
Base Vol:	95	713	135	212	2441	71	123	743	373	211	469	195
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	95	713	135	212	2441	71	123	743	373	211	469	195
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	95	713	135	212	2441	71	123	743	373	211	469	195
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	95	713	135	212	2441	71	123	743	373	211	469	195
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	95	713	135	212	2441	71	123	743	373	211	469	195
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	2.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	3400	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.14	0.08	0.07	0.48	0.04	0.04	0.22	0.23	0.07	0.09	0.12
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #12 Harbor Blvd and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.800
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	52	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	106	647	164	451	2034	122	100	1422	374	125	654	111
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	106	647	164	451	2034	122	100	1422	374	125	654	111
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	106	647	164	451	2034	122	100	1422	374	125	654	111
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	106	647	164	451	2034	122	100	1422	374	125	654	111
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	106	647	164	451	2034	122	100	1422	374	125	654	111
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.39	0.61	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	4029	971	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.16	0.17	0.14	0.40	0.08	0.03	0.28	0.23	0.04	0.13	0.07
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #13 Harbor Blvd and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	1.007
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 0	2	0 1
Volume Module:												
Base Vol:	107	759	48	173	2324	87	90	753	303	146	453	103
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	107	759	48	173	2324	87	90	753	303	146	453	103
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	107	759	48	173	2324	87	90	753	303	146	453	103
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	107	759	48	173	2324	87	90	753	303	146	453	103
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	107	759	48	173	2324	87	90	753	303	146	453	103
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	2.82	0.18	1.00	2.89	0.11	1.00	1.43	0.57	1.00	2.00	1.00
Final Sat.:	3200	4714	286	1600	4827	173	1600	2382	918	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.16	0.17	0.11	0.48	0.50	0.06	0.32	0.33	0.09	0.13	0.06
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #14 MacArthur Blvd and Hyland Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.615
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	30	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	0 1	0	1 0	1 0	1	1 0	3 0	1	1 0	3 0	1
Volume Module:												
Base Vol:	86	4	24	8	2	19	7	1915	760	82	626	26
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	86	4	24	8	2	19	7	1915	760	82	626	26
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	86	4	24	8	2	19	7	1915	760	82	626	26
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	86	4	24	8	2	19	7	1915	760	82	626	26
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	86	4	24	8	2	19	7	1915	760	82	626	26
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	0.14	0.86	1.00	1.00	1.00	1.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	229	1371	1600	1700	1600	1600	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.02	0.02	0.01	0.00	0.01	0.00	0.38	0.48	0.05	0.12	0.02
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #15 MacArthur Blvd and Harbor Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.747
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	43	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	1 0	3 0	1	1 0	3 0	1
Volume Module:												
Base Vol:	128	898	92	289	1662	115	133	1204	363	152	506	151
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	128	898	92	289	1662	115	133	1204	363	152	506	151
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	128	898	92	289	1662	115	133	1204	363	152	506	151
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	128	898	92	289	1662	115	133	1204	363	152	506	151
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	128	898	92	289	1662	115	133	1204	363	152	506	151
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	1.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	1600	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.04	0.18	0.06	0.09	0.33	0.07	0.08	0.24	0.23	0.10	0.10	0.09
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #16 Harbor Blvd and Sunflower Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.705
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	38	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	1 0	1 1	0	1 1	0 1	0
Volume Module:												
Base Vol:	221	1128	209	242	2284	56	9	109	42	149	190	107
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	221	1128	209	242	2284	56	9	109	42	149	190	107
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	221	1128	209	242	2284	56	9	109	42	149	190	107
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	221	1128	209	242	2284	56	9	109	42	149	190	107
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	221	1128	209	242	2284	56	9	109	42	149	190	107
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.00	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	1.00	1.44	0.56	1.00	1.28	0.72
Final Sat.:	3200	5100	1600	3200	5100	1600	1600	2410	890	1604	2045	1151
Capacity Analysis Module:												
Vol/Sat:	0.07	0.22	0.13	0.08	0.45	0.04	0.01	0.05	0.05	0.09	0.09	0.09
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #17 Harbor Blvd and I-405 NB Off-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.573
Loss Time (sec):	5	Average Delay (sec/veh):	17.9
Optimal Cycle:	29	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 4	0 0	0	0 4	0 0	0	0 0	0 0	1	0 1!	0 1
Volume Module:												
Base Vol:	0	1915	0	0	2007	0	0	0	0	399	0	702
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1915	0	0	2007	0	0	0	0	399	0	702
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1915	0	0	2007	0	0	0	0	399	0	702
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1915	0	0	2007	0	0	0	0	399	0	702
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1915	0	0	2007	0	0	0	0	399	0	702
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	1.00	1.00	0.91	1.00	1.00	1.00	1.00	0.89	1.00	0.89
Lanes:	0.00	4.00	0.00	0.00	4.00	0.00	0.00	0.00	0.00	1.36	0.00	1.64
Final Sat.:	0	6916	0	0	6916	0	0	0	0	2298	0	2762
Capacity Analysis Module:												
Vol/Sat:	0.00	0.28	0.00	0.00	0.29	0.00	0.00	0.00	0.00	0.17	0.00	0.25
Crit Moves:	****				****							****
Green/Cycle:	0.00	0.51	0.00	0.00	0.51	0.00	0.00	0.00	0.00	0.44	0.00	0.44
Volume/Cap:	0.00	0.55	0.00	0.00	0.57	0.00	0.00	0.00	0.00	0.39	0.00	0.57
Delay/Veh:	0.0	17.0	0.0	0.0	17.4	0.0	0.0	0.0	0.0	18.8	0.0	21.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	17.0	0.0	0.0	17.4	0.0	0.0	0.0	0.0	18.8	0.0	21.2
LOS by Move:	A	B	A	A	B	A	A	A	A	B	A	C
HCM2k85thQ:	0	17	0	0	18	0	0	0	0	9	0	15

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #18 Harbor Blvd and I-405 SB Off-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.618
Loss Time (sec):	5	Average Delay (sec/veh):	15.0
Optimal Cycle:	31	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Ignore			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 3	0 1	0	0 4	0 0	1	0 1!	0 1	0	0 0	0 0
Volume Module:												
Base Vol:	0	1993	0	0	1278	0	475	0	529	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1993	0	0	1278	0	475	0	529	0	0	0
User Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1993	0	0	1278	0	475	0	529	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1993	0	0	1278	0	475	0	529	0	0	0
PCE Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1993	0	0	1278	0	475	0	529	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	1.00	1.00	0.91	1.00	0.90	1.00	0.90	1.00	1.00	1.00
Lanes:	0.00	3.00	1.00	0.00	4.00	0.00	1.47	0.00	1.53	0.00	0.00	0.00
Final Sat.:	0	5187	1900	0	6916	0	2519	0	2610	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.38	0.00	0.00	0.18	0.00	0.19	0.00	0.20	0.00	0.00	0.00
Crit Moves:		****		****					****			
Green/Cycle:	0.00	0.62	0.00	0.00	0.62	0.00	0.33	0.00	0.33	0.00	0.00	0.00
Volume/Cap:	0.00	0.62	0.00	0.00	0.30	0.00	0.57	0.00	0.62	0.00	0.00	0.00
Delay/Veh:	0.0	12.0	0.0	0.0	8.8	0.0	28.3	0.0	29.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	12.0	0.0	0.0	8.8	0.0	28.3	0.0	29.0	0.0	0.0	0.0
LOS by Move:	A	B	A	A	A	A	C	A	C	A	A	A
HCM2k85thQ:	0	21	0	0	8	0	13	0	15	0	0	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #19 Fairview St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.676
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	35	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	0 1	0	1 0	1 1	0	0 1
Volume Module:												
Base Vol:	7	1435	465	314	1812	8	7	25	25	50	2	26
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	7	1435	465	314	1812	8	7	25	25	50	2	26
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	7	1435	465	314	1812	8	7	25	25	50	2	26
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	7	1435	465	314	1812	8	7	25	25	50	2	26
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	7	1435	465	314	1812	8	7	25	25	50	2	26
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	2.27	0.73	1.00	2.99	0.01	0.24	0.88	0.88	1.92	0.08	1.00
Final Sat.:	1600	3825	1175	1600	4979	21	393	1404	1404	3077	123	1600
Capacity Analysis Module:												
Vol/Sat:	0.00	0.38	0.40	0.20	0.36	0.38	0.02	0.02	0.02	0.02	0.02	0.02
Crit Moves:			****	****				****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #20 Fairview St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.887
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	80	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	2 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	196	1508	284	272	1579	181	170	977	151	180	809	271
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	196	1508	284	272	1579	181	170	977	151	180	809	271
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	196	1508	284	272	1579	181	170	977	151	180	809	271
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	196	1508	284	272	1579	181	170	977	151	180	809	271
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	196	1508	284	272	1579	181	170	977	151	180	809	271
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	1.00	3.00	1.00	1.00	2.69	0.31	1.00	2.60	0.40	1.00	2.25	0.75
Final Sat.:	1600	5100	1600	1600	4506	494	1600	4357	643	1600	3796	1204
Capacity Analysis Module:												
Vol/Sat:	0.12	0.30	0.18	0.17	0.35	0.37	0.11	0.22	0.24	0.11	0.21	0.23
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #21 Fairview St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.800
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	52	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	0 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	138	1479	198	184	2242	162	92	216	50	93	240	19
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	138	1479	198	184	2242	162	92	216	50	93	240	19
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	138	1479	198	184	2242	162	92	216	50	93	240	19
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	138	1479	198	184	2242	162	92	216	50	93	240	19
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	138	1479	198	184	2242	162	92	216	50	93	240	19
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	0.81	0.19	1.00	0.93	0.07
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	1299	301	1600	1483	117
Capacity Analysis Module:												
Vol/Sat:	0.09	0.29	0.12	0.12	0.44	0.10	0.06	0.17	0.17	0.06	0.16	0.16
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #22 Fairview St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.882
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	77	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	212	935	131	225	1576	133	361	1002	268	463	608	121
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	212	935	131	225	1576	133	361	1002	268	463	608	121
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	212	935	131	225	1576	133	361	1002	268	463	608	121
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	212	935	131	225	1576	133	361	1002	268	463	608	121
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	212	935	131	225	1576	133	361	1002	268	463	608	121
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	2.00	2.63	0.37	2.00	2.77	0.23	2.00	2.37	0.63	2.00	2.50	0.50
Final Sat.:	3200	4410	590	3200	4626	374	3200	3987	1013	3200	4203	797
Capacity Analysis Module:												
Vol/Sat:	0.07	0.21	0.22	0.07	0.34	0.36	0.11	0.25	0.26	0.14	0.14	0.15
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #23 Fairview St and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.847
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	64	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	235	1039	136	205	1628	119	69	1010	301	277	836	271
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	235	1039	136	205	1628	119	69	1010	301	277	836	271
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	235	1039	136	205	1628	119	69	1010	301	277	836	271
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	235	1039	136	205	1628	119	69	1010	301	277	836	271
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	235	1039	136	205	1628	119	69	1010	301	277	836	271
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	2.65	0.35	2.00	2.80	0.20	2.00	2.31	0.69	2.00	2.27	0.73
Final Sat.:	3200	4444	556	3200	4673	327	3200	3898	1102	3200	3825	1175
Capacity Analysis Module:												
Vol/Sat:	0.07	0.23	0.24	0.06	0.35	0.36	0.02	0.26	0.27	0.09	0.22	0.23
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #24 Fairview St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.690
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	36	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	280	870	100	269	1357	152	121	984	145	298	627	178
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	280	870	100	269	1357	152	121	984	145	298	627	178
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	280	870	100	269	1357	152	121	984	145	298	627	178
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	280	870	100	269	1357	152	121	984	145	298	627	178
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	280	870	100	269	1357	152	121	984	145	298	627	178
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.69	0.31	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	4505	495	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.09	0.19	0.20	0.08	0.27	0.10	0.04	0.19	0.09	0.09	0.12	0.11
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #25 Fairview Rd and Sunflower Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.687
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	36	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	2 0	1 1	0	2 0	2 0	1
Volume Module:												
Base Vol:	182	1051	160	191	1627	104	44	286	58	357	299	154
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	182	1051	160	191	1627	104	44	286	58	357	299	154
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	182	1051	160	191	1627	104	44	286	58	357	299	154
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	182	1051	160	191	1627	104	44	286	58	357	299	154
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	182	1051	160	191	1627	104	44	286	58	357	299	154
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	2.82	0.18	2.00	1.66	0.34	2.00	2.00	1.00
Final Sat.:	3200	5100	1600	3200	4712	288	3200	2760	540	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.06	0.21	0.10	0.06	0.35	0.36	0.01	0.10	0.11	0.11	0.09	0.10
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #26 Greenville St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.742
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	42	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	0 1	0 0	0	0 0	0 0	2	1 0	1 0	3	0 0
Volume Module:												
Base Vol:	140	0	188	0	0	0	0	1788	436	178	949	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	140	0	188	0	0	0	0	1788	436	178	949	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	140	0	188	0	0	0	0	1788	436	178	949	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	140	0	188	0	0	0	0	1788	436	178	949	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	140	0	188	0	0	0	0	1788	436	178	949	0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	2.41	0.59	1.00	3.00	0.00
Final Sat.:	1600	0	1600	0	0	0	0	4059	941	1600	5100	0
Capacity Analysis Module:												
Vol/Sat:	0.09	0.00	0.12	0.00	0.00	0.00	0.00	0.44	0.46	0.11	0.19	0.00
Crit Moves:			****						****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #27 Greenville St and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.878
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	76	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	1 0	1 0	0	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	47	193	78	154	472	160	50	1127	180	210	847	97
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	47	193	78	154	472	160	50	1127	180	210	847	97
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	47	193	78	154	472	160	50	1127	180	210	847	97
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	47	193	78	154	472	160	50	1127	180	210	847	97
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	47	193	78	154	472	160	50	1127	180	210	847	97
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	0.71	0.29	1.00	0.75	0.25	1.00	2.59	0.41	1.00	2.69	0.31
Final Sat.:	1600	1139	461	1600	1195	405	1600	4339	661	1600	4507	493
Capacity Analysis Module:												
Vol/Sat:	0.03	0.17	0.17	0.10	0.39	0.40	0.03	0.26	0.27	0.13	0.19	0.20
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #28 Raitt St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.704
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	38	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	1 0	1 0	0	1 0	1 0	0	1 0	1 0	0	1 0
Volume Module:												
Base Vol:	34	309	49	69	528	72	52	313	40	60	190	34
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	34	309	49	69	528	72	52	313	40	60	190	34
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	34	309	49	69	528	72	52	313	40	60	190	34
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	34	309	49	69	528	72	52	313	40	60	190	34
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	34	309	49	69	528	72	52	313	40	60	190	34
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	0.86	0.14	1.00	0.88	0.12	1.00	0.89	0.11	1.00	0.85	0.15
Final Sat.:	1600	1381	219	1600	1408	192	1600	1419	181	1600	1357	243
Capacity Analysis Module:												
Vol/Sat:	0.02	0.22	0.22	0.04	0.38	0.38	0.03	0.22	0.22	0.04	0.14	0.14
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #29 Raitt St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.926
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	105	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0 1	0	1 0	0 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	85	228	82	80	376	45	330	2016	290	127	992	59
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	85	228	82	80	376	45	330	2016	290	127	992	59
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	85	228	82	80	376	45	330	2016	290	127	992	59
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	85	228	82	80	376	45	330	2016	290	127	992	59
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	85	228	82	80	376	45	330	2016	290	127	992	59
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.05	1.00	1.00	1.04	1.00
Lanes:	1.00	0.74	0.26	1.00	0.89	0.11	1.00	2.62	0.38	1.00	2.83	0.17
Final Sat.:	1600	1177	423	1600	1429	171	1600	4396	604	1600	4731	269
Capacity Analysis Module:												
Vol/Sat:	0.05	0.19	0.19	0.05	0.26	0.26	0.21	0.46	0.48	0.08	0.21	0.22
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #30 Bear St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.756
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	44	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	1	1 0	2 0	1	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	61	210	89	192	728	289	99	1690	116	103	1735	147
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	61	210	89	192	728	289	99	1690	116	103	1735	147
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	61	210	89	192	728	289	99	1690	116	103	1735	147
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	61	210	89	192	728	289	99	1690	116	103	1735	147
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	61	210	89	192	728	289	99	1690	116	103	1735	147
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.81	0.19	1.00	2.77	0.23
Final Sat.:	1600	3400	1600	1600	3400	1600	1600	4692	308	1600	4625	375
Capacity Analysis Module:												
Vol/Sat:	0.04	0.06	0.06	0.12	0.21	0.18	0.06	0.36	0.38	0.06	0.38	0.39
Crit Moves:	****			****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #31 Bristol St and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.853
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	66	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	2 0	2 1	0	2 0	3 0	1
Volume Module:												
Base Vol:	241	1241	210	449	1845	243	341	1015	164	386	886	293
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	241	1241	210	449	1845	243	341	1015	164	386	886	293
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	241	1241	210	449	1845	243	341	1015	164	386	886	293
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	241	1241	210	449	1845	243	341	1015	164	386	886	293
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	241	1241	210	449	1845	243	341	1015	164	386	886	293
OvlAdjVol:												69
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	2.58	0.42	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	4332	668	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.08	0.24	0.13	0.14	0.36	0.15	0.11	0.23	0.25	0.12	0.17	0.18
OvlAdjV/S:												0.04
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #32 Bristol St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.931
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	109	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	0	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	129	1141	101	396	1597	100	80	404	50	145	481	100
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	129	1141	101	396	1597	100	80	404	50	145	481	100
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	129	1141	101	396	1597	100	80	404	50	145	481	100
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	129	1141	101	396	1597	100	80	404	50	145	481	100
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	129	1141	101	396	1597	100	80	404	50	145	481	100
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.00	1.00	1.00	1.04	1.00
Lanes:	1.00	2.76	0.24	1.00	2.82	0.18	1.00	0.89	0.11	1.00	1.66	0.34
Final Sat.:	1600	4610	390	1600	4717	283	1600	1424	176	1600	2749	551
Capacity Analysis Module:												
Vol/Sat:	0.08	0.25	0.26	0.25	0.34	0.35	0.05	0.28	0.28	0.09	0.17	0.18
Crit Moves:			****	****				****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #33 Bristol St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.721
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	40	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	0	1 0	1 0	0	1 0
Volume Module:												
Base Vol:	68	1482	206	172	1891	51	54	264	23	52	129	22
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	68	1482	206	172	1891	51	54	264	23	52	129	22
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	68	1482	206	172	1891	51	54	264	23	52	129	22
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	68	1482	206	172	1891	51	54	264	23	52	129	22
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	68	1482	206	172	1891	51	54	264	23	52	129	22
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	2.63	0.37	1.00	2.92	0.08	1.00	0.92	0.08	1.00	0.85	0.15
Final Sat.:	1600	4414	586	1600	4874	126	1600	1472	128	1600	1367	233
Capacity Analysis Module:												
Vol/Sat:	0.04	0.34	0.35	0.11	0.39	0.40	0.03	0.18	0.18	0.03	0.09	0.09
Crit Moves:			****	****				****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #34 Bristol St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.902
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	88	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	1 0	2 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	208	1451	184	315	1298	183	206	1268	128	126	949	139
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	208	1451	184	315	1298	183	206	1268	128	126	949	139
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	208	1451	184	315	1298	183	206	1268	128	126	949	139
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	208	1451	184	315	1298	183	206	1268	128	126	949	139
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	208	1451	184	315	1298	183	206	1268	128	126	949	139
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.04	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	1.00	2.72	0.28	1.00	1.74	0.26
Final Sat.:	3200	5100	1600	3200	5100	1600	1600	4560	440	1600	2891	409
Capacity Analysis Module:												
Vol/Sat:	0.07	0.28	0.12	0.10	0.25	0.11	0.13	0.28	0.29	0.08	0.33	0.34
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #35 Bristol St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.947
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	126	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	0 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	130	1159	58	228	2223	229	155	266	131	145	405	47
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	130	1159	58	228	2223	229	155	266	131	145	405	47
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	130	1159	58	228	2223	229	155	266	131	145	405	47
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	130	1159	58	228	2223	229	155	266	131	145	405	47
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	130	1159	58	228	2223	229	155	266	131	145	405	47
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	0.67	0.33	1.00	0.90	0.10
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	1072	528	1600	1434	166
Capacity Analysis Module:												
Vol/Sat:	0.08	0.23	0.04	0.14	0.44	0.14	0.10	0.25	0.25	0.09	0.28	0.28
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #36 Bristol St and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.956
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	139	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	1 0	3 0	1	1 0	3 0	1
Volume Module:												
Base Vol:	221	993	183	643	2178	247	172	958	238	231	1058	180
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	221	993	183	643	2178	247	172	958	238	231	1058	180
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	221	993	183	643	2178	247	172	958	238	231	1058	180
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	221	993	183	643	2178	247	172	958	238	231	1058	180
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	221	993	183	643	2178	247	172	958	238	231	1058	180
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.53	0.47	2.00	2.69	0.31	1.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	4253	747	3200	4511	489	1600	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.07	0.23	0.24	0.20	0.48	0.51	0.11	0.19	0.15	0.14	0.21	0.11
Crit Moves:	****					****		****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #37 Bristol St and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.854
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	66	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	70	778	146	283	1011	119	306	1480	136	157	712	91
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	70	778	146	283	1011	119	306	1480	136	157	712	91
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	70	778	146	283	1011	119	306	1480	136	157	712	91
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	70	778	146	283	1011	119	306	1480	136	157	712	91
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	70	778	146	283	1011	119	306	1480	136	157	712	91
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.53	0.47	1.00	2.68	0.32	1.00	2.75	0.25	1.00	2.66	0.34
Final Sat.:	1600	4242	758	1600	4495	505	1600	4596	404	1600	4456	544
Capacity Analysis Module:												
Vol/Sat:	0.04	0.18	0.19	0.18	0.22	0.24	0.19	0.32	0.34	0.10	0.16	0.17
Crit Moves:			****	****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #38 Bristol St and Alton Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.553
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	26	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	3 0	1	1 0	1 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	10	702	53	224	1551	36	190	117	244	41	87	31
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	10	702	53	224	1551	36	190	117	244	41	87	31
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	10	702	53	224	1551	36	190	117	244	41	87	31
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	10	702	53	224	1551	36	190	117	244	41	87	31
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	10	702	53	224	1551	36	190	117	244	41	87	31
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	1.00	2.79	0.21	1.00	3.00	1.00	1.00	1.00	1.00	1.00	0.74	0.26
Final Sat.:	1600	4663	337	1600	5100	1600	1600	1700	1600	1600	1180	420
Capacity Analysis Module:												
Vol/Sat:	0.01	0.15	0.16	0.14	0.30	0.02	0.12	0.07	0.15	0.03	0.07	0.07
Crit Moves:	****			****			****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #39 Bristol St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.759
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	45	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	95	491	123	319	1389	145	211	1531	311	191	910	132
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	95	491	123	319	1389	145	211	1531	311	191	910	132
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	95	491	123	319	1389	145	211	1531	311	191	910	132
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	95	491	123	319	1389	145	211	1531	311	191	910	132
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	95	491	123	319	1389	145	211	1531	311	191	910	132
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	2.72	0.28	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	4546	454	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.10	0.08	0.10	0.31	0.32	0.07	0.30	0.19	0.06	0.18	0.08
Crit Moves:	****					****		****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #40 Bristol St and Sunflower Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.682
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	35	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 1	2 0	3	0 1	2 0	2	1 1	2 0	3	0 1
Volume Module:												
Base Vol:	119	583	172	232	1316	99	135	1115	451	295	480	166
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	119	583	172	232	1316	99	135	1115	451	295	480	166
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	119	583	172	232	1316	99	135	1115	451	295	480	166
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	119	583	172	232	1316	99	135	1115	451	295	480	166
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	119	583	172	232	1316	99	135	1115	451	295	480	166
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	2.85	1.15	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	4757	1843	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.04	0.11	0.11	0.07	0.26	0.06	0.04	0.23	0.24	0.09	0.09	0.10
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #41 Bristol St and I-405 NB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.568
Loss Time (sec):	5	Average Delay (sec/veh):	20.9
Optimal Cycle:	28	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 4	0 1	0	0 4	1 0	0	0 0	0 2	1	1 1	0 2
Volume Module:												
Base Vol:	0	1638	210	0	2088	8	0	0	36	147	89	807
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1638	210	0	2088	8	0	0	36	147	89	807
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1638	210	0	2088	8	0	0	36	147	89	807
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1638	210	0	2088	8	0	0	36	147	89	807
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1638	210	0	2088	8	0	0	36	147	89	807
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	0.85	1.00	0.91	0.91	1.00	1.00	0.75	0.92	0.92	0.75
Lanes:	0.00	4.00	1.00	0.00	4.98	0.02	0.00	0.00	2.00	1.87	1.13	2.00
Final Sat.:	0	6916	1615	0	8603	33	0	0	2842	3272	1981	2842
Capacity Analysis Module:												
Vol/Sat:	0.00	0.24	0.13	0.00	0.24	0.24	0.00	0.00	0.01	0.04	0.04	0.28
Crit Moves:	****				****				****			****
Green/Cycle:	0.00	0.43	0.43	0.00	0.43	0.43	0.00	0.00	0.02	0.50	0.50	0.50
Volume/Cap:	0.00	0.55	0.30	0.00	0.57	0.57	0.00	0.00	0.57	0.09	0.09	0.57
Delay/Veh:	0.0	21.7	19.1	0.0	21.8	21.8	0.0	0.0	60.0	13.1	13.1	18.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	21.7	19.1	0.0	21.8	21.8	0.0	0.0	60.0	13.1	13.1	18.0
LOS by Move:	A	C	B	A	C	C	A	A	E	B	B	B
HCM2k85thQ:	0	16	7	0	17	17	0	0	2	2	2	15

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #42 Bristol St and I-405 SB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.697
Loss Time (sec):	5	Average Delay (sec/veh):	23.6
Optimal Cycle:	38	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Ignore			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	4 0	0	0 0	2 1	1	3 0	0 0	1	0 0	0 0	0
Volume Module:												
Base Vol:	122	1195	0	0	1097	943	675	0	618	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	122	1195	0	0	1097	943	675	0	618	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	122	1195	0	0	1097	0	675	0	618	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	122	1195	0	0	1097	0	675	0	618	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	122	1195	0	0	1097	0	675	0	618	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	1.00	1.00	0.91	0.91	0.92	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	4.00	0.00	0.00	3.00	1.00	3.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	6916	0	0	5187	1729	5253	0	1615	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.07	0.17	0.00	0.00	0.21	0.00	0.13	0.00	0.38	0.00	0.00	0.00
Crit Moves:	****				****				****			
Green/Cycle:	0.10	0.40	0.00	0.00	0.30	0.00	0.55	0.00	0.55	0.00	0.00	0.00
Volume/Cap:	0.70	0.43	0.00	0.00	0.70	0.00	0.23	0.00	0.70	0.00	0.00	0.00
Delay/Veh:	55.3	21.8	0.0	0.0	32.1	0.0	11.7	0.0	18.9	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	55.3	21.8	0.0	0.0	32.1	0.0	11.7	0.0	18.9	0.0	0.0	0.0
LOS by Move:	E	C	A	A	C	A	B	A	B	A	A	A
HCM2k85thQ:	8	11	0	0	18	0	6	0	22	0	0	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #43 Flower St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.586
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	28	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Prot+Permit			Prot+Permit			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	1	1 0	2 0	1	1 0	2 0	1	1 0	2 0	1
Volume Module:												
Base Vol:	64	593	86	275	929	124	63	394	45	118	377	158
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	64	593	86	275	929	124	63	394	45	118	377	158
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	64	593	86	275	929	124	63	394	45	118	377	158
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	64	593	86	275	929	124	63	394	45	118	377	158
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	64	593	86	275	929	124	63	394	45	118	377	158
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	1600	3400	1600	1600	3400	1600	1600	3400	1600	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.04	0.17	0.05	0.17	0.27	0.08	0.04	0.12	0.03	0.07	0.11	0.10
Crit Moves:		****		****				****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #44 Flower St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.926
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	105	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1 0	1	1 0	1 1	0	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	89	387	102	366	1158	121	107	983	39	161	664	95
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	89	387	102	366	1158	121	107	983	39	161	664	95
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	89	387	102	366	1158	121	107	983	39	161	664	95
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	89	387	102	366	1158	121	107	983	39	161	664	95
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	89	387	102	366	1158	121	107	983	39	161	664	95
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.04	1.00
Lanes:	1.00	1.00	1.00	1.00	1.81	0.19	1.00	1.92	0.08	1.00	1.75	0.25
Final Sat.:	1600	1700	1600	1600	2997	303	1600	3178	122	1600	2899	401
Capacity Analysis Module:												
Vol/Sat:	0.06	0.23	0.06	0.23	0.39	0.40	0.07	0.31	0.32	0.10	0.23	0.24
Crit Moves:		****		****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #45 Flower St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.807
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	54	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	1 0	1 0	0	1 0	1 0	0	1 0	1 0	0	1 0
Volume Module:												
Base Vol:	30	219	19	66	367	53	58	658	50	54	366	49
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	30	219	19	66	367	53	58	658	50	54	366	49
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	30	219	19	66	367	53	58	658	50	54	366	49
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	30	219	19	66	367	53	58	658	50	54	366	49
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	30	219	19	66	367	53	58	658	50	54	366	49
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	0.92	0.08	1.00	0.87	0.13	1.00	0.93	0.07	1.00	0.88	0.12
Final Sat.:	1600	1472	128	1600	1398	202	1600	1487	113	1600	1411	189
Capacity Analysis Module:												
Vol/Sat:	0.02	0.15	0.15	0.04	0.26	0.26	0.04	0.44	0.44	0.03	0.26	0.26
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #46 Flower St and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.818
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	56	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	86	451	100	98	544	120	237	1631	455	115	846	93
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	86	451	100	98	544	120	237	1631	455	115	846	93
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	86	451	100	98	544	120	237	1631	455	115	846	93
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	86	451	100	98	544	120	237	1631	455	115	846	93
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	86	451	100	98	544	120	237	1631	455	115	846	93
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	1.64	0.36	1.00	1.64	0.36	1.00	2.35	0.65	1.00	2.70	0.30
Final Sat.:	1600	2719	581	1600	2722	578	1600	3953	1047	1600	4525	475
Capacity Analysis Module:												
Vol/Sat:	0.05	0.17	0.17	0.06	0.20	0.21	0.15	0.41	0.43	0.07	0.19	0.20
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #47 Flower St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.710
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	38	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	26	170	88	163	352	264	163	1852	79	79	1329	85
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	26	170	88	163	352	264	163	1852	79	79	1329	85
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	26	170	88	163	352	264	163	1852	79	79	1329	85
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	26	170	88	163	352	264	163	1852	79	79	1329	85
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	26	170	88	163	352	264	163	1852	79	79	1329	85
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	1.32	0.68	1.00	1.14	0.86	1.00	2.88	0.12	1.00	2.82	0.18
Final Sat.:	1600	2209	1091	1600	1929	1371	1600	4804	196	1600	4711	289
Capacity Analysis Module:												
Vol/Sat:	0.02	0.08	0.08	0.10	0.18	0.19	0.10	0.39	0.40	0.05	0.28	0.29
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #48 Main St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.536
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	25	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	2 0	2 1	0	2 0	2 0	1
Volume Module:												
Base Vol:	123	844	438	161	843	141	350	495	215	334	291	243
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	123	844	438	161	843	141	350	495	215	334	291	243
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	123	844	438	161	843	141	350	495	215	334	291	243
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	123	844	438	161	843	141	350	495	215	334	291	243
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	123	844	438	161	843	141	350	495	215	334	291	243
OvlAdjVol:			271									163
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	2.57	0.43	2.00	2.09	0.91	2.00	2.00	1.00
Final Sat.:	3200	5100	1600	3200	4312	688	3200	3546	1454	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.04	0.17	0.27	0.05	0.20	0.20	0.11	0.14	0.15	0.10	0.09	0.15
OvlAdjV/S:			0.17									0.10
Crit Moves:	****			****				****	****			

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #49 Main St and Mainplace Dr / Memory Ln

Cycle (sec):	100	Critical Vol./Cap.(X):	0.456
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	22	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 0	2 0	2	1 0	2 0	2	1 0	2 0	2	0 1
Volume Module:												
Base Vol:	35	955	313	27	1066	223	123	194	13	264	301	20
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	35	955	313	27	1066	223	123	194	13	264	301	20
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	35	955	313	27	1066	223	123	194	13	264	301	20
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	35	955	313	27	1066	223	123	194	13	264	301	20
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	35	955	313	27	1066	223	123	194	13	264	301	20
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	2.26	0.74	2.00	2.48	0.52	2.00	2.81	0.19	2.00	2.00	1.00
Final Sat.:	3200	3815	1185	3200	4170	830	3200	4699	301	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.01	0.25	0.26	0.01	0.26	0.27	0.04	0.04	0.04	0.08	0.09	0.01
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #50 Main St and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.845
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	63	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 0	1	2 0	2 0	1	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	156	929	199	225	1251	81	114	1203	60	369	1475	22
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	156	929	199	225	1251	81	114	1203	60	369	1475	22
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	156	929	199	225	1251	81	114	1203	60	369	1475	22
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	156	929	199	225	1251	81	114	1203	60	369	1475	22
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	156	929	199	225	1251	81	114	1203	60	369	1475	22
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	2.00	2.00	1.00	2.00	2.00	1.00	2.00	2.86	0.14	2.00	2.96	0.04
Final Sat.:	3200	3400	1600	3200	3400	1600	3200	4772	228	3200	4929	71
Capacity Analysis Module:												
Vol/Sat:	0.05	0.27	0.12	0.07	0.37	0.05	0.04	0.25	0.26	0.12	0.30	0.31
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #51 Main St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.848
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	64	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	138	901	79	69	1222	186	75	454	83	52	679	41
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	138	901	79	69	1222	186	75	454	83	52	679	41
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	138	901	79	69	1222	186	75	454	83	52	679	41
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	138	901	79	69	1222	186	75	454	83	52	679	41
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	138	901	79	69	1222	186	75	454	83	52	679	41
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.03	1.00
Lanes:	1.00	1.84	0.16	1.00	1.74	0.26	1.00	1.69	0.31	1.00	1.89	0.11
Final Sat.:	1600	3042	258	1600	2877	423	1600	2805	495	1600	3118	182
Capacity Analysis Module:												
Vol/Sat:	0.09	0.30	0.31	0.04	0.42	0.44	0.05	0.16	0.17	0.03	0.22	0.23
Crit Moves:	****					****	****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #52 Main St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.791
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	50	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	0	0 0	1 1	0	0 0	0 0	0	0 1	0 1	0
Volume Module:												
Base Vol:	66	1050	0	0	1162	103	0	0	0	61	848	65
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	66	1050	0	0	1162	103	0	0	0	61	848	65
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	66	1050	0	0	1162	103	0	0	0	61	848	65
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	66	1050	0	0	1162	103	0	0	0	61	848	65
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	66	1050	0	0	1162	103	0	0	0	61	848	65
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.03	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	1.00	2.00	0.00	0.00	1.84	0.16	0.00	0.00	0.00	0.13	1.74	0.13
Final Sat.:	1600	3400	0	0	3039	261	0	0	0	200	2786	214
Capacity Analysis Module:												
Vol/Sat:	0.04	0.31	0.00	0.00	0.38	0.40	0.00	0.00	0.00	0.04	0.30	0.30
Crit Moves:	****					****					****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #53 Main St and 4th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.441
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	21	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	1	1 0	0 0	1	1 0	0 0	1	1 0	0 0	1	1 0
Volume Module:												
Base Vol:	0	1052	25	0	1098	27	0	93	14	0	105	26
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1052	25	0	1098	27	0	93	14	0	105	26
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1052	25	0	1098	27	0	93	14	0	105	26
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1052	25	0	1098	27	0	93	14	0	105	26
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1052	25	0	1098	27	0	93	14	0	105	26
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	0.00	1.95	0.05	0.00	1.95	0.05	0.00	1.74	0.26	0.00	1.60	0.40
Final Sat.:	0	3226	74	0	3223	77	0	2881	419	0	2665	635
Capacity Analysis Module:												
Vol/Sat:	0.00	0.33	0.34	0.00	0.34	0.35	0.00	0.03	0.03	0.00	0.04	0.04
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #54 Main St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.754
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	44	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	2	0 1	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	103	532	54	94	937	69	69	805	85	78	972	54
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	103	532	54	94	937	69	69	805	85	78	972	54
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	103	532	54	94	937	69	69	805	85	78	972	54
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	103	532	54	94	937	69	69	805	85	78	972	54
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	103	532	54	94	937	69	69	805	85	78	972	54
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.06	1.00	1.00	1.03	1.00	1.00	1.03	1.00
Lanes:	1.00	1.82	0.18	1.00	2.00	1.00	1.00	1.81	0.19	1.00	1.89	0.11
Final Sat.:	1600	3005	295	1600	3400	1600	1600	2994	306	1600	3132	168
Capacity Analysis Module:												
Vol/Sat:	0.06	0.18	0.18	0.06	0.28	0.04	0.04	0.27	0.28	0.05	0.31	0.32
Crit Moves:	****				****		****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #55 Main St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.916
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	97	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Prot+Permit			Prot+Permit			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	0 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	93	874	44	184	1434	28	44	419	46	97	259	74
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	93	874	44	184	1434	28	44	419	46	97	259	74
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	93	874	44	184	1434	28	44	419	46	97	259	74
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	93	874	44	184	1434	28	44	419	46	97	259	74
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	93	874	44	184	1434	28	44	419	46	97	259	74
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.90	0.10	1.00	1.96	0.04	1.00	0.90	0.10	1.00	0.78	0.22
Final Sat.:	1600	3147	153	1600	3239	61	1600	1442	158	1600	1244	356
Capacity Analysis Module:												
Vol/Sat:	0.06	0.28	0.29	0.12	0.44	0.46	0.03	0.29	0.29	0.06	0.21	0.21
Crit Moves:	****					****		****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #56 Main St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.876
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	75	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	104	589	64	239	1103	58	99	1288	174	149	867	106
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	104	589	64	239	1103	58	99	1288	174	149	867	106
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	104	589	64	239	1103	58	99	1288	174	149	867	106
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	104	589	64	239	1103	58	99	1288	174	149	867	106
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	104	589	64	239	1103	58	99	1288	174	149	867	106
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	1.80	0.20	1.00	1.90	0.10	1.00	2.64	0.36	1.00	2.67	0.33
Final Sat.:	1600	2986	314	1600	3140	160	1600	4429	571	1600	4477	523
Capacity Analysis Module:												
Vol/Sat:	0.07	0.20	0.20	0.15	0.35	0.36	0.06	0.29	0.30	0.09	0.19	0.20
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #57 Main St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.721
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	40	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	60	344	292	539	709	160	230	1336	242	187	556	279
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	60	344	292	539	709	160	230	1336	242	187	556	279
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	60	344	292	539	709	160	230	1336	242	187	556	279
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	60	344	292	539	709	160	230	1336	242	187	556	279
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	60	344	292	539	709	160	230	1336	242	187	556	279
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.02	0.07	0.18	0.17	0.14	0.10	0.07	0.26	0.15	0.06	0.11	0.17
Crit Moves:			****	****				****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #58 Penn Wy and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.648
Loss Time (sec):	5	Average Delay (sec/veh):	12.1
Optimal Cycle:	34	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	0	0 2	0 0	0	0 0	0 0	2	1 0	1 0	3	0 0
Volume Module:												
Base Vol:	49	0	250	0	0	0	0	1279	652	236	1901	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	49	0	250	0	0	0	0	1279	652	236	1901	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	49	0	250	0	0	0	0	1279	652	236	1901	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	49	0	250	0	0	0	0	1279	652	236	1901	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	49	0	250	0	0	0	0	1279	652	236	1901	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.75	1.00	1.00	1.00	1.00	0.86	0.86	0.95	0.91	1.00
Lanes:	2.00	0.00	2.00	0.00	0.00	0.00	0.00	2.00	1.00	1.00	3.00	0.00
Final Sat.:	3502	0	2842	0	0	0	0	3282	1641	1805	5187	0
Capacity Analysis Module:												
Vol/Sat:	0.01	0.00	0.09	0.00	0.00	0.00	0.00	0.39	0.40	0.13	0.37	0.00
Crit Moves:			****						****	****		
Green/Cycle:	0.14	0.00	0.14	0.00	0.00	0.00	0.00	0.61	0.61	0.20	0.81	0.00
Volume/Cap:	0.10	0.00	0.65	0.00	0.00	0.00	0.00	0.64	0.65	0.65	0.45	0.00
Delay/Veh:	38.0	0.0	44.8	0.0	0.0	0.0	0.0	12.7	13.0	40.7	2.8	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	38.0	0.0	44.8	0.0	0.0	0.0	0.0	12.7	13.0	40.7	2.8	0.0
LOS by Move:	D	A	D	A	A	A	A	B	B	D	A	A
HCM2k85thQ:	1	0	8	0	0	0	0	21	22	12	10	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #59 I-5 NB Off Ramps/17th Street

Cycle (sec):	100	Critical Vol./Cap.(X):	0.792
Loss Time (sec):	5	Average Delay (sec/veh):	31.4
Optimal Cycle:	53	Level Of Service:	C

Street Name:	I-5 NB Off Ramps						17th Street					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Permitted		
Rights:	Include			Include			Ignore			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 1	0	0 1	1	0 0	0 1	1	0	3 0 1	0	0	2 1 0
Volume Module:												
Base Vol:	720	33	21	60	0	312	101	1062	384	0	1508	24
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	720	33	21	60	0	312	101	1062	384	0	1508	24
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Volume:	720	33	21	60	0	312	101	1062	0	0	1508	24
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	720	33	21	60	0	312	101	1062	0	0	1508	24
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
FinalVolume:	720	33	21	60	0	312	101	1062	0	0	1508	24
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	0.85	0.95	1.00	0.85	0.95	0.91	1.00	1.00	0.91	0.91
Lanes:	1.91	0.09	1.00	1.00	0.00	1.00	1.00	3.00	1.00	0.00	2.95	0.05
Final Sat.:	3466	159	1615	1805	0	1615	1805	5187	1900	0	5096	81
Capacity Analysis Module:												
Vol/Sat:	0.21	0.21	0.01	0.03	0.00	0.19	0.06	0.20	0.00	0.00	0.30	0.30
Crit Moves:	****					****	****				****	
Green/Cycle:	0.26	0.26	0.26	0.24	0.00	0.24	0.07	0.44	0.00	0.00	0.37	0.37
Volume/Cap:	0.79	0.79	0.05	0.14	0.00	0.79	0.79	0.46	0.00	0.00	0.79	0.79
Delay/Veh:	39.0	39.0	27.6	29.7	0.0	45.9	73.6	19.6	0.0	0.0	30.2	30.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	39.0	39.0	27.6	29.7	0.0	45.9	73.6	19.6	0.0	0.0	30.2	30.2
LOS by Move:	D	D	C	C	A	D	E	B	A	A	C	C
HCM2k85thQ:	19	19	1	2	0	17	8	13	0	0	25	25

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #60 Penn Wy and I-5 SB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.462
Loss Time (sec):	5	Average Delay (sec/veh):	19.5
Optimal Cycle:	23	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	2	0 1	2 0	2	0 0	0 0	0	0 0	1 0	0	0 2
Volume Module:												
Base Vol:	0	192	170	837	173	0	0	0	0	170	0	95
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	192	170	837	173	0	0	0	0	170	0	95
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	192	170	837	173	0	0	0	0	170	0	95
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	192	170	837	173	0	0	0	0	170	0	95
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	192	170	837	173	0	0	0	0	170	0	95
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.95	0.85	0.92	0.95	1.00	1.00	1.00	1.00	0.95	1.00	0.75
Lanes:	0.00	2.00	1.00	2.00	2.00	0.00	0.00	0.00	0.00	1.00	0.00	2.00
Final Sat.:	0	3610	1615	3502	3610	0	0	0	0	1805	0	2842
Capacity Analysis Module:												
Vol/Sat:	0.00	0.05	0.11	0.24	0.05	0.00	0.00	0.00	0.00	0.09	0.00	0.03
Crit Moves:			****	****						****		
Green/Cycle:	0.00	0.23	0.23	0.52	0.75	0.00	0.00	0.00	0.00	0.20	0.00	0.72
Volume/Cap:	0.00	0.23	0.46	0.46	0.06	0.00	0.00	0.00	0.00	0.46	0.00	0.05
Delay/Veh:	0.0	31.6	34.2	15.5	3.4	0.0	0.0	0.0	0.0	35.9	0.0	4.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	31.6	34.2	15.5	3.4	0.0	0.0	0.0	0.0	35.9	0.0	4.0
LOS by Move:	A	C	C	B	A	A	A	A	A	D	A	A
HCM2k85thQ:	0	4	8	13	1	0	0	0	0	8	0	1

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #61 Santiago St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	1.147
Loss Time (sec):	5	Average Delay (sec/veh):	62.6
Optimal Cycle:	0	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	1 0	0 1	0	1 0	0 1	0	1 0	0 1	0	0 0	1! 0	0
Volume Module:												
Base Vol:	318	221	31	6	335	82	212	81	431	126	116	23
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	318	221	31	6	335	82	212	81	431	126	116	23
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	318	221	31	6	335	82	212	81	431	126	116	23
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	318	221	31	6	335	82	212	81	431	126	116	23
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	318	221	31	6	335	82	212	81	431	126	116	23
Saturation Flow Module:												
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	0.88	0.12	1.00	0.80	0.20	1.00	0.16	0.84	0.47	0.44	0.09
Final Sat.:	387	362	51	383	333	81	393	71	376	177	163	32
Capacity Analysis Module:												
Vol/Sat:	0.82	0.61	0.61	0.02	1.01	1.01	0.54	1.15	1.15	0.71	0.71	0.71
Crit Moves:	****				****			****				****
Delay/Veh:	42.6	24.0	24.0	12.2	75.9	75.9	22.0	116	116.2	32.7	32.7	32.7
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	42.6	24.0	24.0	12.2	75.9	75.9	22.0	116	116.2	32.7	32.7	32.7
LOS by Move:	E	C	C	B	F	F	C	F	F	D	D	D
ApproachDel:	34.4			75.0			88.6			32.7		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	34.4			75.0			88.6			32.7		
LOS by Appr:	D			F			F			D		
AllWayAvgQ:	3.3	1.4	1.4	0.0	7.4	7.4	1.1	13.1	13.1	2.1	2.1	2.1

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #62 Santiago St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	1.444
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1 0	1	1 0	1 0	1	1 0	0 1	0	1 0	2 0	1
Volume Module:												
Base Vol:	188	611	303	409	972	253	118	732	99	297	699	258
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	188	611	303	409	972	253	118	732	99	297	699	258
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	188	611	303	409	972	253	118	732	99	297	699	258
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	188	611	303	409	972	253	118	732	99	297	699	258
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	188	611	303	409	972	253	118	732	99	297	699	258
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.00	1.00	1.00	1.06	1.00
Lanes:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.88	0.12	1.00	2.00	1.00
Final Sat.:	1600	1700	1600	1600	1700	1600	1600	1409	191	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.12	0.36	0.19	0.26	0.57	0.16	0.07	0.52	0.52	0.19	0.21	0.16
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #63 Standard Ave and 4th St

Cycle (sec):	100	Critical Vol./Cap.(X):	1.340
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1 0	1	1 0	1 0	1	1 0	0 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	191	473	248	431	604	332	298	624	219	250	550	340
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	191	473	248	431	604	332	298	624	219	250	550	340
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	191	473	248	431	604	332	298	624	219	250	550	340
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	191	473	248	431	604	332	298	624	219	250	550	340
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	191	473	248	431	604	332	298	624	219	250	550	340
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.74	0.26	1.00	0.62	0.38
Final Sat.:	1600	1700	1600	1600	1700	1600	1600	1184	416	1600	989	611
Capacity Analysis Module:												
Vol/Sat:	0.12	0.28	0.16	0.27	0.36	0.21	0.19	0.53	0.53	0.16	0.56	0.56
Crit Moves:		****			****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #64 Standard Ave and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	1.515
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0 1	0	0 0	1! 0	0	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	45	631	312	123	901	41	161	1959	121	194	1282	120
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	45	631	312	123	901	41	161	1959	121	194	1282	120
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	45	631	312	123	901	41	161	1959	121	194	1282	120
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	45	631	312	123	901	41	161	1959	121	194	1282	120
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	45	631	312	123	901	41	161	1959	121	194	1282	120
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.03	1.00	1.00	1.03	1.00
Lanes:	1.00	0.67	0.33	0.11	0.85	0.04	1.00	1.88	0.12	1.00	1.83	0.17
Final Sat.:	1600	1071	529	185	1354	62	1600	3114	186	1600	3026	274
Capacity Analysis Module:												
Vol/Sat:	0.03	0.59	0.59	0.08	0.67	0.67	0.10	0.63	0.65	0.12	0.42	0.44
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #65 Standard Ave and Mcfadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.843
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	63	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	1 0	1 0	0	1 0	1 0	0	1 0	1 0	0	1 0
Volume Module:												
Base Vol:	44	170	77	123	297	21	21	736	89	81	214	48
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	44	170	77	123	297	21	21	736	89	81	214	48
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	44	170	77	123	297	21	21	736	89	81	214	48
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	44	170	77	123	297	21	21	736	89	81	214	48
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	44	170	77	123	297	21	21	736	89	81	214	48
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	0.69	0.31	1.00	0.93	0.07	1.00	0.89	0.11	1.00	0.82	0.18
Final Sat.:	1600	1101	499	1600	1494	106	1600	1427	173	1600	1307	293
Capacity Analysis Module:												
Vol/Sat:	0.03	0.15	0.15	0.08	0.20	0.20	0.01	0.52	0.52	0.05	0.16	0.16
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #66 Halladay St and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.692
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	36	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	0 1	0	0	0 0 0	0	0	2 1 0	0	1	2 0 0
Volume Module:												
Base Vol:	69	0	160	0	0	0	0	1772	200	210	797	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	69	0	160	0	0	0	0	1772	200	210	797	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	69	0	160	0	0	0	0	1772	200	210	797	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	69	0	160	0	0	0	0	1772	200	210	797	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	69	0	160	0	0	0	0	1772	200	210	797	0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	2.70	0.30	0.63	2.37	0.00
Final Sat.:	1600	0	1600	0	0	0	0	4513	487	1001	3999	0
Capacity Analysis Module:												
Vol/Sat:	0.04	0.00	0.10	0.00	0.00	0.00	0.00	0.39	0.41	0.13	0.20	0.00
Crit Moves:			****						****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #67 Halladay St and Dyer Rd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.668
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	34	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	1 0	1 0	0	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	16	26	71	155	31	33	54	1820	13	126	686	111
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	16	26	71	155	31	33	54	1820	13	126	686	111
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	16	26	71	155	31	33	54	1820	13	126	686	111
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	16	26	71	155	31	33	54	1820	13	126	686	111
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	16	26	71	155	31	33	54	1820	13	126	686	111
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	0.27	0.73	1.00	0.48	0.52	1.00	2.98	0.02	1.00	2.58	0.42
Final Sat.:	1600	429	1171	1600	775	825	1600	4966	34	1600	4331	669
Capacity Analysis Module:												
Vol/Sat:	0.01	0.06	0.06	0.10	0.04	0.04	0.03	0.37	0.38	0.08	0.16	0.17
Crit Moves:			****	****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #68 SR-55 SB Ramps and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.623
Loss Time (sec):	5	Average Delay (sec/veh):	20.2
Optimal Cycle:	32	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Ignore			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	0	2	0	0	0	2	0	0	2	1	1	0	0	3	0	1
Volume Module:																		
Base Vol:	0	0	0	998	0	903	0	1423	1014	0	1363	155
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	998	0	903	0	1423	1014	0	1363	155
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Volume:	0	0	0	998	0	903	0	1423	0	0	1363	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	998	0	903	0	1423	0	0	1363	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
FinalVolume:	0	0	0	998	0	903	0	1423	0	0	1363	0
Saturation Flow Module:																		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.92	1.00	0.75	1.00	0.91	0.91	1.00	0.91	1.00
Lanes:	0.00	0.00	0.00	2.00	0.00	2.00	0.00	3.00	1.00	0.00	3.00	1.00
Final Sat.:	0	0	0	3502	0	2842	0	5187	1729	0	5187	1900
Capacity Analysis Module:																		
Vol/Sat:	0.00	0.00	0.00	0.29	0.00	0.32	0.00	0.27	0.00	0.00	0.26	0.00
Crit Moves:						****		****		****		
Green/Cycle:	0.00	0.00	0.00	0.51	0.00	0.51	0.00	0.44	0.00	0.00	0.44	0.00
Volume/Cap:	0.00	0.00	0.00	0.56	0.00	0.62	0.00	0.62	0.00	0.00	0.60	0.00
Delay/Veh:	0.0	0.0	0.0	17.2	0.0	18.5	0.0	22.1	0.0	0.0	21.7	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	17.2	0.0	18.5	0.0	22.1	0.0	0.0	21.7	0.0
LOS by Move:	A	A	A	B	A	B	A	C	A	A	C	A
HCM2k85thQ:	0	0	0	17	0	18	0	19	0	0	18	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #69 SR-55 NB Ramps and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.728
Loss Time (sec):	5	Average Delay (sec/veh):	19.5
Optimal Cycle:	42	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Ignore			Include			Ignore			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	0	0 1	0 0	0	0 0	0 0	2	0 2	0 0	3	0 1
Volume Module:												
Base Vol:	965	0	1037	0	0	0	0	1503	744	0	591	279
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	965	0	1037	0	0	0	0	1503	744	0	591	279
User Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Volume:	965	0	0	0	0	0	0	1503	0	0	591	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	965	0	0	0	0	0	0	1503	0	0	591	0
PCE Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
FinalVolume:	965	0	0	0	0	0	0	1503	0	0	591	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.88	1.00	0.91	1.00
Lanes:	2.00	0.00	1.00	0.00	0.00	0.00	0.00	2.00	2.00	0.00	3.00	1.00
Final Sat.:	3502	0	1900	0	0	0	0	3610	3344	0	5187	1900
Capacity Analysis Module:												
Vol/Sat:	0.28	0.00	0.00	0.00	0.00	0.00	0.00	0.42	0.00	0.00	0.11	0.00
Crit Moves:	****							****		****		
Green/Cycle:	0.38	0.00	0.00	0.00	0.00	0.00	0.00	0.57	0.00	0.00	0.57	0.00
Volume/Cap:	0.73	0.00	0.00	0.00	0.00	0.00	0.00	0.73	0.00	0.00	0.20	0.00
Delay/Veh:	28.7	0.0	0.0	0.0	0.0	0.0	0.0	17.1	0.0	0.0	10.4	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	28.7	0.0	0.0	0.0	0.0	0.0	0.0	17.1	0.0	0.0	10.4	0.0
LOS by Move:	C	A	A	A	A	A	A	B	A	A	B	A
HCM2k85thQ:	21	0	0	0	0	0	0	27	0	0	5	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #70 SR-55 SB Ramps and Dyer Rd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.610
Loss Time (sec):	5	Average Delay (sec/veh):	25.9
Optimal Cycle:	31	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Ovl			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 1	0 0	2	0 1	1 0	1	1 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	312	37	713	50	242	60	34	1282	273	418	663	111
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	312	37	713	50	242	60	34	1282	273	418	663	111
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	312	37	713	50	242	60	34	1282	273	418	663	111
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	312	37	713	50	242	60	34	1282	273	418	663	111
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	312	37	713	50	242	60	34	1282	273	418	663	111
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.96	0.96	0.75	0.94	0.94	0.85	0.95	0.91	0.85	0.92	0.91	0.85
Lanes:	1.79	0.21	2.00	0.34	1.66	1.00	1.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3251	386	2842	613	2968	1615	1805	5187	1615	3502	5187	1615
Capacity Analysis Module:												
Vol/Sat:	0.10	0.10	0.25	0.08	0.08	0.04	0.02	0.25	0.17	0.12	0.13	0.07
Crit Moves:			****		****			****		****		
Green/Cycle:	0.22	0.22	0.41	0.13	0.13	0.13	0.08	0.41	0.41	0.20	0.52	0.52
Volume/Cap:	0.45	0.45	0.61	0.61	0.61	0.28	0.24	0.61	0.42	0.61	0.24	0.13
Delay/Veh:	34.4	34.4	24.1	43.2	43.2	39.7	44.3	24.0	21.7	38.3	13.1	12.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	34.4	34.4	24.1	43.2	43.2	39.7	44.3	24.0	21.7	38.3	13.1	12.3
LOS by Move:	C	C	C	D	D	D	D	C	C	D	B	B
HCM2k85thQ:	8	8	16	8	8	3	2	18	9	11	6	3

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #71 Glassell St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.826
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	58	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Ovl			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	1 0	1	1 0	1 1	0	1 0	1 0	1	1 0	1 1	0
Volume Module:												
Base Vol:	292	458	139	13	274	60	57	293	563	380	401	10
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	292	458	139	13	274	60	57	293	563	380	401	10
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	292	458	139	13	274	60	57	293	563	380	401	10
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	292	458	139	13	274	60	57	293	563	380	401	10
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	292	458	139	13	274	60	57	293	563	380	401	10
OvlAdjVol:			0						417			
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.03	1.00
Lanes:	2.00	1.00	1.00	1.00	1.64	0.36	1.00	1.00	1.00	1.00	1.95	0.05
Final Sat.:	3200	1700	1600	1600	2725	575	1600	1700	1600	1600	3222	78
Capacity Analysis Module:												
Vol/Sat:	0.09	0.27	0.09	0.01	0.10	0.10	0.04	0.17	0.35	0.24	0.12	0.13
OvlAdjV/S:			0.00						0.26			
Crit Moves:		****		****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #72 Glassell St and SR-22 WB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.841
Loss Time (sec):	5	Average Delay (sec/veh):	30.1
Optimal Cycle:	65	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	0	0 0	2 0	1	0 0	0 0	0	1 0	1! 0	1
Volume Module:												
Base Vol:	568	638	0	0	1017	473	0	0	0	247	4	369
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	568	638	0	0	1017	473	0	0	0	247	4	369
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	568	638	0	0	1017	473	0	0	0	247	4	369
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	568	638	0	0	1017	473	0	0	0	247	4	369
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	568	638	0	0	1017	473	0	0	0	247	4	369
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.95	0.85	1.00	1.00	1.00	0.86	0.86	0.86
Lanes:	1.00	2.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	1.40	0.01	1.59
Final Sat.:	1805	3610	0	0	3610	1615	0	0	0	2271	21	2589
Capacity Analysis Module:												
Vol/Sat:	0.31	0.18	0.00	0.00	0.28	0.29	0.00	0.00	0.00	0.11	0.19	0.14
Crit Moves:	****					****					****	
Green/Cycle:	0.37	0.72	0.00	0.00	0.35	0.35	0.00	0.00	0.00	0.23	0.23	0.23
Volume/Cap:	0.84	0.24	0.00	0.00	0.81	0.84	0.00	0.00	0.00	0.48	0.84	0.63
Delay/Veh:	37.9	4.7	0.0	0.0	33.6	41.0	0.0	0.0	0.0	33.7	45.5	36.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	37.9	4.7	0.0	0.0	33.6	41.0	0.0	0.0	0.0	33.7	45.5	36.0
LOS by Move:	D	A	A	A	C	D	A	A	A	C	D	D
HCM2k85thQ:	27	5	0	0	25	24	0	0	0	8	18	11

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #73 Grand Ave / Glassell St and SR-22 EB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.842
Loss Time (sec):	5	Average Delay (sec/veh):	29.6
Optimal Cycle:	65	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	2	0 1	1 0	2	0 0	1 0	1!	0 1	0 0	0	0 0 0
Volume Module:												
Base Vol:	0	968	258	345	904	0	258	0	1012	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	968	258	345	904	0	258	0	1012	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	968	258	345	904	0	258	0	1012	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	968	258	345	904	0	258	0	1012	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	968	258	345	904	0	258	0	1012	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.95	0.85	0.95	0.95	1.00	0.87	1.00	0.87	1.00	1.00	1.00
Lanes:	0.00	2.00	1.00	1.00	2.00	0.00	1.20	0.00	1.80	0.00	0.00	0.00
Final Sat.:	0	3610	1615	1805	3610	0	1992	0	2974	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.27	0.16	0.19	0.25	0.00	0.13	0.00	0.34	0.00	0.00	0.00
Crit Moves:		****		****					****			
Green/Cycle:	0.00	0.32	0.32	0.23	0.55	0.00	0.40	0.00	0.40	0.00	0.00	0.00
Volume/Cap:	0.00	0.84	0.50	0.84	0.46	0.00	0.32	0.00	0.84	0.00	0.00	0.00
Delay/Veh:	0.0	37.5	28.4	51.4	13.9	0.0	20.4	0.0	31.3	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	37.5	28.4	51.4	13.9	0.0	20.4	0.0	31.3	0.0	0.0	0.0
LOS by Move:	A	D	C	D	B	A	C	A	C	A	A	A
HCM2k85thQ:	0	25	10	19	13	0	7	0	27	0	0	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #74 Grand Ave and Fairhaven Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.569
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	27	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	1 0	1 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	25	1032	148	184	1531	34	99	60	59	152	17	137
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	25	1032	148	184	1531	34	99	60	59	152	17	137
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	25	1032	148	184	1531	34	99	60	59	152	17	137
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	25	1032	148	184	1531	34	99	60	59	152	17	137
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	25	1032	148	184	1531	34	99	60	59	152	17	137
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	1.00	2.62	0.38	1.00	2.93	0.07	1.00	1.01	0.99	1.00	0.11	0.89
Final Sat.:	1600	4398	602	1600	4896	104	1600	1713	1587	1600	177	1423
Capacity Analysis Module:												
Vol/Sat:	0.02	0.23	0.25	0.12	0.31	0.33	0.06	0.04	0.04	0.10	0.10	0.10
Crit Moves:			****	****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #75 Grand Ave and Santa Clara Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.798
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	52	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			WideBypass			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	0	1 0	1 0	1	0 1
Volume Module:												
Base Vol:	92	648	167	307	1503	338	179	291	60	140	270	246
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	92	648	167	307	1503	338	179	291	60	140	270	246
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	92	648	167	307	1503	338	179	291	60	140	270	246
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	92	648	167	307	1503	338	179	291	60	140	270	246
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	92	648	167	307	1503	338	179	291	60	140	270	246
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.00	1.00	1.00	1.06	1.00
Lanes:	1.00	2.39	0.61	1.00	2.45	0.55	1.00	0.83	0.17	1.00	1.00	1.00
Final Sat.:	1600	4016	984	1600	4119	881	1600	1326	274	1600	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.06	0.16	0.17	0.19	0.36	0.38	0.11	0.22	0.22	0.09	0.16	0.15
Crit Moves:	****					****		****		****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #76 Grand Ave and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.856
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	67	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	2 0	2	1 0	2 0	2	1 0
Volume Module:												
Base Vol:	228	666	458	303	1270	249	246	915	116	401	1047	114
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	228	666	458	303	1270	249	246	915	116	401	1047	114
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	228	666	458	303	1270	249	246	915	116	401	1047	114
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	228	666	458	303	1270	249	246	915	116	401	1047	114
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	228	666	458	303	1270	249	246	915	116	401	1047	114
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.00	1.00	1.00	2.51	0.49	2.00	2.66	0.34	2.00	2.71	0.29
Final Sat.:	1600	3400	1600	1600	4213	787	3200	4460	540	3200	4529	471
Capacity Analysis Module:												
Vol/Sat:	0.14	0.20	0.29	0.19	0.30	0.32	0.08	0.21	0.21	0.13	0.23	0.24
Crit Moves:			****	****				****		****		

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #77 Grand Ave and I-5 NB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.845
Loss Time (sec):	5	Average Delay (sec/veh):	15.3
Optimal Cycle:	66	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Ignore			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	2 0	1	1 0	3 0	0	0 0	0 0	0 0	2 0	0 0	1
Volume Module:												
Base Vol:	0	1145	662	104	3338	0	0	0	0	557	0	154
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1145	662	104	3338	0	0	0	0	557	0	154
User Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1145	0	104	3338	0	0	0	0	557	0	154
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1145	0	104	3338	0	0	0	0	557	0	154
PCE Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1145	0	104	3338	0	0	0	0	557	0	154
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.95	1.00	0.95	0.91	1.00	1.00	1.00	1.00	0.92	1.00	0.85
Lanes:	0.00	2.00	1.00	1.00	3.00	0.00	0.00	0.00	0.00	2.00	0.00	1.00
Final Sat.:	0	3610	1900	1805	5187	0	0	0	0	3502	0	1615
Capacity Analysis Module:												
Vol/Sat:	0.00	0.32	0.00	0.06	0.64	0.00	0.00	0.00	0.00	0.16	0.00	0.10
Crit Moves:	****				****					****		
Green/Cycle:	0.00	0.64	0.00	0.12	0.76	0.00	0.00	0.00	0.00	0.19	0.00	0.19
Volume/Cap:	0.00	0.49	0.00	0.49	0.84	0.00	0.00	0.00	0.00	0.84	0.00	0.51
Delay/Veh:	0.0	9.4	0.0	43.2	9.8	0.0	0.0	0.0	0.0	49.0	0.0	37.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	9.4	0.0	43.2	9.8	0.0	0.0	0.0	0.0	49.0	0.0	37.8
LOS by Move:	A	A	A	D	A	A	A	A	A	D	A	D
HCM2k85thQ:	0	15	0	6	39	0	0	0	0	17	0	7

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #78 Grand Ave and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.881
Loss Time (sec):	5	Average Delay (sec/veh):	26.3
Optimal Cycle:	80	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	0 2	2 0	1	0 2	0 1	0	1 0
Volume Module:												
Base Vol:	79	1119	57	231	1908	1684	242	200	394	5	407	37
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	79	1119	57	231	1908	1684	242	200	394	5	407	37
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	79	1119	57	231	1908	1684	242	200	394	5	407	37
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	79	1119	57	231	1908	1684	242	200	394	5	407	37
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	79	1119	57	231	1908	1684	242	200	394	5	407	37
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.90	0.90	0.95	0.95	0.75	0.92	1.00	0.75	0.94	0.94	0.94
Lanes:	1.00	2.85	0.15	1.00	2.00	2.00	2.00	1.00	2.00	0.02	1.82	0.16
Final Sat.:	1805	4901	250	1805	3610	2842	3502	1900	2842	40	3230	294
Capacity Analysis Module:												
Vol/Sat:	0.04	0.23	0.23	0.13	0.53	0.59	0.07	0.11	0.14	0.13	0.13	0.13
Crit Moves:	****				****				****	****		
Green/Cycle:	0.05	0.42	0.42	0.23	0.60	0.76	0.16	0.16	0.16	0.14	0.14	0.14
Volume/Cap:	0.88	0.55	0.55	0.55	0.88	0.78	0.44	0.67	0.88	0.88	0.88	0.88
Delay/Veh:	105.1	22.4	22.4	35.2	21.6	9.2	38.7	45.4	59.3	58.3	58.3	58.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	105.1	22.4	22.4	35.2	21.6	9.2	38.7	45.4	59.3	58.3	58.3	58.3
LOS by Move:	F	C	C	D	C	A	D	D	E	E	E	E
HCM2k85thQ:	7	15	15	10	41	28	6	11	15	15	15	15

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #79 Grand Ave and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.827
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	58	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0 2	1 0		2 0 3	0 1		2 0 2	0 1		2 0 2	0 1	
Volume Module:												
Base Vol:	186	655	31	121	1753	213	318	760	216	268	939	107
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	186	655	31	121	1753	213	318	760	216	268	939	107
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	186	655	31	121	1753	213	318	760	216	268	939	107
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	186	655	31	121	1753	213	318	760	216	268	939	107
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	186	655	31	121	1753	213	318	760	216	268	939	107
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.86	0.14	2.00	3.00	1.00	2.00	2.00	1.00	2.00	2.00	1.00
Final Sat.:	3200	4783	217	3200	5100	1600	3200	3400	1600	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.06	0.14	0.14	0.04	0.34	0.13	0.10	0.22	0.14	0.08	0.28	0.07
Crit Moves:	****				****		****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #80 Grand Ave and Chestnut Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.935
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	113	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	0	1 0	1 0	0	1 0
Volume Module:												
Base Vol:	53	737	163	174	2086	57	64	449	118	81	237	148
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	53	737	163	174	2086	57	64	449	118	81	237	148
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	53	737	163	174	2086	57	64	449	118	81	237	148
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	53	737	163	174	2086	57	64	449	118	81	237	148
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	53	737	163	174	2086	57	64	449	118	81	237	148
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	2.46	0.54	1.00	2.92	0.08	1.00	0.79	0.21	1.00	0.62	0.38
Final Sat.:	1600	4131	869	1600	4872	128	1600	1267	333	1600	985	615
Capacity Analysis Module:												
Vol/Sat:	0.03	0.18	0.19	0.11	0.43	0.45	0.04	0.35	0.35	0.05	0.24	0.24
Crit Moves:	****					****		****		****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #81 Grand Ave and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	1.011
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	1 0	0 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	262	1104	165	294	1974	192	109	352	107	95	340	86
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	262	1104	165	294	1974	192	109	352	107	95	340	86
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	262	1104	165	294	1974	192	109	352	107	95	340	86
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	262	1104	165	294	1974	192	109	352	107	95	340	86
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	262	1104	165	294	1974	192	109	352	107	95	340	86
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.00	1.00	1.00	1.04	1.00
Lanes:	1.00	2.61	0.39	1.00	2.73	0.27	1.00	0.77	0.23	1.00	1.60	0.40
Final Sat.:	1600	4376	624	1600	4575	425	1600	1227	373	1600	2654	646
Capacity Analysis Module:												
Vol/Sat:	0.16	0.25	0.26	0.18	0.43	0.45	0.07	0.29	0.29	0.06	0.13	0.13
Crit Moves:	****					****		****		****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #82 Grand Ave and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.897
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	85	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	65	649	127	253	1293	253	396	1654	159	170	651	142
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	65	649	127	253	1293	253	396	1654	159	170	651	142
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	65	649	127	253	1293	253	396	1654	159	170	651	142
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	65	649	127	253	1293	253	396	1654	159	170	651	142
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	65	649	127	253	1293	253	396	1654	159	170	651	142
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.51	0.49	1.00	2.51	0.49	1.00	2.74	0.26	1.00	2.46	0.54
Final Sat.:	1600	4214	786	1600	4214	786	1600	4579	421	1600	4140	860
Capacity Analysis Module:												
Vol/Sat:	0.04	0.15	0.16	0.16	0.31	0.32	0.25	0.36	0.38	0.11	0.16	0.17
Crit Moves:	****					****			****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #83 Grand Ave and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.664
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	34	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	2 0	3 0	1	2 0	2 1	0
Volume Module:												
Base Vol:	187	564	241	204	589	199	314	1120	440	140	617	208
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	187	564	241	204	589	199	314	1120	440	140	617	208
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	187	564	241	204	589	199	314	1120	440	140	617	208
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	187	564	241	204	589	199	314	1120	440	140	617	208
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	187	564	241	204	589	199	314	1120	440	140	617	208
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	2.10	0.90	1.00	2.24	0.76	2.00	3.00	1.00	2.00	2.24	0.76
Final Sat.:	1600	3563	1437	1600	3788	1212	3200	5100	1600	3200	3790	1210
Capacity Analysis Module:												
Vol/Sat:	0.12	0.16	0.17	0.13	0.16	0.16	0.10	0.22	0.28	0.04	0.16	0.17
Crit Moves:		****	****						****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #84 SR-55 NB Ramps and Dyer Rd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.565
Loss Time (sec):	5	Average Delay (sec/veh):	17.3
Optimal Cycle:	28	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Ignore			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	1! 0	1	0	0	0	0	0	3	0	1	0	0	2	1	0
Volume Module:												
Base Vol:	637	0	521	0	0	0	0	1550	620	0	859	517
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	637	0	521	0	0	0	0	1550	620	0	859	517
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Volume:	637	0	521	0	0	0	0	1550	0	0	859	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	637	0	521	0	0	0	0	1550	0	0	859	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
FinalVolume:	637	0	521	0	0	0	0	1550	0	0	859	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.91	1.00	0.91	1.00	1.00	1.00	1.00	0.91	1.00	1.00	0.91	0.91
Lanes:	1.55	0.00	1.45	0.00	0.00	0.00	0.00	3.00	1.00	0.00	3.00	0.00
Final Sat.:	2674	0	2501	0	0	0	0	5187	1900	0	5187	0
Capacity Analysis Module:												
Vol/Sat:	0.24	0.00	0.21	0.00	0.00	0.00	0.00	0.30	0.00	0.00	0.17	0.00
Crit Moves:	****							****		****		
Green/Cycle:	0.42	0.00	0.42	0.00	0.00	0.00	0.00	0.53	0.00	0.00	0.53	0.00
Volume/Cap:	0.57	0.00	0.49	0.00	0.00	0.00	0.00	0.57	0.00	0.00	0.31	0.00
Delay/Veh:	22.3	0.0	21.3	0.0	0.0	0.0	0.0	16.1	0.0	0.0	13.4	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	22.3	0.0	21.3	0.0	0.0	0.0	0.0	16.1	0.0	0.0	13.4	0.0
LOS by Move:	C	A	C	A	A	A	A	B	A	A	B	A
HCM2k85thQ:	15	0	13	0	0	0	0	18	0	0	8	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #85 Cambridge St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.900
Loss Time (sec):	5	Average Delay (sec/veh):	23.4
Optimal Cycle:	0	Level Of Service:	C

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	T	R		L	T	R		L	T	R		L	T	R	
Control:	Stop Sign				Stop Sign				Stop Sign				Stop Sign			
Rights:	Include				Include				Include				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Lanes:	1	0	1	0	0	0	0	1	0	1	0	0	0	1	0	0
Volume Module:																
Base Vol:	185	235	0		0	480	245		195	0	312		0	0	0	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	185	235	0		0	480	245		195	0	312		0	0	0	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	185	235	0		0	480	245		195	0	312		0	0	0	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	185	235	0		0	480	245		195	0	312		0	0	0	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	185	235	0		0	480	245		195	0	312		0	0	0	
Saturation Flow Module:																
Adjustment:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Lanes:	1.00	1.00	0.00		0.00	1.00	1.00		1.00	0.00	1.00		0.00	0.00	0.00	
Final Sat.:	465	498	0		0	534	586		459	0	541		0	0	0	
Capacity Analysis Module:																
Vol/Sat:	0.40	0.47	xxxx		xxxx	0.90	0.42		0.43	xxxx	0.58		xxxx	xxxx	xxxx	
Crit Moves:		****				****					****					
Delay/Veh:	15.0	15.6	0.0		0.0	42.9	12.8		15.7	0.0	17.1		0.0	0.0	0.0	
Delay Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
AdjDel/Veh:	15.0	15.6	0.0		0.0	42.9	12.8		15.7	0.0	17.1		0.0	0.0	0.0	
LOS by Move:	B	C	*		*	E	B		C	*	C		*	*	*	
ApproachDel:	15.3				32.8				16.6				xxxxxx			
Delay Adj:	1.00				1.00				1.00				xxxxx			
ApprAdjDel:	15.3				32.8				16.6				xxxxxx			
LOS by Appr:	C				D				C				*			
AllWayAvgQ:	0.6	0.8	0.0		0.0	5.0	0.7		0.7	0.0	1.2		0.0	0.0	0.0	

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #86 Cambridge St and Fairhaven Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.624
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	30	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	0	0 0	1 0	0	0 1	1 0	1	0 0	0 0	0	1 0
Volume Module:												
Base Vol:	0	0	0	618	0	199	69	185	0	0	130	101
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	618	0	199	69	185	0	0	130	101
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	618	0	199	69	185	0	0	130	101
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	618	0	199	69	185	0	0	130	101
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	618	0	199	69	185	0	0	130	101
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	0.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	0.00	0.00	0.56	0.44
Final Sat.:	0	0	0	1600	0	1600	1600	1700	0	0	900	700
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.39	0.00	0.12	0.04	0.11	0.00	0.00	0.14	0.14
Crit Moves:				****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #87 Mabury St and 1st Street

Cycle (sec):	100	Critical Vol./Cap.(X):	0.755
Loss Time (sec):	5	Average Delay (sec/veh):	27.4
Optimal Cycle:	46	Level Of Service:	C

Street Name:	Mabury								1st Street			
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	0	1	1	0	1! 0	1	0	0	1	0
Volume Module:												
Base Vol:	5	0	122	276	164	618	0	922	10	46	701	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	5	0	122	276	164	618	0	922	10	46	701	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	5	0	122	276	164	618	0	922	10	46	701	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	5	0	122	276	164	618	0	922	10	46	701	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	5	0	122	276	164	618	0	922	10	46	701	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	0.85	0.90	0.90	0.90	1.00	0.95	0.95	0.95	0.95	0.95
Lanes:	1.00	0.00	1.00	1.23	0.27	1.50	0.00	1.98	0.02	1.00	2.00	0.00
Final Sat.:	1805	0	1615	2097	459	2575	0	3564	39	1805	3610	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.08	0.13	0.36	0.24	0.00	0.26	0.26	0.03	0.19	0.00
Crit Moves:			****		****			****		****		
Green/Cycle:	0.10	0.00	0.10	0.47	0.47	0.47	0.00	0.34	0.34	0.03	0.38	0.00
Volume/Cap:	0.03	0.00	0.75	0.28	0.75	0.51	0.00	0.75	0.75	0.75	0.52	0.00
Delay/Veh:	40.7	0.0	61.9	16.0	24.0	18.5	0.0	31.8	31.8	88.8	24.5	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	40.7	0.0	61.9	16.0	24.0	18.5	0.0	31.8	31.8	88.8	24.5	0.0
LOS by Move:	D	A	E	B	C	B	A	C	C	F	C	A
HCM2k85thQ:	0	0	8	7	25	14	0	22	22	5	14	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #88 Tustin St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.362
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	19	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 1	0	0 1
Volume Module:												
Base Vol:	9	365	105	48	944	3	9	11	35	279	1	43
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	9	365	105	48	944	3	9	11	35	279	1	43
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	9	365	105	48	944	3	9	11	35	279	1	43
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	9	365	105	48	944	3	9	11	35	279	1	43
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	9	365	105	48	944	3	9	11	35	279	1	43
OvlAdjVol:												0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	1.00	2.33	0.67	1.00	2.99	0.01	1.00	1.00	1.00	1.99	0.01	1.00
Final Sat.:	1600	3928	1072	1600	4985	15	1600	1700	1600	3189	11	1600
Capacity Analysis Module:												
Vol/Sat:	0.01	0.09	0.10	0.03	0.19	0.20	0.01	0.01	0.02	0.09	0.09	0.03
OvlAdjV/S:												0.00
Crit Moves:	****					****			****		****	

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #89 Tustin St and SR-22 WB On-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.575
Loss Time (sec):	5	Average Delay (sec/veh):	13.2
Optimal Cycle:	29	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0 2	0 0	0	0 2	1 0	0	0 0	0 0	0	0 0	0 0
Volume Module:												
Base Vol:	762	708	0	0	897	536	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	762	708	0	0	897	536	0	0	0	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	762	708	0	0	897	536	0	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	762	708	0	0	897	536	0	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	762	708	0	0	897	536	0	0	0	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.95	1.00	1.00	0.86	0.86	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	2.00	2.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00
Final Sat.:	3502	3610	0	0	3264	1632	0	0	0	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.22	0.20	0.00	0.00	0.27	0.33	0.00	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****					****						
Green/Cycle:	0.38	0.95	0.00	0.00	0.57	0.57	0.00	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.57	0.21	0.00	0.00	0.48	0.57	0.00	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	25.3	0.2	0.0	0.0	12.8	14.0	0.0	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	25.3	0.2	0.0	0.0	12.8	14.0	0.0	0.0	0.0	0.0	0.0	0.0
LOS by Move:	C	A	A	A	B	B	A	A	A	A	A	A
HCM2k85thQ:	15	1	0	0	14	18	0	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #90 Tustin St and SR-22 EB Off-Ramp / Seba Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.718
Loss Time (sec):	5	Average Delay (sec/veh):	23.8
Optimal Cycle:	41	Level Of Service:	C

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	- T	- R		L	- T	- R		L	- T	- R		L	- T	- R	
Control:	Permitted				Permitted				Split Phase				Split Phase			
Rights:	Include				Include				Include				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Y+R:	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0	
Lanes:	0	0 2	1 0		1	0 3	0 0		1	0 1!	0 1		0	0 1!	0 0	
Volume Module:																
Base Vol:	0	1213	6		13	779	0		252	25	1084		6	0	43	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	0	1213	6		13	779	0		252	25	1084		6	0	43	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	0	1213	6		13	779	0		252	25	1084		6	0	43	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	0	1213	6		13	779	0		252	25	1084		6	0	43	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	0	1213	6		13	779	0		252	25	1084		6	0	43	
Saturation Flow Module:																
Sat/Lane:	1900	1900	1900		1900	1900	1900		1900	1900	1900		1900	1900	1900	
Adjustment:	1.00	0.91	0.91		0.12	0.91	1.00		0.87	0.87	0.87		0.88	1.00	0.88	
Lanes:	0.00	2.99	0.01		1.00	3.00	0.00		1.18	0.04	1.78		0.12	0.00	0.88	
Final Sat.:	0	5156	26		232	5187	0		1960	60	2956		204	0	1460	
Capacity Analysis Module:																
Vol/Sat:	0.00	0.24	0.24		0.06	0.15	0.00		0.13	0.42	0.37		0.03	0.00	0.03	
Crit Moves:		****								****			****			
Green/Cycle:	0.00	0.33	0.33		0.33	0.33	0.00		0.58	0.58	0.58		0.04	0.00	0.04	
Volume/Cap:	0.00	0.72	0.72		0.17	0.46	0.00		0.22	0.72	0.63		0.72	0.00	0.72	
Delay/Veh:	0.0	31.1	31.1		25.0	26.8	0.0		10.1	16.4	14.4		78.0	0.0	78.0	
User DelAdj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
AdjDel/Veh:	0.0	31.1	31.1		25.0	26.8	0.0		10.1	16.4	14.4		78.0	0.0	78.0	
LOS by Move:	A	C	C		C	C	A		B	B	B		E	A	E	
HCM2k85thQ:	0	20	20		1	11	0		5	24	19		5	0	5	

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #91 Tustin Ave and Fairhaven Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	1.242
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	1 0	0 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	125	720	126	249	2148	42	78	297	511	306	338	442
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	125	720	126	249	2148	42	78	297	511	306	338	442
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	125	720	126	249	2148	42	78	297	511	306	338	442
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	125	720	126	249	2148	42	78	297	511	306	338	442
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	125	720	126	249	2148	42	78	297	511	306	338	442
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	2.00	2.55	0.45	2.00	2.94	0.06	1.00	0.37	0.63	1.00	0.43	0.57
Final Sat.:	3200	4285	715	3200	4908	92	1600	588	1012	1600	693	907
Capacity Analysis Module:												
Vol/Sat:	0.04	0.17	0.18	0.08	0.44	0.46	0.05	0.51	0.50	0.19	0.49	0.49
Crit Moves:	****					****		****		****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #92 Tustin Ave and Santa Clara Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	1.097
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	- T	- R		L	- T	- R		L	- T	- R		L	- T	- R	
Control:	Protected				Protected				Permitted				Permitted			
Rights:	Include				Include				Include				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Y+R:	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0	
Lanes:	1	0 2	1	0	2	0 2	1	0	1	0 0	1	0	1	0 0	1	0
Volume Module:																
Base Vol:	102	556	104		135	2756	87		116	293	214		118	244	122	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	102	556	104		135	2756	87		116	293	214		118	244	122	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	102	556	104		135	2756	87		116	293	214		118	244	122	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	102	556	104		135	2756	87		116	293	214		118	244	122	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	102	556	104		135	2756	87		116	293	214		118	244	122	
Saturation Flow Module:																
Sat/Lane:	1600	1600	1600		1600	1600	1600		1600	1600	1600		1600	1600	1600	
Adjustment:	1.00	1.05	1.00		1.00	1.04	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Lanes:	1.00	2.53	0.47		2.00	2.91	0.09		1.00	0.58	0.42		1.00	0.67	0.33	
Final Sat.:	1600	4244	756		3200	4853	147		1600	925	675		1600	1067	533	
Capacity Analysis Module:																
Vol/Sat:	0.06	0.13	0.14		0.04	0.57	0.59		0.07	0.32	0.32		0.07	0.23	0.23	
Crit Moves:	****						****			****				****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #93 Tustin Ave and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.805
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	53	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 0	1	2 0	2 1	0	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	155	277	330	638	1841	20	215	849	145	488	770	188
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	155	277	330	638	1841	20	215	849	145	488	770	188
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	155	277	330	638	1841	20	215	849	145	488	770	188
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	155	277	330	638	1841	20	215	849	145	488	770	188
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	155	277	330	638	1841	20	215	849	145	488	770	188
OvlAdjVol:			86									0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.00	1.00	2.00	2.97	0.03	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	3400	1600	3200	4948	52	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.05	0.08	0.21	0.20	0.37	0.39	0.07	0.17	0.09	0.15	0.15	0.12
OvlAdjV/S:			0.05									0.00
Crit Moves:	****					****		****			****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #94 Tustin Ave and 4th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.791
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	50	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	2 0	2	0 1	1 0	2	1 1	1 0	2	1 0
Volume Module:												
Base Vol:	40	204	183	743	946	293	159	942	39	129	727	472
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	40	204	183	743	946	293	159	942	39	129	727	472
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	40	204	183	743	946	293	159	942	39	129	727	472
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	40	204	183	743	946	293	159	942	39	129	727	472
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	40	204	183	743	946	293	159	942	39	129	727	472
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	2.00	1.00	2.00	2.00	1.00	1.00	3.00	1.00	1.00	2.00	1.00
Final Sat.:	1600	3400	1600	3200	3400	1600	1600	5100	1600	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.06	0.11	0.23	0.28	0.18	0.10	0.18	0.02	0.08	0.21	0.30
Crit Moves:			****	****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #95 SR-55 SB Ramps / Auto Mall Dr and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.653
Loss Time (sec):	5	Average Delay (sec/veh):	25.1
Optimal Cycle:	34	Level Of Service:	C

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	- T	- R		L	- T	- R		L	- T	- R		L	- T	- R	
Control:	Split Phase				Split Phase				Protected				Protected			
Rights:	Include				Include				Ignore				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Y+R:	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0	
Lanes:	1	1 0	0	2	1	0 1	0	1	1	0 3	0	1	2	0 2	1	0
Volume Module:																
Base Vol:	456	68	540		50	30	8		34	1334	493		510	1202	157	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	456	68	540		50	30	8		34	1334	493		510	1202	157	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	0.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	0.00		1.00	1.00	1.00	
PHF Volume:	456	68	540		50	30	8		34	1334	0		510	1202	157	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	456	68	540		50	30	8		34	1334	0		510	1202	157	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	0.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	0.00		1.00	1.00	1.00	
FinalVolume:	456	68	540		50	30	8		34	1334	0		510	1202	157	
Saturation Flow Module:																
Sat/Lane:	1900	1900	1900		1900	1900	1900		1900	1900	1900		1900	1900	1900	
Adjustment:	0.96	0.96	0.75		0.95	1.00	0.85		0.95	0.91	1.00		0.92	0.89	0.89	
Lanes:	1.74	0.26	2.00		1.00	1.00	1.00		1.00	3.00	1.00		2.00	2.65	0.35	
Final Sat.:	3168	472	2842		1805	1900	1615		1805	5187	1900		3502	4510	589	
Capacity Analysis Module:																
Vol/Sat:	0.14	0.14	0.19		0.03	0.02	0.00		0.02	0.26	0.00		0.15	0.27	0.27	
Crit Moves:			****		****					****			****			
Green/Cycle:	0.29	0.29	0.29		0.04	0.04	0.04		0.04	0.39	0.00		0.22	0.58	0.58	
Volume/Cap:	0.49	0.49	0.65		0.65	0.37	0.12		0.46	0.65	0.00		0.65	0.46	0.46	
Delay/Veh:	29.7	29.7	32.9		65.6	49.5	46.8		51.5	25.5	0.0		37.3	12.4	12.4	
User DelAdj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
AdjDel/Veh:	29.7	29.7	32.9		65.6	49.5	46.8		51.5	25.5	0.0		37.3	12.4	12.4	
LOS by Move:	C	C	C		E	D	D		D	C	A		D	B	B	
HCM2k85thQ:	11	11	14		4	2	1		3	19	0		13	13	13	

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Note: Queue reported is the number of cars per lane.
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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #96 SR-55 NB Ramps / Del Amo Ave and Newport Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	0.665
Loss Time (sec):	5	Average Delay (sec/veh):	25.6
Optimal Cycle:	35	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0 2		1 0	1 0 3		0 1	2 0 1		1 0	1 0 1		1 0
Volume Module:												
Base Vol:	413	109	69	20	497	728	269	163	155	8	223	4
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	413	109	69	20	497	728	269	163	155	8	223	4
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	413	109	69	20	497	728	269	163	155	8	223	4
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	413	109	69	20	497	728	269	163	155	8	223	4
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	413	109	69	20	497	728	269	163	155	8	223	4
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.86	0.86	0.95	0.91	0.85	0.92	0.88	0.88	0.95	0.95	0.95
Lanes:	2.00	2.00	1.00	1.00	3.00	1.00	2.00	1.03	0.97	1.00	1.96	0.04
Final Sat.:	3502	3257	1629	1805	5187	1615	3502	1715	1631	1805	3536	63
Capacity Analysis Module:												
Vol/Sat:	0.12	0.03	0.04	0.01	0.10	0.45	0.08	0.10	0.10	0.00	0.06	0.06
Crit Moves:	****					****	****				****	
Green/Cycle:	0.18	0.59	0.59	0.15	0.56	0.68	0.12	0.20	0.20	0.01	0.09	0.09
Volume/Cap:	0.67	0.06	0.07	0.07	0.17	0.67	0.67	0.47	0.47	0.47	0.67	0.67
Delay/Veh:	41.1	8.9	8.9	36.4	10.6	11.0	46.5	35.8	35.8	68.7	48.6	48.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	41.1	8.9	8.9	36.4	10.6	11.0	46.5	35.8	35.8	68.7	48.6	48.6
LOS by Move:	D	A	A	D	B	B	D	D	D	E	D	D
HCM2k85thQ:	11	1	2	1	4	21	8	8	8	1	8	8

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #97 Red Hill Ave and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.619
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	30	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Ovl			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0 3	0 1	2	0 3	0 1	2	0 3	0 1	2	0 3	0 1
Volume Module:												
Base Vol:	122	449	128	203	861	403	397	1276	271	250	1213	170
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	122	449	128	203	861	403	397	1276	271	250	1213	170
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	122	449	128	203	861	403	397	1276	271	250	1213	170
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	122	449	128	203	861	403	397	1276	271	250	1213	170
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	122	449	128	203	861	403	397	1276	271	250	1213	170
OvlAdjVol:						205			210			
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.04	0.09	0.08	0.06	0.17	0.25	0.12	0.25	0.17	0.08	0.24	0.11
OvlAdjV/S:						0.13			0.13			
Crit Moves:	****				****		****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #98 Red Hill Ave and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.580
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	28	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 1	0	2 0	2 1	0	1 0	2 1	0	2 0	3 0	1
Volume Module:												
Base Vol:	118	716	97	20	949	191	239	407	300	152	543	96
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	118	716	97	20	949	191	239	407	300	152	543	96
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	118	716	97	20	949	191	239	407	300	152	543	96
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	118	716	97	20	949	191	239	407	300	152	543	96
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	118	716	97	20	949	191	239	407	300	152	543	96
OvlAdjVol:												86
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.52	0.48	2.00	2.50	0.50	1.00	2.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5936	764	3200	4196	804	1600	3400	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.04	0.12	0.13	0.01	0.23	0.24	0.15	0.12	0.19	0.05	0.11	0.06
OvlAdjV/S:												0.05
Crit Moves:	****					****	****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #99 Red Hill Ave and Dyer Rd / barranca Pkwy

Cycle (sec):	100	Critical Vol./Cap.(X):	0.577
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	27	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	4 0	1	2 0	4 0	1	2 0	3 1	0	2 0	4 0	1
Volume Module:												
Base Vol:	184	458	151	184	809	106	126	758	300	522	583	114
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	184	458	151	184	809	106	126	758	300	522	583	114
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	184	458	151	184	809	106	126	758	300	522	583	114
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	184	458	151	184	809	106	126	758	300	522	583	114
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	184	458	151	184	809	106	126	758	300	522	583	114
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	4.00	1.00	2.00	4.00	1.00	2.00	3.00	1.00	2.00	4.00	1.00
Final Sat.:	3200	6800	1600	3200	6800	1600	3200	5100	1600	3200	6800	1600
Capacity Analysis Module:												
Vol/Sat:	0.06	0.07	0.09	0.06	0.12	0.07	0.04	0.15	0.19	0.16	0.09	0.07
Crit Moves:	****				****				****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #100 Red Hill Ave and Alton Pkwy

Cycle (sec):	100	Critical Vol./Cap.(X):	0.930
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	109	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	2 0	1	2 0	1 0	1
Volume Module:												
Base Vol:	235	693	434	233	1059	170	185	574	574	335	284	159
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	235	693	434	233	1059	170	185	574	574	335	284	159
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	235	693	434	233	1059	170	185	574	574	335	284	159
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	235	693	434	233	1059	170	185	574	574	335	284	159
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	235	693	434	233	1059	170	185	574	574	335	284	159
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	2.00	1.00	2.00	1.00	1.00
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	3400	1600	3200	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.15	0.14	0.27	0.15	0.21	0.11	0.12	0.17	0.36	0.10	0.17	0.10
Crit Moves:			****	****					****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #101 Red Hill Ave and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.709
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	38	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ignore			Include			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0 2	1 0		2 0 3	0 1		2 0 3	0 1		1 0 3	0 1	
Volume Module:												
Base Vol:	83	742	13	205	529	485	1194	617	101	34	329	843
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	83	742	13	205	529	485	1194	617	101	34	329	843
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Volume:	83	742	13	205	529	0	1194	617	101	34	329	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	83	742	13	205	529	0	1194	617	101	34	329	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
FinalVolume:	83	742	13	205	529	0	1194	617	101	34	329	0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.95	0.05	2.00	3.00	1.00	2.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	4917	83	3200	5100	1600	3200	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.15	0.16	0.06	0.10	0.00	0.37	0.12	0.06	0.02	0.06	0.00
Crit Moves:			****	****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #102 Red Hill Ave and Main St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.733
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	41	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0 2	0 1	2	0 2	0 1	2	0 2	1 0	2	0 2	1 0
Volume Module:												
Base Vol:	201	691	449	78	365	126	139	1309	254	167	335	79
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	201	691	449	78	365	126	139	1309	254	167	335	79
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	201	691	449	78	365	126	139	1309	254	167	335	79
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	201	691	449	78	365	126	139	1309	254	167	335	79
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	201	691	449	78	365	126	139	1309	254	167	335	79
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	2.00	2.00	1.00	2.00	2.00	1.00	2.00	2.51	0.49	2.00	2.43	0.57
Final Sat.:	3200	3400	1600	3200	3400	1600	3200	4220	780	3200	4084	916
Capacity Analysis Module:												
Vol/Sat:	0.06	0.20	0.28	0.02	0.11	0.08	0.04	0.31	0.33	0.05	0.08	0.09
Crit Moves:			****	****					****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #103 I-5 SB Ramps and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.480
Loss Time (sec):	5	Average Delay (sec/veh):	17.8
Optimal Cycle:	24	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	0	2	0	0	2	0	3	0	2	1
Volume Module:												
Base Vol:	0	0	0	382	0	38	287	492	0	0	1027	300
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	382	0	38	287	492	0	0	1027	300
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	382	0	38	287	492	0	0	1027	300
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	382	0	38	287	492	0	0	1027	300
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	382	0	38	287	492	0	0	1027	300
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.92	1.00	0.85	0.92	0.91	1.00	1.00	0.88	0.88
Lanes:	0.00	0.00	0.00	2.00	0.00	1.00	2.00	3.00	0.00	0.00	2.32	0.68
Final Sat.:	0	0	0	3502	0	1615	3502	5187	0	0	3878	1133
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.11	0.00	0.02	0.08	0.09	0.00	0.00	0.26	0.26
Crit Moves:				****			****				****	
Green/Cycle:	0.00	0.00	0.00	0.23	0.00	0.40	0.17	0.72	0.00	0.00	0.55	0.55
Volume/Cap:	0.00	0.00	0.00	0.48	0.00	0.06	0.48	0.13	0.00	0.00	0.48	0.48
Delay/Veh:	0.0	0.0	0.0	34.0	0.0	18.6	38.1	4.3	0.0	0.0	13.8	13.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

AdjDel/Veh:	0.0	0.0	0.0	34.0	0.0	18.6	38.1	4.3	0.0	0.0	13.8	13.8
LOS by Move:	A	A	A	C	A	B	D	A	A	A	B	B
HCM2k85thQ:	0	0	0	9	0	1	7	3	0	0	14	14

****

Note: Queue reported is the number of cars per lane.

****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #104 Tustin Ranch Rd and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.530
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	25	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	0 0	0	2 0	0 0	1	2 0	3 0	0	0 0	3 0	1
Volume Module:												
Base Vol:	0	0	0	174	0	22	27	357	0	0	2130	490
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	174	0	22	27	357	0	0	2130	490
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	174	0	22	27	357	0	0	2130	490
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	174	0	22	27	357	0	0	2130	490
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	174	0	22	27	357	0	0	2130	490
OvlAdjVol:												403
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	0.00	0.00	0.00	2.00	0.00	1.00	2.00	3.00	0.00	0.00	3.00	1.00
Final Sat.:	0	0	0	3200	0	1600	3200	5100	0	0	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.05	0.00	0.01	0.01	0.07	0.00	0.00	0.42	0.31
OvlAdjV/S:												0.25
Crit Moves:				****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #105 Von Karman Ave and Barranca Pkwy

Cycle (sec):	100	Critical Vol./Cap.(X):	0.930
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	108	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0 1	1 0		2 0 2			2 0 3	0 1		2 0 4	0 1	
Volume Module:												
Base Vol:	375	497	298	98	960	420	247	928	310	917	1506	47
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	375	497	298	98	960	420	247	928	310	917	1506	47
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	375	497	298	98	960	420	247	928	310	917	1506	47
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	375	497	298	98	960	420	247	928	310	917	1506	47
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	375	497	298	98	960	420	247	928	310	917	1506	47
OvlAdjVol:						173						
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	1.25	0.75	2.00	2.00	2.00	2.00	3.00	1.00	2.00	4.00	1.00
Final Sat.:	3200	2101	1199	3200	3400	3200	3200	5100	1600	3200	6800	1600
Capacity Analysis Module:												
Vol/Sat:	0.12	0.24	0.25	0.03	0.28	0.13	0.08	0.18	0.19	0.29	0.22	0.03
OvlAdjV/S:						0.05						
Crit Moves:	****			****					****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #106 Red Hill Avenue and El Camino Real

Cycle (sec):	100	Critical Vol./Cap.(X):	0.640
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	32	Level Of Service:	B

Street Name:	Red Hill Avenue						El Camino Real					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0	3	0	1	1	0	2	1	0	1	0	1	0	1	0	0	1	0
Volume Module:												
Base Vol:	222	569	183	20	1440	88	53	120	126	197	161	12
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	222	569	183	20	1440	88	53	120	126	197	161	12
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	222	569	183	20	1440	88	53	120	126	197	161	12
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	222	569	183	20	1440	88	53	120	126	197	161	12
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	222	569	183	20	1440	88	53	120	126	197	161	12
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	2.00	3.00	1.00	1.00	2.83	0.17	1.00	1.00	1.00	1.00	0.93	0.07
Final Sat.:	3200	5100	1600	1600	4724	276	1600	1700	1600	1600	1489	111
Capacity Analysis Module:												
Vol/Sat:	0.07	0.11	0.11	0.01	0.30	0.32	0.03	0.07	0.08	0.12	0.11	0.11
Crit Moves:	****					****			****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #107 Red Hill Avenue and I-5 NB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.631
Loss Time (sec):	5	Average Delay (sec/veh):	20.1
Optimal Cycle:	32	Level Of Service:	C

Street Name:	Red Hill Avenue						I-5 NB Ramps					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	0	0 0	3 0	1	0 0	0 0	0	2 0	0 0	1
Volume Module:												
Base Vol:	266	883	0	0	1217	458	0	0	0	238	0	272
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	266	883	0	0	1217	458	0	0	0	238	0	272
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	266	883	0	0	1217	458	0	0	0	238	0	272
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	266	883	0	0	1217	458	0	0	0	238	0	272
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	266	883	0	0	1217	458	0	0	0	238	0	272
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	1.00	1.00	0.91	0.85	1.00	1.00	1.00	0.60	1.00	0.85
Lanes:	1.00	3.00	0.00	0.00	3.00	1.00	0.00	0.00	0.00	2.00	0.00	1.00
Final Sat.:	1805	5187	0	0	5187	1615	0	0	0	2271	0	1615
Capacity Analysis Module:												
Vol/Sat:	0.15	0.17	0.00	0.00	0.23	0.28	0.00	0.00	0.00	0.10	0.00	0.17
Crit Moves:	****					****						****
Green/Cycle:	0.23	0.68	0.00	0.00	0.45	0.45	0.00	0.00	0.00	0.27	0.00	0.27
Volume/Cap:	0.63	0.25	0.00	0.00	0.52	0.63	0.00	0.00	0.00	0.39	0.00	0.63
Delay/Veh:	37.5	6.1	0.0	0.0	20.0	23.0	0.0	0.0	0.0	30.4	0.0	35.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

AdjDel/Veh:	37.5	6.1	0.0	0.0	20.0	23.0	0.0	0.0	0.0	30.4	0.0	35.3
LOS by Move:	D	A	A	A	C	C	A	A	A	C	A	D
HCM2k85thQ:	13	6	0	0	15	17	0	0	0	5	0	13

****

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #108 Red Hill Avenue and I-5 SB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.824
Loss Time (sec):	5	Average Delay (sec/veh):	24.7
Optimal Cycle:	60	Level Of Service:	C

Street Name:	Red Hill Avenue						I-5 SB Ramps					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	4 0	1	1 0	3 0	0	0 1	0	1	0 0	0 0	0
Volume Module:												
Base Vol:	0	1191	527	392	1125	0	210	5	386	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1191	527	392	1125	0	210	5	386	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1191	527	392	1125	0	210	5	386	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1191	527	392	1125	0	210	5	386	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1191	527	392	1125	0	210	5	386	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	0.85	0.95	0.91	1.00	0.95	0.95	0.85	1.00	1.00	1.00
Lanes:	0.00	4.00	1.00	1.00	3.00	0.00	0.98	0.02	1.00	0.00	0.00	0.00
Final Sat.:	0	6916	1615	1805	5187	0	1759	42	1615	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.17	0.33	0.22	0.22	0.00	0.12	0.12	0.24	0.00	0.00	0.00
Crit Moves:			****	****					****			
Green/Cycle:	0.00	0.40	0.40	0.26	0.66	0.00	0.29	0.29	0.29	0.00	0.00	0.00
Volume/Cap:	0.00	0.43	0.82	0.82	0.33	0.00	0.41	0.41	0.82	0.00	0.00	0.00
Delay/Veh:	0.0	22.1	35.6	45.8	7.4	0.0	29.1	29.1	44.4	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	22.1	35.6	45.8	7.4	0.0	29.1	29.1	44.4	0.0	0.0	0.0
LOS by Move:	A	C	D	D	A	A	C	C	D	A	A	A
HCM2k85thQ:	0	11	25	20	8	0	9	9	20	0	0	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #109 Red Hill Avenue and Nisson Road

Cycle (sec):	100	Critical Vol./Cap.(X):	0.655
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	33	Level Of Service:	B

Street Name:	Red Hill Avenue						Nisson Road					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 1	0	1 0	2 1	0	1 0	0 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	16	1244	23	169	1274	134	242	59	48	49	26	215
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	16	1244	23	169	1274	134	242	59	48	49	26	215
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	16	1244	23	169	1274	134	242	59	48	49	26	215
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	16	1244	23	169	1274	134	242	59	48	49	26	215
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	16	1244	23	169	1274	134	242	59	48	49	26	215
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	3.93	0.07	1.00	2.71	0.29	1.00	0.55	0.45	1.00	0.11	0.89
Final Sat.:	1600	6584	116	1600	4543	457	1600	882	718	1600	173	1427
Capacity Analysis Module:												
Vol/Sat:	0.01	0.19	0.20	0.11	0.28	0.29	0.15	0.07	0.07	0.03	0.15	0.15
Crit Moves:			****	****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #110 Red Hill Avenue and Walnut Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	0.643
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	32	Level Of Service:	B

Street Name:	Red Hill Avenue						Walnut Avenue					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	2 1	0	1	0	2 1	0	1	0	1	1 0	2	0	1	1	0
Volume Module:												
Base Vol:	135	525	99	117	1180	132	99	247	244	263	348	166
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	135	525	99	117	1180	132	99	247	244	263	348	166
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	135	525	99	117	1180	132	99	247	244	263	348	166
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	135	525	99	117	1180	132	99	247	244	263	348	166
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	135	525	99	117	1180	132	99	247	244	263	348	166
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.05	1.00
Lanes:	1.00	2.52	0.48	1.00	2.70	0.30	1.00	1.01	0.99	2.00	1.35	0.65
Final Sat.:	1600	4238	762	1600	4517	483	1600	1710	1590	3200	2267	1033
Capacity Analysis Module:												
Vol/Sat:	0.08	0.12	0.13	0.07	0.26	0.27	0.06	0.14	0.15	0.08	0.15	0.16
Crit Moves:	****					****			****	****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #111 Red Hill Avenue and Valencia Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	0.677
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	35	Level Of Service:	B

Street Name:	Red Hill Avenue						Valencia Avenue					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 1	0	1 0	2 1	0	1 0	1 0	1	2 0	1 0	1
Volume Module:												
Base Vol:	128	542	175	132	1132	76	48	335	130	315	433	153
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	128	542	175	132	1132	76	48	335	130	315	433	153
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	128	542	175	132	1132	76	48	335	130	315	433	153
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	128	542	175	132	1132	76	48	335	130	315	433	153
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	128	542	175	132	1132	76	48	335	130	315	433	153
OvlAdjVol:			21									21
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	3.02	0.98	1.00	2.81	0.19	1.00	1.00	1.00	2.00	1.00	1.00
Final Sat.:	1600	5138	1562	1600	4698	302	1600	1700	1600	3200	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.08	0.11	0.11	0.08	0.24	0.25	0.03	0.20	0.08	0.10	0.25	0.10
OvlAdjV/S:			0.01									0.01
Crit Moves:	****					****		****		****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #112 Tustin Ranch Road and Warner Avenue North

Cycle (sec):	100	Critical Vol./Cap.(X):	0.459
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	22	Level Of Service:	A

Street Name:	Tustin Ranch Road						Warner Avenue North					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	3 0 1	2	0	3 0 0	0	0	0 0 0	3	0	0 0 2
Volume Module:												
Base Vol:	0	396	383	412	1165	0	0	0	0	865	0	255
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	396	383	412	1165	0	0	0	0	865	0	255
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	396	383	412	1165	0	0	0	0	865	0	255
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	396	383	412	1165	0	0	0	0	865	0	255
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	396	383	412	1165	0	0	0	0	865	0	255
OvlAdjVol:			95									0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	0.00	3.00	1.00	2.00	3.00	0.00	0.00	0.00	0.00	3.00	0.00	2.00
Final Sat.:	0	5100	1600	3200	5100	0	0	0	0	4800	0	3200
Capacity Analysis Module:												
Vol/Sat:	0.00	0.08	0.24	0.13	0.23	0.00	0.00	0.00	0.00	0.18	0.00	0.08
OvlAdjV/S:			0.06									0.00
Crit Moves:	****			****					****			

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #113 Tustin Ranch Road and Walnut Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	1.313
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Street Name:	Tustin Ranch Road						Walnut Avenue					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Ovl			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0	3	0	1	2	0	3	0	1	2	0	1	1	0	2	0	2	0	1
Volume Module:																				
Base Vol:	103	630	314	1123	3139	418	856	1118	675	176	260	219								
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00								
Initial Bse:	103	630	314	1123	3139	418	856	1118	675	176	260	219								
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00								
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00								
PHF Volume:	103	630	314	1123	3139	418	856	1118	675	176	260	219								
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0								
Reduced Vol:	103	630	314	1123	3139	418	856	1118	675	176	260	219								
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00								
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00								
FinalVolume:	103	630	314	1123	3139	418	856	1118	675	176	260	219								
OvlAdjVol:			226			0						0								
Saturation Flow Module:																				
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600								
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00								
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	1.25	0.75	2.00	2.00	1.00								
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	2095	1205	3200	3400	1600								
Capacity Analysis Module:																				
Vol/Sat:	0.03	0.12	0.20	0.35	0.62	0.26	0.27	0.53	0.56	0.06	0.08	0.14								
OvlAdjV/S:			0.14			0.00						0.00								
Crit Moves:	****			****					****	****										

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #114 SR-55 SB Ramps and Irvine Boulevard

Cycle (sec):	100	Critical Vol./Cap.(X):	1.176
Loss Time (sec):	5	Average Delay (sec/veh):	55.4
Optimal Cycle:	180	Level Of Service:	E

Street Name:	SR-55 SB Ramps						Irvine Boulevard					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	0	1	0	1! 0	1	0	0	2	1	0	1	0	3	0	0
Volume Module:												
Base Vol:	0	0	0	64	0	70	0	719	1290	480	1201	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	64	0	70	0	719	1290	480	1201	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	64	0	70	0	719	1290	480	1201	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	64	0	70	0	719	1290	480	1201	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	64	0	70	0	719	1290	480	1201	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.88	1.00	0.79	1.00	0.82	0.82	0.95	0.91	1.00
Lanes:	0.00	0.00	0.00	1.45	0.00	1.55	0.00	2.00	1.00	1.00	3.00	0.00
Final Sat.:	0	0	0	2426	0	2321	0	3126	1563	1805	5187	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.03	0.00	0.03	0.00	0.23	0.83	0.27	0.23	0.00
Crit Moves:				****					****	****		
Green/Cycle:	0.00	0.00	0.00	0.02	0.00	0.04	0.00	0.70	0.70	0.23	0.93	0.00
Volume/Cap:	0.00	0.00	0.00	0.89	0.00	0.69	0.00	0.33	1.18	1.18	0.25	0.00
Delay/Veh:	0.0	0.0	0.0	91.2	0.0	56.8	0.0	5.8	100.6	140.8	0.4	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	91.2	0.0	56.8	0.0	5.8	100.6	140.8	0.4	0.0
LOS by Move:	A	A	A	F	A	E	A	A	F	F	A	A

```
HCM2k85thQ:     0    0     0     5    0     4     0    7    95    38    2     0
********************************************************************************
Note: Queue reported is the number of cars per lane.
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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #115 SR-55 NB Ramps and Irvine Boulevard

Cycle (sec):	100	Critical Vol./Cap.(X):	0.824
Loss Time (sec):	5	Average Delay (sec/veh):	20.3
Optimal Cycle:	60	Level Of Service:	C

Street Name:	SR-55 NB Ramps						Irvine Boulevard					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	1! 0	1	0	0	0	0	1	0	3	0	0	0	0	2	1	0
Volume Module:																		
Base Vol:	395	0	379	0	0	0	204	485	0	0	1133	788
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	395	0	379	0	0	0	204	485	0	0	1133	788
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	395	0	379	0	0	0	204	485	0	0	1133	788
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	395	0	379	0	0	0	204	485	0	0	1133	788
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	395	0	379	0	0	0	204	485	0	0	1133	788
Saturation Flow Module:																		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.75	1.00	0.74	1.00	1.00	1.00	0.95	0.91	1.00	1.00	0.85	0.85
Lanes:	1.51	0.00	1.49	0.00	0.00	0.00	1.00	3.00	0.00	0.00	2.00	1.00
Final Sat.:	2138	0	2097	0	0	0	1805	5187	0	0	3247	1624
Capacity Analysis Module:																		
Vol/Sat:	0.18	0.00	0.18	0.00	0.00	0.00	0.11	0.09	0.00	0.00	0.35	0.49
Crit Moves:	****						****					****
Green/Cycle:	0.22	0.00	0.45	0.00	0.00	0.00	0.14	0.73	0.00	0.00	0.59	0.59
Volume/Cap:	0.83	0.00	0.40	0.00	0.00	0.00	0.82	0.13	0.00	0.00	0.59	0.82
Delay/Veh:	43.0	0.0	18.8	0.0	0.0	0.0	61.5	4.2	0.0	0.0	13.3	18.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	43.0	0.0	18.8	0.0	0.0	0.0	61.5	4.2	0.0	0.0	13.3	18.9
LOS by Move:	D	A	B	A	A	A	E	A	A	A	B	B
HCM2k85thQ:	15	0	8	0	0	0	13	3	0	0	18	33

Note: Queue reported is the number of cars per lane.

APPENDIX D.6

ANALYSIS WORKSHEETS –
2045 WP (PM Peak Hour)

Santa Ana Circulation Element

Scenario Report

Scenario:	2045 WP PM Peak Hour
Command:	Default Command
Volume:	2045 PM
Geometry:	SACE
Impact Fee:	Default Impact Fee
Trip Generation:	Default Trip Generation
Trip Distribution:	Default Trip Distribution
Paths:	Default Path
Routes:	Default Route
Configuration:	Default Configuration

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Santa Ana Circulation Element

Impact Analysis Report
Level Of Service

Intersection	Base LOS	Base Del/ Veh	Base V/ C	Future LOS	Future Del/ Veh	Future V/ C	Change in V/C
# 1 Euclid St and 1st St	C	xxxxx	0.787	C	xxxxx	0.787	+ 0.000 V/C
# 2 Euclid St and McFadden Ave	C	xxxxx	0.793	C	xxxxx	0.793	+ 0.000 V/C
# 3 Euclid St and Edinger Ave	D	xxxxx	0.860	D	xxxxx	0.860	+ 0.000 V/C
# 4 Newhope St and Hazard Ave	A	xxxxx	0.516	A	xxxxx	0.516	+ 0.000 V/C
# 5 Newhope St and McFadden Ave	B	xxxxx	0.684	B	xxxxx	0.684	+ 0.000 V/C
# 6 Harbor Blvd and SR-22 WB Off-R	C	23.8	0.769	C	23.8	0.769	+ 0.000 D/V
# 7 Trask Avenue and SR-22 EB On-R	B	11.0	0.381	B	11.0	0.381	+ 0.000 D/V
# 8 Harbor Blvd and Westminster Av	D	xxxxx	0.842	D	xxxxx	0.842	+ 0.000 V/C
# 9 Harbor Blvd and 1st St	C	xxxxx	0.753	C	xxxxx	0.753	+ 0.000 V/C
# 10 Harbor Blvd and McFadden Ave	C	xxxxx	0.711	C	xxxxx	0.711	+ 0.000 V/C
# 11 Harbor Blvd and Edinger Ave	C	xxxxx	0.722	C	xxxxx	0.722	+ 0.000 V/C
# 12 Harbor Blvd and Warner Ave	F	xxxxx	1.539	F	xxxxx	1.539	+ 0.000 V/C
# 13 Harbor Blvd and Segerstrom Ave	E	xxxxx	0.934	E	xxxxx	0.934	+ 0.000 V/C
# 14 MacArthur Blvd and Hyland Ave	D	xxxxx	0.891	D	xxxxx	0.891	+ 0.000 V/C
# 15 MacArthur Blvd and Harbor Blvd	C	xxxxx	0.788	C	xxxxx	0.788	+ 0.000 V/C
# 16 Harbor Blvd and Sunflower Ave	C	xxxxx	0.711	C	xxxxx	0.711	+ 0.000 V/C
# 17 Harbor Blvd and I-405 NB Off-R	B	18.8	0.710	B	18.8	0.710	+ 0.000 D/V
# 18 Harbor Blvd and I-405 SB Off-R	B	18.1	0.673	B	18.1	0.673	+ 0.000 D/V
# 19 Fairview St and Civic Center D	C	xxxxx	0.707	C	xxxxx	0.707	+ 0.000 V/C
# 20 Fairview St and 1st St	D	xxxxx	0.894	D	xxxxx	0.894	+ 0.000 V/C
# 21 Fairview St and McFadden Ave	B	xxxxx	0.684	B	xxxxx	0.684	+ 0.000 V/C
# 22 Fairview St and Edinger Ave	E	xxxxx	0.920	E	xxxxx	0.920	+ 0.000 V/C
# 23 Fairview St and Warner Ave	D	xxxxx	0.805	D	xxxxx	0.805	+ 0.000 V/C
# 24 Fairview St and MacArthur Blvd	C	xxxxx	0.761	C	xxxxx	0.761	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Base Del/ Veh	Base V/ C	Future LOS	Future Del/ Veh	Future V/ C	Change in
# 25 Fairview Rd and Sunflower Ave	B	xxxxx	0.691	B	xxxxx	0.691	+ 0.000 V/C
# 26 Greenville St and Edinger Ave	B	xxxxx	0.647	B	xxxxx	0.647	+ 0.000 V/C
# 27 Greenville St and Segerstrom A	C	xxxxx	0.797	C	xxxxx	0.797	+ 0.000 V/C
# 28 Raitt St and McFadden Ave	B	xxxxx	0.676	B	xxxxx	0.676	+ 0.000 V/C
# 29 Raitt St and Edinger Ave	E	xxxxx	0.988	E	xxxxx	0.988	+ 0.000 V/C
# 30 Bear St and MacArthur Blvd	E	xxxxx	0.904	E	xxxxx	0.904	+ 0.000 V/C
# 31 Bristol St and 17th St	D	xxxxx	0.811	D	xxxxx	0.811	+ 0.000 V/C
# 32 Bristol St and Civic Center Dr	D	xxxxx	0.898	D	xxxxx	0.898	+ 0.000 V/C
# 33 Bristol St and Santa Ana Blvd	C	xxxxx	0.732	C	xxxxx	0.732	+ 0.000 V/C
# 34 Bristol St and 1st St	E	xxxxx	0.972	E	xxxxx	0.972	+ 0.000 V/C
# 35 Bristol St and McFadden Ave	D	xxxxx	0.890	D	xxxxx	0.890	+ 0.000 V/C
# 36 Bristol St and Warner Ave	D	xxxxx	0.871	D	xxxxx	0.871	+ 0.000 V/C
# 37 Bristol St and Segerstrom Ave	E	xxxxx	0.915	E	xxxxx	0.915	+ 0.000 V/C
# 38 Bristol St and Alton Ave	A	xxxxx	0.583	A	xxxxx	0.583	+ 0.000 V/C
# 39 Bristol St and MacArthur Blvd	C	xxxxx	0.796	C	xxxxx	0.796	+ 0.000 V/C
# 40 Bristol St and Sunflower Ave	C	xxxxx	0.744	C	xxxxx	0.744	+ 0.000 V/C
# 41 Bristol St and I-405 NB Ramps	C	29.0	0.854	C	29.0	0.854	+ 0.000 D/V
# 42 Bristol St and I-405 SB Ramps	B	18.2	0.631	B	18.2	0.631	+ 0.000 D/V
# 43 Flower St and Santa Ana Blvd	B	xxxxx	0.668	B	xxxxx	0.668	+ 0.000 V/C
# 44 Flower St and 1st St	F	xxxxx	1.215	F	xxxxx	1.215	+ 0.000 V/C
# 45 Flower St and McFadden Ave	E	xxxxx	0.968	E	xxxxx	0.968	+ 0.000 V/C
# 46 Flower St and Segerstrom Ave	E	xxxxx	0.916	E	xxxxx	0.916	+ 0.000 V/C
# 47 Flower St and MacArthur Blvd	E	xxxxx	0.908	E	xxxxx	0.908	+ 0.000 V/C
# 48 Main St and La Veta Ave	C	xxxxx	0.764	C	xxxxx	0.764	+ 0.000 V/C
# 49 Main St and Mainplace Dr / Mem	A	xxxxx	0.504	A	xxxxx	0.504	+ 0.000 V/C
# 50 Main St and 17th St	C	xxxxx	0.799	C	xxxxx	0.799	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Base Del/ Veh	Base V/ C	Future LOS	Future Del/ Veh	Future V/ C	Change in
# 51 Main St and Civic Center Dr	D	xxxxx	0.856	D	xxxxx	0.856	+ 0.000 V/C
# 52 Main St and Santa Ana Blvd	C	xxxxx	0.700	C	xxxxx	0.700	+ 0.000 V/C
# 53 Main St and 4th St	A	xxxxx	0.472	A	xxxxx	0.472	+ 0.000 V/C
# 54 Main St and 1st St	D	xxxxx	0.803	D	xxxxx	0.803	+ 0.000 V/C
# 55 Main St and McFadden Ave	E	xxxxx	0.935	E	xxxxx	0.935	+ 0.000 V/C
# 56 Main St and Edinger Ave	E	xxxxx	0.977	E	xxxxx	0.977	+ 0.000 V/C
# 57 Main St and MacArthur Blvd	C	xxxxx	0.781	C	xxxxx	0.781	+ 0.000 V/C
# 58 Penn Wy and 17th St	C	20.3	0.790	C	20.3	0.790	+ 0.000 D/V
# 59 I-5 NB Off Ramps/17th Street	C	20.3	0.630	C	20.3	0.630	+ 0.000 D/V
# 60 Penn Wy and I-5 SB Ramps	C	22.8	0.433	C	22.8	0.433	+ 0.000 D/V
# 61 Santiago St and Civic Center D	D	27.6	0.896	D	27.6	0.896	+ 0.000 V/C
# 62 Santiago St and Santa Ana Blvd	F	xxxxx	1.411	F	xxxxx	1.411	+ 0.000 V/C
# 63 Standard Ave and 4th St	F	xxxxx	1.341	F	xxxxx	1.341	+ 0.000 V/C
# 64 Standard Ave and 1st St	F	xxxxx	1.484	F	xxxxx	1.484	+ 0.000 V/C
# 65 Standard Ave and Mcfadden Ave	C	xxxxx	0.769	C	xxxxx	0.769	+ 0.000 V/C
# 66 Halladay St and Warner Ave	D	xxxxx	0.824	D	xxxxx	0.824	+ 0.000 V/C
# 67 Halladay St and Dyer Rd	E	xxxxx	0.945	E	xxxxx	0.945	+ 0.000 V/C
# 68 SR-55 SB Ramps and MacArthur B	B	18.4	0.602	B	18.4	0.602	+ 0.000 D/V
# 69 SR-55 NB Ramps and MacArthur B	B	16.2	0.511	B	16.2	0.511	+ 0.000 D/V
# 70 SR-55 SB Ramps and Dyer Rd	C	26.5	0.679	C	26.5	0.679	+ 0.000 D/V
# 71 Glassell St and La Veta Ave	C	xxxxx	0.766	C	xxxxx	0.766	+ 0.000 V/C
# 72 Glassell St and SR-22 WB Ramps	C	33.4	0.943	C	33.4	0.943	+ 0.000 D/V
# 73 Grand Ave / Glassell St and SR	C	31.8	0.927	C	31.8	0.927	+ 0.000 D/V
# 74 Grand Ave and Fairhaven Ave	B	xxxxx	0.665	B	xxxxx	0.665	+ 0.000 V/C
# 75 Grand Ave and Santa Clara Ave	C	xxxxx	0.731	C	xxxxx	0.731	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Base Del/ Veh	Base V/ C	Future LOS	Future Del/ Veh	Future V/ C	Change in
# 76 Grand Ave and 17th St	D	xxxxx	0.854	D	xxxxx	0.854	+ 0.000 V/C
# 77 Grand Ave and I-5 NB Ramps	B	11.3	0.764	B	11.3	0.764	+ 0.000 D/V
# 78 Grand Ave and Santa Ana Blvd	C	28.5	0.827	C	28.5	0.827	+ 0.000 D/V
# 79 Grand Ave and 1st St	C	xxxxx	0.718	C	xxxxx	0.718	+ 0.000 V/C
# 80 Grand Ave and Chestnut Ave	C	xxxxx	0.783	C	xxxxx	0.783	+ 0.000 V/C
# 81 Grand Ave and McFadden Ave	D	xxxxx	0.813	D	xxxxx	0.813	+ 0.000 V/C
# 82 Grand Ave and Edinger Ave	F	xxxxx	1.079	F	xxxxx	1.079	+ 0.000 V/C
# 83 Grand Ave and Warner Ave	D	xxxxx	0.899	D	xxxxx	0.899	+ 0.000 V/C
# 84 SR-55 NB Ramps and Dyer Rd	A	6.0	0.349	A	6.0	0.349	+ 0.000 D/V
# 85 Cambridge St and La Veta Ave	D	26.1	0.938	D	26.1	0.938	+ 0.000 V/C
# 86 Cambridge St and Fairhaven Ave	A	xxxxx	0.531	A	xxxxx	0.531	+ 0.000 V/C
# 87 Mabury St and 1st Street	C	27.2	0.752	C	27.2	0.752	+ 0.000 D/V
# 88 Tustin St and La Veta Ave	A	xxxxx	0.344	A	xxxxx	0.344	+ 0.000 V/C
# 89 Tustin St and SR-22 WB On-Ramp	A	7.7	0.321	A	7.7	0.321	+ 0.000 D/V
# 90 Tustin St and SR-22 EB Off-Ram	C	23.3	0.625	C	23.3	0.625	+ 0.000 D/V
# 91 Tustin Ave and Fairhaven Ave	C	xxxxx	0.757	C	xxxxx	0.757	+ 0.000 V/C
# 92 Tustin Ave and Santa Clara Ave	C	xxxxx	0.779	C	xxxxx	0.779	+ 0.000 V/C
# 93 Tustin Ave and 17th St	C	xxxxx	0.741	C	xxxxx	0.741	+ 0.000 V/C
# 94 Tustin Ave and 4th St	C	xxxxx	0.784	C	xxxxx	0.784	+ 0.000 V/C
# 95 SR-55 SB Ramps / Auto Mall Dr	C	25.4	0.687	C	25.4	0.687	+ 0.000 D/V
# 96 SR-55 NB Ramps / Del Amo Ave a	D	46.2	0.951	D	46.2	0.951	+ 0.000 D/V
# 97 Red Hill Ave and Edinger Ave	D	xxxxx	0.865	D	xxxxx	0.865	+ 0.000 V/C
# 98 Red Hill Ave and Warner Ave	E	xxxxx	0.952	E	xxxxx	0.952	+ 0.000 V/C
# 99 Red Hill Ave and Dyer Rd / bar	A	xxxxx	0.591	A	xxxxx	0.591	+ 0.000 V/C
#100 Red Hill Ave and Alton Pkwy	F	xxxxx	1.494	F	xxxxx	1.494	+ 0.000 V/C
#101 Red Hill Ave and MacArthur Blv	C	xxxxx	0.722	C	xxxxx	0.722	+ 0.000 V/C

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Santa Ana Circulation Element

Intersection	Base LOS	Base Del/ Veh	Base V/ C	Future LOS	Future Del/ Veh	Future V/ C	Change in
#102 Red Hill Ave and Main St	D	xxxxx	0.836	D	xxxxx	0.836	+ 0.000 V/C
#103 I-5 SB Ramps and Santa Ana Blv	B	19.9	0.444	B	19.9	0.444	+ 0.000 D/V
#104 Tustin Ranch Rd and Warner Ave	B	xxxxx	0.671	B	xxxxx	0.671	+ 0.000 V/C
#105 Von Karman Ave and Barranca Pk	F	xxxxx	1.284	F	xxxxx	1.284	+ 0.000 V/C
#106 Red Hill Avenue and El Camino	B	xxxxx	0.696	B	xxxxx	0.696	+ 0.000 V/C
#107 Red Hill Avenue and I-5 NB Ram	C	23.6	0.623	C	23.6	0.623	+ 0.000 D/V
#108 Red Hill Avenue and I-5 SB Ram	C	21.8	0.713	C	21.8	0.713	+ 0.000 D/V
#109 Red Hill Avenue and Nisson Roa	C	xxxxx	0.715	C	xxxxx	0.715	+ 0.000 V/C
#110 Red Hill Avenue and Walnut Ave	C	xxxxx	0.749	C	xxxxx	0.749	+ 0.000 V/C
#111 Red Hill Avenue and Valencia A	C	xxxxx	0.753	C	xxxxx	0.753	+ 0.000 V/C
#112 Tustin Ranch Road and Warner A	E	xxxxx	0.924	E	xxxxx	0.924	+ 0.000 V/C
#113 Tustin Ranch Road and Walnut A	F	xxxxx	1.772	F	xxxxx	1.772	+ 0.000 V/C
#114 SR-55 SB Ramps and Irvine Boul	C	26.5	0.929	C	26.5	0.929	+ 0.000 D/V
#115 SR-55 NB Ramps and Irvine Boul	C	22.3	0.841	C	22.3	0.841	+ 0.000 D/V

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #1 Euclid St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.787
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	49	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			WideBypass			WideBypass		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	165	1532	123	137	781	148	201	641	100	168	702	168
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	165	1532	123	137	781	148	201	641	100	168	702	168
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	165	1532	123	137	781	148	201	641	100	168	702	168
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	165	1532	123	137	781	148	201	641	100	168	702	168
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	165	1532	123	137	781	148	201	641	100	168	702	168
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.78	0.22	1.00	2.52	0.48	1.00	2.60	0.40	1.00	2.42	0.58
Final Sat.:	1600	4643	357	1600	4235	765	1600	4352	648	1600	4073	927
Capacity Analysis Module:												
Vol/Sat:	0.10	0.33	0.34	0.09	0.18	0.19	0.13	0.15	0.15	0.11	0.17	0.18
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #2 Euclid St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.793
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	51	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	248	2020	130	87	1070	130	134	307	136	86	359	166
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	248	2020	130	87	1070	130	134	307	136	86	359	166
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	248	2020	130	87	1070	130	134	307	136	86	359	166
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	248	2020	130	87	1070	130	134	307	136	86	359	166
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	248	2020	130	87	1070	130	134	307	136	86	359	166
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.82	0.18	1.00	2.68	0.32	1.00	1.39	0.61	1.00	1.37	0.63
Final Sat.:	1600	4710	290	1600	4480	520	1600	2318	982	1600	2288	1012
Capacity Analysis Module:												
Vol/Sat:	0.16	0.43	0.45	0.05	0.24	0.25	0.08	0.13	0.14	0.05	0.16	0.16
Crit Moves:			****	****			****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #3 Euclid St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.860
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	69	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	1 0	2 0	1	1 0	2 0	1
Volume Module:												
Base Vol:	551	1648	117	136	897	228	121	467	140	119	1103	179
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	551	1648	117	136	897	228	121	467	140	119	1103	179
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	551	1648	117	136	897	228	121	467	140	119	1103	179
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	551	1648	117	136	897	228	121	467	140	119	1103	179
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	551	1648	117	136	897	228	121	467	140	119	1103	179
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.80	0.20	2.00	2.39	0.61	1.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	3200	4682	318	3200	4027	973	1600	3400	1600	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.17	0.35	0.37	0.04	0.22	0.23	0.08	0.14	0.09	0.07	0.32	0.11
Crit Moves:			****	****			****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #4 Newhope St and Hazard Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.516
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	24	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Prot+Permit			Prot+Permit			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	0 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	102	1129	44	45	954	114	33	66	40	8	51	15
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	102	1129	44	45	954	114	33	66	40	8	51	15
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	102	1129	44	45	954	114	33	66	40	8	51	15
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	102	1129	44	45	954	114	33	66	40	8	51	15
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	102	1129	44	45	954	114	33	66	40	8	51	15
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.92	0.08	1.00	1.79	0.21	1.00	0.62	0.38	1.00	0.77	0.23
Final Sat.:	1600	3180	120	1600	2958	342	1600	996	604	1600	1236	364
Capacity Analysis Module:												
Vol/Sat:	0.06	0.36	0.37	0.03	0.32	0.33	0.02	0.07	0.07	0.01	0.04	0.04
Crit Moves:			****	****				****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #5 Newhope St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.684
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	36	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	188	1009	177	121	539	104	45	273	40	72	398	113
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	188	1009	177	121	539	104	45	273	40	72	398	113
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	188	1009	177	121	539	104	45	273	40	72	398	113
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	188	1009	177	121	539	104	45	273	40	72	398	113
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	188	1009	177	121	539	104	45	273	40	72	398	113
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	1.70	0.30	1.00	1.68	0.32	1.00	1.74	0.26	1.00	1.56	0.44
Final Sat.:	1600	2822	478	1600	2782	518	1600	2891	409	1600	2592	708
Capacity Analysis Module:												
Vol/Sat:	0.12	0.36	0.37	0.08	0.19	0.20	0.03	0.09	0.10	0.05	0.15	0.16
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #6 Harbor Blvd and SR-22 WB Off-Ramp / Banner Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.769
Loss Time (sec):	5	Average Delay (sec/veh):	23.8
Optimal Cycle:	48	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0 3	0 0	0	0 2	1 0	0	0 1!	0 0	1	1 0	0 1
Volume Module:												
Base Vol:	103	2324	0	0	1823	59	102	0	86	558	75	160
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	103	2324	0	0	1823	59	102	0	86	558	75	160
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	103	2324	0	0	1823	59	102	0	86	558	75	160
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	103	2324	0	0	1823	59	102	0	86	558	75	160
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	103	2324	0	0	1823	59	102	0	86	558	75	160
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	1.00	1.00	0.91	0.91	0.91	1.00	0.91	0.96	0.96	0.85
Lanes:	1.00	3.00	0.00	0.00	2.91	0.09	0.54	0.00	0.46	1.76	0.24	1.00
Final Sat.:	1805	5187	0	0	4999	162	942	0	794	3209	431	1615
Capacity Analysis Module:												
Vol/Sat:	0.06	0.45	0.00	0.00	0.36	0.36	0.11	0.00	0.11	0.17	0.17	0.10
Crit Moves:		****		****			****				****	
Green/Cycle:	0.08	0.58	0.00	0.00	0.50	0.50	0.14	0.00	0.14	0.23	0.23	0.23
Volume/Cap:	0.72	0.77	0.00	0.00	0.72	0.72	0.77	0.00	0.77	0.77	0.77	0.44
Delay/Veh:	61.7	17.0	0.0	0.0	20.4	20.4	55.1	0.0	55.1	40.7	40.7	34.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	61.7	17.0	0.0	0.0	20.4	20.4	55.1	0.0	55.1	40.7	40.7	34.1
LOS by Move:	E	B	A	A	C	C	E	A	E	D	D	C
HCM2k85thQ:	7	30	0	0	26	26	11	0	11	17	17	7

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #7 Trask Avenue and SR-22 EB On-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.381
Loss Time (sec):	5	Average Delay (sec/veh):	11.0
Optimal Cycle:	20	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	0 0	0	0 0	0 0	0	2 0	1 0	0	0 0	1 1	0
Volume Module:												
Base Vol:	0	0	0	0	0	0	785	588	0	0	449	42
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	0	0	0	785	588	0	0	449	42
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	0	0	0	785	588	0	0	449	42
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	0	0	0	785	588	0	0	449	42
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	0	0	0	785	588	0	0	449	42
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	0.92	1.00	1.00	1.00	0.94	0.94
Lanes:	0.00	0.00	0.00	0.00	0.00	0.00	2.00	1.00	0.00	0.00	1.83	0.17
Final Sat.:	0	0	0	0	0	0	3502	1900	0	0	3258	305
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.00	0.00	0.00	0.22	0.31	0.00	0.00	0.14	0.14
Crit Moves:							****				****	
Green/Cycle:	0.00	0.00	0.00	0.00	0.00	0.00	0.59	0.95	0.00	0.00	0.36	0.36
Volume/Cap:	0.00	0.00	0.00	0.00	0.00	0.00	0.38	0.33	0.00	0.00	0.38	0.38
Delay/Veh:	0.0	0.0	0.0	0.0	0.0	0.0	11.0	0.3	0.0	0.0	23.8	23.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	0.0	0.0	0.0	11.0	0.3	0.0	0.0	23.8	23.8
LOS by Move:	A	A	A	A	A	A	B	A	A	A	C	C
HCM2k85thQ:	0	0	0	0	0	0	10	3	0	0	9	9

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #8 Harbor Blvd and Westminster Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.842
Loss Time (sec):	0	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	145	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	2 1	0	1 0	3 0	1
Volume Module:												
Base Vol:	222	1106	262	254	1752	202	227	808	39	294	1018	231
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	222	1106	262	254	1752	202	227	808	39	294	1018	231
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	222	1106	262	254	1752	202	227	808	39	294	1018	231
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	222	1106	262	254	1752	202	227	808	39	294	1018	231
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	222	1106	262	254	1752	202	227	808	39	294	1018	231
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	2.86	0.14	1.00	3.00	1.00
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	4779	221	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.14	0.22	0.16	0.16	0.34	0.13	0.14	0.17	0.18	0.18	0.20	0.14
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #9 Harbor Blvd and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.753
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	44	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	280	1446	160	240	1053	142	192	784	148	185	865	141
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	280	1446	160	240	1053	142	192	784	148	185	865	141
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	280	1446	160	240	1053	142	192	784	148	185	865	141
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	280	1446	160	240	1053	142	192	784	148	185	865	141
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	280	1446	160	240	1053	142	192	784	148	185	865	141
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	2.00	2.52	0.48	2.00	2.58	0.42
Final Sat.:	1600	5100	1600	1600	5100	1600	3200	4238	762	3200	4327	673
Capacity Analysis Module:												
Vol/Sat:	0.17	0.28	0.10	0.15	0.21	0.09	0.06	0.19	0.19	0.06	0.20	0.21
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #10 Harbor Blvd and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.711
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	38	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	1 0	1 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	182	1866	136	261	1114	101	147	358	45	63	151	43
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	182	1866	136	261	1114	101	147	358	45	63	151	43
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	182	1866	136	261	1114	101	147	358	45	63	151	43
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	182	1866	136	261	1114	101	147	358	45	63	151	43
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	182	1866	136	261	1114	101	147	358	45	63	151	43
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.00	1.00
Lanes:	2.00	3.00	1.00	2.00	2.75	0.25	1.00	1.78	0.22	1.00	0.78	0.22
Final Sat.:	3200	5100	1600	3200	4601	399	1600	2943	357	1600	1245	355
Capacity Analysis Module:												
Vol/Sat:	0.06	0.37	0.09	0.08	0.24	0.25	0.09	0.12	0.13	0.04	0.12	0.12
Crit Moves:		****			****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #11 Harbor Blvd and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.722
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	40	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	2 0	2 1	0	2 0	3 0	1
Volume Module:												
Base Vol:	262	1645	199	283	822	75	109	460	81	188	1089	363
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	262	1645	199	283	822	75	109	460	81	188	1089	363
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	262	1645	199	283	822	75	109	460	81	188	1089	363
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	262	1645	199	283	822	75	109	460	81	188	1089	363
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	262	1645	199	283	822	75	109	460	81	188	1089	363
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	2.55	0.45	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	4281	719	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.08	0.32	0.12	0.09	0.16	0.05	0.03	0.11	0.11	0.06	0.21	0.23
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #12 Harbor Blvd and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	1.539
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	768	3685	277	250	1389	166	198	1399	320	253	2626	838
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	768	3685	277	250	1389	166	198	1399	320	253	2626	838
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	768	3685	277	250	1389	166	198	1399	320	253	2626	838
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	768	3685	277	250	1389	166	198	1399	320	253	2626	838
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	768	3685	277	250	1389	166	198	1399	320	253	2626	838
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.79	0.21	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	4664	336	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.24	0.79	0.83	0.08	0.27	0.10	0.06	0.27	0.20	0.08	0.51	0.52
Crit Moves:		****	****				****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #13 Harbor Blvd and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.934
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	113	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 0	2	0 1
Volume Module:												
Base Vol:	275	1732	55	86	973	86	118	585	192	121	1308	475
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	275	1732	55	86	973	86	118	585	192	121	1308	475
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	275	1732	55	86	973	86	118	585	192	121	1308	475
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	275	1732	55	86	973	86	118	585	192	121	1308	475
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	275	1732	55	86	973	86	118	585	192	121	1308	475
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	2.91	0.09	1.00	2.76	0.24	1.00	1.51	0.49	1.00	2.00	1.00
Final Sat.:	3200	4852	148	1600	4610	390	1600	2509	791	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.09	0.36	0.37	0.05	0.21	0.22	0.07	0.23	0.24	0.08	0.38	0.30
Crit Moves:			****	****			****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #14 MacArthur Blvd and Hyland Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.891
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	82	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	0 1	0	1 0	1 0	1	1 0	3 0	1	1 0	3 0	1
Volume Module:												
Base Vol:	1139	7	66	12	4	14	35	869	156	25	2320	12
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	1139	7	66	12	4	14	35	869	156	25	2320	12
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	1139	7	66	12	4	14	35	869	156	25	2320	12
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	1139	7	66	12	4	14	35	869	156	25	2320	12
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	1139	7	66	12	4	14	35	869	156	25	2320	12
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	0.10	0.90	1.00	1.00	1.00	1.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	153	1447	1600	1700	1600	1600	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.36	0.05	0.05	0.01	0.00	0.01	0.02	0.17	0.10	0.02	0.45	0.01
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #15 MacArthur Blvd and Harbor Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.788
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	50	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	1 0	3 0	1	1 0	3 0	1
Volume Module:												
Base Vol:	527	1362	79	206	1039	170	143	501	185	85	1429	269
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	527	1362	79	206	1039	170	143	501	185	85	1429	269
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	527	1362	79	206	1039	170	143	501	185	85	1429	269
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	527	1362	79	206	1039	170	143	501	185	85	1429	269
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	527	1362	79	206	1039	170	143	501	185	85	1429	269
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	1.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	1600	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.16	0.27	0.05	0.06	0.20	0.11	0.09	0.10	0.12	0.05	0.28	0.17
Crit Moves:	****				****		****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #16 Harbor Blvd and Sunflower Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.711
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	38	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	1 0	1 1	0	1 1	0 1	0
Volume Module:												
Base Vol:	156	1552	230	118	1464	44	85	230	239	282	592	221
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	156	1552	230	118	1464	44	85	230	239	282	592	221
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	156	1552	230	118	1464	44	85	230	239	282	592	221
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	156	1552	230	118	1464	44	85	230	239	282	592	221
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	156	1552	230	118	1464	44	85	230	239	282	592	221
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	1.00	1.00	1.00	1.00	1.39	0.61
Final Sat.:	3200	5100	1600	3200	5100	1600	1600	1700	1600	1600	2322	974
Capacity Analysis Module:												
Vol/Sat:	0.05	0.30	0.14	0.04	0.29	0.03	0.05	0.14	0.15	0.18	0.25	0.23
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #17 Harbor Blvd and I-405 NB Off-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.710
Loss Time (sec):	5	Average Delay (sec/veh):	18.8
Optimal Cycle:	40	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	4	0 0	0 0	4	0 0	0 0	0	0 0	1 0	1! 0	1
Volume Module:												
Base Vol:	0	1488	0	0	2570	0	0	0	0	513	0	828
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1488	0	0	2570	0	0	0	0	513	0	828
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1488	0	0	2570	0	0	0	0	513	0	828
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1488	0	0	2570	0	0	0	0	513	0	828
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1488	0	0	2570	0	0	0	0	513	0	828
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	1.00	1.00	0.91	1.00	1.00	1.00	1.00	0.89	1.00	0.89
Lanes:	0.00	4.00	0.00	0.00	4.00	0.00	0.00	0.00	0.00	1.38	0.00	1.62
Final Sat.:	0	6916	0	0	6916	0	0	0	0	2337	0	2734
Capacity Analysis Module:												
Vol/Sat:	0.00	0.22	0.00	0.00	0.37	0.00	0.00	0.00	0.00	0.22	0.00	0.30
Crit Moves:	****				****							****
Green/Cycle:	0.00	0.52	0.00	0.00	0.52	0.00	0.00	0.00	0.00	0.43	0.00	0.43
Volume/Cap:	0.00	0.41	0.00	0.00	0.71	0.00	0.00	0.00	0.00	0.51	0.00	0.71
Delay/Veh:	0.0	14.5	0.0	0.0	18.7	0.0	0.0	0.0	0.0	21.2	0.0	24.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	14.5	0.0	0.0	18.7	0.0	0.0	0.0	0.0	21.2	0.0	24.9
LOS by Move:	A	B	A	A	B	A	A	A	A	C	A	C
HCM2k85thQ:	0	12	0	0	25	0	0	0	0	13	0	21

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #18 Harbor Blvd and I-405 SB Off-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.673
Loss Time (sec):	5	Average Delay (sec/veh):	18.1
Optimal Cycle:	36	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Ignore			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 3	0 1	0	0 4	0 0	1	0 1!	0 1	0	0 0	0 0
Volume Module:												
Base Vol:	0	1881	0	0	2254	0	181	0	833	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1881	0	0	2254	0	181	0	833	0	0	0
User Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1881	0	0	2254	0	181	0	833	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1881	0	0	2254	0	181	0	833	0	0	0
PCE Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1881	0	0	2254	0	181	0	833	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	1.00	1.00	0.91	1.00	0.87	1.00	0.87	1.00	1.00	1.00
Lanes:	0.00	3.00	1.00	0.00	4.00	0.00	1.18	0.00	1.82	0.00	0.00	0.00
Final Sat.:	0	5187	1900	0	6916	0	1946	0	3008	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.36	0.00	0.00	0.33	0.00	0.09	0.00	0.28	0.00	0.00	0.00
Crit Moves:		****			****				****			
Green/Cycle:	0.00	0.54	0.00	0.00	0.54	0.00	0.41	0.00	0.41	0.00	0.00	0.00
Volume/Cap:	0.00	0.67	0.00	0.00	0.61	0.00	0.23	0.00	0.67	0.00	0.00	0.00
Delay/Veh:	0.0	17.4	0.0	0.0	16.1	0.0	19.1	0.0	25.2	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	17.4	0.0	0.0	16.1	0.0	19.1	0.0	25.2	0.0	0.0	0.0
LOS by Move:	A	B	A	A	B	A	B	A	C	A	A	A
HCM2k85thQ:	0	23	0	0	20	0	5	0	18	0	0	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #19 Fairview St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.707
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	38	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	0 1	0	1 0	1 1	0	0 1
Volume Module:												
Base Vol:	6	1777	499	205	1903	0	2	6	4	164	0	61
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	6	1777	499	205	1903	0	2	6	4	164	0	61
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	6	1777	499	205	1903	0	2	6	4	164	0	61
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	6	1777	499	205	1903	0	2	6	4	164	0	61
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	6	1777	499	205	1903	0	2	6	4	164	0	61
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.00	1.00	1.00	1.06	1.00
Lanes:	1.00	2.34	0.66	1.00	3.00	0.00	0.33	1.00	0.67	2.00	0.00	1.00
Final Sat.:	1600	3948	1052	1600	5000	0	533	1600	1067	3200	0	1600
Capacity Analysis Module:												
Vol/Sat:	0.00	0.45	0.47	0.13	0.38	0.00	0.00	0.00	0.00	0.05	0.00	0.04
Crit Moves:			****	****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #20 Fairview St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.894
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	83	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	2 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	182	1339	121	236	1468	291	215	870	151	126	952	147
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	182	1339	121	236	1468	291	215	870	151	126	952	147
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	182	1339	121	236	1468	291	215	870	151	126	952	147
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	182	1339	121	236	1468	291	215	870	151	126	952	147
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	182	1339	121	236	1468	291	215	870	151	126	952	147
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	3.00	1.00	1.00	2.50	0.50	1.00	2.56	0.44	1.00	2.60	0.40
Final Sat.:	1600	5100	1600	1600	4206	794	1600	4290	710	1600	4358	642
Capacity Analysis Module:												
Vol/Sat:	0.11	0.26	0.08	0.15	0.35	0.37	0.13	0.20	0.21	0.08	0.22	0.23
Crit Moves:	****					****	****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #21 Fairview St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.684
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	35	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	0 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	174	1596	139	195	1577	246	79	214	53	51	177	43
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	174	1596	139	195	1577	246	79	214	53	51	177	43
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	174	1596	139	195	1577	246	79	214	53	51	177	43
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	174	1596	139	195	1577	246	79	214	53	51	177	43
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	174	1596	139	195	1577	246	79	214	53	51	177	43
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	0.80	0.20	1.00	0.80	0.20
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	1282	318	1600	1287	313
Capacity Analysis Module:												
Vol/Sat:	0.11	0.31	0.09	0.12	0.31	0.15	0.05	0.17	0.17	0.03	0.14	0.14
Crit Moves:		****			****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #22 Fairview St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.920
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	100	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	190	1810	111	198	616	151	205	628	168	639	1353	297
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	190	1810	111	198	616	151	205	628	168	639	1353	297
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	190	1810	111	198	616	151	205	628	168	639	1353	297
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	190	1810	111	198	616	151	205	628	168	639	1353	297
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	190	1810	111	198	616	151	205	628	168	639	1353	297
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	2.00	2.83	0.17	2.00	2.41	0.59	2.00	2.37	0.63	2.00	2.46	0.54
Final Sat.:	3200	4723	277	3200	4055	945	3200	3987	1013	3200	4136	864
Capacity Analysis Module:												
Vol/Sat:	0.06	0.38	0.40	0.06	0.15	0.16	0.06	0.16	0.17	0.20	0.33	0.34
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #23 Fairview St and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.805
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	53	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	211	1679	156	179	906	92	163	792	123	178	1049	227
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	211	1679	156	179	906	92	163	792	123	178	1049	227
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	211	1679	156	179	906	92	163	792	123	178	1049	227
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	211	1679	156	179	906	92	163	792	123	178	1049	227
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	211	1679	156	179	906	92	163	792	123	178	1049	227
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	2.00	2.74	0.26	2.00	2.72	0.28	2.00	2.60	0.40	2.00	2.47	0.53
Final Sat.:	3200	4592	408	3200	4558	442	3200	4355	645	3200	4146	854
Capacity Analysis Module:												
Vol/Sat:	0.07	0.37	0.38	0.06	0.20	0.21	0.05	0.18	0.19	0.06	0.25	0.27
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #24 Fairview St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.761
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	45	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	228	1568	85	162	902	83	253	661	223	159	1210	212
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	228	1568	85	162	902	83	253	661	223	159	1210	212
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	228	1568	85	162	902	83	253	661	223	159	1210	212
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	228	1568	85	162	902	83	253	661	223	159	1210	212
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	228	1568	85	162	902	83	253	661	223	159	1210	212
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.85	0.15	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	4753	247	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.07	0.33	0.34	0.05	0.18	0.05	0.08	0.13	0.14	0.05	0.24	0.13
Crit Moves:			****	****			****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #25 Fairview Rd and Sunflower Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.691
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	36	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	2 0	1 1	0	2 0	2 0	1
Volume Module:												
Base Vol:	190	1705	371	138	1145	79	245	520	115	208	522	136
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	190	1705	371	138	1145	79	245	520	115	208	522	136
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	190	1705	371	138	1145	79	245	520	115	208	522	136
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	190	1705	371	138	1145	79	245	520	115	208	522	136
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	190	1705	371	138	1145	79	245	520	115	208	522	136
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	2.81	0.19	2.00	1.64	0.36	2.00	2.00	1.00
Final Sat.:	3200	5100	1600	3200	4690	310	3200	2720	580	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.06	0.33	0.23	0.04	0.24	0.26	0.08	0.19	0.20	0.07	0.15	0.09
Crit Moves:		****			****				****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #26 Greenville St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.647
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	32	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	0 1	0 0	0	0 0	0 0	2	1 0	1 0	3	0 0
Volume Module:												
Base Vol:	327	0	218	0	0	0	0	1178	240	155	1840	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	327	0	218	0	0	0	0	1178	240	155	1840	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	327	0	218	0	0	0	0	1178	240	155	1840	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	327	0	218	0	0	0	0	1178	240	155	1840	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	327	0	218	0	0	0	0	1178	240	155	1840	0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.00	2.49	0.51	1.00	3.00	0.00
Final Sat.:	1600	0	1600	0	0	0	0	4188	812	1600	5100	0
Capacity Analysis Module:												
Vol/Sat:	0.20	0.00	0.14	0.00	0.00	0.00	0.00	0.28	0.30	0.10	0.36	0.00
Crit Moves:	****								****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #27 Greenville St and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.797
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	51	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0 1	0	1 0	0 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	50	414	31	44	143	44	138	1049	60	39	1490	213
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	50	414	31	44	143	44	138	1049	60	39	1490	213
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	50	414	31	44	143	44	138	1049	60	39	1490	213
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	50	414	31	44	143	44	138	1049	60	39	1490	213
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	50	414	31	44	143	44	138	1049	60	39	1490	213
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	0.93	0.07	1.00	0.76	0.24	1.00	2.84	0.16	1.00	2.62	0.38
Final Sat.:	1600	1489	111	1600	1224	376	1600	4740	260	1600	4400	600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.28	0.28	0.03	0.12	0.12	0.09	0.22	0.23	0.02	0.34	0.35
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #28 Raitt St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.676
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	35	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	1 0	1 0	0	1 0	1 0	0	1 0	1 0	0	1 0
Volume Module:												
Base Vol:	34	482	75	24	252	40	59	347	30	38	318	44
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	34	482	75	24	252	40	59	347	30	38	318	44
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	34	482	75	24	252	40	59	347	30	38	318	44
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	34	482	75	24	252	40	59	347	30	38	318	44
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	34	482	75	24	252	40	59	347	30	38	318	44
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	0.87	0.13	1.00	0.86	0.14	1.00	0.92	0.08	1.00	0.88	0.12
Final Sat.:	1600	1385	215	1600	1381	219	1600	1473	127	1600	1406	194
Capacity Analysis Module:												
Vol/Sat:	0.02	0.35	0.35	0.02	0.18	0.18	0.04	0.24	0.24	0.02	0.23	0.23
Crit Moves:		****			****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #29 Raitt St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.988
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	1 0	1 0	0	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	84	361	101	61	209	52	177	1226	175	195	2203	197
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	84	361	101	61	209	52	177	1226	175	195	2203	197
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	84	361	101	61	209	52	177	1226	175	195	2203	197
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	84	361	101	61	209	52	177	1226	175	195	2203	197
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	84	361	101	61	209	52	177	1226	175	195	2203	197
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	0.78	0.22	1.00	0.80	0.20	1.00	2.63	0.37	1.00	2.75	0.25
Final Sat.:	1600	1250	350	1600	1281	319	1600	4400	600	1600	4606	394
Capacity Analysis Module:												
Vol/Sat:	0.05	0.29	0.29	0.04	0.16	0.16	0.11	0.28	0.29	0.12	0.48	0.50
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #30 Bear St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.904
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	89	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	1	1 0	2 0	1	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	262	1258	328	115	278	84	90	847	88	82	1475	235
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	262	1258	328	115	278	84	90	847	88	82	1475	235
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	262	1258	328	115	278	84	90	847	88	82	1475	235
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	262	1258	328	115	278	84	90	847	88	82	1475	235
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	262	1258	328	115	278	84	90	847	88	82	1475	235
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.72	0.28	1.00	2.59	0.41
Final Sat.:	1600	3400	1600	1600	3400	1600	1600	4548	452	1600	4340	660
Capacity Analysis Module:												
Vol/Sat:	0.16	0.37	0.21	0.07	0.08	0.05	0.06	0.19	0.19	0.05	0.34	0.36
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #31 Bristol St and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.811
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	54	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	2 0	2 1	0	2 0	3 0	1
Volume Module:												
Base Vol:	370	1801	300	347	1847	314	243	543	98	280	1055	357
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	370	1801	300	347	1847	314	243	543	98	280	1055	357
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	370	1801	300	347	1847	314	243	543	98	280	1055	357
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	370	1801	300	347	1847	314	243	543	98	280	1055	357
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	370	1801	300	347	1847	314	243	543	98	280	1055	357
OvlAdjVol:												183
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	2.54	0.46	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	4266	734	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.12	0.35	0.19	0.11	0.36	0.20	0.08	0.13	0.13	0.09	0.21	0.22
OvlAdjV/S:												0.11
Crit Moves:	****				****		****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #32 Bristol St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.898
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	85	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	0	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	214	1494	78	173	1589	163	130	366	43	149	635	72
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	214	1494	78	173	1589	163	130	366	43	149	635	72
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	214	1494	78	173	1589	163	130	366	43	149	635	72
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	214	1494	78	173	1589	163	130	366	43	149	635	72
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	214	1494	78	173	1589	163	130	366	43	149	635	72
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.00	1.00	1.00	1.03	1.00
Lanes:	1.00	2.85	0.15	1.00	2.72	0.28	1.00	0.89	0.11	1.00	1.80	0.20
Final Sat.:	1600	4762	238	1600	4553	447	1600	1432	168	1600	2974	326
Capacity Analysis Module:												
Vol/Sat:	0.13	0.31	0.33	0.11	0.35	0.37	0.08	0.26	0.26	0.09	0.21	0.22
Crit Moves:	****					****		****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #33 Bristol St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.732
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	41	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	0	1 0	1 0	0	1 0
Volume Module:												
Base Vol:	85	1722	98	120	1597	60	70	194	45	125	199	54
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	85	1722	98	120	1597	60	70	194	45	125	199	54
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	85	1722	98	120	1597	60	70	194	45	125	199	54
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	85	1722	98	120	1597	60	70	194	45	125	199	54
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	85	1722	98	120	1597	60	70	194	45	125	199	54
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	2.84	0.16	1.00	2.89	0.11	1.00	0.81	0.19	1.00	0.79	0.21
Final Sat.:	1600	4742	258	1600	4826	174	1600	1299	301	1600	1258	342
Capacity Analysis Module:												
Vol/Sat:	0.05	0.36	0.38	0.08	0.33	0.35	0.04	0.15	0.15	0.08	0.16	0.16
Crit Moves:			****	****				****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #34 Bristol St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.972
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	167	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	1 0	2 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	434	1872	276	216	1169	264	259	1454	186	155	953	90
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	434	1872	276	216	1169	264	259	1454	186	155	953	90
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	434	1872	276	216	1169	264	259	1454	186	155	953	90
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	434	1872	276	216	1169	264	259	1454	186	155	953	90
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	434	1872	276	216	1169	264	259	1454	186	155	953	90
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.03	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	1.00	2.66	0.34	1.00	1.83	0.17
Final Sat.:	3200	5100	1600	3200	5100	1600	1600	4456	544	1600	3024	276
Capacity Analysis Module:												
Vol/Sat:	0.14	0.37	0.17	0.07	0.23	0.17	0.16	0.33	0.34	0.10	0.32	0.33
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #35 Bristol St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.890
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	81	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	0 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	249	1833	99	116	1618	373	114	222	44	82	424	50
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	249	1833	99	116	1618	373	114	222	44	82	424	50
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	249	1833	99	116	1618	373	114	222	44	82	424	50
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	249	1833	99	116	1618	373	114	222	44	82	424	50
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	249	1833	99	116	1618	373	114	222	44	82	424	50
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	0.83	0.17	1.00	0.89	0.11
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	1335	265	1600	1431	169
Capacity Analysis Module:												
Vol/Sat:	0.16	0.36	0.06	0.07	0.32	0.23	0.07	0.17	0.17	0.05	0.30	0.30
Crit Moves:	****			****			****			****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #36 Bristol St and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.871
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	72	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	1 0	3 0	1	1 0	3 0	1
Volume Module:												
Base Vol:	312	1170	213	343	1144	223	159	715	126	282	1662	487
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	312	1170	213	343	1144	223	159	715	126	282	1662	487
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	312	1170	213	343	1144	223	159	715	126	282	1662	487
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	312	1170	213	343	1144	223	159	715	126	282	1662	487
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	312	1170	213	343	1144	223	159	715	126	282	1662	487
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.54	0.46	2.00	2.51	0.49	1.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200	4261	739	3200	4217	783	1600	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.10	0.27	0.29	0.11	0.27	0.28	0.10	0.14	0.08	0.18	0.33	0.30
Crit Moves:		****	****				****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

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Intersection #37 Bristol St and Segerstrom Ave

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Cycle (sec):	100	Critical Vol./Cap.(X):	0.915	
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx	
Optimal Cycle:	96	Level Of Service:	E	

****

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	177	1309	231	98	894	187	279	998	79	156	1408	71
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	177	1309	231	98	894	187	279	998	79	156	1408	71
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	177	1309	231	98	894	187	279	998	79	156	1408	71
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	177	1309	231	98	894	187	279	998	79	156	1408	71
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	177	1309	231	98	894	187	279	998	79	156	1408	71
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	2.55	0.45	1.00	2.48	0.52	1.00	2.78	0.22	1.00	2.86	0.14
Final Sat.:	1600	4280	720	1600	4170	830	1600	4648	352	1600	4770	230
Capacity Analysis Module:												
Vol/Sat:	0.11	0.31	0.32	0.06	0.21	0.23	0.17	0.21	0.22	0.10	0.30	0.31
Crit Moves:			****	****			****					****

****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #38 Bristol St and Alton Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.583
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	28	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	3 0	1	1 0	1 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	43	1667	157	114	858	54	50	70	130	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	43	1667	157	114	858	54	50	70	130	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	43	1667	157	114	858	54	50	70	130	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	43	1667	157	114	858	54	50	70	130	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	43	1667	157	114	858	54	50	70	130	0	0	0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	1.00	2.74	0.26	1.00	3.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
Final Sat.:	1600	4587	413	1600	5100	1600	1600	1700	1600	1600	1600	0
Capacity Analysis Module:												
Vol/Sat:	0.03	0.36	0.38	0.07	0.17	0.03	0.03	0.04	0.08	0.00	0.00	0.00
Crit Moves:			****	****					****			

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #39 Bristol St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.796
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	51	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	335	1428	260	237	871	137	357	796	169	247	1432	270
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	335	1428	260	237	871	137	357	796	169	247	1432	270
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	335	1428	260	237	871	137	357	796	169	247	1432	270
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	335	1428	260	237	871	137	357	796	169	247	1432	270
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	335	1428	260	237	871	137	357	796	169	247	1432	270
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	2.59	0.41	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	4348	652	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.10	0.28	0.16	0.07	0.20	0.21	0.11	0.16	0.11	0.08	0.28	0.17
Crit Moves:		****			****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #40 Bristol St and Sunflower Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.744
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	42	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	1	2 0	3 0	1	2 0	2 1	1	2 0	3 0	1
Volume Module:												
Base Vol:	687	1452	234	226	844	242	391	764	296	254	976	311
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	687	1452	234	226	844	242	391	764	296	254	976	311
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	687	1452	234	226	844	242	391	764	296	254	976	311
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	687	1452	234	226	844	242	391	764	296	254	976	311
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	687	1452	234	226	844	242	391	764	296	254	976	311
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	2.88	1.12	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	4813	1787	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.21	0.28	0.15	0.07	0.17	0.15	0.12	0.16	0.17	0.08	0.19	0.19
Crit Moves:	****				****		****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #41 Bristol St and I-405 NB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.854
Loss Time (sec):	5	Average Delay (sec/veh):	29.0
Optimal Cycle:	69	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	4	0 1	0 0	4	1 0	0 0	0	0 2	1 1	1	0 2
Volume Module:												
Base Vol:	0	2235	195	0	2458	23	0	0	204	357	306	1184
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	2235	195	0	2458	23	0	0	204	357	306	1184
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	2235	195	0	2458	23	0	0	204	357	306	1184
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	2235	195	0	2458	23	0	0	204	357	306	1184
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	2235	195	0	2458	23	0	0	204	357	306	1184
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	0.85	1.00	0.91	0.91	1.00	1.00	0.75	0.93	0.93	0.75
Lanes:	0.00	4.00	1.00	0.00	4.95	0.05	0.00	0.00	2.00	1.62	1.38	2.00
Final Sat.:	0	6916	1615	0	8556	80	0	0	2842	2840	2434	2842
Capacity Analysis Module:												
Vol/Sat:	0.00	0.32	0.12	0.00	0.29	0.29	0.00	0.00	0.07	0.13	0.13	0.42
Crit Moves:		****			****				****			****
Green/Cycle:	0.00	0.38	0.38	0.00	0.38	0.38	0.00	0.00	0.08	0.49	0.49	0.49
Volume/Cap:	0.00	0.85	0.32	0.00	0.76	0.76	0.00	0.00	0.85	0.26	0.26	0.85
Delay/Veh:	0.0	31.5	22.3	0.0	28.2	28.2	0.0	0.0	69.8	15.1	15.1	27.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	31.5	22.3	0.0	28.2	28.2	0.0	0.0	69.8	15.1	15.1	27.9
LOS by Move:	A	C	C	A	C	C	A	A	E	B	B	C
HCM2k85thQ:	0	29	7	0	24	24	0	0	9	6	6	30

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #42 Bristol St and I-405 SB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.631
Loss Time (sec):	5	Average Delay (sec/veh):	18.2
Optimal Cycle:	32	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Ignore			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	4 0	0	0 0	2 1	1	3 0	0 0	1	0 0	0 0	0
Volume Module:												
Base Vol:	135	1647	0	0	1657	968	740	0	331	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	135	1647	0	0	1657	968	740	0	331	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	135	1647	0	0	1657	0	740	0	331	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	135	1647	0	0	1657	0	740	0	331	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	135	1647	0	0	1657	0	740	0	331	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	1.00	1.00	0.91	0.91	0.92	1.00	0.85	1.00	1.00	1.00
Lanes:	1.00	4.00	0.00	0.00	3.00	1.00	3.00	0.00	1.00	0.00	0.00	0.00
Final Sat.:	1805	6916	0	0	5187	1729	5253	0	1615	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.07	0.24	0.00	0.00	0.32	0.00	0.14	0.00	0.20	0.00	0.00	0.00
Crit Moves:	****				****				****			
Green/Cycle:	0.12	0.63	0.00	0.00	0.51	0.00	0.32	0.00	0.32	0.00	0.00	0.00
Volume/Cap:	0.63	0.38	0.00	0.00	0.63	0.00	0.43	0.00	0.63	0.00	0.00	0.00
Delay/Veh:	47.9	9.3	0.0	0.0	18.4	0.0	26.7	0.0	31.1	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	47.9	9.3	0.0	0.0	18.4	0.0	26.7	0.0	31.1	0.0	0.0	0.0
LOS by Move:	D	A	A	A	B	A	C	A	C	A	A	A
HCM2k85thQ:	8	10	0	0	21	0	10	0	14	0	0	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #43 Flower St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.668
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	34	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Prot+Permit			Prot+Permit			Prot+Permit			Prot+Permit		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	1	1 0	2 0	1	1 0	2 0	1	1 0	2 0	1
Volume Module:												
Base Vol:	215	1307	107	95	486	44	104	300	49	106	371	110
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	215	1307	107	95	486	44	104	300	49	106	371	110
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	215	1307	107	95	486	44	104	300	49	106	371	110
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	215	1307	107	95	486	44	104	300	49	106	371	110
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	215	1307	107	95	486	44	104	300	49	106	371	110
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	1600	3400	1600	1600	3400	1600	1600	3400	1600	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.13	0.38	0.07	0.06	0.14	0.03	0.07	0.09	0.03	0.07	0.11	0.07
Crit Moves:		****			****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #44 Flower St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	1.215
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1 0	1	1 0	1 1	0	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	211	1000	238	140	383	111	135	1181	98	144	969	101
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	211	1000	238	140	383	111	135	1181	98	144	969	101
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	211	1000	238	140	383	111	135	1181	98	144	969	101
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	211	1000	238	140	383	111	135	1181	98	144	969	101
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	211	1000	238	140	383	111	135	1181	98	144	969	101
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.03	1.00	1.00	1.03	1.00
Lanes:	1.00	1.00	1.00	1.00	1.55	0.45	1.00	1.85	0.15	1.00	1.81	0.19
Final Sat.:	1600	1700	1600	1600	2581	719	1600	3055	245	1600	2998	302
Capacity Analysis Module:												
Vol/Sat:	0.13	0.59	0.15	0.09	0.15	0.15	0.08	0.39	0.40	0.09	0.32	0.33
Crit Moves:		****		****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #45 Flower St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.968
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	159	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	1 0	1 0	0	1 0	1 0	0	1 0	1 0	0	1 0
Volume Module:												
Base Vol:	54	734	45	26	201	33	56	422	17	43	532	76
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	54	734	45	26	201	33	56	422	17	43	532	76
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	54	734	45	26	201	33	56	422	17	43	532	76
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	54	734	45	26	201	33	56	422	17	43	532	76
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	54	734	45	26	201	33	56	422	17	43	532	76
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	0.94	0.06	1.00	0.86	0.14	1.00	0.96	0.04	1.00	0.88	0.12
Final Sat.:	1600	1508	92	1600	1374	226	1600	1538	62	1600	1400	200
Capacity Analysis Module:												
Vol/Sat:	0.03	0.49	0.49	0.02	0.15	0.15	0.04	0.27	0.27	0.03	0.38	0.38
Crit Moves:		****			****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #46 Flower St and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.916
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	97	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	120	843	80	80	449	129	241	1198	126	101	1699	109
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	120	843	80	80	449	129	241	1198	126	101	1699	109
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	120	843	80	80	449	129	241	1198	126	101	1699	109
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	120	843	80	80	449	129	241	1198	126	101	1699	109
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	120	843	80	80	449	129	241	1198	126	101	1699	109
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.04	1.00
Lanes:	1.00	1.83	0.17	1.00	1.55	0.45	1.00	2.71	0.29	1.00	2.82	0.18
Final Sat.:	1600	3023	277	1600	2586	714	1600	4543	457	1600	4711	289
Capacity Analysis Module:												
Vol/Sat:	0.08	0.28	0.29	0.05	0.17	0.18	0.15	0.26	0.28	0.06	0.36	0.38
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #47 Flower St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.908
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	92	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	116	553	65	97	233	224	264	1254	80	61	1967	175
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	116	553	65	97	233	224	264	1254	80	61	1967	175
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	116	553	65	97	233	224	264	1254	80	61	1967	175
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	116	553	65	97	233	224	264	1254	80	61	1967	175
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	116	553	65	97	233	224	264	1254	80	61	1967	175
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	1.79	0.21	1.00	1.02	0.98	1.00	2.82	0.18	1.00	2.75	0.25
Final Sat.:	1600	2963	337	1600	1732	1568	1600	4712	288	1600	4608	392
Capacity Analysis Module:												
Vol/Sat:	0.07	0.19	0.19	0.06	0.13	0.14	0.17	0.27	0.28	0.04	0.43	0.45
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #48 Main St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.764
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	45	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	2 1	0	2 0	2 1	0	2 0	2 0	1
Volume Module:												
Base Vol:	310	948	444	191	1199	309	347	495	222	332	662	266
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	310	948	444	191	1199	309	347	495	222	332	662	266
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	310	948	444	191	1199	309	347	495	222	332	662	266
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	310	948	444	191	1199	309	347	495	222	332	662	266
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	310	948	444	191	1199	309	347	495	222	332	662	266
OvlAdjVol:			278									171
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	2.39	0.61	2.00	2.07	0.93	2.00	2.00	1.00
Final Sat.:	3200	5100	1600	3200	4016	984	3200	3514	1486	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.10	0.19	0.28	0.06	0.30	0.31	0.11	0.14	0.15	0.10	0.19	0.17
OvlAdjV/S:			0.17									0.11
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #49 Main St and Mainplace Dr / Memory Ln

Cycle (sec):	100	Critical Vol./Cap.(X):	0.504
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	24	Level Of Service:	A

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	- T	- R		L	- T	- R		L	- T	- R		L	- T	- R	
Control:	Protected				Protected				Protected				Protected			
Rights:	Include				Include				Include				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Y+R:	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0	
Lanes:	2	0 2	1	0	2	0 2	1	0	2	0 2	1	0	2	0 2	0	1
Volume Module:																
Base Vol:	102	1075	204		48	999	132		195	217	58		370	302	146	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	102	1075	204		48	999	132		195	217	58		370	302	146	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	102	1075	204		48	999	132		195	217	58		370	302	146	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	102	1075	204		48	999	132		195	217	58		370	302	146	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	102	1075	204		48	999	132		195	217	58		370	302	146	
Saturation Flow Module:																
Sat/Lane:	1600	1600	1600		1600	1600	1600		1600	1600	1600		1600	1600	1600	
Adjustment:	1.00	1.05	1.00		1.00	1.05	1.00		1.00	1.05	1.00		1.00	1.06	1.00	
Lanes:	2.00	2.52	0.48		2.00	2.65	0.35		2.00	2.37	0.63		2.00	2.00	1.00	
Final Sat.:	3200	4234	766		3200	4440	560		3200	3988	1012		3200	3400	1600	
Capacity Analysis Module:																
Vol/Sat:	0.03	0.25	0.27		0.02	0.23	0.24		0.06	0.05	0.06		0.12	0.09	0.09	
Crit Moves:			****		****						****		****			

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #50 Main St and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.799
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	52	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 0	1	2 0	2 0	1	2 0	2 1	0	2 0	2 1	0
Volume Module:												
Base Vol:	239	1237	270	243	1062	164	174	975	58	187	1147	76
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	239	1237	270	243	1062	164	174	975	58	187	1147	76
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	239	1237	270	243	1062	164	174	975	58	187	1147	76
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	239	1237	270	243	1062	164	174	975	58	187	1147	76
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	239	1237	270	243	1062	164	174	975	58	187	1147	76
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	2.00	2.00	1.00	2.00	2.00	1.00	2.00	2.83	0.17	2.00	2.81	0.19
Final Sat.:	3200	3400	1600	3200	3400	1600	3200	4730	270	3200	4702	298
Capacity Analysis Module:												
Vol/Sat:	0.07	0.36	0.17	0.08	0.31	0.10	0.05	0.21	0.22	0.06	0.24	0.25
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #51 Main St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.856
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	67	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	98	1234	103	63	1031	65	168	930	111	38	384	39
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	98	1234	103	63	1031	65	168	930	111	38	384	39
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	98	1234	103	63	1031	65	168	930	111	38	384	39
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	98	1234	103	63	1031	65	168	930	111	38	384	39
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	98	1234	103	63	1031	65	168	930	111	38	384	39
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.03	1.00
Lanes:	1.00	1.85	0.15	1.00	1.88	0.12	1.00	1.79	0.21	1.00	1.82	0.18
Final Sat.:	1600	3053	247	1600	3110	190	1600	2959	341	1600	3005	295
Capacity Analysis Module:												
Vol/Sat:	0.06	0.40	0.42	0.04	0.33	0.34	0.11	0.31	0.33	0.02	0.13	0.13
Crit Moves:			****	****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #52 Main St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.700
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	37	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	0	0 0	1 1	0	0 0	0 0	0	0 1	0 1	0
Volume Module:												
Base Vol:	56	1280	0	0	1108	54	0	0	0	65	656	85
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	56	1280	0	0	1108	54	0	0	0	65	656	85
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	56	1280	0	0	1108	54	0	0	0	65	656	85
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	56	1280	0	0	1108	54	0	0	0	65	656	85
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	56	1280	0	0	1108	54	0	0	0	65	656	85
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.03	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	1.00	2.00	0.00	0.00	1.91	0.09	0.00	0.00	0.00	0.16	1.63	0.21
Final Sat.:	1600	3400	0	0	3151	149	0	0	0	258	2604	337
Capacity Analysis Module:												
Vol/Sat:	0.04	0.38	0.00	0.00	0.35	0.36	0.00	0.00	0.00	0.04	0.25	0.25
Crit Moves:	****					****					****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #53 Main St and 4th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.472
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	22	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	1 1	0	0 0	1 1	0	0 0	1 1	0	0 0	1 1	0
Volume Module:												
Base Vol:	0	1108	53	0	951	43	0	112	63	0	157	40
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1108	53	0	951	43	0	112	63	0	157	40
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1108	53	0	951	43	0	112	63	0	157	40
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1108	53	0	951	43	0	112	63	0	157	40
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1108	53	0	951	43	0	112	63	0	157	40
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.05	1.00	1.00	1.04	1.00
Lanes:	0.00	1.91	0.09	0.00	1.91	0.09	0.00	1.28	0.72	0.00	1.59	0.41
Final Sat.:	0	3154	146	0	3162	138	0	2148	1152	0	2650	650
Capacity Analysis Module:												
Vol/Sat:	0.00	0.35	0.36	0.00	0.30	0.31	0.00	0.05	0.05	0.00	0.06	0.06
Crit Moves:		****	****				****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #54 Main St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.803
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	53	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	2	0 1	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	179	875	76	160	615	131	91	921	68	75	834	58
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	179	875	76	160	615	131	91	921	68	75	834	58
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	179	875	76	160	615	131	91	921	68	75	834	58
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	179	875	76	160	615	131	91	921	68	75	834	58
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	179	875	76	160	615	131	91	921	68	75	834	58
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.06	1.00	1.00	1.03	1.00	1.00	1.03	1.00
Lanes:	1.00	1.84	0.16	1.00	2.00	1.00	1.00	1.86	0.14	1.00	1.87	0.13
Final Sat.:	1600	3044	256	1600	3400	1600	1600	3080	220	1600	3092	208
Capacity Analysis Module:												
Vol/Sat:	0.11	0.29	0.30	0.10	0.18	0.08	0.06	0.30	0.31	0.05	0.27	0.28
Crit Moves:			****	****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #55 Main St and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.935
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	113	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Prot+Permit			Prot+Permit			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	0 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	149	1169	72	138	933	68	78	345	42	71	400	180
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	149	1169	72	138	933	68	78	345	42	71	400	180
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	149	1169	72	138	933	68	78	345	42	71	400	180
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	149	1169	72	138	933	68	78	345	42	71	400	180
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	149	1169	72	138	933	68	78	345	42	71	400	180
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.88	0.12	1.00	1.86	0.14	1.00	0.89	0.11	1.00	0.69	0.31
Final Sat.:	1600	3114	186	1600	3083	217	1600	1426	174	1600	1103	497
Capacity Analysis Module:												
Vol/Sat:	0.09	0.38	0.39	0.09	0.30	0.31	0.05	0.24	0.24	0.04	0.36	0.36
Crit Moves:			****	****			****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #56 Main St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.977
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	177	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1	1 0	1 0	1	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	111	1181	54	129	646	88	159	940	105	117	1569	162
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	111	1181	54	129	646	88	159	940	105	117	1569	162
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	111	1181	54	129	646	88	159	940	105	117	1569	162
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	111	1181	54	129	646	88	159	940	105	117	1569	162
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	111	1181	54	129	646	88	159	940	105	117	1569	162
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.03	1.00	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	1.91	0.09	1.00	1.76	0.24	1.00	2.70	0.30	1.00	2.72	0.28
Final Sat.:	1600	3160	140	1600	2916	384	1600	4518	482	1600	4551	449
Capacity Analysis Module:												
Vol/Sat:	0.07	0.37	0.39	0.08	0.22	0.23	0.10	0.21	0.22	0.07	0.34	0.36
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #57 Main St and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.781
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	48	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	488	1169	321	271	352	266	385	792	76	188	1488	432
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	488	1169	321	271	352	266	385	792	76	188	1488	432
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	488	1169	321	271	352	266	385	792	76	188	1488	432
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	488	1169	321	271	352	266	385	792	76	188	1488	432
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	488	1169	321	271	352	266	385	792	76	188	1488	432
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.15	0.23	0.20	0.08	0.07	0.17	0.12	0.16	0.05	0.06	0.29	0.27
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #58 Penn Wy and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.790
Loss Time (sec):	5	Average Delay (sec/veh):	20.3
Optimal Cycle:	52	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	0	0 2	0 0	0	0 0	0 0	2	1 0	1 0	3	0 0
Volume Module:												
Base Vol:	795	0	388	0	0	0	0	1712	552	126	1480	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	795	0	388	0	0	0	0	1712	552	126	1480	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	795	0	388	0	0	0	0	1712	552	126	1480	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	795	0	388	0	0	0	0	1712	552	126	1480	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	795	0	388	0	0	0	0	1712	552	126	1480	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.75	1.00	1.00	1.00	1.00	0.88	0.88	0.95	0.91	1.00
Lanes:	2.00	0.00	2.00	0.00	0.00	0.00	0.00	2.27	0.73	1.00	3.00	0.00
Final Sat.:	3502	0	2842	0	0	0	0	3777	1218	1805	5187	0
Capacity Analysis Module:												
Vol/Sat:	0.23	0.00	0.14	0.00	0.00	0.00	0.00	0.45	0.45	0.07	0.29	0.00
Crit Moves:	****							****		****		
Green/Cycle:	0.29	0.00	0.29	0.00	0.00	0.00	0.00	0.57	0.57	0.09	0.66	0.00
Volume/Cap:	0.79	0.00	0.47	0.00	0.00	0.00	0.00	0.79	0.79	0.79	0.43	0.00
Delay/Veh:	37.1	0.0	29.8	0.0	0.0	0.0	0.0	18.1	18.1	67.4	8.1	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	37.1	0.0	29.8	0.0	0.0	0.0	0.0	18.1	18.1	67.4	8.1	0.0
LOS by Move:	D	A	C	A	A	A	A	B	B	E	A	A
HCM2k85thQ:	20	0	9	0	0	0	0	31	31	9	12	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #59 I-5 NB Off Ramps/17th Street

Cycle (sec):	100	Critical Vol./Cap.(X):	0.630
Loss Time (sec):	5	Average Delay (sec/veh):	20.3
Optimal Cycle:	32	Level Of Service:	C

Street Name:	I-5 NB Off Ramps						17th Street					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Permitted		
Rights:	Include			Include			Ignore			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 1	0	0 1	1	0 0	0 1	1	0 3	0 1	0	0 2	1 0
Volume Module:												
Base Vol:	375	50	20	55	0	142	128	1168	767	0	1622	44
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	375	50	20	55	0	142	128	1168	767	0	1622	44
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Volume:	375	50	20	55	0	142	128	1168	0	0	1622	44
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	375	50	20	55	0	142	128	1168	0	0	1622	44
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
FinalVolume:	375	50	20	55	0	142	128	1168	0	0	1622	44
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.96	0.96	0.85	0.95	1.00	0.85	0.95	0.91	1.00	1.00	0.91	0.91
Lanes:	1.76	0.24	1.00	1.00	0.00	1.00	1.00	3.00	1.00	0.00	2.92	0.08
Final Sat.:	3212	428	1615	1805	0	1615	1805	5187	1900	0	5030	136
Capacity Analysis Module:												
Vol/Sat:	0.12	0.12	0.01	0.03	0.00	0.09	0.07	0.23	0.00	0.00	0.32	0.32
Crit Moves:		****				****	****				****	
Green/Cycle:	0.19	0.19	0.19	0.14	0.00	0.14	0.11	0.62	0.00	0.00	0.51	0.51
Volume/Cap:	0.63	0.63	0.07	0.22	0.00	0.63	0.63	0.36	0.00	0.00	0.63	0.63
Delay/Veh:	39.5	39.5	33.7	38.6	0.0	46.2	48.6	9.1	0.0	0.0	18.1	18.1
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	39.5	39.5	33.7	38.6	0.0	46.2	48.6	9.1	0.0	0.0	18.1	18.1
LOS by Move:	D	D	C	D	A	D	D	A	A	A	B	B
HCM2k85thQ:	11	11	1	3	0	8	8	10	0	0	21	21

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #60 Penn Wy and I-5 SB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.433
Loss Time (sec):	5	Average Delay (sec/veh):	22.8
Optimal Cycle:	22	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	2	0 1	2 0	2	0 0	0 0	0	0 0	1 0	0	0 2
Volume Module:												
Base Vol:	0	363	238	508	117	0	0	0	0	214	0	183
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	363	238	508	117	0	0	0	0	214	0	183
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	363	238	508	117	0	0	0	0	214	0	183
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	363	238	508	117	0	0	0	0	214	0	183
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	363	238	508	117	0	0	0	0	214	0	183
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.95	0.85	0.92	0.95	1.00	1.00	1.00	1.00	0.95	1.00	0.75
Lanes:	0.00	2.00	1.00	2.00	2.00	0.00	0.00	0.00	0.00	1.00	0.00	2.00
Final Sat.:	0	3610	1615	3502	3610	0	0	0	0	1805	0	2842
Capacity Analysis Module:												
Vol/Sat:	0.00	0.10	0.15	0.15	0.03	0.00	0.00	0.00	0.00	0.12	0.00	0.06
Crit Moves:			****	****						****		
Green/Cycle:	0.00	0.34	0.34	0.34	0.68	0.00	0.00	0.00	0.00	0.27	0.00	0.61
Volume/Cap:	0.00	0.30	0.43	0.43	0.05	0.00	0.00	0.00	0.00	0.43	0.00	0.11
Delay/Veh:	0.0	24.3	26.0	26.1	5.4	0.0	0.0	0.0	0.0	30.5	0.0	8.2
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	24.3	26.0	26.1	5.4	0.0	0.0	0.0	0.0	30.5	0.0	8.2
LOS by Move:	A	C	C	C	A	A	A	A	A	C	A	A
HCM2k85thQ:	0	7	9	10	1	0	0	0	0	9	0	2

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #61 Santiago St and Civic Center Dr

Cycle (sec):	100	Critical Vol./Cap.(X):	0.896
Loss Time (sec):	5	Average Delay (sec/veh):	27.6
Optimal Cycle:	0	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Stop Sign			Stop Sign			Stop Sign			Stop Sign		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	1 0	0 1	0	1 0	0 1	0	1 0	0 1	0	0 0	1! 0	0
Volume Module:												
Base Vol:	133	411	41	20	281	72	266	76	269	14	19	18
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	133	411	41	20	281	72	266	76	269	14	19	18
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	133	411	41	20	281	72	266	76	269	14	19	18
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	133	411	41	20	281	72	266	76	269	14	19	18
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	133	411	41	20	281	72	266	76	269	14	19	18
Saturation Flow Module:												
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	0.91	0.09	1.00	0.80	0.20	1.00	0.22	0.78	0.27	0.38	0.35
Final Sat.:	462	459	46	437	385	99	452	115	406	107	145	138
Capacity Analysis Module:												
Vol/Sat:	0.29	0.90	0.90	0.05	0.73	0.73	0.59	0.66	0.66	0.13	0.13	0.13
Crit Moves:		****			****				****		****	
Delay/Veh:	13.4	44.0	44.0	11.0	26.6	26.6	20.7	21.2	21.2	12.7	12.7	12.7
Delay Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	13.4	44.0	44.0	11.0	26.6	26.6	20.7	21.2	21.2	12.7	12.7	12.7
LOS by Move:	B	E	E	B	D	D	C	C	C	B	B	B
ApproachDel:	37.0			25.8			21.0			12.7		
Delay Adj:	1.00			1.00			1.00			1.00		
ApprAdjDel:	37.0			25.8			21.0			12.7		
LOS by Appr:	E			D			C			B		
AllWayAvgQ:	0.4	4.8	4.8	0.0	2.2	2.2	1.3	1.7	1.7	0.1	0.1	0.1

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #62 Santiago St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	1.411
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1 0	1	1 0	1 0	1	1 0	0 1	0	1 0	2 0	1
Volume Module:												
Base Vol:	135	833	236	378	639	103	164	673	87	255	546	347
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	135	833	236	378	639	103	164	673	87	255	546	347
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	135	833	236	378	639	103	164	673	87	255	546	347
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	135	833	236	378	639	103	164	673	87	255	546	347
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	135	833	236	378	639	103	164	673	87	255	546	347
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.00	1.00	1.00	1.06	1.00
Lanes:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.89	0.11	1.00	2.00	1.00
Final Sat.:	1600	1700	1600	1600	1700	1600	1600	1417	183	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.08	0.49	0.15	0.24	0.38	0.06	0.10	0.47	0.48	0.16	0.16	0.22
Crit Moves:		****		****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #63 Standard Ave and 4th St

Cycle (sec):	100	Critical Vol./Cap.(X):	1.341
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1 0	1	1 0	1 0	1	1 0	0 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	251	543	214	254	428	297	302	477	201	233	648	351
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	251	543	214	254	428	297	302	477	201	233	648	351
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	251	543	214	254	428	297	302	477	201	233	648	351
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	251	543	214	254	428	297	302	477	201	233	648	351
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	251	543	214	254	428	297	302	477	201	233	648	351
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.70	0.30	1.00	0.65	0.35
Final Sat.:	1600	1700	1600	1600	1700	1600	1600	1126	474	1600	1038	562
Capacity Analysis Module:												
Vol/Sat:	0.16	0.32	0.13	0.16	0.25	0.19	0.19	0.42	0.42	0.15	0.62	0.62
Crit Moves:		****			****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #64 Standard Ave and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	1.484
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0 1	0	0 0	1! 0	0	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	109	724	259	142	679	47	168	1279	65	264	1888	116
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	109	724	259	142	679	47	168	1279	65	264	1888	116
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	109	724	259	142	679	47	168	1279	65	264	1888	116
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	109	724	259	142	679	47	168	1279	65	264	1888	116
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	109	724	259	142	679	47	168	1279	65	264	1888	116
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.03	1.00	1.00	1.03	1.00
Lanes:	1.00	0.74	0.26	0.16	0.79	0.05	1.00	1.90	0.10	1.00	1.88	0.12
Final Sat.:	1600	1178	422	262	1252	87	1600	3145	155	1600	3115	185
Capacity Analysis Module:												
Vol/Sat:	0.07	0.61	0.61	0.09	0.54	0.54	0.11	0.41	0.42	0.17	0.61	0.63
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #65 Standard Ave and Mcfadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.769
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	46	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	1 0	1 0	0	1 0	1 0	0	1 0	1 0	0	1 0
Volume Module:												
Base Vol:	103	407	50	110	240	42	53	351	60	68	412	119
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	103	407	50	110	240	42	53	351	60	68	412	119
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	103	407	50	110	240	42	53	351	60	68	412	119
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	103	407	50	110	240	42	53	351	60	68	412	119
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	103	407	50	110	240	42	53	351	60	68	412	119
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	0.89	0.11	1.00	0.85	0.15	1.00	0.85	0.15	1.00	0.78	0.22
Final Sat.:	1600	1425	175	1600	1362	238	1600	1366	234	1600	1241	359
Capacity Analysis Module:												
Vol/Sat:	0.06	0.29	0.29	0.07	0.18	0.18	0.03	0.26	0.26	0.04	0.33	0.33
Crit Moves:		****			****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #66 Halladay St and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.824
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	57	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	0 1	0 0	0	0 0	0 1	1	1 0	0 1	2	0 0
Volume Module:												
Base Vol:	200	0	417	0	0	0	2	955	96	287	2284	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	200	0	417	0	0	0	2	955	96	287	2284	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	200	0	417	0	0	0	2	955	96	287	2284	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	200	0	417	0	0	0	2	955	96	287	2284	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	200	0	417	0	0	0	2	955	96	287	2284	0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.02	1.00	1.00	1.05	1.00
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	0.01	2.72	0.27	0.33	2.67	0.00
Final Sat.:	1600	0	1600	0	0	0	9	4453	438	536	4464	0
Capacity Analysis Module:												
Vol/Sat:	0.13	0.00	0.26	0.00	0.00	0.00	0.00	0.21	0.22	0.18	0.51	0.00
Crit Moves:			****				****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #67 Halladay St and Dyer Rd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.945
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	124	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	0	1 0	1 0	0	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	57	249	287	125	17	35	166	1433	10	52	1680	134
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	57	249	287	125	17	35	166	1433	10	52	1680	134
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	57	249	287	125	17	35	166	1433	10	52	1680	134
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	57	249	287	125	17	35	166	1433	10	52	1680	134
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	57	249	287	125	17	35	166	1433	10	52	1680	134
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	0.46	0.54	1.00	0.33	0.67	1.00	2.98	0.02	1.00	2.78	0.22
Final Sat.:	1600	743	857	1600	523	1077	1600	4967	33	1600	4645	355
Capacity Analysis Module:												
Vol/Sat:	0.04	0.34	0.33	0.08	0.03	0.03	0.10	0.29	0.30	0.03	0.36	0.38
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #68 SR-55 SB Ramps and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.602
Loss Time (sec):	5	Average Delay (sec/veh):	18.4
Optimal Cycle:	30	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Ignore			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	0	2	0	0	0	2	0	0	2	1	1	0	0	3	0	1
Volume Module:												
Base Vol:	0	0	0	321	0	777	0	1305	1163	0	1547	608
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	321	0	777	0	1305	1163	0	1547	608
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Volume:	0	0	0	321	0	777	0	1305	0	0	1547	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	321	0	777	0	1305	0	0	1547	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
FinalVolume:	0	0	0	321	0	777	0	1305	0	0	1547	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.92	1.00	0.75	1.00	0.91	0.91	1.00	0.91	1.00
Lanes:	0.00	0.00	0.00	2.00	0.00	2.00	0.00	3.00	1.00	0.00	3.00	1.00
Final Sat.:	0	0	0	3502	0	2842	0	5187	1729	0	5187	1900
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.09	0.00	0.27	0.00	0.25	0.00	0.00	0.30	0.00
Crit Moves:						****	****				****	
Green/Cycle:	0.00	0.00	0.00	0.45	0.00	0.45	0.00	0.50	0.00	0.00	0.50	0.00
Volume/Cap:	0.00	0.00	0.00	0.20	0.00	0.60	0.00	0.51	0.00	0.00	0.60	0.00
Delay/Veh:	0.0	0.0	0.0	16.5	0.0	21.3	0.0	17.2	0.0	0.0	18.5	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	16.5	0.0	21.3	0.0	17.2	0.0	0.0	18.5	0.0
LOS by Move:	A	A	A	B	A	C	A	B	A	A	B	A
HCM2k85thQ:	0	0	0	5	0	16	0	15	0	0	19	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #69 SR-55 NB Ramps and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.511
Loss Time (sec):	5	Average Delay (sec/veh):	16.2
Optimal Cycle:	25	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Ignore			Include			Ignore			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	0	0 1	0 0	0	0 0	0 0	2	0 2	0 0	3	0 1
Volume Module:												
Base Vol:	726	0	483	0	0	0	0	764	785	0	1444	1040
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	726	0	483	0	0	0	0	764	785	0	1444	1040
User Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Volume:	726	0	0	0	0	0	0	764	0	0	1444	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	726	0	0	0	0	0	0	764	0	0	1444	0
PCE Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
FinalVolume:	726	0	0	0	0	0	0	764	0	0	1444	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.88	1.00	0.91	1.00
Lanes:	2.00	0.00	1.00	0.00	0.00	0.00	0.00	2.00	2.00	0.00	3.00	1.00
Final Sat.:	3502	0	1900	0	0	0	0	3610	3344	0	5187	1900
Capacity Analysis Module:												
Vol/Sat:	0.21	0.00	0.00	0.00	0.00	0.00	0.00	0.21	0.00	0.00	0.28	0.00
Crit Moves:	****						****				****	
Green/Cycle:	0.41	0.00	0.00	0.00	0.00	0.00	0.00	0.54	0.00	0.00	0.54	0.00
Volume/Cap:	0.51	0.00	0.00	0.00	0.00	0.00	0.00	0.39	0.00	0.00	0.51	0.00
Delay/Veh:	22.6	0.0	0.0	0.0	0.0	0.0	0.0	13.3	0.0	0.0	14.5	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	22.6	0.0	0.0	0.0	0.0	0.0	0.0	13.3	0.0	0.0	14.5	0.0
LOS by Move:	C	A	A	A	A	A	A	B	A	A	B	A
HCM2k85thQ:	13	0	0	0	0	0	0	11	0	0	16	0

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #70 SR-55 SB Ramps and Dyer Rd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.679
Loss Time (sec):	5	Average Delay (sec/veh):	26.5
Optimal Cycle:	37	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Ovl			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 1	0	0 2	0 1	1	0 1	1 0	3	0 1	2 0	3	0 1
Volume Module:												
Base Vol:	386	12	291	103	226	110	66	1544	210	507	904	58
Growth Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
Initial Bse:	386	12	291	103	226	110	66	1544	210	507	904	58
User Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
PHF Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
PHF Volume:	386	12	291	103	226	110	66	1544	210	507	904	58
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	386	12	291	103	226	110	66	1544	210	507	904	58
PCE Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
MLF Adj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
FinalVolume:	386	12	291	103	226	110	66	1544	210	507	904	58
Saturation Flow Module:												
Sat/Lane:	1900 1900		1900	1900 1900		1900	1900 1900		1900	1900 1900		1900
Adjustment:	0.95 0.95		0.75	0.94 0.94		0.85	0.95 0.91		0.85	0.92 0.91		0.85
Lanes:	1.94 0.06		2.00	0.63 1.37		1.00	1.00 3.00		1.00	2.00 3.00		1.00
Final Sat.:	3516	109	2842	1113 2443		1615	1805 5187		1615	3502 5187		1615
Capacity Analysis Module:												
Vol/Sat:	0.11 0.11		0.10	0.09 0.09		0.07	0.04 0.30		0.13	0.14 0.17		0.04
Crit Moves:	****			****			****			****		
Green/Cycle:	0.16 0.16		0.38	0.14 0.14		0.14	0.11 0.44		0.44	0.21 0.54		0.54
Volume/Cap:	0.68 0.68		0.27	0.68 0.68		0.50	0.32 0.68		0.30	0.68 0.32		0.07
Delay/Veh:	42.7 42.7		21.9	45.0 45.0		41.8	41.8 23.3		18.4	38.7 12.9		11.1
User DelAdj:	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00	1.00 1.00		1.00
AdjDel/Veh:	42.7 42.7		21.9	45.0 45.0		41.8	41.8 23.3		18.4	38.7 12.9		11.1
LOS by Move:	D	D	C	D	D	D	D	C	B	D	B	B
HCM2k85thQ:	11	11	6	10	10	6	3	22	7	13	9	1

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #71 Glassell St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.766
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	46	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Ovl			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	1 0	1	1 0	1 1	0	1 0	1 0	1	1 0	1 1	0
Volume Module:												
Base Vol:	573	595	194	15	499	37	112	367	631	225	407	16
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	573	595	194	15	499	37	112	367	631	225	407	16
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	573	595	194	15	499	37	112	367	631	225	407	16
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	573	595	194	15	499	37	112	367	631	225	407	16
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	573	595	194	15	499	37	112	367	631	225	407	16
OvlAdjVol:			0						344			
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.03	1.00	1.00	1.06	1.00	1.00	1.03	1.00
Lanes:	2.00	1.00	1.00	1.00	1.86	0.14	1.00	1.00	1.00	1.00	1.92	0.08
Final Sat.:	3200	1700	1600	1600	3079	221	1600	1700	1600	1600	3179	121
Capacity Analysis Module:												
Vol/Sat:	0.18	0.35	0.12	0.01	0.16	0.17	0.07	0.22	0.39	0.14	0.13	0.13
OvlAdjV/S:			0.00						0.22			
Crit Moves:	****			****				****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #72 Glassell St and SR-22 WB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.943
Loss Time (sec):	5	Average Delay (sec/veh):	33.4
Optimal Cycle:	125	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	0	0 0	2 0	1	0 0	0 0	0	1 0	1! 0	1
Volume Module:												
Base Vol:	640	1045	0	0	1110	323	0	0	0	268	9	476
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	640	1045	0	0	1110	323	0	0	0	268	9	476
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	640	1045	0	0	1110	323	0	0	0	268	9	476
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	640	1045	0	0	1110	323	0	0	0	268	9	476
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	640	1045	0	0	1110	323	0	0	0	268	9	476
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.95	1.00	1.00	0.95	0.85	1.00	1.00	1.00	0.86	0.86	0.86
Lanes:	1.00	2.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	1.35	0.02	1.63
Final Sat.:	1805	3610	0	0	3610	1615	0	0	0	2201	38	2646
Capacity Analysis Module:												
Vol/Sat:	0.35	0.29	0.00	0.00	0.31	0.20	0.00	0.00	0.00	0.12	0.23	0.18
Crit Moves:	****				****						****	
Green/Cycle:	0.38	0.70	0.00	0.00	0.33	0.33	0.00	0.00	0.00	0.25	0.25	0.25
Volume/Cap:	0.94	0.41	0.00	0.00	0.94	0.61	0.00	0.00	0.00	0.49	0.94	0.73
Delay/Veh:	51.9	6.4	0.0	0.0	47.5	30.5	0.0	0.0	0.0	32.4	56.2	37.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	51.9	6.4	0.0	0.0	47.5	30.5	0.0	0.0	0.0	32.4	56.2	37.0
LOS by Move:	D	A	A	A	D	C	A	A	A	C	E	D
HCM2k85thQ:	34	11	0	0	32	14	0	0	0	9	24	15

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #73 Grand Ave / Glassell St and SR-22 EB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.927
Loss Time (sec):	5	Average Delay (sec/veh):	31.8
Optimal Cycle:	109	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 2	0 1	1	0 2	0 0	1	0 1!	0 1	0	0 0	0 0
Volume Module:												
Base Vol:	0	1316	336	505	903	0	238	4	522	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1316	336	505	903	0	238	4	522	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1316	336	505	903	0	238	4	522	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1316	336	505	903	0	238	4	522	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1316	336	505	903	0	238	4	522	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.95	0.85	0.95	0.95	1.00	0.86	0.86	0.86	1.00	1.00	1.00
Lanes:	0.00	2.00	1.00	1.00	2.00	0.00	1.31	0.01	1.68	0.00	0.00	0.00
Final Sat.:	0	3610	1615	1805	3610	0	2130	17	2731	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.36	0.21	0.28	0.25	0.00	0.11	0.24	0.19	0.00	0.00	0.00
Crit Moves:		****		****				****				
Green/Cycle:	0.00	0.39	0.39	0.30	0.70	0.00	0.25	0.25	0.25	0.00	0.00	0.00
Volume/Cap:	0.00	0.93	0.53	0.93	0.36	0.00	0.44	0.93	0.75	0.00	0.00	0.00
Delay/Veh:	0.0	39.7	24.1	55.9	6.3	0.0	31.4	52.7	37.5	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	39.7	24.1	55.9	6.3	0.0	31.4	52.7	37.5	0.0	0.0	0.0
LOS by Move:	A	D	C	E	A	A	C	D	D	A	A	A
HCM2k85thQ:	0	36	13	28	9	0	8	23	16	0	0	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #74 Grand Ave and Fairhaven Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.665
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	34	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	1 0	1 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	50	1311	262	242	974	89	79	45	33	153	29	109
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	50	1311	262	242	974	89	79	45	33	153	29	109
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	50	1311	262	242	974	89	79	45	33	153	29	109
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	50	1311	262	242	974	89	79	45	33	153	29	109
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	50	1311	262	242	974	89	79	45	33	153	29	109
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.00	1.00
Lanes:	1.00	2.50	0.50	1.00	2.75	0.25	1.00	1.15	0.85	1.00	0.21	0.79
Final Sat.:	1600	4201	799	1600	4598	402	1600	1946	1354	1600	336	1264
Capacity Analysis Module:												
Vol/Sat:	0.03	0.31	0.33	0.15	0.21	0.22	0.05	0.02	0.02	0.10	0.09	0.09
Crit Moves:			****	****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #75 Grand Ave and Santa Clara Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.731
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	41	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			WideBypass			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	0	1 0	1 0	1	0 1
Volume Module:												
Base Vol:	95	1280	266	240	946	240	114	139	26	115	206	220
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	95	1280	266	240	946	240	114	139	26	115	206	220
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	95	1280	266	240	946	240	114	139	26	115	206	220
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	95	1280	266	240	946	240	114	139	26	115	206	220
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	95	1280	266	240	946	240	114	139	26	115	206	220
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.00	1.00	1.00	1.06	1.00
Lanes:	1.00	2.48	0.52	1.00	2.39	0.61	1.00	0.84	0.16	1.00	1.00	1.00
Final Sat.:	1600	4174	826	1600	4029	971	1600	1348	252	1600	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.06	0.31	0.32	0.15	0.23	0.25	0.07	0.10	0.10	0.07	0.12	0.14
Crit Moves:			****	****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #76 Grand Ave and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.854
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	66	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	2 0	2	1 0	2 0	2	1 0
Volume Module:												
Base Vol:	266	947	239	294	634	140	371	949	160	284	1046	190
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	266	947	239	294	634	140	371	949	160	284	1046	190
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	266	947	239	294	634	140	371	949	160	284	1046	190
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	266	947	239	294	634	140	371	949	160	284	1046	190
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	266	947	239	294	634	140	371	949	160	284	1046	190
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.40	0.60	1.00	2.46	0.54	2.00	2.57	0.43	2.00	2.54	0.46
Final Sat.:	1600	4033	967	1600	4132	868	3200	4307	693	3200	4262	738
Capacity Analysis Module:												
Vol/Sat:	0.17	0.23	0.25	0.18	0.15	0.16	0.12	0.22	0.23	0.09	0.25	0.26
Crit Moves:			****	****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #77 Grand Ave and I-5 NB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.764
Loss Time (sec):	5	Average Delay (sec/veh):	11.3
Optimal Cycle:	47	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Ignore			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	2 0	1	1 0	3 0	0	0 0	0 0	0 0	2 0	0 0	1
Volume Module:												
Base Vol:	0	2036	1571	67	1623	0	0	0	0	168	0	202
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	2036	1571	67	1623	0	0	0	0	168	0	202
User Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	2036	0	67	1623	0	0	0	0	168	0	202
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	2036	0	67	1623	0	0	0	0	168	0	202
PCE Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	2036	0	67	1623	0	0	0	0	168	0	202
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.95	1.00	0.95	0.91	1.00	1.00	1.00	1.00	0.92	1.00	0.85
Lanes:	0.00	2.00	1.00	1.00	3.00	0.00	0.00	0.00	0.00	2.00	0.00	1.00
Final Sat.:	0	3610	1900	1805	5187	0	0	0	0	3502	0	1615
Capacity Analysis Module:												
Vol/Sat:	0.00	0.56	0.00	0.04	0.31	0.00	0.00	0.00	0.00	0.05	0.00	0.13
Crit Moves:		****		****								****
Green/Cycle:	0.00	0.74	0.00	0.05	0.79	0.00	0.00	0.00	0.00	0.16	0.00	0.16
Volume/Cap:	0.00	0.76	0.00	0.76	0.40	0.00	0.00	0.00	0.00	0.29	0.00	0.76
Delay/Veh:	0.0	9.2	0.0	79.2	3.4	0.0	0.0	0.0	0.0	37.0	0.0	52.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	9.2	0.0	79.2	3.4	0.0	0.0	0.0	0.0	37.0	0.0	52.4
LOS by Move:	A	A	A	E	A	A	A	A	A	D	A	D
HCM2k85thQ:	0	30	0	6	9	0	0	0	0	4	0	12

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #78 Grand Ave and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.827
Loss Time (sec):	5	Average Delay (sec/veh):	28.5
Optimal Cycle:	61	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	0 2	2 0	1	0 2	0 1	0	1 0
Volume Module:												
Base Vol:	213	2411	40	44	964	684	370	226	356	44	295	213
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	213	2411	40	44	964	684	370	226	356	44	295	213
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	213	2411	40	44	964	684	370	226	356	44	295	213
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	213	2411	40	44	964	684	370	226	356	44	295	213
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	213	2411	40	44	964	684	370	226	356	44	295	213
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	0.91	0.95	0.95	0.75	0.92	1.00	0.75	0.89	0.89	0.89
Lanes:	1.00	2.95	0.05	1.00	2.00	2.00	2.00	1.00	2.00	0.16	1.07	0.77
Final Sat.:	1805	5092	84	1805	3610	2842	3502	1900	2842	270	1810	1307
Capacity Analysis Module:												
Vol/Sat:	0.12	0.47	0.47	0.02	0.27	0.24	0.11	0.12	0.13	0.16	0.16	0.16
Crit Moves:		****		****					****		****	
Green/Cycle:	0.18	0.57	0.57	0.03	0.42	0.57	0.15	0.15	0.15	0.20	0.20	0.20
Volume/Cap:	0.64	0.83	0.83	0.83	0.64	0.42	0.70	0.79	0.83	0.83	0.83	0.83
Delay/Veh:	41.9	19.4	19.4	112.6	24.1	12.4	44.4	54.2	53.7	47.0	47.0	47.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	41.9	19.4	19.4	112.6	24.1	12.4	44.4	54.2	53.7	47.0	47.0	47.0
LOS by Move:	D	B	B	F	C	B	D	D	D	D	D	D
HCM2k85thQ:	11	35	35	5	19	11	11	13	13	17	17	17

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #79 Grand Ave and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.718
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	39	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	3 0	1	2 0	2 0	1	2 0	2 0	1
Volume Module:												
Base Vol:	195	1493	45	133	784	313	272	900	89	121	753	138
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	195	1493	45	133	784	313	272	900	89	121	753	138
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	195	1493	45	133	784	313	272	900	89	121	753	138
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	195	1493	45	133	784	313	272	900	89	121	753	138
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	195	1493	45	133	784	313	272	900	89	121	753	138
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.91	0.09	2.00	3.00	1.00	2.00	2.00	1.00	2.00	2.00	1.00
Final Sat.:	3200	4860	140	3200	5100	1600	3200	3400	1600	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.06	0.31	0.32	0.04	0.15	0.20	0.09	0.26	0.06	0.04	0.22	0.09
Crit Moves:			****	****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #80 Grand Ave and Chestnut Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.783
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	49	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	1 0	0 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	180	1352	234	149	664	112	47	143	31	91	283	165
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	180	1352	234	149	664	112	47	143	31	91	283	165
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	180	1352	234	149	664	112	47	143	31	91	283	165
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	180	1352	234	149	664	112	47	143	31	91	283	165
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	180	1352	234	149	664	112	47	143	31	91	283	165
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	2.56	0.44	1.00	2.57	0.43	1.00	0.82	0.18	1.00	0.63	0.37
Final Sat.:	1600	4292	708	1600	4307	693	1600	1315	285	1600	1011	589
Capacity Analysis Module:												
Vol/Sat:	0.11	0.32	0.33	0.09	0.15	0.16	0.03	0.11	0.11	0.06	0.28	0.28
Crit Moves:			****	****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #81 Grand Ave and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.813
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	55	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	1 0	0 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	232	1320	227	176	800	215	64	238	42	96	762	168
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	232	1320	227	176	800	215	64	238	42	96	762	168
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	232	1320	227	176	800	215	64	238	42	96	762	168
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	232	1320	227	176	800	215	64	238	42	96	762	168
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	232	1320	227	176	800	215	64	238	42	96	762	168
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.00	1.00	1.00	1.04	1.00
Lanes:	1.00	2.56	0.44	1.00	2.36	0.64	1.00	0.85	0.15	1.00	1.64	0.36
Final Sat.:	1600	4296	704	1600	3983	1017	1600	1360	240	1600	2722	578
Capacity Analysis Module:												
Vol/Sat:	0.15	0.31	0.32	0.11	0.20	0.21	0.04	0.17	0.17	0.06	0.28	0.29
Crit Moves:		****	****				****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #82 Grand Ave and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	1.079
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	1 0	2 1	0	1 0	2 1	0
Volume Module:												
Base Vol:	259	1494	129	236	583	327	207	895	72	98	1758	228
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	259	1494	129	236	583	327	207	895	72	98	1758	228
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	259	1494	129	236	583	327	207	895	72	98	1758	228
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	259	1494	129	236	583	327	207	895	72	98	1758	228
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	259	1494	129	236	583	327	207	895	72	98	1758	228
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.76	0.24	1.00	2.00	1.00	1.00	2.78	0.22	1.00	2.66	0.34
Final Sat.:	1600	4618	382	1600	3400	1600	1600	4643	357	1600	4449	551
Capacity Analysis Module:												
Vol/Sat:	0.16	0.32	0.34	0.15	0.17	0.20	0.13	0.19	0.20	0.06	0.40	0.41
Crit Moves:		****	****				****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #83 Grand Ave and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.899
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	86	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	2 0	3 0	1	2 0	2 1	0
Volume Module:												
Base Vol:	218	788	174	178	575	265	342	656	142	244	1556	510
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	218	788	174	178	575	265	342	656	142	244	1556	510
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	218	788	174	178	575	265	342	656	142	244	1556	510
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	218	788	174	178	575	265	342	656	142	244	1556	510
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	218	788	174	178	575	265	342	656	142	244	1556	510
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	2.46	0.54	1.00	2.05	0.95	2.00	3.00	1.00	2.00	2.26	0.74
Final Sat.:	1600	4132	868	1600	3486	1514	3200	5100	1600	3200	3815	1185
Capacity Analysis Module:												
Vol/Sat:	0.14	0.19	0.20	0.11	0.16	0.18	0.11	0.13	0.09	0.08	0.41	0.43
Crit Moves:	****					****	****					****

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #84 SR-55 NB Ramps and Dyer Rd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.349
Loss Time (sec):	5	Average Delay (sec/veh):	6.0
Optimal Cycle:	19	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Ignore			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	1! 0	1	0	0	0	0	0	3	0	1	0	0	2	1	0
Volume Module:												
Base Vol:	188	0	51	0	0	0	0	1355	689	0	1411	882
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	188	0	51	0	0	0	0	1355	689	0	1411	882
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
PHF Volume:	188	0	51	0	0	0	0	1355	0	0	1411	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	188	0	51	0	0	0	0	1355	0	0	1411	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00
FinalVolume:	188	0	51	0	0	0	0	1355	0	0	1411	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.93	1.00	0.93	1.00	1.00	1.00	1.00	0.91	1.00	1.00	0.91	0.91
Lanes:	1.79	0.00	1.21	0.00	0.00	0.00	0.00	3.00	1.00	0.00	3.00	0.00
Final Sat.:	3161	0	2147	0	0	0	0	5187	1900	0	5187	0
Capacity Analysis Module:												
Vol/Sat:	0.06	0.00	0.02	0.00	0.00	0.00	0.00	0.26	0.00	0.00	0.27	0.00
Crit Moves:	****						****				****	
Green/Cycle:	0.17	0.00	0.17	0.00	0.00	0.00	0.00	0.78	0.00	0.00	0.78	0.00
Volume/Cap:	0.35	0.00	0.14	0.00	0.00	0.00	0.00	0.34	0.00	0.00	0.35	0.00
Delay/Veh:	36.9	0.0	35.3	0.0	0.0	0.0	0.0	3.3	0.0	0.0	3.4	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	36.9	0.0	35.3	0.0	0.0	0.0	0.0	3.3	0.0	0.0	3.4	0.0
LOS by Move:	D	A	D	A	A	A	A	A	A	A	A	A
HCM2k85thQ:	5	0	2	0	0	0	0	7	0	0	8	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM 4-Way Stop Method (Base Volume Alternative)

Intersection #85 Cambridge St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.938
Loss Time (sec):	5	Average Delay (sec/veh):	26.1
Optimal Cycle:	0	Level Of Service:	D

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	T	R		L	T	R		L	T	R		L	T	R	
Control:	Stop Sign				Stop Sign				Stop Sign				Stop Sign			
Rights:	Include				Include				Include				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Lanes:	1	0	1	0	0	0	0	1	0	1	0	0	0	1	0	0
Volume Module:																
Base Vol:	176	246	0		0	206	191		469	0	309		0	0	0	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	176	246	0		0	206	191		469	0	309		0	0	0	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	176	246	0		0	206	191		469	0	309		0	0	0	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	176	246	0		0	206	191		469	0	309		0	0	0	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	176	246	0		0	206	191		469	0	309		0	0	0	
Saturation Flow Module:																
Adjustment:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Lanes:	1.00	1.00	0.00		0.00	1.00	1.00		1.00	0.00	1.00		0.00	0.00	0.00	
Final Sat.:	454	486	0		0	479	528		500	0	592		0	0	0	
Capacity Analysis Module:																
Vol/Sat:	0.39	0.51	xxxx		xxxx	0.43	0.36		0.94	xxxx	0.52		xxxx	xxxx	xxxx	
Crit Moves:		****				****			****							
Delay/Veh:	15.1	16.9	0.0		0.0	15.3	12.9		52.4	0.0	14.9		0.0	0.0	0.0	
Delay Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
AdjDel/Veh:	15.1	16.9	0.0		0.0	15.3	12.9		52.4	0.0	14.9		0.0	0.0	0.0	
LOS by Move:	C	C	*		*	C	B		F	*	B		*	*	*	
ApproachDel:	16.2				14.1				37.5				xxxxxx			
Delay Adj:	1.00				1.00				1.00				xxxxx			
ApprAdjDel:	16.2				14.1				37.5				xxxxxx			
LOS by Appr:	C				B				E				*			
AllWayAvgQ:	0.6	0.9	0.0		0.0	0.7	0.5		5.9	0.0	1.0		0.0	0.0	0.0	

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #86 Cambridge St and Fairhaven Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.531
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	25	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	0	0 0	1 0	0	0 1	1 0	1	0 0	0 0	0	1 0
Volume Module:												
Base Vol:	0	0	0	195	0	68	85	148	0	0	218	272
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	195	0	68	85	148	0	0	218	272
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	195	0	68	85	148	0	0	218	272
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	195	0	68	85	148	0	0	218	272
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	195	0	68	85	148	0	0	218	272
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	0.00	0.00	0.00	1.00	0.00	1.00	1.00	1.00	0.00	0.00	0.44	0.56
Final Sat.:	0	0	0	1600	0	1600	1600	1700	0	0	712	888
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.12	0.00	0.04	0.05	0.09	0.00	0.00	0.31	0.31
Crit Moves:				****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #87 Mabury St and 1st Street

Cycle (sec):	100	Critical Vol./Cap.(X):	0.752
Loss Time (sec):	5	Average Delay (sec/veh):	27.2
Optimal Cycle:	46	Level Of Service:	C

Street Name:	Mabury								1st Street							
Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	-	T	- R	L	-	T	- R	L	-	T	- R	L	-	T	- R
Control:	Split Phase				Split Phase				Protected				Protected			
Rights:	Include				Include				Include				Include			
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	0	0 1	1	0	1!	0 1	0	0	1	1 0	1	0	1	1 0
Volume Module:																
Base Vol:	25	0	256	164	70	310	0	1275	20	31	423	0				
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
Initial Bse:	25	0	256	164	70	310	0	1275	20	31	423	0				
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
PHF Volume:	25	0	256	164	70	310	0	1275	20	31	423	0				
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0				
Reduced Vol:	25	0	256	164	70	310	0	1275	20	31	423	0				
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
FinalVolume:	25	0	256	164	70	310	0	1275	20	31	423	0				
Saturation Flow Module:																
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900				
Adjustment:	0.95	1.00	0.85	0.90	0.90	0.90	1.00	0.95	0.95	0.95	0.95	0.95				
Lanes:	1.00	0.00	1.00	1.27	0.23	1.50	0.00	1.97	0.03	1.00	2.00	0.00				
Final Sat.:	1805	0	1615	2170	390	2577	0	3547	56	1805	3610	0				
Capacity Analysis Module:																
Vol/Sat:	0.01	0.00	0.16	0.08	0.18	0.12	0.00	0.36	0.36	0.02	0.12	0.00				
Crit Moves:			****		****			****		****						
Green/Cycle:	0.21	0.00	0.21	0.24	0.24	0.24	0.00	0.48	0.48	0.02	0.50	0.00				
Volume/Cap:	0.07	0.00	0.75	0.32	0.75	0.50	0.00	0.75	0.75	0.75	0.23	0.00				
Delay/Veh:	31.7	0.0	46.1	31.5	39.8	33.4	0.0	23.2	23.2	103.1	14.2	0.0				
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00				
AdjDel/Veh:	31.7	0.0	46.1	31.5	39.8	33.4	0.0	23.2	23.2	103.1	14.2	0.0				
LOS by Move:	C	A	D	C	D	C	A	C	C	F	B	A				
HCM2k85thQ:	1	0	14	5	16	9	0	27	27	4	6	0				

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #88 Tustin St and La Veta Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.344
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	18	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 1	0	0 1
Volume Module:												
Base Vol:	31	693	164	96	851	16	13	5	20	134	4	82
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	31	693	164	96	851	16	13	5	20	134	4	82
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	31	693	164	96	851	16	13	5	20	134	4	82
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	31	693	164	96	851	16	13	5	20	134	4	82
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	31	693	164	96	851	16	13	5	20	134	4	82
OvlAdjVol:												0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	1.00	2.43	0.57	1.00	2.94	0.06	1.00	1.00	1.00	1.94	0.06	1.00
Final Sat.:	1600	4081	919	1600	4911	89	1600	1700	1600	3107	93	1600
Capacity Analysis Module:												
Vol/Sat:	0.02	0.17	0.18	0.06	0.17	0.18	0.01	0.00	0.01	0.04	0.04	0.05
OvlAdjV/S:												0.00
Crit Moves:		****	****					****		****		

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #89 Tustin St and SR-22 WB On-Ramp

Cycle (sec):	100	Critical Vol./Cap.(X):	0.321
Loss Time (sec):	5	Average Delay (sec/veh):	7.7
Optimal Cycle:	19	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0 2	0 0	0	0 2	1 0	0	0 0	0 0	0	0 0	0 0
Volume Module:												
Base Vol:	383	1100	0	0	642	330	0	0	0	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	383	1100	0	0	642	330	0	0	0	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	383	1100	0	0	642	330	0	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	383	1100	0	0	642	330	0	0	0	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	383	1100	0	0	642	330	0	0	0	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.95	1.00	1.00	0.86	0.86	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	2.00	2.00	0.00	0.00	2.00	1.00	0.00	0.00	0.00	0.00	0.00	0.00
Final Sat.:	3502	3610	0	0	3282	1641	0	0	0	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.11	0.30	0.00	0.00	0.20	0.20	0.00	0.00	0.00	0.00	0.00	0.00
Crit Moves:	****				****							
Green/Cycle:	0.34	0.95	0.00	0.00	0.61	0.61	0.00	0.00	0.00	0.00	0.00	0.00
Volume/Cap:	0.32	0.32	0.00	0.00	0.32	0.33	0.00	0.00	0.00	0.00	0.00	0.00
Delay/Veh:	24.6	0.2	0.0	0.0	9.6	9.6	0.0	0.0	0.0	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	24.6	0.2	0.0	0.0	9.6	9.6	0.0	0.0	0.0	0.0	0.0	0.0
LOS by Move:	C	A	A	A	A	A	A	A	A	A	A	A
HCM2k85thQ:	7	3	0	0	8	8	0	0	0	0	0	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #90 Tustin St and SR-22 EB Off-Ramp / Seba Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.625
Loss Time (sec):	5	Average Delay (sec/veh):	23.3
Optimal Cycle:	32	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Permitted			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 2	1 0	1	0 3	0 0	1	0 1!	0 1	0	0 1!	0 0
Volume Module:												
Base Vol:	0	1155	3	43	589	0	440	39	558	28	0	67
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1155	3	43	589	0	440	39	558	28	0	67
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1155	3	43	589	0	440	39	558	28	0	67
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1155	3	43	589	0	440	39	558	28	0	67
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1155	3	43	589	0	440	39	558	28	0	67
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	0.91	0.11	0.91	1.00	0.90	0.90	0.90	0.89	1.00	0.89
Lanes:	0.00	2.99	0.01	1.00	3.00	0.00	1.41	0.07	1.52	0.29	0.00	0.71
Final Sat.:	0	5174	13	217	5187	0	2408	124	2596	499	0	1195
Capacity Analysis Module:												
Vol/Sat:	0.00	0.22	0.22	0.20	0.11	0.00	0.18	0.31	0.21	0.06	0.00	0.06
Crit Moves:		****						****		****		
Green/Cycle:	0.00	0.36	0.36	0.36	0.36	0.00	0.50	0.50	0.50	0.09	0.00	0.09
Volume/Cap:	0.00	0.63	0.63	0.56	0.32	0.00	0.36	0.63	0.43	0.63	0.00	0.63
Delay/Veh:	0.0	27.3	27.3	34.4	23.4	0.0	15.2	18.8	15.8	51.8	0.0	51.8
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	27.3	27.3	34.4	23.4	0.0	15.2	18.8	15.8	51.8	0.0	51.8
LOS by Move:	A	C	C	C	C	A	B	B	B	D	A	D
HCM2k85thQ:	0	17	17	3	7	0	9	19	11	6	0	6

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #91 Tustin Ave and Fairhaven Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.757
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	44	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	1 0	0 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	258	1377	122	356	940	108	82	185	151	86	191	180
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	258	1377	122	356	940	108	82	185	151	86	191	180
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	258	1377	122	356	940	108	82	185	151	86	191	180
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	258	1377	122	356	940	108	82	185	151	86	191	180
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	258	1377	122	356	940	108	82	185	151	86	191	180
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	2.00	2.76	0.24	2.00	2.69	0.31	1.00	0.55	0.45	1.00	0.51	0.49
Final Sat.:	3200	4609	391	3200	4505	495	1600	881	719	1600	824	776
Capacity Analysis Module:												
Vol/Sat:	0.08	0.30	0.31	0.11	0.21	0.22	0.05	0.21	0.21	0.05	0.23	0.23
Crit Moves:		****	****				****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #92 Tustin Ave and Santa Clara Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.779
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	48	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	2 0	2	1 0	1 0	0	1 0	1 0	0	1 0
Volume Module:												
Base Vol:	176	852	131	199	1574	109	63	198	116	115	144	136
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	176	852	131	199	1574	109	63	198	116	115	144	136
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	176	852	131	199	1574	109	63	198	116	115	144	136
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	176	852	131	199	1574	109	63	198	116	115	144	136
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	176	852	131	199	1574	109	63	198	116	115	144	136
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	2.60	0.40	2.00	2.81	0.19	1.00	0.63	0.37	1.00	0.51	0.49
Final Sat.:	1600	4360	640	3200	4689	311	1600	1009	591	1600	823	777
Capacity Analysis Module:												
Vol/Sat:	0.11	0.20	0.20	0.06	0.34	0.35	0.04	0.20	0.20	0.07	0.18	0.17
Crit Moves:	****					****		****		****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #93 Tustin Ave and 17th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.741
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	42	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 0	1	2 0	2 1	0	2 0	3 0	1	2 0	3 0	1
Volume Module:												
Base Vol:	233	1000	514	377	404	40	457	898	122	211	693	385
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	233	1000	514	377	404	40	457	898	122	211	693	385
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	233	1000	514	377	404	40	457	898	122	211	693	385
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	233	1000	514	377	404	40	457	898	122	211	693	385
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	233	1000	514	377	404	40	457	898	122	211	693	385
OvlAdjVol:			409									197
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.00	1.00	2.00	2.73	0.27	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	3400	1600	3200	4568	432	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.07	0.29	0.32	0.12	0.09	0.09	0.14	0.18	0.08	0.07	0.14	0.24
OvlAdjV/S:			0.26									0.12
Crit Moves:	****			****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #94 Tustin Ave and 4th St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.784
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	49	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	2 0	2	0 1	1 0	2	1 1	1 0	2	1 0
Volume Module:												
Base Vol:	35	381	123	536	396	145	226	806	41	123	833	513
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	35	381	123	536	396	145	226	806	41	123	833	513
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	35	381	123	536	396	145	226	806	41	123	833	513
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	35	381	123	536	396	145	226	806	41	123	833	513
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	35	381	123	536	396	145	226	806	41	123	833	513
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	2.27	0.73	2.00	2.00	1.00	1.00	3.00	1.00	1.00	2.00	1.00
Final Sat.:	1600	3829	1171	3200	3400	1600	1600	5100	1600	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.02	0.10	0.11	0.17	0.12	0.09	0.14	0.16	0.03	0.08	0.25	0.32
Crit Moves:			****	****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #95 SR-55 SB Ramps / Auto Mall Dr and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.687
Loss Time (sec):	5	Average Delay (sec/veh):	25.4
Optimal Cycle:	37	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Ignore			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 1	0	0 2	1	0 1	0 1	1	0 3	0 1	2	0 2	1 0
Volume Module:												
Base Vol:	401	32	423	110	50	37	46	1649	464	437	1139	136
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	401	32	423	110	50	37	46	1649	464	437	1139	136
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Volume:	401	32	423	110	50	37	46	1649	0	437	1139	136
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	401	32	423	110	50	37	46	1649	0	437	1139	136
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
FinalVolume:	401	32	423	110	50	37	46	1649	0	437	1139	136
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.96	0.96	0.75	0.95	1.00	0.85	0.95	0.91	1.00	0.92	0.90	0.90
Lanes:	1.85	0.15	2.00	1.00	1.00	1.00	1.00	3.00	1.00	2.00	2.68	0.32
Final Sat.:	3364	268	2842	1805	1900	1615	1805	5187	1900	3502	4560	544
Capacity Analysis Module:												
Vol/Sat:	0.12	0.12	0.15	0.06	0.03	0.02	0.03	0.32	0.00	0.12	0.25	0.25
Crit Moves:			****	****				****		****		
Green/Cycle:	0.22	0.22	0.22	0.09	0.09	0.09	0.06	0.46	0.00	0.18	0.58	0.58
Volume/Cap:	0.55	0.55	0.69	0.69	0.30	0.26	0.43	0.69	0.00	0.69	0.43	0.43
Delay/Veh:	35.7	35.7	39.3	56.0	43.6	43.5	48.1	22.0	0.0	41.4	11.6	11.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	35.7	35.7	39.3	56.0	43.6	43.5	48.1	22.0	0.0	41.4	11.6	11.6
LOS by Move:	D	D	D	E	D	D	D	C	A	D	B	B
HCM2k85thQ:	10	10	12	7	3	2	3	23	0	12	12	12

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Note: Queue reported is the number of cars per lane.
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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #96 SR-55 NB Ramps / Del Amo Ave and Newport Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	0.951
Loss Time (sec):	5	Average Delay (sec/veh):	46.2
Optimal Cycle:	134	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0 2	1 0		1 0 3	0 1		2 0 1	1 0		1 0 1	1 0	
Volume Module:												
Base Vol:	879	262	51	19	231	889	165	57	33	23	348	16
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	879	262	51	19	231	889	165	57	33	23	348	16
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	879	262	51	19	231	889	165	57	33	23	348	16
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	879	262	51	19	231	889	165	57	33	23	348	16
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	879	262	51	19	231	889	165	57	33	23	348	16
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.89	0.89	0.95	0.91	0.85	0.92	0.90	0.90	0.95	0.94	0.94
Lanes:	2.00	2.51	0.49	1.00	3.00	1.00	2.00	1.27	0.73	1.00	1.91	0.09
Final Sat.:	3502	4238	825	1805	5187	1615	3502	2161	1251	1805	3427	158
Capacity Analysis Module:												
Vol/Sat:	0.25	0.06	0.06	0.01	0.04	0.55	0.05	0.03	0.03	0.01	0.10	0.10
Crit Moves:	****					****	****				****	
Green/Cycle:	0.26	0.68	0.68	0.12	0.53	0.58	0.05	0.11	0.11	0.05	0.11	0.11
Volume/Cap:	0.95	0.09	0.09	0.09	0.08	0.95	0.95	0.25	0.25	0.25	0.95	0.95
Delay/Veh:	54.9	5.5	5.5	39.7	11.6	38.3	101.1	41.5	41.5	47.0	77.7	77.7
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	54.9	5.5	5.5	39.7	11.6	38.3	101.1	41.5	41.5	47.0	77.7	77.7
LOS by Move:	D	A	A	D	B	D	F	D	D	D	E	E
HCM2k85thQ:	27	2	2	1	2	44	8	2	2	1	15	15

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #97 Red Hill Ave and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.865
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	70	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Ovl			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0 3	0 1	2	0 3	0 1	2	0 3	0 1	2	0 3	0 1
Volume Module:												
Base Vol:	208	1501	238	108	214	173	410	1051	128	91	1176	574
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	208	1501	238	108	214	173	410	1051	128	91	1176	574
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	208	1501	238	108	214	173	410	1051	128	91	1176	574
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	208	1501	238	108	214	173	410	1051	128	91	1176	574
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	208	1501	238	108	214	173	410	1051	128	91	1176	574
OvlAdjVol:						0			24			
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00	2.00	3.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	5100	1600	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.07	0.29	0.15	0.03	0.04	0.11	0.13	0.21	0.08	0.03	0.23	0.36
OvlAdjV/S:						0.00			0.02			
Crit Moves:	****			****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #98 Red Hill Ave and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.952
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	132	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0 3	1 0	2	0 2	1 0	1	0 2	1 0	2	0 3	0 1
Volume Module:												
Base Vol:	343	1175	135	89	341	432	324	880	137	78	1643	216
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	343	1175	135	89	341	432	324	880	137	78	1643	216
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	343	1175	135	89	341	432	324	880	137	78	1643	216
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	343	1175	135	89	341	432	324	880	137	78	1643	216
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	343	1175	135	89	341	432	324	880	137	78	1643	216
OvlAdjVol:												172
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	3.59	0.41	2.00	2.00	1.00	1.00	2.60	0.40	2.00	3.00	1.00
Final Sat.:	3200	6040	660	3200	3400	1600	1600	4353	647	3200	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.11	0.19	0.20	0.03	0.10	0.27	0.20	0.20	0.21	0.02	0.32	0.14
OvlAdjV/S:												0.11
Crit Moves:	****					****	****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #99 Red Hill Ave and Dyer Rd / barranca Pkwy

Cycle (sec):	100	Critical Vol./Cap.(X):	0.591
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	28	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	4 0	1	2 0	4 0	1	2 0	3 1	0	2 0	4 0	1
Volume Module:												
Base Vol:	344	1293	333	314	531	249	178	662	87	129	979	286
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	344	1293	333	314	531	249	178	662	87	129	979	286
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	344	1293	333	314	531	249	178	662	87	129	979	286
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	344	1293	333	314	531	249	178	662	87	129	979	286
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	344	1293	333	314	531	249	178	662	87	129	979	286
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	4.00	1.00	2.00	4.00	1.00	2.00	3.54	0.46	2.00	4.00	1.00
Final Sat.:	3200	6800	1600	3200	6800	1600	3200	5957	743	3200	6800	1600
Capacity Analysis Module:												
Vol/Sat:	0.11	0.19	0.21	0.10	0.08	0.16	0.06	0.11	0.12	0.04	0.14	0.18
Crit Moves:			****	****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #100 Red Hill Ave and Alton Pkwy

Cycle (sec):	100	Critical Vol./Cap.(X):	1.494
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 0	1	1 0	3 0	1	1 0	2 0	1	2 0	1 0	1
Volume Module:												
Base Vol:	27	1513	219	148	628	5	453	1111	569	2003	342	1235
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	27	1513	219	148	628	5	453	1111	569	2003	342	1235
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	27	1513	219	148	628	5	453	1111	569	2003	342	1235
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	27	1513	219	148	628	5	453	1111	569	2003	342	1235
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	27	1513	219	148	628	5	453	1111	569	2003	342	1235
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	3.00	1.00	1.00	3.00	1.00	1.00	2.00	1.00	2.00	1.00	1.00
Final Sat.:	1600	5100	1600	1600	5100	1600	1600	3400	1600	3200	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.02	0.30	0.14	0.09	0.12	0.00	0.28	0.33	0.36	0.63	0.20	0.77
Crit Moves:		****			****			****				****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #101 Red Hill Ave and MacArthur Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.722		
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx		
Optimal Cycle:	40	Level Of Service:	C		

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ignore			Include			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0 2	1 0		2 0 3	0 1		2 0 3	0 1		1 0 3	0 1	
Volume Module:												
Base Vol:	179 1160		14	340	707	1240	515	237	36	39	818	574
Growth Adj:	1.00 1.00		1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	179 1160		14	340	707	1240	515	237	36	39	818	574
User Adj:	1.00 1.00		1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00 1.00		1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Volume:	179 1160		14	340	707	0	515	237	36	39	818	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	179 1160		14	340	707	0	515	237	36	39	818	0
PCE Adj:	1.00 1.00		1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00 1.00		1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	0.00
FinalVolume:	179 1160		14	340	707	0	515	237	36	39	818	0
Saturation Flow Module:												
Sat/Lane:	1600 1600		1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00 1.04		1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00 2.96		0.04	2.00	3.00	1.00	2.00	3.00	1.00	1.00	3.00	1.00
Final Sat.:	3200 4943		57	3200	5100	1600	3200	5100	1600	1600	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.06 0.23		0.24	0.11	0.14	0.00	0.16	0.05	0.02	0.02	0.16	0.00
Crit Moves:			****	****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #102 Red Hill Ave and Main St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.836
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	61	Level Of Service:	D

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	- T	- R		L	- T	- R		L	- T	- R		L	- T	- R	
Control:	Protected				Protected				Protected				Protected			
Rights:	Include				Include				Include				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Y+R:	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0	
Lanes:	2	0 2	0	1	2	0 2	0	1	2	0 2	1	0	2	0 2	1	0
Volume Module:																
Base Vol:	467	1006	272		88	640	348		212	757	205		284	1617	91	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	467	1006	272		88	640	348		212	757	205		284	1617	91	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	467	1006	272		88	640	348		212	757	205		284	1617	91	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	467	1006	272		88	640	348		212	757	205		284	1617	91	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	467	1006	272		88	640	348		212	757	205		284	1617	91	
Saturation Flow Module:																
Sat/Lane:	1600	1600	1600		1600	1600	1600		1600	1600	1600		1600	1600	1600	
Adjustment:	1.00	1.06	1.00		1.00	1.06	1.00		1.00	1.05	1.00		1.00	1.04	1.00	
Lanes:	2.00	2.00	1.00		2.00	2.00	1.00		2.00	2.36	0.64		2.00	2.84	0.16	
Final Sat.:	3200	3400	1600		3200	3400	1600		3200	3977	1023		3200	4744	256	
Capacity Analysis Module:																
Vol/Sat:	0.15	0.30	0.17		0.03	0.19	0.22		0.07	0.19	0.20		0.09	0.34	0.36	
Crit Moves:	****						****		****						****	

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #103 I-5 SB Ramps and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	0.444
Loss Time (sec):	5	Average Delay (sec/veh):	19.9
Optimal Cycle:	22	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	0	2	0	0	2	0	3	0	2	1
Volume Module:												
Base Vol:	0	0	0	357	0	78	595	672	0	0	590	162
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	357	0	78	595	672	0	0	590	162
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	357	0	78	595	672	0	0	590	162
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	357	0	78	595	672	0	0	590	162
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	357	0	78	595	672	0	0	590	162
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.92	1.00	0.85	0.92	0.91	1.00	1.00	0.88	0.88
Lanes:	0.00	0.00	0.00	2.00	0.00	1.00	2.00	3.00	0.00	0.00	2.35	0.65
Final Sat.:	0	0	0	3502	0	1615	3502	5187	0	0	3939	1082
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.10	0.00	0.05	0.17	0.13	0.00	0.00	0.15	0.15
Crit Moves:				****			****				****	
Green/Cycle:	0.00	0.00	0.00	0.23	0.00	0.61	0.38	0.72	0.00	0.00	0.34	0.34
Volume/Cap:	0.00	0.00	0.00	0.44	0.00	0.08	0.44	0.18	0.00	0.00	0.44	0.44
Delay/Veh:	0.0	0.0	0.0	33.4	0.0	7.9	23.2	4.5	0.0	0.0	26.0	26.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

AdjDel/Veh:	0.0	0.0	0.0	33.4	0.0	7.9	23.2	4.5	0.0	0.0	26.0	26.0
LOS by Move:	A	A	A	C	A	A	C	A	A	A	C	C
HCM2k85thQ:	0	0	0	8	0	2	11	4	0	0	10	10

****

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #104 Tustin Ranch Rd and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.671
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	34	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Protected			Protected		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	0 0	0	2 0	0 0	1	2 0	3 0	0	0 0	3 0	1
Volume Module:												
Base Vol:	0	0	0	653	0	45	64	2124	0	0	925	398
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	653	0	45	64	2124	0	0	925	398
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	653	0	45	64	2124	0	0	925	398
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	653	0	45	64	2124	0	0	925	398
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	653	0	45	64	2124	0	0	925	398
OvlAdjVol:												71
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	0.00	0.00	0.00	2.00	0.00	1.00	2.00	3.00	0.00	0.00	3.00	1.00
Final Sat.:	0	0	0	3200	0	1600	3200	5100	0	0	5100	1600
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.20	0.00	0.03	0.02	0.42	0.00	0.00	0.18	0.25
OvlAdjV/S:												0.04
Crit Moves:				****				****		****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #105 Von Karman Ave and Barranca Pkwy

Cycle (sec):	100	Critical Vol./Cap.(X):	1.284
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0 1	1 0		2 0 2			2 0 3	0 1		2 0 4	0 1	
Volume Module:												
Base Vol:	362	2082	454	80	407	280	785	1435	201	254	805	274
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	362	2082	454	80	407	280	785	1435	201	254	805	274
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	362	2082	454	80	407	280	785	1435	201	254	805	274
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	362	2082	454	80	407	280	785	1435	201	254	805	274
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	362	2082	454	80	407	280	785	1435	201	254	805	274
OvlAdjVol:						0						
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	1.64	0.36	2.00	2.00	2.00	2.00	3.00	1.00	2.00	4.00	1.00
Final Sat.:	3200	2727	573	3200	3400	3200	3200	5100	1600	3200	6800	1600
Capacity Analysis Module:												
Vol/Sat:	0.11	0.76	0.79	0.03	0.12	0.09	0.25	0.28	0.13	0.08	0.12	0.17
OvlAdjV/S:						0.00						
Crit Moves:		****	****				****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #106 Red Hill Avenue and El Camino Real

Cycle (sec):	100	Critical Vol./Cap.(X):	0.696
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	37	Level Of Service:	B

Street Name:	Red Hill Avenue						El Camino Real					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0	3	0	1	1	0	2	1	0	1	0	1	0	1	0	0	1	0
Volume Module:												
Base Vol:	433	1372	253	20	728	96	52	113	148	265	439	51
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	433	1372	253	20	728	96	52	113	148	265	439	51
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	433	1372	253	20	728	96	52	113	148	265	439	51
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	433	1372	253	20	728	96	52	113	148	265	439	51
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	433	1372	253	20	728	96	52	113	148	265	439	51
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.00	1.00
Lanes:	2.00	3.00	1.00	1.00	2.65	0.35	1.00	1.00	1.00	1.00	0.90	0.10
Final Sat.:	3200	5100	1600	1600	4441	559	1600	1700	1600	1600	1433	167
Capacity Analysis Module:												
Vol/Sat:	0.14	0.27	0.16	0.01	0.16	0.17	0.03	0.07	0.09	0.17	0.31	0.31
Crit Moves:	****					****	****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #107 Red Hill Avenue and I-5 NB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.623
Loss Time (sec):	5	Average Delay (sec/veh):	23.6
Optimal Cycle:	32	Level Of Service:	C

Street Name:	Red Hill Avenue						I-5 NB Ramps					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	3	0	0	0	0	3	0	1	0	0	0	0	0	1	1	0	1	0
Volume Module:												
Base Vol:	292	1670	0	0	865	175	0	0	0	256	96	415
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	292	1670	0	0	865	175	0	0	0	256	96	415
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	292	1670	0	0	865	175	0	0	0	256	96	415
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	292	1670	0	0	865	175	0	0	0	256	96	415
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	292	1670	0	0	865	175	0	0	0	256	96	415
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.91	1.00	1.00	0.91	0.85	1.00	1.00	1.00	0.83	0.83	0.83
Lanes:	1.00	3.00	0.00	0.00	3.00	1.00	0.00	0.00	0.00	1.45	0.55	1.00
Final Sat.:	1805	5187	0	0	5187	1615	0	0	0	2292	860	1576
Capacity Analysis Module:												
Vol/Sat:	0.16	0.32	0.00	0.00	0.17	0.11	0.00	0.00	0.00	0.11	0.11	0.26
Crit Moves:	****				****							****
Green/Cycle:	0.26	0.53	0.00	0.00	0.27	0.27	0.00	0.00	0.00	0.42	0.42	0.42
Volume/Cap:	0.62	0.61	0.00	0.00	0.62	0.40	0.00	0.00	0.00	0.26	0.26	0.62
Delay/Veh:	35.3	16.9	0.0	0.0	33.1	30.7	0.0	0.0	0.0	18.8	18.8	23.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

AdjDel/Veh:	35.3	16.9	0.0	0.0	33.1	30.7	0.0	0.0	0.0	18.8	18.8	23.6
LOS by Move:	D	B	A	A	C	C	A	A	A	B	B	C
HCM2k85thQ:	13	20	0	0	14	7	0	0	0	6	6	17

****

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #108 Red Hill Avenue and I-5 SB Ramps

Cycle (sec):	100	Critical Vol./Cap.(X):	0.713
Loss Time (sec):	5	Average Delay (sec/veh):	21.8
Optimal Cycle:	40	Level Of Service:	C

Street Name:	Red Hill Avenue						I-5 SB Ramps					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0 4	0 1	1	0 3	0 0	0	1 0	0 1	0	0 0	0 0
Volume Module:												
Base Vol:	0	1700	540	310	744	0	296	3	265	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1700	540	310	744	0	296	3	265	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1700	540	310	744	0	296	3	265	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1700	540	310	744	0	296	3	265	0	0	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1700	540	310	744	0	296	3	265	0	0	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	0.91	0.85	0.95	0.91	1.00	0.92	0.92	0.85	1.00	1.00	1.00
Lanes:	0.00	4.00	1.00	1.00	3.00	0.00	0.99	0.01	1.00	0.00	0.00	0.00
Final Sat.:	0	6916	1615	1805	5187	0	1732	18	1615	0	0	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.25	0.33	0.17	0.14	0.00	0.17	0.17	0.16	0.00	0.00	0.00
Crit Moves:			****	****				****				
Green/Cycle:	0.00	0.47	0.47	0.24	0.71	0.00	0.24	0.24	0.24	0.00	0.00	0.00
Volume/Cap:	0.00	0.52	0.71	0.71	0.20	0.00	0.71	0.71	0.68	0.00	0.00	0.00
Delay/Veh:	0.0	18.8	24.4	40.3	4.9	0.0	40.5	40.5	39.6	0.0	0.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	18.8	24.4	40.3	4.9	0.0	40.5	40.5	39.6	0.0	0.0	0.0
LOS by Move:	A	B	C	D	A	A	D	D	D	A	A	A
HCM2k85thQ:	0	15	21	15	4	0	15	15	13	0	0	0

Note: Queue reported is the number of cars per lane.

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #109 Red Hill Avenue and Nisson Road

Cycle (sec):	100	Critical Vol./Cap.(X):	0.715
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	39	Level Of Service:	C

Street Name:	Red Hill Avenue						Nisson Road					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 1	0	1 0	2 1	0	1 0	0 1	0	1 0	0 1	0
Volume Module:												
Base Vol:	34	1769	36	142	730	73	247	90	40	48	83	141
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	34	1769	36	142	730	73	247	90	40	48	83	141
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	34	1769	36	142	730	73	247	90	40	48	83	141
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	34	1769	36	142	730	73	247	90	40	48	83	141
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	34	1769	36	142	730	73	247	90	40	48	83	141
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lanes:	1.00	3.92	0.08	1.00	2.73	0.27	1.00	0.69	0.31	1.00	0.37	0.63
Final Sat.:	1600	6572	128	1600	4564	436	1600	1108	492	1600	593	1007
Capacity Analysis Module:												
Vol/Sat:	0.02	0.27	0.28	0.09	0.16	0.17	0.15	0.08	0.08	0.03	0.14	0.14
Crit Moves:			****	****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #110 Red Hill Avenue and Walnut Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	0.749
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	43	Level Of Service:	C

Street Name:	Red Hill Avenue						Walnut Avenue					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	2	1	0	1	0	2	1	0	1	0	1	0	2	0	1	1	0
Volume Module:												
Base Vol:	283	1378	99	123	382	160	91	218	90	196	654	171
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	283	1378	99	123	382	160	91	218	90	196	654	171
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	283	1378	99	123	382	160	91	218	90	196	654	171
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	283	1378	99	123	382	160	91	218	90	196	654	171
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	283	1378	99	123	382	160	91	218	90	196	654	171
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	2.80	0.20	1.00	2.11	0.89	1.00	1.42	0.58	2.00	1.59	0.41
Final Sat.:	1600	4678	322	1600	3583	1417	1600	2365	935	3200	2637	663
Capacity Analysis Module:												
Vol/Sat:	0.18	0.29	0.31	0.08	0.11	0.11	0.06	0.09	0.10	0.06	0.25	0.26
Crit Moves:			****	****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #111 Red Hill Avenue and Valencia Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	0.753
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	44	Level Of Service:	C

Street Name:	Red Hill Avenue						Valencia Avenue					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	3 1	0	1 0	2 1	0	1 0	1 0	1	2 0	1 0	1
Volume Module:												
Base Vol:	248	1849	305	116	467	52	110	297	130	284	412	272
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	248	1849	305	116	467	52	110	297	130	284	412	272
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	248	1849	305	116	467	52	110	297	130	284	412	272
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	248	1849	305	116	467	52	110	297	130	284	412	272
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	248	1849	305	116	467	52	110	297	130	284	412	272
OvlAdjVol:			225									156
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	3.43	0.57	1.00	2.70	0.30	1.00	1.00	1.00	2.00	1.00	1.00
Final Sat.:	1600	5794	906	1600	4519	481	1600	1700	1600	3200	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.16	0.32	0.34	0.07	0.10	0.11	0.07	0.17	0.08	0.09	0.24	0.17
OvlAdjV/S:			0.25									0.10
Crit Moves:	****			****			****				****	

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #112 Tustin Ranch Road and Warner Avenue North

Cycle (sec):	100	Critical Vol./Cap.(X):	0.924
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	103	Level Of Service:	E

Street Name:	Tustin Ranch Road						Warner Avenue North					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	3 0 1	2	0	3 0 0	0	0	0 0 0	3	0	0 0 2
Volume Module:												
Base Vol:	0	1995	1175	447	657	0	0	0	0	792	0	716
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1995	1175	447	657	0	0	0	0	792	0	716
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1995	1175	447	657	0	0	0	0	792	0	716
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1995	1175	447	657	0	0	0	0	792	0	716
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1995	1175	447	657	0	0	0	0	792	0	716
OvlAdjVol:			911									269
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	0.00	3.00	1.00	2.00	3.00	0.00	0.00	0.00	0.00	3.00	0.00	2.00
Final Sat.:	0	5100	1600	3200	5100	0	0	0	0	4800	0	3200
Capacity Analysis Module:												
Vol/Sat:	0.00	0.39	0.73	0.14	0.13	0.00	0.00	0.00	0.00	0.17	0.00	0.22
OvlAdjV/S:			0.57									0.08
Crit Moves:			****	****						****		

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #113 Tustin Ranch Road and Walnut Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	1.772
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Street Name:	Tustin Ranch Road						Walnut Avenue					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Ovl			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2	0	3	0	1	2	0	3	0	1	2	0	1	1	0	2	0	2	0	1
Volume Module:																				
Base Vol:	876	3814	401	550	1460	947	329	402	230	307	1760	1394
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	876	3814	401	550	1460	947	329	402	230	307	1760	1394
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	876	3814	401	550	1460	947	329	402	230	307	1760	1394
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	876	3814	401	550	1460	947	329	402	230	307	1760	1394
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	876	3814	401	550	1460	947	329	402	230	307	1760	1394
OvlAdjVol:			247			783						1119
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	1.27	0.73	2.00	2.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	2135	1165	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.27	0.75	0.25	0.17	0.29	0.59	0.10	0.19	0.20	0.10	0.52	0.87
OvlAdjV/S:			0.15			0.49						0.70
Crit Moves:	****			****			****					****

Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #114 SR-55 SB Ramps and Irvine Boulevard

Cycle (sec):	100	Critical Vol./Cap.(X):	0.929
Loss Time (sec):	5	Average Delay (sec/veh):	26.5
Optimal Cycle:	111	Level Of Service:	C

Street Name:	SR-55 SB Ramps						Irvine Boulevard					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Permitted			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0	0	0	1	0	1! 0	1	0	0	2	1	0	1	0	3	0	0
Volume Module:												
Base Vol:	0	0	0	484	0	289	0	1049	707	425	1222	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	484	0	289	0	1049	707	425	1222	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	0	0	484	0	289	0	1049	707	425	1222	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	484	0	289	0	1049	707	425	1222	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	0	0	484	0	289	0	1049	707	425	1222	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.74	1.00	0.73	1.00	0.86	0.86	0.95	0.91	1.00
Lanes:	0.00	0.00	0.00	1.62	0.00	1.38	0.00	2.00	1.00	1.00	3.00	0.00
Final Sat.:	0	0	0	2278	0	1915	0	3251	1625	1805	5187	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.21	0.00	0.15	0.00	0.32	0.44	0.24	0.24	0.00
Crit Moves:				****					****	****		
Green/Cycle:	0.00	0.00	0.00	0.23	0.00	0.46	0.00	0.47	0.47	0.25	0.72	0.00
Volume/Cap:	0.00	0.00	0.00	0.93	0.00	0.33	0.00	0.69	0.93	0.93	0.33	0.00
Delay/Veh:	0.0	0.0	0.0	54.5	0.0	17.5	0.0	21.7	33.7	61.9	5.1	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	54.5	0.0	17.5	0.0	21.7	33.7	61.9	5.1	0.0
LOS by Move:	A	A	A	D	A	B	A	C	C	E	A	A

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HCM2k85thQ:     0    0     0    19    0     7     0   22    39    25    8     0
********************************************************************************
Note: Queue reported is the number of cars per lane.
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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #115 SR-55 NB Ramps and Irvine Boulevard

Cycle (sec):	100	Critical Vol./Cap.(X):	0.841
Loss Time (sec):	5	Average Delay (sec/veh):	22.3
Optimal Cycle:	65	Level Of Service:	C

Street Name:	SR-55 NB Ramps						Irvine Boulevard					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Permitted			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1	0	1! 0	1	0	0	0	0	1	0	3	0	0	0	0	2	1	0
Volume Module:																		
Base Vol:	424	21	386	0	0	0	239	1094	0	0	1043	660
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	424	21	386	0	0	0	239	1094	0	0	1043	660
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	424	21	386	0	0	0	239	1094	0	0	1043	660
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	424	21	386	0	0	0	239	1094	0	0	1043	660
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	424	21	386	0	0	0	239	1094	0	0	1043	660
Saturation Flow Module:																		
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.86	0.86	0.86	1.00	1.00	1.00	0.95	0.91	1.00	1.00	0.86	0.86
Lanes:	1.50	0.05	1.45	0.00	0.00	0.00	1.00	3.00	0.00	0.00	2.00	1.00
Final Sat.:	2445	80	2372	0	0	0	1805	5187	0	0	3257	1629
Capacity Analysis Module:																		
Vol/Sat:	0.17	0.26	0.16	0.00	0.00	0.00	0.13	0.21	0.00	0.00	0.32	0.41
Crit Moves:	****						****					****
Green/Cycle:	0.31	0.31	0.31	0.00	0.00	0.00	0.16	0.64	0.00	0.00	0.48	0.48
Volume/Cap:	0.56	0.84	0.52	0.00	0.00	0.00	0.84	0.33	0.00	0.00	0.66	0.84
Delay/Veh:	29.2	38.7	28.7	0.0	0.0	0.0	60.5	8.3	0.0	0.0	20.4	25.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	29.2	38.7	28.7	0.0	0.0	0.0	60.5	8.3	0.0	0.0	20.4	25.9
LOS by Move:	C	D	C	A	A	A	E	A	A	A	C	C
HCM2k85thQ:	12	22	11	0	0	0	15	9	0	0	21	31

Note: Queue reported is the number of cars per lane.

APPENDIX E
MITIGATION WORKSHEETS

Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #3 Euclid St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.995
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 0	2 0	2	1 0	1 0	2	0 1	1 0	2	1 0
Volume Module:												
Base Vol:	127	801	68	180	2613	240	136	595	364	133	812	148
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	127	801	68	180	2613	240	136	595	364	133	812	148
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	127	801	68	180	2613	240	136	595	364	133	812	148
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	127	801	68	180	2613	240	136	595	364	133	812	148
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	127	801	68	180	2613	240	136	595	364	133	812	148
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	127	801	68	180	2613	240	136	595	364	133	812	148
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.05	1.00
Lanes:	2.00	2.77	0.23	2.00	2.75	0.25	1.00	2.00	1.00	1.00	2.54	0.46
Final Sat.:	3200	4624	376	3200	4596	404	1600	3400	1600	1600	4260	740
Capacity Analysis Module:												
Vol/Sat:	0.04	0.17	0.18	0.06	0.57	0.59	0.09	0.17	0.23	0.08	0.19	0.20
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #3 Euclid St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.803
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	53	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 0	2 0	2	1 0	1 0	2	0 1	1 0	2	1 0
Volume Module:												
Base Vol:	551	1648	117	136	897	228	121	467	140	119	1103	179
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	551	1648	117	136	897	228	121	467	140	119	1103	179
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	551	1648	117	136	897	228	121	467	140	119	1103	179
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	551	1648	117	136	897	228	121	467	140	119	1103	179
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	551	1648	117	136	897	228	121	467	140	119	1103	179
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	551	1648	117	136	897	228	121	467	140	119	1103	179
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.05	1.00
Lanes:	2.00	2.80	0.20	2.00	2.39	0.61	1.00	2.00	1.00	1.00	2.58	0.42
Final Sat.:	3200	4682	318	3200	4027	973	1600	3400	1600	1600	4330	670
Capacity Analysis Module:												
Vol/Sat:	0.17	0.35	0.37	0.04	0.22	0.23	0.08	0.14	0.09	0.07	0.25	0.27
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #3 Euclid St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.995
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	2 0	2 1	0	2 0	2 0	1	1 0	2 0	1
Volume Module:												
Base Vol:	127	801	68	180	2613	240	136	595	364	133	812	148
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	127	801	68	180	2613	240	136	595	364	133	812	148
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	127	801	68	180	2613	240	136	595	364	133	812	148
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	127	801	68	180	2613	240	136	595	364	133	812	148
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	127	801	68	180	2613	240	136	595	364	133	812	148
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	127	801	68	180	2613	240	136	595	364	133	812	148
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.77	0.23	2.00	2.75	0.25	2.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	3200	4624	376	3200	4596	404	3200	3400	1600	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.04	0.17	0.18	0.06	0.57	0.59	0.04	0.17	0.23	0.08	0.24	0.09
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #3 Euclid St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.822
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	57	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 0	2 0	2	1 0	2 0	2	0 1	1 0	2	0 1
Volume Module:												
Base Vol:	551	1648	117	136	897	228	121	467	140	119	1103	179
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	551	1648	117	136	897	228	121	467	140	119	1103	179
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	551	1648	117	136	897	228	121	467	140	119	1103	179
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	551	1648	117	136	897	228	121	467	140	119	1103	179
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	551	1648	117	136	897	228	121	467	140	119	1103	179
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	551	1648	117	136	897	228	121	467	140	119	1103	179
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.80	0.20	2.00	2.39	0.61	2.00	2.00	1.00	1.00	2.00	1.00
Final Sat.:	3200	4682	318	3200	4027	973	3200	3400	1600	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.17	0.35	0.37	0.04	0.22	0.23	0.04	0.14	0.09	0.07	0.32	0.11
Crit Moves:			****	****			****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #3 Euclid St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.965
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	154	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 0	2 0	2	1 0	2 0	2	0 1	2 0	2	0 1
Volume Module:												
Base Vol:	127	801	68	180	2613	240	136	595	364	133	812	148
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	127	801	68	180	2613	240	136	595	364	133	812	148
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	127	801	68	180	2613	240	136	595	364	133	812	148
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	127	801	68	180	2613	240	136	595	364	133	812	148
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	127	801	68	180	2613	240	136	595	364	133	812	148
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	127	801	68	180	2613	240	136	595	364	133	812	148
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.77	0.23	2.00	2.75	0.25	2.00	2.00	1.00	2.00	2.00	1.00
Final Sat.:	3200	4624	376	3200	4596	404	3200	3400	1600	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.04	0.17	0.18	0.06	0.57	0.59	0.04	0.17	0.23	0.04	0.24	0.09
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #3 Euclid St and Edinger Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.822
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	57	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 0	2 0	2	1 0	2 0	2	0 1	2 0	2	0 1
Volume Module:												
Base Vol:	551	1648	117	136	897	228	121	467	140	119	1103	179
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	551	1648	117	136	897	228	121	467	140	119	1103	179
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	551	1648	117	136	897	228	121	467	140	119	1103	179
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	551	1648	117	136	897	228	121	467	140	119	1103	179
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	551	1648	117	136	897	228	121	467	140	119	1103	179
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	551	1648	117	136	897	228	121	467	140	119	1103	179
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.80	0.20	2.00	2.39	0.61	2.00	2.00	1.00	2.00	2.00	1.00
Final Sat.:	3200	4682	318	3200	4027	973	3200	3400	1600	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.17	0.35	0.37	0.04	0.22	0.23	0.04	0.14	0.09	0.04	0.32	0.11
Crit Moves:			****	****			****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #13 Harbor Blvd and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.961
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	147	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	1 0	2 1	0	1 0	1 1	0	2 0	2 0	1
Volume Module:												
Base Vol:	107	759	48	173	2324	87	90	753	303	146	453	103
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	107	759	48	173	2324	87	90	753	303	146	453	103
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	107	759	48	173	2324	87	90	753	303	146	453	103
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	107	759	48	173	2324	87	90	753	303	146	453	103
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	107	759	48	173	2324	87	90	753	303	146	453	103
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	2.82	0.18	1.00	2.89	0.11	1.00	1.43	0.57	2.00	2.00	1.00
Final Sat.:	3200	4714	286	1600	4827	173	1600	2382	918	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.16	0.17	0.11	0.48	0.50	0.06	0.32	0.33	0.05	0.13	0.06
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #13 Harbor Blvd and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.934
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	113	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	1 0	2 1	0	1 0	1 1	0	2 0	2 0	1
Volume Module:												
Base Vol:	275	1732	55	86	973	86	118	585	192	121	1308	475
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	275	1732	55	86	973	86	118	585	192	121	1308	475
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	275	1732	55	86	973	86	118	585	192	121	1308	475
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	275	1732	55	86	973	86	118	585	192	121	1308	475
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	275	1732	55	86	973	86	118	585	192	121	1308	475
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.06	1.00
Lanes:	2.00	2.91	0.09	1.00	2.76	0.24	1.00	1.51	0.49	2.00	2.00	1.00
Final Sat.:	3200	4852	148	1600	4610	390	1600	2509	791	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.09	0.36	0.37	0.05	0.21	0.22	0.07	0.23	0.24	0.04	0.38	0.30
Crit Moves:			****	****			****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #34 Bristol St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.788
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	50	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	1 0	2 1	1	1 0	2 1	0
Volume Module:												
Base Vol:	208	1451	184	315	1298	183	206	1268	128	126	949	139
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	208	1451	184	315	1298	183	206	1268	128	126	949	139
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	208	1451	184	315	1298	183	206	1268	128	126	949	139
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	208	1451	184	315	1298	183	206	1268	128	126	949	139
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	208	1451	184	315	1298	183	206	1268	128	126	949	139
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	1.00	3.00	1.00	1.00	2.62	0.38
Final Sat.:	3200	5100	1600	3200	5100	1600	1600	5100	1600	1600	4387	613
Capacity Analysis Module:												
Vol/Sat:	0.07	0.28	0.12	0.10	0.25	0.11	0.13	0.25	0.08	0.08	0.22	0.23
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #34 Bristol St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.867
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	71	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	1 0	2 1	1	1 0	2 1	0
Volume Module:												
Base Vol:	434	1872	276	216	1169	264	259	1454	186	155	953	90
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	434	1872	276	216	1169	264	259	1454	186	155	953	90
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	434	1872	276	216	1169	264	259	1454	186	155	953	90
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	434	1872	276	216	1169	264	259	1454	186	155	953	90
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	434	1872	276	216	1169	264	259	1454	186	155	953	90
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	1.00	3.00	1.00	1.00	2.74	0.26
Final Sat.:	3200	5100	1600	3200	5100	1600	1600	5100	1600	1600	4586	414
Capacity Analysis Module:												
Vol/Sat:	0.14	0.37	0.17	0.07	0.23	0.17	0.16	0.29	0.12	0.10	0.21	0.22
Crit Moves:		****			****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #37 Bristol St and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.814
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	55	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 1	1 0	2	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	70	778	146	283	1011	119	306	1480	136	157	712	91
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	70	778	146	283	1011	119	306	1480	136	157	712	91
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	70	778	146	283	1011	119	306	1480	136	157	712	91
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	70	778	146	283	1011	119	306	1480	136	157	712	91
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	70	778	146	283	1011	119	306	1480	136	157	712	91
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	3.00	1.00	1.00	2.68	0.32	1.00	2.75	0.25	1.00	2.66	0.34
Final Sat.:	1600	5100	1600	1600	4495	505	1600	4596	404	1600	4456	544
Capacity Analysis Module:												
Vol/Sat:	0.04	0.15	0.09	0.18	0.22	0.24	0.19	0.32	0.34	0.10	0.16	0.17
Crit Moves:		****		****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #37 Bristol St and Segerstrom Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.868
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	72	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	WideBypass			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 1	1 0	2	1 0	1 0	2	1 0	1 0	2	1 0
Volume Module:												
Base Vol:	177	1309	231	98	894	187	279	998	79	156	1408	71
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	177	1309	231	98	894	187	279	998	79	156	1408	71
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	177	1309	231	98	894	187	279	998	79	156	1408	71
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	177	1309	231	98	894	187	279	998	79	156	1408	71
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	177	1309	231	98	894	187	279	998	79	156	1408	71
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	3.00	1.00	1.00	2.48	0.52	1.00	2.78	0.22	1.00	2.86	0.14
Final Sat.:	1600	5100	1600	1600	4170	830	1600	4648	352	1600	4770	230
Capacity Analysis Module:												
Vol/Sat:	0.11	0.26	0.14	0.06	0.21	0.23	0.17	0.21	0.22	0.10	0.30	0.31
Crit Moves:	****					****	****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #44 Flower St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.925
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	104	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	1	1 0	1 1	0	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	89	387	102	366	1158	121	107	983	39	161	664	95
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	89	387	102	366	1158	121	107	983	39	161	664	95
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	89	387	102	366	1158	121	107	983	39	161	664	95
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	89	387	102	366	1158	121	107	983	39	161	664	95
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	89	387	102	366	1158	121	107	983	39	161	664	95
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	89	387	102	366	1158	121	107	983	39	161	664	95
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.03	1.00	1.00	1.03	1.00	1.00	1.04	1.00
Lanes:	1.00	2.00	1.00	1.00	1.81	0.19	1.00	1.92	0.08	1.00	1.75	0.25
Final Sat.:	1600	3400	1600	1600	2997	303	1600	3178	122	1600	2899	401
Capacity Analysis Module:												
Vol/Sat:	0.06	0.11	0.06	0.23	0.39	0.40	0.07	0.31	0.32	0.10	0.23	0.24
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #44 Flower St and 1st St

Cycle (sec):	100	Critical Vol./Cap.(X):	0.921
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	101	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 0	1	1 0	1 1	0	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	211	1000	238	140	383	111	135	1181	98	144	969	101
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	211	1000	238	140	383	111	135	1181	98	144	969	101
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	211	1000	238	140	383	111	135	1181	98	144	969	101
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	211	1000	238	140	383	111	135	1181	98	144	969	101
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	211	1000	238	140	383	111	135	1181	98	144	969	101
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	211	1000	238	140	383	111	135	1181	98	144	969	101
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.03	1.00	1.00	1.03	1.00
Lanes:	1.00	2.00	1.00	1.00	1.55	0.45	1.00	1.85	0.15	1.00	1.81	0.19
Final Sat.:	1600	3400	1600	1600	2581	719	1600	3055	245	1600	2998	302
Capacity Analysis Module:												
Vol/Sat:	0.13	0.29	0.15	0.09	0.15	0.15	0.08	0.39	0.40	0.09	0.32	0.33
Crit Moves:		****		****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #62 Santiago St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	1.241
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1 0	1	1 0	1 0	1	0 1	0 1	0	1 0	2 0	1
Volume Module:												
Base Vol:	188	611	303	409	972	253	118	732	99	297	699	258
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	188	611	303	409	972	253	118	732	99	297	699	258
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	188	611	303	409	972	253	118	732	99	297	699	258
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	188	611	303	409	972	253	118	732	99	297	699	258
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	188	611	303	409	972	253	118	732	99	297	699	258
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.00	1.00	1.00	1.06	1.00
Lanes:	1.00	1.00	1.00	1.00	1.00	1.00	0.25	1.54	0.21	1.00	2.00	1.00
Final Sat.:	1600	1700	1600	1600	1700	1600	398	2468	334	1600	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.12	0.36	0.19	0.26	0.57	0.16	0.30	0.30	0.30	0.19	0.21	0.16
Crit Moves:	****				****			****			****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #62 Santiago St and Santa Ana Blvd

Cycle (sec):	100	Critical Vol./Cap.(X):	1.282
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound				South Bound				East Bound				West Bound			
Movement:	L	-	T	- R	L	-	T	- R	L	-	T	- R	L	-	T	- R
Control:	Protected				Protected				Split Phase				Split Phase			
Rights:	Include				Include				Include				Include			
Min. Green:	0	0	0		0	0	0		0	0	0		0	0	0	
Y+R:	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0	4.0	
Lanes:	1	0	1	0 1	1	0	1	0 1	0	1	0	1 0	1	0	2	0 1
Volume Module:																
Base Vol:	135	833	236		378	639	103		164	673	87		255	546	347	
Growth Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
Initial Bse:	135	833	236		378	639	103		164	673	87		255	546	347	
User Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
PHF Volume:	135	833	236		378	639	103		164	673	87		255	546	347	
Reduct Vol:	0	0	0		0	0	0		0	0	0		0	0	0	
Reduced Vol:	135	833	236		378	639	103		164	673	87		255	546	347	
PCE Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
MLF Adj:	1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00		1.00	1.00	1.00	
FinalVolume:	135	833	236		378	639	103		164	673	87		255	546	347	
Saturation Flow Module:																
Sat/Lane:	1600	1600	1600		1600	1600	1600		1600	1600	1600		1600	1600	1600	
Adjustment:	1.00	1.06	1.00		1.00	1.06	1.00		1.00	1.00	1.00		1.00	1.06	1.00	
Lanes:	1.00	1.00	1.00		1.00	1.00	1.00		0.35	1.46	0.19		1.00	2.00	1.00	
Final Sat.:	1600	1700	1600		1600	1700	1600		568	2331	301		1600	3400	1600	
Capacity Analysis Module:																
Vol/Sat:	0.08	0.49	0.15		0.24	0.38	0.06		0.29	0.29	0.29		0.16	0.16	0.22	
Crit Moves:		****			****						****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #63 Standard Ave and 4th St

Cycle (sec):	100	Critical Vol./Cap.(X):	1.062
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1 0	1	1 0	1 0	1	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	191	473	248	431	604	332	298	624	219	250	550	340
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	191	473	248	431	604	332	298	624	219	250	550	340
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	191	473	248	431	604	332	298	624	219	250	550	340
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	191	473	248	431	604	332	298	624	219	250	550	340
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	191	473	248	431	604	332	298	624	219	250	550	340
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	191	473	248	431	604	332	298	624	219	250	550	340
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.48	0.52	1.00	1.24	0.76
Final Sat.:	1600	1700	1600	1600	1700	1600	1600	2469	831	1600	2078	1222
Capacity Analysis Module:												
Vol/Sat:	0.12	0.28	0.16	0.27	0.36	0.21	0.19	0.25	0.26	0.16	0.26	0.28
Crit Moves:	****			****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #63 Standard Ave and 4th St

Cycle (sec):	100	Critical Vol./Cap.(X):	1.029
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	1 0	1	1 0	1 0	1	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	251	543	214	254	428	297	302	477	201	233	648	351
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	251	543	214	254	428	297	302	477	201	233	648	351
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	251	543	214	254	428	297	302	477	201	233	648	351
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	251	543	214	254	428	297	302	477	201	233	648	351
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	251	543	214	254	428	297	302	477	201	233	648	351
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	251	543	214	254	428	297	302	477	201	233	648	351
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.04	1.00	1.00	1.05	1.00
Lanes:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.41	0.59	1.00	1.30	0.70
Final Sat.:	1600	1700	1600	1600	1700	1600	1600	2351	949	1600	2176	1124
Capacity Analysis Module:												
Vol/Sat:	0.16	0.32	0.13	0.16	0.25	0.19	0.19	0.20	0.21	0.15	0.30	0.31
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #81 Grand Ave and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.861
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	69	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	1 0	2	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	262	1104	165	294	1974	192	109	352	107	95	340	86
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	262	1104	165	294	1974	192	109	352	107	95	340	86
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	262	1104	165	294	1974	192	109	352	107	95	340	86
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	262	1104	165	294	1974	192	109	352	107	95	340	86
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	262	1104	165	294	1974	192	109	352	107	95	340	86
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	2.61	0.39	1.00	2.73	0.27	1.00	1.53	0.47	1.00	1.60	0.40
Final Sat.:	1600	4376	624	1600	4575	425	1600	2554	746	1600	2654	646
Capacity Analysis Module:												
Vol/Sat:	0.16	0.25	0.26	0.18	0.43	0.45	0.07	0.14	0.14	0.06	0.13	0.13
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #81 Grand Ave and McFadden Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.813
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	55	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2 1	0	1 0	2 1	0	1 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	232	1320	227	176	800	215	64	238	42	96	762	168
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	232	1320	227	176	800	215	64	238	42	96	762	168
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	232	1320	227	176	800	215	64	238	42	96	762	168
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	232	1320	227	176	800	215	64	238	42	96	762	168
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	232	1320	227	176	800	215	64	238	42	96	762	168
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.04	1.00
Lanes:	1.00	2.56	0.44	1.00	2.36	0.64	1.00	1.70	0.30	1.00	1.64	0.36
Final Sat.:	1600	4296	704	1600	3983	1017	1600	2820	480	1600	2722	578
Capacity Analysis Module:												
Vol/Sat:	0.15	0.31	0.32	0.11	0.20	0.21	0.04	0.08	0.09	0.06	0.28	0.29
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #91 Tustin Ave and Fairhaven Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	1.056
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 0	2 0	2	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	125	720	126	249	2148	42	78	297	511	306	338	442
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	125	720	126	249	2148	42	78	297	511	306	338	442
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	125	720	126	249	2148	42	78	297	511	306	338	442
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	125	720	126	249	2148	42	78	297	511	306	338	442
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	125	720	126	249	2148	42	78	297	511	306	338	442
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	125	720	126	249	2148	42	78	297	511	306	338	442
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.55	0.45	2.00	2.94	0.06	1.00	1.00	1.00	1.00	1.00	1.00
Final Sat.:	3200	4285	715	3200	4908	92	1600	1700	1600	1600	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.04	0.17	0.18	0.08	0.44	0.46	0.05	0.17	0.32	0.19	0.20	0.28
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #91 Tustin Ave and Fairhaven Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.641
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	32	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2	1 0	2 0	2	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	258	1377	122	356	940	108	82	185	151	86	191	180
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	258	1377	122	356	940	108	82	185	151	86	191	180
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	258	1377	122	356	940	108	82	185	151	86	191	180
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	258	1377	122	356	940	108	82	185	151	86	191	180
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	258	1377	122	356	940	108	82	185	151	86	191	180
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	258	1377	122	356	940	108	82	185	151	86	191	180
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	2.76	0.24	2.00	2.69	0.31	1.00	1.10	0.90	1.00	1.03	0.97
Final Sat.:	3200	4609	391	3200	4505	495	1600	1862	1438	1600	1747	1553
Capacity Analysis Module:												
Vol/Sat:	0.08	0.30	0.31	0.11	0.21	0.22	0.05	0.10	0.10	0.05	0.11	0.12
Crit Moves:			****	****			****					****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #92 Tustin Ave and Santa Clara Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.938
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	116	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	2 0	2	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	102	556	104	135	2756	87	116	293	214	118	244	122
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	102	556	104	135	2756	87	116	293	214	118	244	122
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	102	556	104	135	2756	87	116	293	214	118	244	122
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	102	556	104	135	2756	87	116	293	214	118	244	122
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	102	556	104	135	2756	87	116	293	214	118	244	122
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	102	556	104	135	2756	87	116	293	214	118	244	122
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	1.00	2.53	0.47	2.00	2.91	0.09	1.00	1.16	0.84	1.00	1.33	0.67
Final Sat.:	1600	4244	756	3200	4853	147	1600	1949	1351	1600	2233	1067
Capacity Analysis Module:												
Vol/Sat:	0.06	0.13	0.14	0.04	0.57	0.59	0.07	0.15	0.16	0.07	0.11	0.11
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #92 Tustin Ave and Santa Clara Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.681
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	35	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Permitted			Permitted		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	2	1 0	2 0	2	1 0	1 0	1	1 0	1 0	1	1 0
Volume Module:												
Base Vol:	176	852	131	199	1574	109	63	198	116	115	144	136
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	176	852	131	199	1574	109	63	198	116	115	144	136
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	176	852	131	199	1574	109	63	198	116	115	144	136
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	176	852	131	199	1574	109	63	198	116	115	144	136
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	176	852	131	199	1574	109	63	198	116	115	144	136
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	176	852	131	199	1574	109	63	198	116	115	144	136
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.04	1.00	1.00	1.05	1.00	1.00	1.06	1.00
Lanes:	1.00	2.60	0.40	2.00	2.81	0.19	1.00	1.26	0.74	1.00	1.03	0.97
Final Sat.:	1600	4360	640	3200	4689	311	1600	2118	1182	1600	1746	1554
Capacity Analysis Module:												
Vol/Sat:	0.11	0.20	0.20	0.06	0.34	0.35	0.04	0.09	0.10	0.07	0.08	0.09
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #96 SR-55 NB Ramps / Del Amo Ave and Newport Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	0.460
Loss Time (sec):	5	Average Delay (sec/veh):	24.4
Optimal Cycle:	23	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0 2		1 0	1 0 3		0 2	2 0 1		1 0	1 0 1		1 0
Volume Module:												
Base Vol:	413	109	69	20	497	728	269	163	155	8	223	4
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	413	109	69	20	497	728	269	163	155	8	223	4
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	413	109	69	20	497	728	269	163	155	8	223	4
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	413	109	69	20	497	728	269	163	155	8	223	4
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	413	109	69	20	497	728	269	163	155	8	223	4
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.86	0.86	0.95	0.91	0.75	0.92	0.88	0.88	0.95	0.95	0.95
Lanes:	2.00	2.00	1.00	1.00	3.00	2.00	2.00	1.03	0.97	1.00	1.96	0.04
Final Sat.:	3502	3257	1629	1805	5187	2842	3502	1715	1631	1805	3536	63
Capacity Analysis Module:												
Vol/Sat:	0.12	0.03	0.04	0.01	0.10	0.26	0.08	0.10	0.10	0.00	0.06	0.06
Crit Moves:	****					****	****				****	
Green/Cycle:	0.26	0.51	0.51	0.13	0.39	0.56	0.17	0.29	0.29	0.01	0.14	0.14
Volume/Cap:	0.46	0.07	0.08	0.08	0.25	0.46	0.46	0.33	0.33	0.33	0.46	0.46
Delay/Veh:	31.7	12.3	12.4	38.1	20.7	13.4	38.2	28.0	28.0	56.5	40.4	40.4
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	31.7	12.3	12.4	38.1	20.7	13.4	38.2	28.0	28.0	56.5	40.4	40.4
LOS by Move:	C	B	B	D	C	B	D	C	C	E	D	D
HCM2k85thQ:	9	1	2	1	6	12	7	6	6	1	6	6

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #96 SR-55 NB Ramps / Del Amo Ave and Newport Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	0.700
Loss Time (sec):	5	Average Delay (sec/veh):	28.5
Optimal Cycle:	39	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	2 1	0	1 0	3 0	2	2 0	1 1	0	1 0	1 1	0
Volume Module:												
Base Vol:	879	262	51	19	231	889	165	57	33	23	348	16
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	879	262	51	19	231	889	165	57	33	23	348	16
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	879	262	51	19	231	889	165	57	33	23	348	16
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	879	262	51	19	231	889	165	57	33	23	348	16
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	879	262	51	19	231	889	165	57	33	23	348	16
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	0.89	0.89	0.95	0.91	0.75	0.92	0.90	0.90	0.95	0.94	0.94
Lanes:	2.00	2.51	0.49	1.00	3.00	2.00	2.00	1.27	0.73	1.00	1.91	0.09
Final Sat.:	3502	4238	825	1805	5187	2842	3502	2161	1251	1805	3427	158
Capacity Analysis Module:												
Vol/Sat:	0.25	0.06	0.06	0.01	0.04	0.31	0.05	0.03	0.03	0.01	0.10	0.10
Crit Moves:	****					****	****				****	
Green/Cycle:	0.36	0.63	0.63	0.11	0.38	0.45	0.07	0.14	0.14	0.07	0.14	0.14
Volume/Cap:	0.70	0.10	0.10	0.10	0.12	0.70	0.70	0.18	0.18	0.18	0.70	0.70
Delay/Veh:	29.3	7.3	7.3	40.5	20.2	24.1	54.7	37.9	37.9	44.6	44.9	44.9
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	29.3	7.3	7.3	40.5	20.2	24.1	54.7	37.9	37.9	44.6	44.9	44.9
LOS by Move:	C	A	A	D	C	C	D	D	D	D	D	D
HCM2k85thQ:	19	2	2	1	3	20	6	2	2	1	11	11

Note: Queue reported is the number of cars per lane.

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #98 Red Hill Ave and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.559
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	26	Level Of Service:	A

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 1	0	2 0	2 1	0	2 0	2 1	0	2 0	3 1	0
Volume Module:												
Base Vol:	118	716	97	20	949	191	239	407	300	152	543	96
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	118	716	97	20	949	191	239	407	300	152	543	96
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	118	716	97	20	949	191	239	407	300	152	543	96
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	118	716	97	20	949	191	239	407	300	152	543	96
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	118	716	97	20	949	191	239	407	300	152	543	96
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	118	716	97	20	949	191	239	407	300	152	543	96
OvlAdjVol:												90
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.52	0.48	2.00	2.50	0.50	2.00	2.00	1.00	2.00	3.40	0.60
Final Sat.:	3200	5936	764	3200	4196	804	3200	3400	1600	3200	5738	962
Capacity Analysis Module:												
Vol/Sat:	0.04	0.12	0.13	0.01	0.23	0.24	0.07	0.12	0.19	0.05	0.09	0.10
OvlAdjV/S:												0.09
Crit Moves:	****					****			****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #98 Red Hill Ave and Warner Ave

Cycle (sec):	100	Critical Vol./Cap.(X):	0.804
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	53	Level Of Service:	D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 1	0	2 0	2 1	0	2 0	2 1	0	2 0	3 1	0
Volume Module:												
Base Vol:	343	1175	135	89	341	432	324	880	137	78	1643	216
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	343	1175	135	89	341	432	324	880	137	78	1643	216
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	343	1175	135	89	341	432	324	880	137	78	1643	216
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	343	1175	135	89	341	432	324	880	137	78	1643	216
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	343	1175	135	89	341	432	324	880	137	78	1643	216
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	343	1175	135	89	341	432	324	880	137	78	1643	216
OvlAdjVol:												195
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.05	1.00	1.00	1.05	1.00
Lanes:	2.00	3.59	0.41	2.00	2.00	1.00	2.00	2.60	0.40	2.00	3.54	0.46
Final Sat.:	3200	6040	660	3200	3400	1600	3200	4353	647	3200	5956	744
Capacity Analysis Module:												
Vol/Sat:	0.11	0.19	0.20	0.03	0.10	0.27	0.10	0.20	0.21	0.02	0.28	0.29
OvlAdjV/S:												0.26
Crit Moves:	****					****	****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #100 Red Hill Ave and Alton Pkwy

Cycle (sec):	100	Critical Vol./Cap.(X):	0.684
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	36	Level Of Service:	B

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Ignore			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	4 0	1	1 0	3 0	1	2 0	2 1	1	2 0	1 1	0
Volume Module:												
Base Vol:	235	693	434	233	1059	170	185	574	574	335	284	159
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	235	693	434	233	1059	170	185	574	574	335	284	159
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	235	693	434	233	1059	170	185	574	574	335	284	159
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Volume:	235	693	434	233	1059	170	185	574	0	335	284	159
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	235	693	434	233	1059	170	185	574	0	335	284	159
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
FinalVolume:	235	693	434	233	1059	170	185	574	0	335	284	159
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00
Lanes:	1.00	4.00	1.00	1.00	3.00	1.00	2.00	3.00	1.00	2.00	1.28	0.72
Final Sat.:	1600	6800	1600	1600	5100	1600	3200	5100	1600	3200	2151	1149
Capacity Analysis Module:												
Vol/Sat:	0.15	0.10	0.27	0.15	0.21	0.11	0.06	0.11	0.00	0.10	0.13	0.14
Crit Moves:			****	****				****		****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #100 Red Hill Ave and Alton Pkwy

Cycle (sec):	100	Critical Vol./Cap.(X):	1.278
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Include			Ignore			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	1 0	4 0	1	1 0	3 0	1	2 0	2 1	1	2 0	1 1	0
Volume Module:												
Base Vol:	27	1513	219	148	628	5	453	1111	569	2003	342	1235
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	27	1513	219	148	628	5	453	1111	569	2003	342	1235
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	27	1513	219	148	628	5	453	1111	569	2003	342	1235
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
PHF Volume:	27	1513	219	148	628	5	453	1111	0	2003	342	1235
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	27	1513	219	148	628	5	453	1111	0	2003	342	1235
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00
FinalVolume:	27	1513	219	148	628	5	453	1111	0	2003	342	1235
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	1.00	4.00	1.00	1.00	3.00	1.00	2.00	3.00	1.00	2.00	1.00	1.00
Final Sat.:	1600	6800	1600	1600	5100	1600	3200	5100	1600	3200	1700	1600
Capacity Analysis Module:												
Vol/Sat:	0.02	0.22	0.14	0.09	0.12	0.00	0.14	0.22	0.00	0.63	0.20	0.77
Crit Moves:		****			****			****				****

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #105 Von Karman Ave and Barranca Pkwy

Cycle (sec):	100	Critical Vol./Cap.(X):	0.930
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	108	Level Of Service:	E

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	1	1 0	2 0	2 0	2	2 0	3 0	1	2 0	3 1	1
Volume Module:												
Base Vol:	375	497	298	98	960	420	247	928	310	917	1506	47
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	375	497	298	98	960	420	247	928	310	917	1506	47
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	375	497	298	98	960	420	247	928	310	917	1506	47
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	375	497	298	98	960	420	247	928	310	917	1506	47
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	375	497	298	98	960	420	247	928	310	917	1506	47
OvlAdjVol:						173						
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.05	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	1.25	0.75	2.00	2.00	2.00	2.00	3.00	1.00	2.00	4.00	1.00
Final Sat.:	3200	2101	1199	3200	3400	3200	3200	5100	1600	3200	6800	1600
Capacity Analysis Module:												
Vol/Sat:	0.12	0.24	0.25	0.03	0.28	0.13	0.08	0.18	0.19	0.29	0.22	0.03
OvlAdjV/S:						0.05						
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Base Volume Alternative)

Intersection #105 Von Karman Ave and Barranca Pkwy

Cycle (sec):	100	Critical Vol./Cap.(X):	1.241
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Include			Ovl			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	1	1 0	2 0	2 0	2	2 0	3 0	1	2 0	3	1 1
Volume Module:												
Base Vol:	362	2082	454	80	407	280	785	1435	201	254	805	274
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	362	2082	454	80	407	280	785	1435	201	254	805	274
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	362	2082	454	80	407	280	785	1435	201	254	805	274
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	362	2082	454	80	407	280	785	1435	201	254	805	274
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	362	2082	454	80	407	280	785	1435	201	254	805	274
OvlAdjVol:						0						
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.04	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.05	1.00
Lanes:	2.00	1.64	0.36	2.00	2.00	2.00	2.00	3.00	1.00	2.00	3.73	1.27
Final Sat.:	3200	2727	573	3200	3400	3200	3200	5100	1600	3200	6268	2032
Capacity Analysis Module:												
Vol/Sat:	0.11	0.76	0.79	0.03	0.12	0.09	0.25	0.28	0.13	0.08	0.13	0.13
OvlAdjV/S:						0.00						
Crit Moves:		****	****				****				****	

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #112 Tustin Ranch Road and Warner Avenue North

Cycle (sec):	100	Critical Vol./Cap.(X):	0.459
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	22	Level Of Service:	A

Street Name:	Tustin Ranch Road						Warner Avenue North					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	3 0	2	2 0	3 0	0	0 0	0 0	0	3 0	0 0	2
Volume Module:												
Base Vol:	0	396	383	412	1165	0	0	0	0	865	0	255
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	396	383	412	1165	0	0	0	0	865	0	255
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	396	383	412	1165	0	0	0	0	865	0	255
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	396	383	412	1165	0	0	0	0	865	0	255
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	396	383	412	1165	0	0	0	0	865	0	255
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	396	383	412	1165	0	0	0	0	865	0	255
OvlAdjVol:			0									0
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	0.00	3.00	2.00	2.00	3.00	0.00	0.00	0.00	0.00	3.00	0.00	2.00
Final Sat.:	0	5100	3200	3200	5100	0	0	0	0	4800	0	3200
Capacity Analysis Module:												
Vol/Sat:	0.00	0.08	0.12	0.13	0.23	0.00	0.00	0.00	0.00	0.18	0.00	0.08
OvlAdjV/S:			0.00									0.00
Crit Moves:	****			****						****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #112 Tustin Ranch Road and Warner Avenue North

Cycle (sec):	100	Critical Vol./Cap.(X):	0.746
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	43	Level Of Service:	C

Street Name:	Tustin Ranch Road						Warner Avenue North					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Ovl			Include			Include			Ovl		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	0 0	3 0	2	2 0	3 0	0	0 0	0 0	0	3 0	0 0	2
Volume Module:												
Base Vol:	0	1995	1175	447	657	0	0	0	0	792	0	716
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	1995	1175	447	657	0	0	0	0	792	0	716
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	0	1995	1175	447	657	0	0	0	0	792	0	716
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Volume:	0	1995	1175	447	657	0	0	0	0	792	0	716
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	1995	1175	447	657	0	0	0	0	792	0	716
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	0	1995	1175	447	657	0	0	0	0	792	0	716
OvlAdjVol:			647									269
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	0.00	3.00	2.00	2.00	3.00	0.00	0.00	0.00	0.00	3.00	0.00	2.00
Final Sat.:	0	5100	3200	3200	5100	0	0	0	0	4800	0	3200
Capacity Analysis Module:												
Vol/Sat:	0.00	0.39	0.37	0.14	0.13	0.00	0.00	0.00	0.00	0.17	0.00	0.22
OvlAdjV/S:			0.20									0.08
Crit Moves:	****			****						****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #113 Tustin Ranch Road and Walnut Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	1.175
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Street Name:	Tustin Ranch Road						Walnut Avenue					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Ovl			Include			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	2 0	2 0	1	2 0	2 0	1
Volume Module:												
Base Vol:	103	630	314	1123	3139	418	856	1118	675	176	260	219
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	103	630	314	1123	3139	418	856	1118	675	176	260	219
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	103	630	314	1123	3139	418	856	1118	675	176	260	219
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Volume:	103	630	314	1123	3139	418	856	1118	675	176	260	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	103	630	314	1123	3139	418	856	1118	675	176	260	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
FinalVolume:	103	630	314	1123	3139	418	856	1118	675	176	260	0
OvlAdjVol:			226			0						
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	2.00	1.00	2.00	2.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	3400	1600	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.03	0.12	0.20	0.35	0.62	0.26	0.27	0.33	0.42	0.06	0.08	0.00
OvlAdjV/S:			0.14			0.00						
Crit Moves:	****			****					****	****		

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Santa Ana Circulation Element

Level Of Service Computation Report
ICU 1(Loss as Cycle Length %) Method (Future Volume Alternative)

Intersection #113 Tustin Ranch Road and Walnut Avenue

Cycle (sec):	100	Critical Vol./Cap.(X):	1.590
Loss Time (sec):	5	Average Delay (sec/veh):	xxxxxx
Optimal Cycle:	180	Level Of Service:	F

Street Name:	Tustin Ranch Road						Walnut Avenue					
Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Protected			Protected		
Rights:	Ovl			Ovl			Include			Ignore		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lanes:	2 0	3 0	1	2 0	3 0	1	2 0	2 0	1	2 0	2 0	1
Volume Module:												
Base Vol:	876	3814	401	550	1460	947	329	402	230	307	1760	1394
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	876	3814	401	550	1460	947	329	402	230	307	1760	1394
Added Vol:	0	0	0	0	0	0	0	0	0	0	0	0
PasserByVol:	0	0	0	0	0	0	0	0	0	0	0	0
Initial Fut:	876	3814	401	550	1460	947	329	402	230	307	1760	1394
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
PHF Volume:	876	3814	401	550	1460	947	329	402	230	307	1760	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	876	3814	401	550	1460	947	329	402	230	307	1760	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.00
FinalVolume:	876	3814	401	550	1460	947	329	402	230	307	1760	0
OvlAdjVol:			247			783						
Saturation Flow Module:												
Sat/Lane:	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600	1600
Adjustment:	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00	1.00	1.06	1.00
Lanes:	2.00	3.00	1.00	2.00	3.00	1.00	2.00	2.00	1.00	2.00	2.00	1.00
Final Sat.:	3200	5100	1600	3200	5100	1600	3200	3400	1600	3200	3400	1600
Capacity Analysis Module:												
Vol/Sat:	0.27	0.75	0.25	0.17	0.29	0.59	0.10	0.12	0.14	0.10	0.52	0.00
OvlAdjV/S:			0.15			0.49						
Crit Moves:	****			****			****				****	

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APPENDIX F
VMT CALCULATIONS

Base Year 2016										
SCENARIO	I-I	I-X	X-I	Total VMT	TOT_POP	Total Employees	VMT/Capita	TOT_POP	Total Employees - Reduced	VMT / Capita
Citywide	698,590	5,295,496	5,291,968	11,286,054	364,042	176,354	20.9	364,042	139,672	22.4
Countywide	47,482,030	24,809,328	24,994,227	97,285,585	3,179,626	1,710,147	19.9	3,179,626	593,421	25.8

Existing Year 2020										
SCENARIO	I-I	I-X	X-I	Total VMT	TOT_POP	Total Employees	VMT/Capita	TOT_POP	Total Employees - Reduced	VMT / Capita
Citywide	697,779	5,356,504	5,352,841	11,407,124	367,064	177,854	20.9	367,064	140,860	22.5
Countywide	48,242,429	25,463,887	25,637,825	99,344,141	3,228,591	1,747,428	20.0	3,228,591	606,358	25.9

Future Year 2045 Baseline										
SCENARIO	I-I	I-X	X-I	Total VMT	TOT_POP	Total Employees	VMT/Capita	TOT_POP	Total Employees - Reduced	VMT / Capita
Citywide	692,704	5,737,798	5,733,292	12,163,794	385,955	187,226	21.2	385,955	148,283	22.8
Countywide	52,994,916	29,554,879	29,660,308	112,210,103	3,534,620	1,980,433	20.3	3,534,620	687,210	26.6

Future Year 2045 WP - 80% Buildout										
SCENARIO	I-I	I-X	X-I	Total VMT	TOT_POP	Total Employees	VMT/Capita	TOT_POP	Total Employees - Reduced	VMT / Capita
Citywide	637,655	5,432,337	5,448,967	11,518,959	432,421	169,438	19.1	432,421	134,195	20.3
Countywide	52,931,787	29,642,498	29,748,229	112,322,514	3,581,086	1,962,645	20.3	3,581,086	681,038	26.4

*In order to mitigate double-counting, a reduction factor was applied to the employee population to account for the amount of employees who are also residents of the City. Per SCAG Local Profile Report (2019), 20.8% of commuters live in the City. Thus, a reduction of 20.8% was applied to the employee population. (https://www.scag.ca.gov/Documents/SantaAna.pdf)

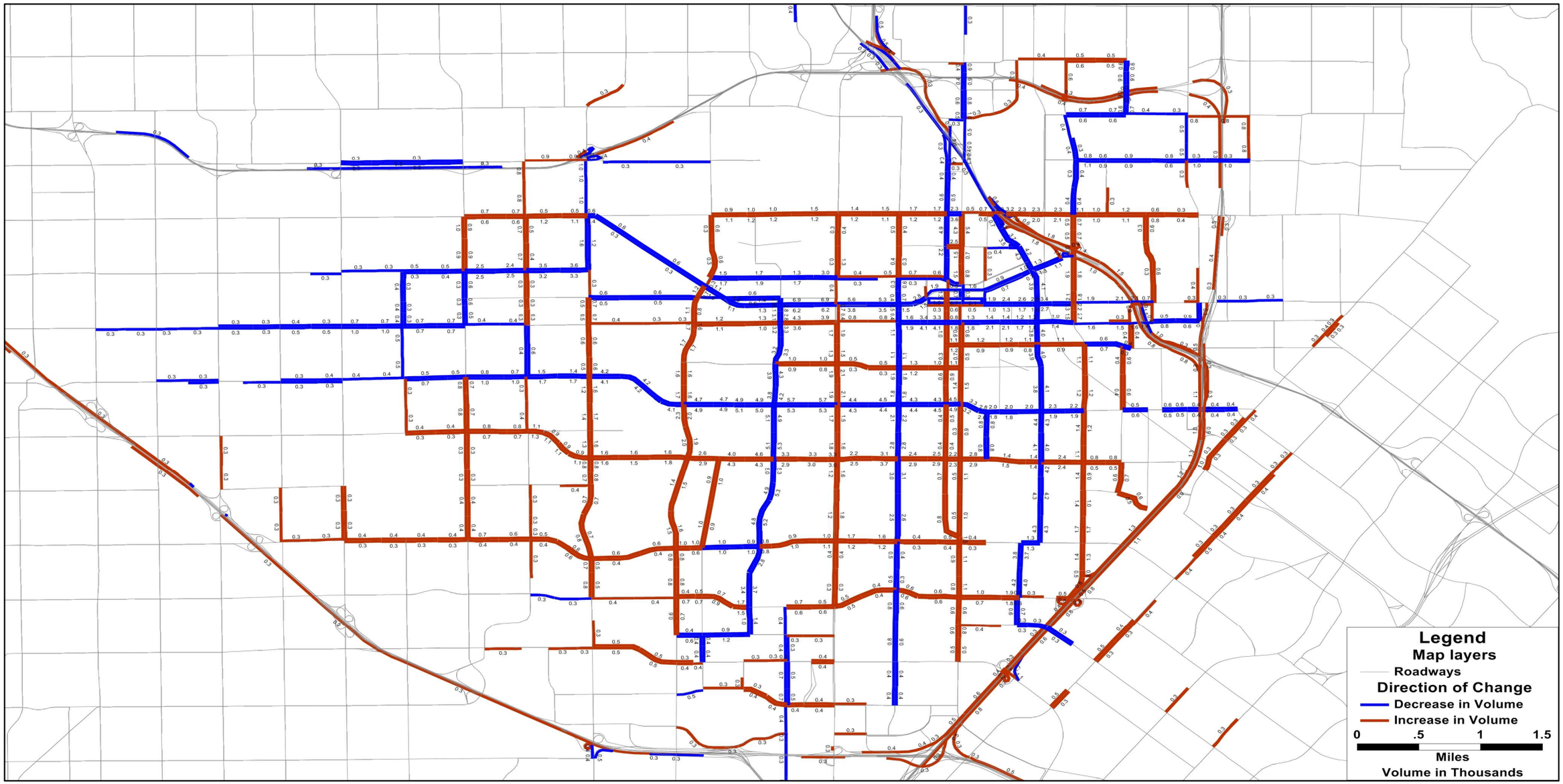
*In order to mitigate double-counting, a reduction factor was applied to the employee population to account for the amount of employees who are also residents of the County. Per SCAG Local Profile Report (2019), 65.3% of commuters live in the County. Thus, a reduction of 65.3% was applied to the employee population. (https://www.scag.ca.gov/Documents/OrangeCountyLP.pdf)

APPENDIX G

OCTAM VOLUME COMPARISON PLOTS

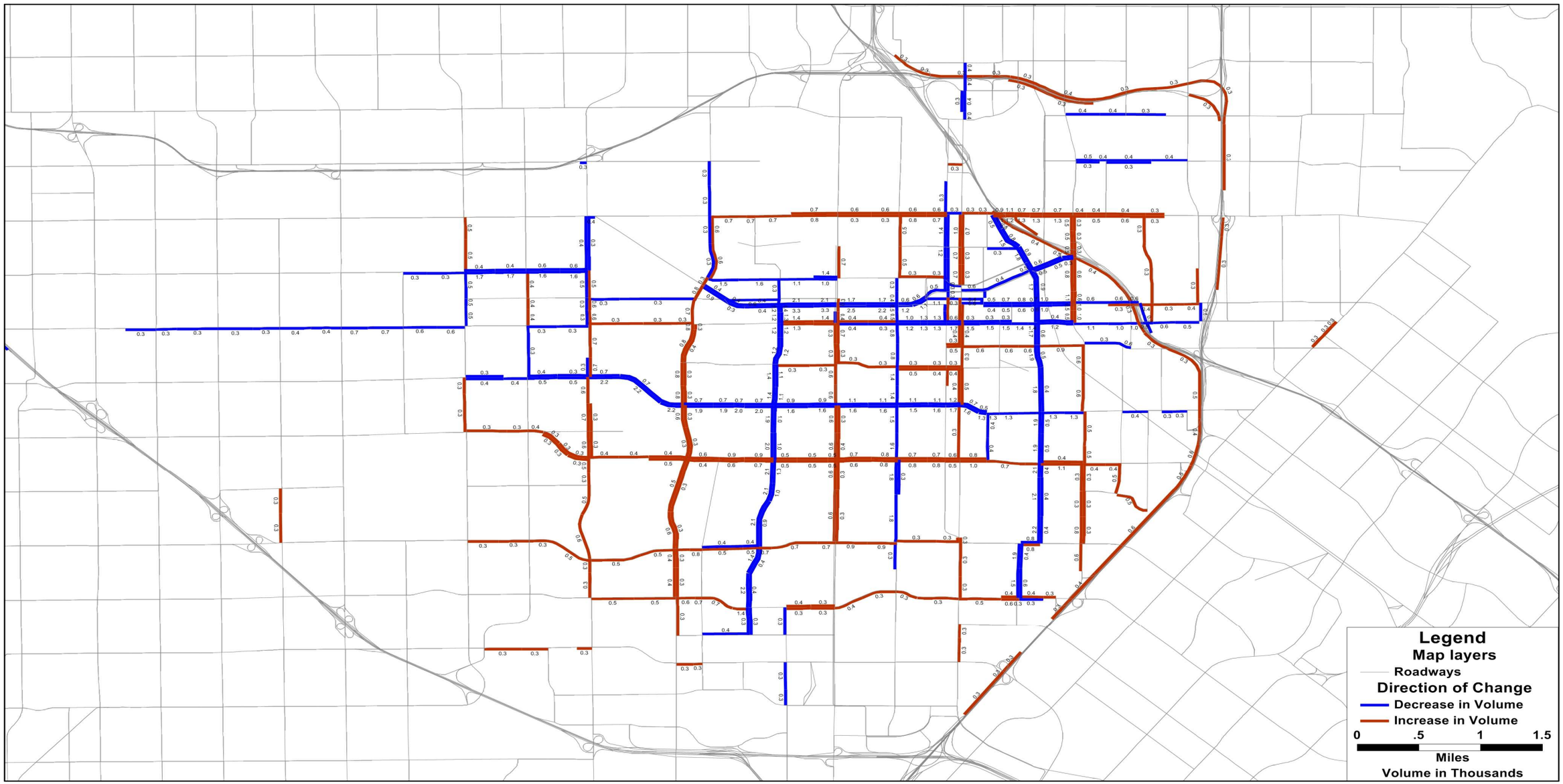
OCTAM 5.0 – Daily Traffic Volume Comparison Between 2045 80% Buildout and 2045 80% Buildout with Baseline Network

Volume in Thousands



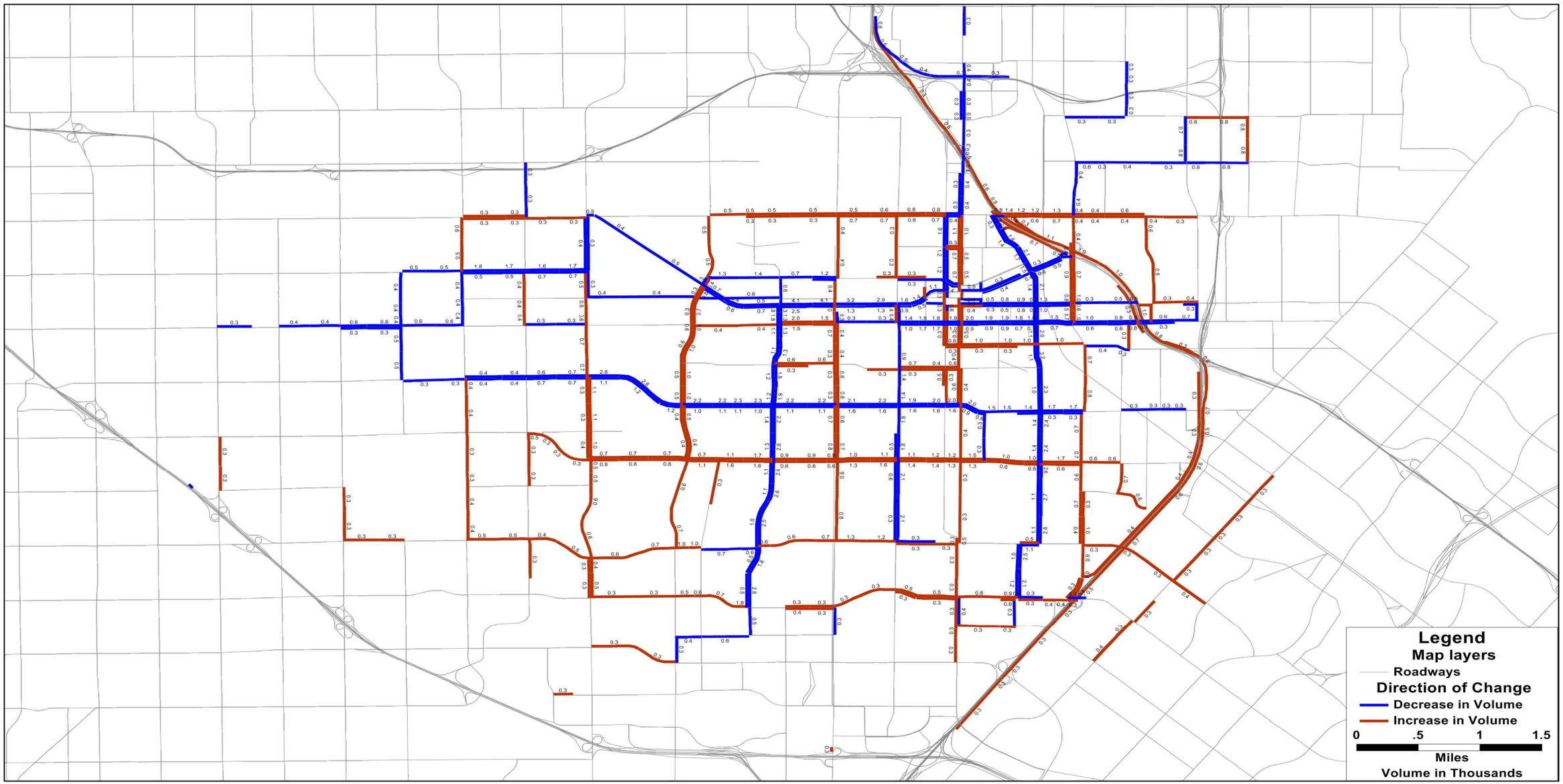
OCTAM 5.0 – AM Peak Traffic Volume Comparison Between 2045 80% Buildout and 2045 80% Buildout with Baseline Network

Volume in Thousands



OCTAM 5.0 – PM Peak Traffic Volume Comparison Between 2045 80% Buildout and 2045 80% Buildout with Baseline Network

Volume in Thousands



APPENDIX H
COUNTS

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE:	LOCATION:	Santa Ana	PROJECT #:	SC2183
Tue, May 14, 19	NORTH & SOUTH:	Euclid	LOCATION #:	1
	EAST & WEST:	1st	CONTROL:	SIGNAL

NOTES:

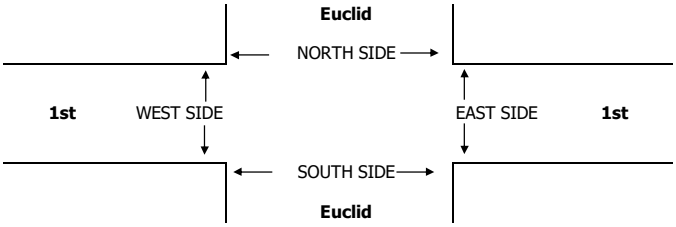
AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

Add U-Turns to Left Turns

		NORTHBOUND (Euclid)			SOUTHBOUND (Euclid)			EASTBOUND (1st)			WESTBOUND (1st)			
	LANES:	NL 1	NT 3	NR 0	SL 1	ST 3	SR 0	EL 1	ET 3	ER 0	WL 1	WT 3	WR 0	TOTAL
AM	7:00 AM	7	180	40	42	475	14	33	136	22	23	71	18	1,061
	7:15 AM	10	203	43	53	504	12	36	138	32	37	109	29	1,206
	7:30 AM	28	245	48	48	503	19	44	147	38	60	125	42	1,347
	7:45 AM	34	271	44	47	473	15	62	205	41	47	173	49	1,461
	8:00 AM	33	264	41	60	432	23	52	210	43	44	168	49	1,419
	8:15 AM	27	253	44	49	417	45	57	240	29	31	114	46	1,352
	8:30 AM	21	235	43	58	400	35	41	168	22	37	123	32	1,215
	8:45 AM	35	179	31	48	377	39	56	117	23	35	141	34	1,115
	VOLUMES	195	1,830	334	405	3,581	202	381	1,361	250	314	1,024	299	10,176
	APPROACH %	8%	78%	14%	10%	86%	5%	19%	68%	13%	19%	63%	18%	
	APP/DEPART	2,359	/	2,508	4,188	/	4,139	1,992	/	2,110	1,637	/	1,419	0
	BEGIN PEAK HR		7:30 AM											
	VOLUMES	122	1,033	177	204	1,825	102	215	802	151	182	580	186	5,579
	APPROACH %	9%	78%	13%	10%	86%	5%	18%	69%	13%	19%	61%	20%	
	PEAK HR FACTOR		0.954			0.935			0.896			0.881		0.955
	APP/DEPART	1,332	/	1,433	2,131	/	2,157	1,168	/	1,186	948	/	803	0
PM	4:00 PM	46	364	41	36	244	43	55	161	20	44	162	58	1,274
	4:15 PM	45	404	28	44	213	42	44	121	19	39	174	52	1,225
	4:30 PM	23	318	73	50	283	48	57	161	20	46	170	43	1,292
	4:45 PM	36	367	29	39	258	47	53	154	20	48	196	61	1,308
	5:00 PM	34	363	33	43	289	47	46	162	28	59	225	55	1,384
	5:15 PM	43	371	28	49	261	46	58	169	28	56	226	55	1,390
	5:30 PM	41	370	23	38	245	56	53	162	25	45	223	58	1,339
	5:45 PM	39	358	33	53	247	48	52	178	23	54	226	46	1,357
	VOLUMES	307	2,915	288	352	2,040	377	418	1,268	183	391	1,602	428	10,569
	APPROACH %	9%	83%	8%	13%	74%	14%	22%	68%	10%	16%	66%	18%	
	APP/DEPART	3,510	/	3,753	2,769	/	2,610	1,869	/	1,912	2,421	/	2,294	0
	BEGIN PEAK HR		5:00 PM											
	VOLUMES	157	1,462	117	183	1,042	197	209	671	104	214	900	214	5,470
	APPROACH %	9%	84%	7%	13%	73%	14%	21%	68%	11%	16%	68%	16%	
	PEAK HR FACTOR		0.982			0.938			0.965			0.979		0.984
	APP/DEPART	1,736	/	1,880	1,422	/	1,361	984	/	971	1,328	/	1,258	0

U-TURNS

NB	SB	EB	WB	TTL
0	0	0	0	
1	0	0	2	3
0	0	0	2	2
0	0	0	1	1
0	0	1	0	1
1	0	0	0	1
1	0	0	2	3
0	0	1	3	4
1	0	0	0	1
4	0	2	10	16
0	1	1	1	3
0	0	0	1	1
0	0	1	1	2
0	0	2	2	4
0	0	3	0	3
0	0	2	0	2
1	0	0	0	1
0	0	0	0	0
1	1	9	5	16



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:30 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	5:00 PM					0	0	0	0	0					

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

DATE: Wed, Feb 10, 16

LOCATION: Santa Ana
NORTH & SOUTH: Euclid
EAST & WEST: McFadden

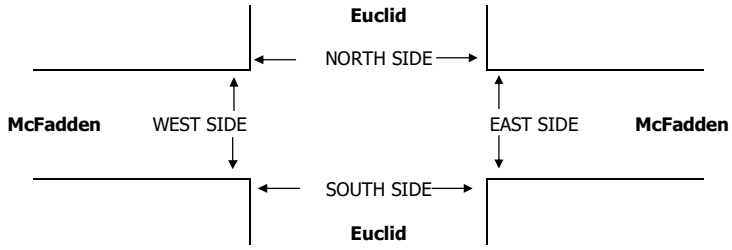
PROJECT #: SC0846
LOCATION #: 30
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Euclid			SOUTHBOUND Euclid			EASTBOUND McFadden			WESTBOUND McFadden				U-TURNS				
		NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL	NB	SB	EB	WB	TTL
	LANES:	1	2	0	1	3	0	1	2	0	1	2	0		0	0	0	0	
AM	7:00 AM	15	164	8	14	479	23	19	68	16	23	52	12	893	0	0	0	0	0
	7:15 AM	16	272	20	26	548	20	27	78	41	17	64	25	1,154	0	0	0	0	0
	7:30 AM	29	263	38	32	395	18	51	118	71	45	65	39	1,164	0	0	0	0	0
	7:45 AM	34	265	27	38	470	34	58	101	81	49	75	38	1,270	0	0	0	0	0
	8:00 AM	43	258	30	26	431	58	55	111	62	41	133	35	1,283	0	0	0	0	0
	8:15 AM	33	271	30	35	535	41	54	114	61	24	80	22	1,300	0	0	0	0	0
	8:30 AM	22	263	15	22	469	31	39	118	48	17	73	34	1,151	0	0	0	0	0
	8:45 AM	20	229	17	20	432	18	34	90	28	20	56	25	989	0	0	0	0	0
	VOLUMES	212	1,985	185	213	3,759	243	337	798	408	236	598	230	9,204	0	0	0	0	0
	APPROACH %	9%	83%	8%	5%	89%	6%	22%	52%	26%	22%	56%	22%						
	APP/DEPART	2,382	/	2,552	4,215	/	4,403	1,543	/	1,196	1,064	/	1,053	0					
	BEGIN PEAK HR		7:30 AM																
	VOLUMES	139	1,057	125	131	1,831	151	218	444	275	159	353	134	5,017					
	APPROACH %	11%	80%	9%	6%	87%	7%	23%	47%	29%	25%	55%	21%						
	PEAK HR FACTOR		0.989			0.865			0.976			0.773		0.965					
	APP/DEPART	1,321	/	1,409	2,113	/	2,265	937	/	700	646	/	643	0					
PM	4:00 PM	29	286	20	25	278	27	45	83	35	14	96	25	963	0	0	0	0	0
	4:15 PM	30	297	15	27	284	35	53	96	47	17	109	30	1,040	0	0	0	0	0
	4:30 PM	34	315	21	22	287	40	55	101	43	21	123	35	1,097	0	0	0	0	0
	4:45 PM	36	327	20	26	300	39	49	98	52	27	130	47	1,151	0	0	0	0	0
	5:00 PM	44	335	23	24	285	35	51	104	40	30	134	53	1,158	0	0	0	0	0
	5:15 PM	34	374	19	22	365	40	40	100	41	34	130	63	1,262	0	0	0	0	0
	5:30 PM	54	334	26	33	351	43	35	100	44	31	117	73	1,241	0	0	0	0	0
	5:45 PM	51	352	17	28	267	36	36	115	29	21	121	64	1,137	0	0	0	0	0
	VOLUMES	312	2,620	161	207	2,417	295	364	797	331	195	960	390	9,049	0	0	0	0	0
	APPROACH %	10%	85%	5%	7%	83%	10%	24%	53%	22%	13%	62%	25%						
	APP/DEPART	3,093	/	3,374	2,919	/	2,943	1,492	/	1,165	1,545	/	1,567	0					
	BEGIN PEAK HR		4:45 PM																
	VOLUMES	168	1,370	88	105	1,301	157	175	402	177	122	511	236	4,812					
	APPROACH %	10%	84%	5%	7%	83%	10%	23%	53%	23%	14%	59%	27%						
	PEAK HR FACTOR		0.952			0.915			0.947			0.957		0.953					
	APP/DEPART	1,626	/	1,781	1,563	/	1,600	754	/	595	869	/	836	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	8	4	2	8	22	7	1	2	5	15	1	3	0	3	7
	7:15 AM	8	13	6	27	54	6	12	5	27	50	2	1	1	0	4
	7:30 AM	21	52	21	55	149	20	50	19	52	141	1	2	2	3	8
	7:45 AM	7	12	11	4	34	6	8	8	3	25	1	4	3	1	9
	8:00 AM	9	20	4	11	44	3	15	4	7	29	6	5	0	4	15
	8:15 AM	6	4	5	9	24	6	3	4	9	22	0	1	1	0	2
	8:30 AM	4	6	4	2	16	3	5	4	2	14	1	1	0	0	2
	8:45 AM	1	2	2	0	5	1	1	2	0	4	0	1	0	0	1
	TOTAL	64	113	55	116	348	52	95	48	105	300	12	18	7	11	48
PM	4:00 PM	8	14	9	7	38	5	12	7	5	29	3	2	2	2	9
	4:15 PM	8	12	13	4	37	7	8	8	3	26	1	4	5	1	11
	4:30 PM	10	15	8	10	43	4	10	6	6	26	6	5	2	4	17
	4:45 PM	8	4	5	9	26	6	3	4	9	22	2	1	1	0	4
	5:00 PM	35	14	11	51	111	31	12	10	47	100	4	2	1	4	11
	5:15 PM	19	46	15	34	114	16	44	11	33	104	3	2	4	1	10
	5:30 PM	15	24	6	42	87	14	22	6	41	83	1	2	0	1	4
	5:45 PM	6	7	4	11	28	5	7	2	10	24	1	0	2	1	4
	TOTAL	109	136	71	168	484	88	118	54	154	414	21	18	17	14	70

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Tue, May 14, 19

LOCATION: Santa Ana
NORTH & SOUTH: Euclid
EAST & WEST: Edinger

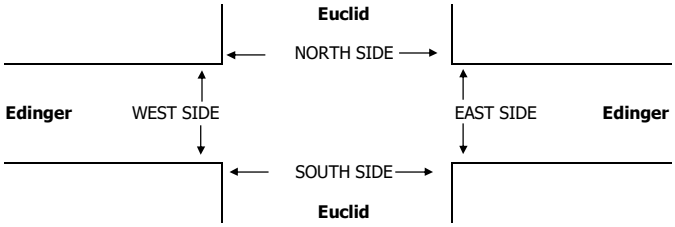
PROJECT #: SC2183
LOCATION #: 2
CONTROL: SIGNAL

NOTES:

AM PM MD OTHER OTHER — ▲ N ◄ W E ► S ▼

Add U-Turns to Left Turns

		NORTHBOUND Euclid			SOUTHBOUND Euclid			EASTBOUND Edinger			WESTBOUND Edinger				U-TURNS				
	LANES:	NL 2	NT 2.5	NR 0.5	SL 2	ST 2.5	SR 0.5	EL 1	ET 2	ER 1	WL 1	WT 2	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	15	137	5	17	426	24	33	131	88	25	101	18	1,020	0	0	0	1	1
	7:15 AM	23	134	8	27	485	31	50	141	86	18	119	24	1,146	0	0	3	1	4
	7:30 AM	30	214	8	35	494	39	42	152	124	27	165	32	1,362	3	0	2	0	5
	7:45 AM	47	199	18	27	478	57	50	188	138	29	155	29	1,415	3	0	2	1	6
	8:00 AM	30	221	16	34	424	30	37	204	109	27	149	30	1,311	3	1	4	0	8
	8:15 AM	25	202	29	29	427	41	41	201	84	16	133	18	1,246	0	0	1	1	2
	8:30 AM	27	161	13	26	456	45	50	144	87	27	117	21	1,174	1	1	2	0	4
	8:45 AM	33	152	19	26	397	33	31	140	86	20	125	26	1,088	2	0	5	0	7
	VOLUMES	230	1,420	116	221	3,587	300	334	1,301	802	189	1,064	198	9,762	12	2	19	4	37
	APPROACH %	13%	80%	7%	5%	87%	7%	14%	53%	33%	13%	73%	14%						
	APP/DEPART	1,766	/	1,935	4,108	/	4,586	2,437	/	1,640	1,451	/	1,601	0					
	BEGIN PEAK HR		7:30 AM																
	VOLUMES	132	836	71	125	1,823	167	170	745	455	99	602	109	5,334					
	APPROACH %	13%	80%	7%	6%	86%	8%	12%	54%	33%	12%	74%	13%						
	PEAK HR FACTOR		0.973			0.931			0.911			0.904		0.942					
	APP/DEPART	1,039	/	1,107	2,115	/	2,384	1,370	/	942	810	/	901	0					
PM	4:00 PM	86	347	11	24	179	36	39	162	39	23	186	22	1,154	2	3	1	1	7
	4:15 PM	104	390	27	25	179	41	37	150	36	18	214	21	1,242	1	0	2	0	3
	4:30 PM	107	350	19	28	178	47	40	150	46	17	208	32	1,222	0	1	5	1	7
	4:45 PM	102	415	34	32	191	46	46	139	41	20	203	35	1,304	1	1	2	0	4
	5:00 PM	140	374	27	36	195	48	37	130	55	21	217	36	1,316	3	0	8	0	11
	5:15 PM	136	401	29	27	216	63	43	172	45	19	191	29	1,371	6	1	4	1	12
	5:30 PM	142	366	20	25	193	44	33	174	43	30	220	34	1,324	3	0	1	0	4
	5:45 PM	112	313	25	32	191	43	40	171	38	17	207	32	1,221	0	2	8	0	10
	VOLUMES	929	2,956	192	229	1,522	368	315	1,248	343	165	1,646	241	10,154	16	8	31	3	58
	APPROACH %	23%	73%	5%	11%	72%	17%	17%	65%	18%	8%	80%	12%						
	APP/DEPART	4,077	/	3,489	2,119	/	2,043	1,906	/	1,664	2,052	/	2,958	0					
	BEGIN PEAK HR		4:45 PM																
	VOLUMES	520	1,556	110	120	795	201	159	615	184	90	831	134	5,315					
	APPROACH %	24%	71%	5%	11%	71%	18%	17%	64%	19%	9%	79%	13%						
	PEAK HR FACTOR		0.966			0.912			0.921			0.929		0.969					
	APP/DEPART	2,186	/	1,836	1,116	/	1,081	958	/	844	1,055	/	1,554	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR			7:30 AM			0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR			4:45 PM			0	0	0	0	0					

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

DATE: Wed, Feb 10, 16

LOCATION: Santa Ana
NORTH & SOUTH: Newhope
EAST & WEST: Hazard

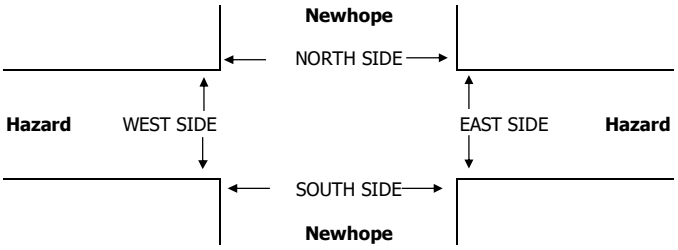
PROJECT #: SC0846
LOCATION #: 11
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND (Newhope)			SOUTHBOUND (Newhope)			EASTBOUND (Hazard)			WESTBOUND (Hazard)				U-TURNS				
	LANES:	NL 1	NT 2	NR 0	SL 1	ST 2	SR 0	EL 1	ET 2	ER 0	WL 1	WT 2	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	10	146	6	7	257	4	13	28	16	4	15	9	515	0	0	0	0	0
	7:15 AM	15	178	15	15	352	21	10	36	28	8	39	9	726	0	0	0	0	0
	7:30 AM	24	190	22	21	308	31	19	50	25	17	39	13	759	0	0	0	0	0
	7:45 AM	40	165	48	37	281	41	26	57	59	10	45	19	828	0	0	0	0	0
	8:00 AM	15	176	15	14	347	25	19	48	29	14	40	24	766	0	0	0	0	0
	8:15 AM	20	174	4	17	316	16	30	59	22	1	23	11	693	0	0	0	0	0
	8:30 AM	13	163	3	17	322	23	22	48	24	9	32	10	686	0	0	0	0	0
	8:45 AM	29	161	11	9	282	13	24	32	26	4	29	12	632	0	0	0	0	0
	VOLUMES	166	1,353	124	137	2,465	174	163	358	229	67	262	107	5,605	0	0	0	0	0
	APPROACH %	10%	82%	8%	5%	89%	6%	22%	48%	31%	15%	60%	25%						
	APP/DEPART	1,643	/	1,623	2,776	/	2,761	750	/	619	436	/	602	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	94	709	100	87	1,288	118	74	191	141	49	163	65	3,079					
	APPROACH %	10%	79%	11%	6%	86%	8%	18%	47%	35%	18%	59%	23%						
	PEAK HR FACTOR		0.892			0.962			0.715			0.888		0.930					
	APP/DEPART	903	/	848	1,493	/	1,478	406	/	378	277	/	375	0					
PM	4:00 PM	20	292	8	11	223	24	24	30	18	3	42	38	733	0	0	0	0	0
	4:15 PM	18	304	12	16	194	17	27	35	17	10	30	37	717	0	0	0	0	0
	4:30 PM	21	340	12	10	199	27	29	49	14	6	41	13	761	0	0	0	0	0
	4:45 PM	27	299	18	13	193	19	24	51	23	8	59	12	746	0	0	0	0	0
	5:00 PM	22	315	9	15	267	35	19	48	29	10	52	7	828	0	0	0	0	0
	5:15 PM	31	298	7	9	252	31	29	49	29	5	54	19	813	0	0	0	0	0
	5:30 PM	28	294	15	14	251	26	29	56	37	12	37	13	812	0	0	0	0	0
	5:45 PM	30	324	17	9	227	27	26	52	30	3	51	16	812	0	0	0	0	0
	VOLUMES	197	2,466	98	97	1,806	206	207	370	197	57	366	155	6,222	0	0	0	0	0
	APPROACH %	7%	89%	4%	5%	86%	10%	27%	48%	25%	10%	63%	27%						
	APP/DEPART	2,761	/	2,828	2,109	/	2,060	774	/	565	578	/	769	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	111	1,231	48	47	997	119	103	205	125	30	194	55	3,265					
	APPROACH %	8%	89%	3%	4%	86%	10%	24%	47%	29%	11%	70%	20%						
	PEAK HR FACTOR		0.937			0.917			0.887			0.894		0.986					
	APP/DEPART	1,390	/	1,389	1,163	/	1,152	433	/	300	279	/	424	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	1	1	1	0	3	0	0	0	0	0	1	1	1	0	3
	7:15 AM	2	3	1	0	6	1	2	1	0	4	1	1	0	0	2
	7:30 AM	4	3	5	0	12	4	3	4	0	11	0	0	1	0	1
	7:45 AM	1	3	8	2	14	1	3	6	1	11	0	0	2	1	3
	8:00 AM	1	12	8	0	21	1	11	5	0	17	0	1	3	0	4
	8:15 AM	1	6	7	4	18	0	6	5	2	13	1	0	2	2	5
	8:30 AM	14	6	5	0	25	14	6	4	0	24	0	0	1	0	1
	8:45 AM	1	12	2	0	15	1	12	1	0	14	0	0	1	0	1
	TOTAL	25	46	37	6	114	22	43	26	3	94	3	3	11	3	20
PM	4:00 PM	1	0	3	2	6	0	0	3	2	5	1	0	0	0	1
	4:15 PM	1	2	1	3	7	0	0	0	0	0	1	2	1	3	7
	4:30 PM	4	0	2	4	10	4	0	1	3	8	0	0	1	1	2
	4:45 PM	3	0	0	5	8	3	0	0	3	6	0	0	0	2	2
	5:00 PM	0	2	4	1	7	0	1	4	0	5	0	1	0	1	2
	5:15 PM	2	3	5	2	12	1	2	2	0	5	1	1	3	2	7
	5:30 PM	1	4	2	1	8	0	2	1	0	3	1	2	1	1	5
	5:45 PM	2	2	2	2	8	2	1	1	0	4	0	1	1	2	4
	TOTAL	14	13	19	20	66	10	6	12	8	36	4	7	7	12	30

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

DATE: Wed, Feb 10, 16

LOCATION: Santa Ana
NORTH & SOUTH: Newhope
EAST & WEST: McFadden

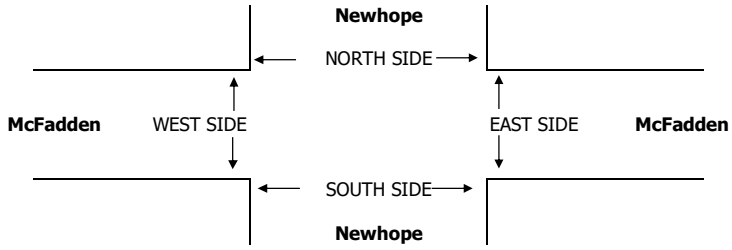
PROJECT #: SC0846
LOCATION #: 31
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND (Newhope)			SOUTHBOUND (Newhope)			EASTBOUND (McFadden)			WESTBOUND (McFadden)				U-TURNS				
	LANES:	NL 1	NT 2	NR 0	SL 1	ST 2	SR 0	EL 1	ET 2	ER 0	WL 1	WT 2	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	5	97	11	18	281	11	10	54	17	11	67	21	603	0	0	0	0	0
	7:15 AM	14	91	15	22	301	19	8	73	38	17	72	17	687	0	0	0	0	0
	7:30 AM	17	94	28	18	318	17	33	113	37	37	112	22	846	0	0	0	0	0
	7:45 AM	38	110	45	29	247	20	24	102	43	49	110	34	851	0	0	0	0	0
	8:00 AM	31	144	60	31	249	37	52	129	38	53	133	22	979	0	0	0	0	0
	8:15 AM	19	123	33	36	322	31	20	128	35	30	76	18	871	0	0	0	0	0
	8:30 AM	9	127	28	22	291	17	25	108	30	35	88	25	805	0	0	0	0	0
	8:45 AM	23	162	38	26	355	23	10	83	21	40	61	31	873	0	0	0	0	0
	VOLUMES	156	948	258	202	2,364	175	182	790	259	272	719	190	6,515	0	0	0	0	0
	APPROACH %	11%	70%	19%	7%	86%	6%	15%	64%	21%	23%	61%	16%						
	APP/DEPART	1,362	/	1,320	2,741	/	2,895	1,231	/	1,250	1,181	/	1,050	0					
	BEGIN PEAK HR		7:30 AM																
	VOLUMES	105	471	166	114	1,136	105	129	472	153	169	431	96	3,547					
	APPROACH %	14%	63%	22%	8%	84%	8%	17%	63%	20%	24%	62%	14%						
	PEAK HR FACTOR		0.789			0.871			0.861			0.837		0.906					
	APP/DEPART	742	/	696	1,355	/	1,458	754	/	752	696	/	641	0					
PM	4:00 PM	37	275	38	37	140	30	15	103	11	35	104	51	876	0	0	0	0	0
	4:15 PM	26	283	41	37	158	26	22	118	17	27	99	40	894	0	0	0	0	0
	4:30 PM	35	278	42	30	134	31	11	104	13	31	103	38	850	0	0	0	0	0
	4:45 PM	42	274	39	22	146	18	11	106	16	26	130	46	876	0	0	0	0	0
	5:00 PM	43	262	42	26	158	32	26	110	18	25	138	39	919	0	0	0	0	0
	5:15 PM	52	260	53	37	169	40	19	108	11	24	135	39	947	0	0	0	0	0
	5:30 PM	50	245	42	48	191	36	20	122	23	32	138	39	986	0	0	0	0	0
	5:45 PM	49	274	45	46	186	28	12	136	17	21	155	44	1,013	0	0	0	0	0
	VOLUMES	334	2,151	342	283	1,282	241	136	907	126	221	1,002	336	7,361	0	0	0	0	0
	APPROACH %	12%	76%	12%	16%	71%	13%	12%	78%	11%	14%	64%	22%						
	APP/DEPART	2,827	/	2,623	1,806	/	1,629	1,169	/	1,532	1,559	/	1,577	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	194	1,041	182	157	704	136	77	476	69	102	566	161	3,865					
	APPROACH %	14%	73%	13%	16%	71%	14%	12%	77%	11%	12%	68%	19%						
	PEAK HR FACTOR		0.963			0.906			0.942			0.942		0.954					
	APP/DEPART	1,417	/	1,279	997	/	875	622	/	815	829	/	896	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	1	5	4	0	10	1	4	3	0	8	0	1	1	0	2
	7:15 AM	4	4	7	0	15	1	2	6	0	9	3	2	1	0	6
	7:30 AM	4	24	19	1	48	3	21	18	0	42	1	3	1	1	6
	7:45 AM	13	17	20	32	82	13	16	19	31	79	0	1	1	1	3
	8:00 AM	36	57	47	40	180	35	55	43	38	171	1	2	4	2	9
	8:15 AM	11	30	34	17	92	9	30	32	15	86	2	0	2	2	6
	8:30 AM	5	7	19	4	35	5	6	17	3	31	0	1	2	1	4
	8:45 AM	3	3	14	0	20	3	2	13	0	18	0	1	1	0	2
	TOTAL	77	147	164	94	482	70	136	151	87	444	7	11	13	7	38
PM	4:00 PM	2	2	5	4	13	2	1	5	4	12	0	1	0	0	1
	4:15 PM	4	6	10	6	26	3	2	5	4	14	1	4	5	2	12
	4:30 PM	2	6	1	2	11	2	3	0	2	7	0	3	1	0	4
	4:45 PM	10	17	4	8	39	10	15	4	8	37	0	2	0	0	2
	5:00 PM	15	9	4	4	32	9	6	3	3	21	6	3	1	1	11
	5:15 PM	4	7	8	4	23	4	3	7	4	18	0	4	1	0	5
	5:30 PM	7	4	5	5	21	6	2	3	5	16	1	2	2	0	5
	5:45 PM	5	5	6	1	17	3	3	3	0	9	2	2	3	1	8
	TOTAL	49	56	43	34	182	39	35	30	30	134	10	21	13	4	48

ITM Peak Hour Summary

Prepared by:

National Data & Surveying Services

Harbor Blvd and Banner Dr , Garden Grove

Date: 2014-10-16
Day: Thursday
Project #: Historical
City: Garden Grove

AM Peak Hour	715 AM
NOON Peak Hour	
PM Peak Hour	500 PM

Peak Hour Summary

Southbound Approach (Harbor Blvd)

	Lanes 0	3	0		
AM	17	1301	0	1407	AM
NOON	0	0	0	0	NOON
PM	47	1459	0	2166	PM

Eastbound Approach (Banner Dr)

Lanes	AM	NOON	PM
	126	0	207
0	71	0	89
1	0	0	0
0	116	0	74
Lanes	AM	NOON	PM

Westbound Approach

	AM	NOON	PM	Lanes
	112	0	163	1
	42	0	76	.5
	803	0	569	1.5
	0	0	0	
	AM	NOON	PM	

CONTROL: Signalized

Northbound Approach

AM	2220	67	1224	0	AM
NOON	0	0	0	0	NOON
PM	2102	84	1914	0	PM
		1	3	1	Lanes

Count Periods	Start	End
AM	7:00 AM	9:00 AM
NOON		
PM	4:00 PM	6:00 PM

Total Ins & Outs

	North Leg		
AM	1318	1407	AM
NOON	0	0	NOON
PM	1506	2166	PM

West Leg AM	West Leg NOON	West Leg PM	East Leg	East Leg AM	East Leg NOON	East Leg PM
126	0	207		957	0	808
187	0	163		0	0	0

	South Leg	
AM	2220	1291
NOON	0	0
PM	2102	1998

Total Volume Per Leg

	North Leg	
AM	2725	AM
NOON	0	NOON
PM	3672	PM

West Leg AM	West Leg NOON	West Leg PM	East Leg	East Leg AM	East Leg NOON	East Leg PM
313	0	370		957	0	808

	South Leg	
AM	3511	
NOON	0	
PM	4100	

ITM Peak Hour Summary

Prepared by:

National Data & Surveying Services

SR-22 EB On-Ramp and Trask Ave , City of Santa Ana

Date: 2013-02-05

Day: Tuesday

Project #: Historical

AM Peak Hour	715 AM
NOON Peak Hour	
PM Peak Hour	400 PM

Peak Hour Summary

Southbound Approach (SR-22 EB On-Ramp)

Lanes	0	0	0		
AM	0	0	0	735	AM
NOON	0	0	0	0	NOON
PM	0	0	0	834	PM

Eastbound Approach (Trask Ave)

Lanes	AM	NOON	PM
	688	0	565
2	607	0	781
1	760	0	585
0	0	0	0
Lanes	AM	NOON	PM

Westbound Approach

AM	NOON	PM	Lanes
128	0	53	0
688	0	565	2
0	0	0	0
760	0	585	
AM	NOON	PM	

CONTROL: Signalized

Count Periods	Start	End
AM	6:00 AM	9:00 AM
NOON		
PM	4:00 PM	7:00 PM

Northbound Approach

AM	0	0	0	0	AM
NOON	0	0	0	0	NOON
PM	0	0	0	0	PM
		0	0	0	Lanes

Total Ins & Outs

	North Leg		
	0	735	AM
	0	0	NOON
	0	834	PM

West Leg				East Leg		
AM	NOON	PM		AM	NOON	PM
688	0	565		816	0	618
1367	0	1366		760	0	585

	South Leg	
AM	0	0
NOON	0	0
PM	0	0

Total Volume Per Leg

	North Leg	
	735	AM
	0	NOON
	834	PM

West Leg			East Leg		
AM	NOON	PM	AM	NOON	PM
2055	0	1931	1576	0	1203

	South Leg
AM	0
NOON	0
PM	0

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

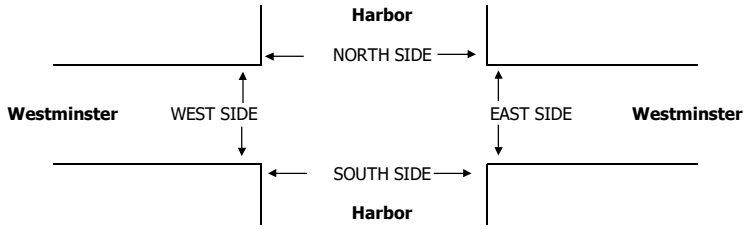
DATE:	LOCATION:	Santa Ana	PROJECT #:	SC2183
Thu, May 16, 19	NORTH & SOUTH:	Harbor	LOCATION #:	3
	EAST & WEST:	Westminster	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND (Harbor)			SOUTHBOUND (Harbor)			EASTBOUND (Westminster)			WESTBOUND (Westminster)				U-TURNS				
	LANES:	NL 1	NT 3	NR 1	SL 1	ST 3	SR 1	EL 1	ET 3	ER 0	WL 1	WT 3	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	40	258	41	40	419	20	24	160	3	51	91	25	1,172	7	3	3	4	17
	7:15 AM	28	298	43	43	428	13	23	214	6	50	115	32	1,293	5	3	4	6	18
	7:30 AM	41	336	67	56	439	24	36	237	10	61	156	33	1,496	6	4	8	7	25
	7:45 AM	49	299	74	56	391	22	43	236	2	52	156	38	1,418	9	8	10	5	32
	8:00 AM	48	288	55	55	390	28	43	234	9	48	138	45	1,381	9	4	9	5	27
	8:15 AM	38	238	52	42	426	20	52	210	11	48	118	38	1,293	10	6	13	6	35
	8:30 AM	52	282	41	56	383	36	55	182	8	40	113	35	1,283	13	8	14	5	40
	8:45 AM	44	234	24	35	340	34	37	145	8	48	103	48	1,100	9	2	6	4	21
	VOLUMES	340	2,233	397	383	3,216	197	313	1,618	57	398	990	294	10,436	68	38	67	42	215
	APPROACH %	11%	75%	13%	10%	85%	5%	16%	81%	3%	24%	59%	17%						
	APP/DEPART	2,970	/	2,811	3,796	/	3,697	1,988	/	2,402	1,682	/	1,526	0					
	BEGIN PEAK HR		7:30 AM																
	VOLUMES	176	1,161	248	209	1,646	94	174	917	32	209	568	154	5,588					
	APPROACH %	11%	73%	16%	11%	84%	5%	15%	82%	3%	22%	61%	17%						
	PEAK HR FACTOR		0.892			0.939			0.982			0.931		0.934					
	APP/DEPART	1,585	/	1,471	1,949	/	1,898	1,123	/	1,375	931	/	844	0					
PM	4:00 PM	59	331	66	47	345	48	60	170	4	63	171	50	1,414	9	3	13	8	33
	4:15 PM	60	331	46	44	321	37	45	136	8	54	180	48	1,310	6	2	9	6	23
	4:30 PM	63	336	70	58	299	46	45	127	11	62	181	46	1,344	6	2	7	12	27
	4:45 PM	50	363	72	48	320	41	47	153	14	59	204	49	1,420	8	7	9	5	29
	5:00 PM	69	341	65	48	370	40	49	142	3	57	194	52	1,430	10	4	10	4	28
	5:15 PM	69	320	84	55	369	39	49	177	10	59	216	42	1,489	9	7	13	7	36
	5:30 PM	85	336	100	53	356	43	40	188	4	52	175	36	1,468	12	6	12	3	33
	5:45 PM	70	337	77	57	340	31	39	170	7	50	173	38	1,389	10	1	11	4	26
	VOLUMES	525	2,695	580	410	2,720	325	374	1,263	61	456	1,494	361	11,264	70	32	84	49	235
	APPROACH %	14%	71%	15%	12%	79%	9%	22%	74%	4%	20%	65%	16%						
	APP/DEPART	3,800	/	3,378	3,455	/	3,258	1,698	/	2,270	2,311	/	2,358	0					
	BEGIN PEAK HR		4:45 PM																
	VOLUMES	273	1,360	321	204	1,415	163	185	660	31	227	789	179	5,807					
	APPROACH %	14%	70%	16%	11%	79%	9%	21%	75%	4%	19%	66%	15%						
	PEAK HR FACTOR		0.938			0.962			0.928			0.942		0.975					
	APP/DEPART	1,954	/	1,704	1,782	/	1,693	876	/	1,180	1,195	/	1,230	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:30 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	4:45 PM					0	0	0	0	0					

INTERSECTION TURNING MOVEMENT COUNTS AVERAGE SHEET

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: AVERAGE FOR THREE DAYS

LOCATION: Santa Ana
NORTH & SOUTH: Harbor
EAST & WEST: 1st
PROJECT #: CMP2019
LOCATION #: 190
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N ◄ W E ► S ▼

Add U-Turns to Left Turns

		NORTHBOUND (Harbor)			SOUTHBOUND (Harbor)			EASTBOUND (1st)			WESTBOUND (1st)			
	LANES:	NL 1	NT 3	NR 1	SL 1	ST 3	SR 1	EL 2	ET 3	ER 0	WL 2	WT 3	WR 0	TOTAL
AM	6:00 AM	11	108	14	18	225	11	21	67	22	11	54	21	583
	6:15 AM	10	125	22	25	289	11	19	108	34	23	78	30	775
	6:30 AM	13	143	22	37	451	15	20	137	33	22	93	30	1,015
	6:45 AM	23	148	20	53	452	15	23	186	36	33	92	30	1,109
	7:00 AM	19	156	32	53	487	20	23	207	42	36	93	35	1,203
	7:15 AM	22	227	45	53	493	25	26	230	40	56	115	38	1,371
	7:30 AM	19	237	61	53	510	26	30	229	47	78	149	36	1,478
	7:45 AM	27	228	52	51	445	38	32	224	44	60	188	37	1,428
	8:00 AM	24	232	42	60	446	32	37	239	47	47	162	31	1,400
	8:15 AM	28	200	38	58	428	20	37	264	56	48	118	32	1,327
	8:30 AM	24	184	35	53	439	24	33	253	38	37	115	36	1,270
	8:45 AM	32	177	34	41	408	22	35	213	40	36	128	32	1,197
	VOLUMES	254	2,165	416	554	5,073	259	338	2,355	480	486	1,385	388	14,272
	APPROACH %	9%	75%	14%	9%	86%	4%	11%	73%	15%	22%	61%	17%	
	APP/DEPART	2,898	/	2,914	5,910	/	6,102	3,205	/	3,326	2,259	/	1,930	
	BEGIN PEAK HR		7:15 AM											
	VOLUMES	115	925	200	224	1,895	122	138	922	179	242	614	143	5,717
	APPROACH %	9%	75%	16%	10%	85%	5%	11%	74%	14%	24%	61%	14%	
	PEAK HR FACTOR		0.958			0.948			0.950			0.875		0.960
	APP/DEPART	1,239	/	1,200	2,240	/	2,337	1,238	/	1,339	999	/	840	
PM	03:00 PM	62	351	56	54	252	31	38	154	31	50	170	36	1,285
	3:15 PM	56	342	61	62	261	33	36	170	29	49	178	35	1,311
	3:30 PM	53	340	54	64	252	34	45	171	33	51	203	41	1,342
	3:45 PM	62	342	39	66	289	39	31	186	30	48	217	45	1,394
	4:00 PM	59	343	48	67	271	34	36	169	35	49	232	49	1,393
	4:15 PM	62	364	45	60	273	36	30	180	28	53	230	38	1,398
	4:30 PM	62	363	33	60	256	34	34	196	38	46	235	41	1,399
	4:45 PM	61	372	39	64	292	34	39	176	28	65	231	36	1,438
	5:00 PM	63	366	35	64	309	37	42	183	29	61	263	42	1,494
	5:15 PM	63	371	42	67	298	44	35	189	31	57	276	39	1,512
	5:30 PM	64	363	40	63	303	43	34	192	44	56	280	43	1,524
	5:45 PM	66	364	46	59	295	39	41	194	39	53	243	49	1,489
	6:00 PM	71	350	43	57	265	35	41	190	35	51	244	41	1,423
	6:15 PM	66	364	40	59	275	34	45	173	39	45	231	47	1,418
	6:30 PM	64	350	49	69	247	48	36	164	36	45	208	37	1,354
	6:45 PM	58	317	45	62	257	36	39	134	35	47	172	40	1,244
	VOLUMES	991	5,664	717	998	4,394	590	602	2,822	541	827	3,613	659	22,717
	APPROACH %	13%	76%	10%	16%	72%	10%	15%	69%	13%	16%	71%	13%	
	APP/DEPART	7,478	/	7,019	6,075	/	5,868	4,063	/	4,537	5,100	/	5,293	
	BEGIN PEAK HR		5:00 PM											
	VOLUMES	284	1,465	162	275	1,205	162	186	758	143	227	1,062	173	6,102
	APPROACH %	15%	77%	8%	17%	73%	10%	17%	70%	13%	16%	73%	12%	
	PEAK HR FACTOR		0.988			0.990			0.969			0.964		0.986
	APP/DEPART	1,911	/	1,811	1,642	/	1,603	1,087	/	1,174	1,462	/	1,514	

U-TURNS

NB	SB	EB	WB	TTL
0	0	0	0	
2	2	0	0	4
2	1	0	0	3
3	2	1	0	6
4	2	3	0	8
4	0	3	0	7
5	1	2	0	8
6	1	4	0	10
5	1	4	0	10
6	3	2	0	12
8	3	2	0	14
9	3	7	0	19
8	4	3	0	16
62	23	32	0	117
7	5	5	0	17
4	10	5	0	19
7	7	4	0	18
3	6	4	0	13
5	7	7	0	19
6	7	5	0	18
8	6	6	0	21
6	6	5	0	17
8	5	8	0	21
7	4	10	0	20
6	6	11	0	23
7	7	5	0	19
9	5	7	0	20
5	6	7	0	18
9	3	5	0	17
8	5	6	0	19
106	94	99	1	299

Right-turn Overlap

NRR	SRR	ERR	WRR
X	X	X	X
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0

Harbor — NORTH SIDE

1st — WEST SIDE — EAST SIDE — 1st

SOUTH SIDE — Harbor

ALL PED AND BIKE

	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL
6:00 AM	4	19	4	12	38
6:15 AM	1	14	3	10	28
6:30 AM	2	17	2	12	33
6:45 AM	3	19	4	16	42
7:00 AM	5	19	7	11	42
7:15 AM	7	25	11	30	74
7:30 AM	6	28	9	29	72
7:45 AM	5	21	6	18	51
8:00 AM	5	18	4	17	45
8:15 AM	7	16	7	17	47
8:30 AM	8	16	4	13	41
8:45 AM	8	22	9	17	55
TOTAL	63	234	71	201	568
AM BEGIN PEAK HR			7:15 AM		
3:00 PM	9	26	3	23	62
3:15 PM	16	35	13	25	90
3:30 PM	15	23	8	28	73
3:45 PM	14	29	9	28	80
4:00 PM	13	27	10	28	78
4:15 PM	13	33	5	25	77
4:30 PM	18	20	10	24	72
4:45 PM	13	27	7	22	69
5:00 PM	7	26	3	26	62
5:15 PM	11	33	7	27	78
5:30 PM	12	18	7	19	56
5:45 PM	13	29	9	29	81
6:00 PM	16	19	10	25	70
6:15 PM	10	23	7	22	62
6:30 PM	10	18	3	18	49
6:45 PM	7	21	4	19	52
TOTAL	197	408	116	390	1,110
PM BEGIN PEAK HR			5:00 PM		

PEDESTRIAN CROSSINGS

N SIDE	S SIDE	E SIDE	W SIDE	TOTAL
3	17	4	10	34
1	12	2	10	25
2	15	2	11	30
2	17	3	14	36
4	17	6	9	36
7	24	11	28	71
6	26	8	26	66
3	19	6	17	45
5	16	4	15	39
5	14	6	15	40
5	14	4	12	34
6	19	8	15	47
48	210	64	181	503
21	85	29	85	220
7	23	3	22	55
15	34	12	25	86
13	22	7	25	67
12	27	8	27	74
11	24	8	24	67
12	32	4	24	72
16	18	10	22	66
12	25	6	19	62
4	24	3	23	55
10	29	5	25	69
10	16	5	17	49
11	26	9	25	70
14	17	9	24	64
8	18	7	19	52
8	16	2	13	40
6	18	4	18	46
170	370	102	352	994
35	95	22	91	243

BICYCLE CROSSINGS

NS	SS	ES	WS	TOTAL
1	1	0	2	5
0	2	1	0	3
0	2	0	1	3
1	2	1	3	6
1	2	0	2	5
0	1	0	2	3
1	2	1	3	7
2	2	1	1	6
0	3	0	2	6
2	2	1	1	6
3	2	1	1	7
2	3	1	1	8
15	24	7	20	65
3	8	2	9	22
2	3	0	2	7
1	1	1	0	4
1	2	0	3	6
3	1	1	1	6
2	3	2	4	11
1	2	1	1	5
2	1	1	2	6
2	2	1	3	7
2	1	1	3	7
1	4	2	2	9
2	2	2	1	7
3	3	1	4	11
1	2	1	1	6
1	5	0	4	10
1	2	1	5	9
1	3	0	2	6
27	38	13	38	116
8	11	5	10	34

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

DATE: Wed, Feb 10, 16

LOCATION: Santa Ana
NORTH & SOUTH: Harbor
EAST & WEST: McFadden

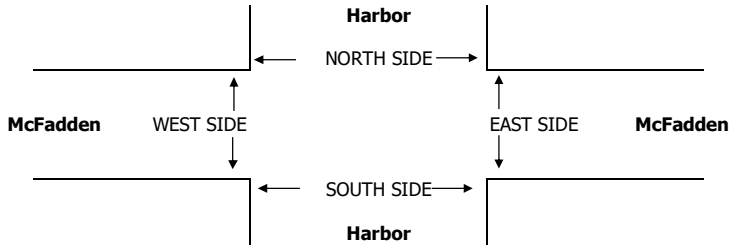
PROJECT #: SC0846
LOCATION #: 32
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Harbor			SOUTHBOUND Harbor			EASTBOUND McFadden			WESTBOUND McFadden				U-TURNS				
	LANES:	NL 2	NT 3	NR 1	SL 2	ST 3	SR 0	EL 1	ET 2	ER 0	WL 1	WT 2	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	21	170	13	33	480	10	31	54	24	35	57	17	945	5	6	0	0	11
	7:15 AM	20	214	17	34	533	10	24	67	18	45	74	21	1,077	3	9	0	0	12
	7:30 AM	19	238	18	26	536	15	36	95	27	51	102	27	1,190	3	4	0	0	7
	7:45 AM	25	260	34	45	488	12	39	94	30	48	117	30	1,222	3	3	0	0	6
	8:00 AM	32	250	25	55	425	22	47	102	36	44	110	33	1,181	5	10	0	0	15
	8:15 AM	23	269	20	48	395	12	38	127	31	39	84	28	1,114	7	9	0	0	16
	8:30 AM	21	245	30	71	407	26	36	95	27	47	85	18	1,108	8	13	0	0	21
	8:45 AM	15	198	28	47	456	15	21	96	30	47	76	23	1,052	7	11	0	0	18
	VOLUMES	176	1,844	185	359	3,720	122	272	730	223	356	705	197	8,889	41	65	0	0	106
	APPROACH %	8%	84%	8%	9%	89%	3%	22%	60%	18%	28%	56%	16%						
	APP/DEPART	2,205	/	2,378	4,201	/	4,340	1,225	/	1,209	1,258	/	962	0					
	BEGIN PEAK HR		7:30 AM																
	VOLUMES	99	1,017	97	174	1,844	61	160	418	124	182	413	118	4,707					
	APPROACH %	8%	84%	8%	8%	89%	3%	23%	60%	18%	26%	58%	17%						
	PEAK HR FACTOR		0.951			0.901			0.895			0.914		0.963					
	APP/DEPART	1,213	/	1,321	2,079	/	2,168	702	/	663	713	/	555	0					
PM	4:00 PM	51	346	34	67	266	22	34	135	19	44	109	47	1,174	5	12	0	0	17
	4:15 PM	35	416	33	67	277	30	50	130	18	53	125	29	1,263	5	13	0	1	19
	4:30 PM	35	421	36	62	284	24	43	122	22	50	99	26	1,224	5	11	0	1	17
	4:45 PM	38	405	36	74	317	29	46	115	14	44	117	31	1,266	3	14	0	0	17
	5:00 PM	35	425	27	74	286	27	50	117	11	49	121	40	1,262	6	18	0	0	24
	5:15 PM	38	429	29	71	305	15	56	129	15	52	125	38	1,302	5	13	0	0	18
	5:30 PM	47	411	34	66	300	32	52	127	20	48	134	35	1,306	5	10	0	1	16
	5:45 PM	40	384	30	75	332	36	47	126	16	53	106	27	1,272	4	10	0	0	14
	VOLUMES	319	3,237	259	556	2,367	215	378	1,001	135	393	936	273	10,069	38	101	0	3	142
	APPROACH %	8%	85%	7%	18%	75%	7%	25%	66%	9%	25%	58%	17%						
	APP/DEPART	3,815	/	3,989	3,138	/	2,930	1,514	/	1,718	1,602	/	1,432	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	160	1,649	120	286	1,223	110	205	499	62	202	486	140	5,142					
	APPROACH %	8%	85%	6%	18%	76%	7%	27%	65%	8%	24%	59%	17%						
	PEAK HR FACTOR		0.972			0.914			0.958			0.954		0.984					
	APP/DEPART	1,929	/	2,045	1,619	/	1,506	766	/	855	828	/	736	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	3	18	5	4	30	3	14	5	3	25	0	4	0	1	5
	7:15 AM	8	22	14	6	50	6	19	12	2	39	2	3	2	4	11
	7:30 AM	11	8	14	9	42	10	8	12	7	37	1	0	2	2	5
	7:45 AM	16	16	13	8	53	15	12	13	6	46	1	4	0	2	7
	8:00 AM	14	27	22	13	76	12	24	21	11	68	2	3	1	2	8
	8:15 AM	12	17	11	12	52	11	17	11	9	48	1	0	0	3	4
	8:30 AM	9	14	9	6	38	8	12	9	3	32	1	2	0	3	6
	8:45 AM	20	14	14	10	58	16	13	10	9	48	4	1	4	1	10
	TOTAL	93	136	102	68	399	81	119	93	50	343	12	17	9	18	56
PM	4:00 PM	4	27	15	11	57	3	25	13	6	47	1	2	2	5	10
	4:15 PM	17	24	33	19	93	14	22	30	13	79	3	2	3	6	14
	4:30 PM	22	27	19	23	91	14	20	15	16	65	8	7	4	7	26
	4:45 PM	22	28	31	19	100	18	26	27	14	85	4	2	4	5	15
	5:00 PM	14	31	21	7	73	8	27	16	3	54	6	4	5	4	19
	5:15 PM	18	32	22	21	93	17	30	18	20	85	1	2	4	1	8
	5:30 PM	19	17	21	16	73	14	14	15	13	56	5	3	6	3	17
	5:45 PM	9	47	18	13	87	7	42	9	13	71	2	5	9	0	16
	TOTAL	125	233	180	129	667	95	206	143	98	542	30	27	37	31	125

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

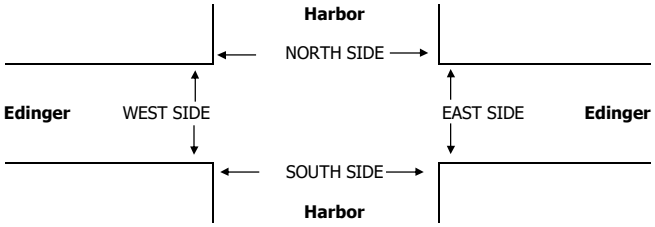
DATE:	LOCATION:	Santa Ana	PROJECT #:	SC2183
Tue, May 14, 19	NORTH & SOUTH:	Harbor	LOCATION #:	5
	EAST & WEST:	Edinger	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ◄ W — N ▲ — S ▼ — E ►

Add U-Turns to Left Turns

		NORTHBOUND Harbor			SOUTHBOUND Harbor			EASTBOUND Edinger			WESTBOUND Edinger				U-TURNS				
	LANES:	NL 2	NT 3	NR 1	SL 2	ST 3	SR 1	EL 2	ET 2.5	ER 0.5	WL 2	WT 3	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	20	139	20	58	478	5	16	144	56	27	90	24	1,077	4	4	0	0	8
	7:15 AM	24	159	40	44	513	12	23	165	79	33	118	49	1,259	5	2	2	1	10
	7:30 AM	28	202	37	55	538	15	25	147	82	63	120	55	1,367	5	5	0	0	10
	7:45 AM	20	226	43	43	552	18	27	138	65	74	125	53	1,384	2	4	1	2	9
	8:00 AM	29	177	24	39	481	15	30	187	93	48	123	44	1,290	3	2	3	1	9
	8:15 AM	20	149	21	43	530	11	33	150	56	39	101	32	1,185	6	3	0	0	9
	8:30 AM	13	164	22	43	489	10	37	129	66	27	105	25	1,130	0	4	1	0	5
	8:45 AM	17	148	25	50	477	6	25	117	47	50	98	28	1,088	2	6	1	0	9
	VOLUMES	171	1,364	232	375	4,058	92	216	1,177	544	361	880	310	9,780	27	30	8	4	69
	APPROACH %	10%	77%	13%	8%	90%	2%	11%	61%	28%	23%	57%	20%						
	APP/DEPART	1,767	/	1,912	4,525	/	4,986	1,937	/	1,758	1,551	/	1,124	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	101	764	144	181	2,084	60	105	637	319	218	486	201	5,300					
	APPROACH %	10%	76%	14%	8%	90%	3%	10%	60%	30%	24%	54%	22%						
	PEAK HR FACTOR		0.873			0.948			0.856			0.898		0.957					
	APP/DEPART	1,009	/	1,077	2,325	/	2,632	1,061	/	953	905	/	638	0					
PM	4:00 PM	58	404	47	60	205	21	41	134	26	47	164	62	1,269	1	5	2	1	9
	4:15 PM	52	416	37	74	229	15	31	127	20	40	203	81	1,325	0	9	1	1	11
	4:30 PM	68	450	43	73	210	20	36	137	26	30	179	72	1,344	1	13	6	1	21
	4:45 PM	65	451	48	63	208	17	35	137	23	25	205	65	1,342	4	4	3	1	12
	5:00 PM	66	434	53	66	197	20	32	128	28	31	190	66	1,311	2	11	7	0	20
	5:15 PM	83	443	48	77	197	18	33	155	18	44	222	62	1,400	4	7	3	0	14
	5:30 PM	65	427	62	74	211	19	32	138	30	38	187	75	1,358	1	14	3	0	18
	5:45 PM	63	410	47	82	198	11	22	154	29	48	190	86	1,340	1	5	3	0	9
	VOLUMES	520	3,435	385	569	1,655	141	262	1,110	200	303	1,540	569	10,689	14	68	28	4	114
	APPROACH %	12%	79%	9%	24%	70%	6%	17%	71%	13%	13%	64%	24%						
	APP/DEPART	4,340	/	4,306	2,365	/	2,168	1,572	/	2,000	2,412	/	2,215	0					
	BEGIN PEAK HR		4:45 PM																
	VOLUMES	279	1,755	211	280	813	74	132	558	99	138	804	268	5,411					
	APPROACH %	12%	78%	9%	24%	70%	6%	17%	71%	13%	11%	66%	22%						
	PEAK HR FACTOR		0.978			0.960			0.958			0.922		0.966					
	APP/DEPART	2,245	/	2,175	1,167	/	1,060	789	/	1,014	1,210	/	1,162	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:15 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	4:45 PM					0	0	0	0	0					

INTERSECTION TURNING MOVEMENT COUNTS

T319

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Tue, Feb 26, 19

LOCATION: Santa Ana
NORTH & SOUTH: Harbor
EAST & WEST: Warner

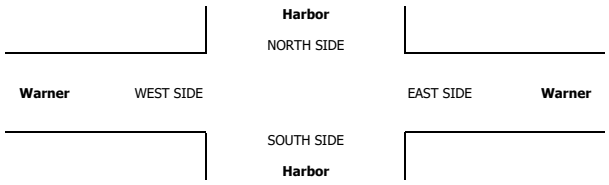
PROJECT #: CMP2019
LOCATION #: 191
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

Add U-Turns to Left Turns

		NORTHBOUND Harbor			SOUTHBOUND Harbor			EASTBOUND Warner			WESTBOUND Warner				U-TURNS					Right-turn Overlap			
	LANES:	NL 2	NT 3	NR 0	SL 2	ST 3	SR 1	EL 2	ET 3	ER 1	WL 2	WT 3	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL	NRR X	SRR X	ERR X	WRR X
AM	6:00 AM	10	71	22	50	246	3	2	129	28	26	87	9	683	5	1	0	2	8	0	0	0	0
AM	6:15 AM	15	56	15	47	302	12	9	136	36	40	99	17	784	3	1	0	5	9	0	0	0	0
AM	6:30 AM	19	70	11	95	470	27	7	166	35	35	113	10	1,058	6	0	0	2	8	0	0	0	0
AM	6:45 AM	19	98	24	99	473	22	12	233	58	41	138	14	1,231	4	0	0	2	6	0	0	0	0
AM	7:00 AM	24	132	33	111	491	13	15	272	73	32	121	14	1,331	9	1	0	6	16	0	0	0	0
AM	7:15 AM	25	154	40	117	523	16	24	358	89	31	194	22	1,593	3	1	1	1	6	0	0	0	0
AM	7:30 AM	40	196	40	104	527	32	26	318	106	33	216	30	1,668	3	0	0	3	6	0	0	0	0
AM	7:45 AM	17	163	38	114	509	36	23	350	78	44	210	37	1,619	5	2	1	3	11	0	0	0	0
AM	8:00 AM	31	181	58	117	479	37	23	345	87	41	164	43	1,606	3	0	1	5	9	0	0	0	0
AM	8:15 AM	28	174	54	107	515	29	25	316	69	38	182	23	1,560	2	0	2	3	7	0	0	0	0
AM	8:30 AM	28	141	42	106	476	29	23	280	95	34	162	27	1,443	1	0	2	3	6	0	0	0	0
AM	8:45 AM	27	140	42	83	512	29	17	224	85	24	188	22	1,393	5	0	0	2	7	0	0	0	0
AM	VOLUMES	283	1,576	419	1,150	5,523	285	206	3,127	839	419	1,874	268	15,969	49	6	7	37	99	0	0	0	0
AM	APPROACH %	12%	69%	18%	17%	79%	4%	5%	75%	20%	16%	73%	10%										
AM	APP/DEPART	2,278	/	2,049	6,958	/	6,793	4,172	/	4,727	2,561	/	2,400	0									
AM	BEGIN PEAK HR		7:15 AM																				
AM	VOLUMES	113	694	176	452	2,038	121	96	1,371	360	149	784	132	6,486						0	0	0	0
AM	APPROACH %	11%	71%	18%	17%	78%	5%	5%	75%	20%	14%	74%	12%										
AM	PEAK HR FACTOR		0.890			0.985			0.970			0.915		0.972									
AM	APP/DEPART	983	/	922	2,611	/	2,549	1,827	/	2,008	1,065	/	1,007	0									
PM	03:00 PM	77	408	43	57	185	22	37	238	49	54	250	91	1,511	6	1	2	2	11	0	0	0	0
PM	3:15 PM	67	424	26	53	198	27	51	227	41	44	267	87	1,512	0	0	2	1	3	0	0	0	0
PM	3:30 PM	82	446	34	39	186	30	21	182	53	50	299	106	1,528	3	0	2	6	11	0	0	0	0
PM	3:45 PM	82	422	30	36	207	24	23	221	53	35	350	120	1,603	0	0	2	1	3	0	0	0	0
PM	4:00 PM	101	467	46	28	163	22	12	224	46	45	337	94	1,585	3	0	0	5	8	0	0	0	0
PM	4:15 PM	84	439	40	37	149	19	32	213	49	22	320	94	1,498	3	1	2	0	6	0	0	0	0
PM	4:30 PM	107	446	53	28	179	21	26	197	40	28	326	106	1,557	2	0	4	0	6	0	0	0	0
PM	4:45 PM	90	478	24	47	214	17	43	190	38	29	321	109	1,600	2	0	3	0	5	0	0	0	0
PM	5:00 PM	100	454	28	32	196	28	52	254	50	43	320	122	1,679	5	0	2	2	9	0	0	0	0
PM	5:15 PM	106	472	31	34	193	27	39	237	47	35	342	99	1,662	8	1	5	0	14	0	0	0	0
PM	5:30 PM	104	498	24	30	208	22	19	154	63	26	357	109	1,614	0	0	0	1	1	0	0	0	0
PM	5:45 PM	117	456	29	32	199	22	28	149	43	34	323	90	1,522	1	0	4	1	6	0	0	0	0
PM	6:00 PM	112	438	27	26	175	20	31	197	50	34	308	73	1,491	1	0	2	2	5	0	0	0	0
PM	6:15 PM	123	381	26	36	166	26	28	163	35	23	313	91	1,411	5	1	0	2	8	0	0	0	0
PM	6:30 PM	110	412	26	33	155	31	18	196	29	29	317	53	1,409	2	1	3	1	7	0	0	0	0
PM	6:45 PM	92	349	32	32	153	19	19	165	30	28	229	54	1,202	1	0	0	1	2	0	0	0	0
PM	VOLUMES	1,554	6,990	519	580	2,926	377	479	3,207	716	559	4,979	1,498	24,384	42	5	33	25	105	0	0	0	0
PM	APPROACH %	17%	77%	6%	15%	75%	10%	11%	73%	16%	8%	71%	21%										
PM	APP/DEPART	9,063	/	8,939	3,883	/	4,218	4,402	/	4,326	7,036	/	6,901	0									
PM	BEGIN PEAK HR		4:45 PM																				
PM	VOLUMES	766	3,676	276	271	1,509	180	238	1,690	386	257	2,673	853	12,775						0	0	0	0
PM	APPROACH %	16%	78%	6%	14%	77%	9%	10%	73%	17%	7%	71%	23%										
PM	PEAK HR FACTOR		1.884			1.763			1.625			1.922		1.902									
PM	APP/DEPART	4,718	/	4,758	1,960	/	2,164	2,314	/	2,239	3,783	/	3,614	0									



		ALL PED AND BIKE					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	6:00 AM	1	0	1	1	3	1	0	1	0	2	0	0	0	1	1
AM	6:15 AM	1	1	2	3	7	1	1	1	0	3	0	0	1	3	4
AM	6:30 AM	0	8	1	0	9	0	7	1	0	8	0	1	0	0	1
AM	6:45 AM	0	6	0	2	8	0	5	0	2	7	0	1	0	0	1
AM	7:00 AM	0	2	1	4	7	0	2	1	4	7	0	0	0	0	0
AM	7:15 AM	6	6	4	1	17	6	6	3	1	16	0	0	1	0	1
AM	7:30 AM	3	7	2	2	14	2	5	1	2	10	1	2	1	0	4
AM	7:45 AM	1	0	0	1	2	0	0	0	1	1	1	0	0	0	1
AM	8:00 AM	4	13	4	6	27	3	13	3	6	25	1	0	1	0	2
AM	8:15 AM	1	0	4	0	5	1	0	4	0	5	0	0	0	0	0
AM	8:30 AM	1	2	0	3	6	1	2	0	3	6	0	0	0	0	0
AM	8:45 AM	1	0	4	5	10	1	0	3	4	8	0	0	1	1	2
AM	TOTAL	19	45	23	28	115	16	41	18	23	98	3	4	5	5	17
AM	AM BEGIN PEAK HR			7:15 AM			11	24	7	10	52	3	2	3	0	8
PM	3:00 PM	0	5	5	0	10	0	5	4	0	9	0	0	1	0	1
PM	3:15 PM	7	2	2	3	14	7	2	2	3	14	0	0	0	0	0
PM	3:30 PM	4	4	6	2	16	3	3	6	2	14	1	1	0	0	2
PM	3:45 PM	1	3	3	3	10	1	1	1	3	6	0	2	2	0	4
PM	4:00 PM	4	8	5	0	17	4	7	5	0	16	0	1	0	0	1
PM	4:15 PM	2	2	0	4	8	2	1	0	3	6	0	1	0	1	2
PM	4:30 PM	1	9	9	2	21	0	9	8	1	18	1	0	1	1	3
PM	4:45 PM	2	2	5	2	11	2	2	4	2	10	0	0	1	0	1
PM	5:00 PM	2	6	8	1	17	0	6	6	1	13	2	0	2	0	4
PM	5:15 PM	0	3	8	1	12	0	2	8	1	11	0	1	0	0	1
PM	5:30 PM	4	2	7	0	13	4	2	7	0	13	0	0	0	0	0
PM	5:45 PM	0	1	2	1	4	0	1	0	1	2	0	0	2	0	2
PM	6:00 PM	2	3	6	3	14	1	3	4	2	10	1	0	2	1	4
PM	6:15 PM	1	1	0	1	3	1	0	0	1	2	0	1	0	0	1
PM	6:30 PM	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0
PM	6:45 PM	2	1	0	1	4	2	0	0	0	2	0	1	0	1	2
PM	TOTAL	33	52	66	24	175	28	44	55	20	147	5	8	11	4	28
PM	PM BEGIN PEAK HR			4:45 PM			6	12	25	4	47	2	1	3	0	6

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

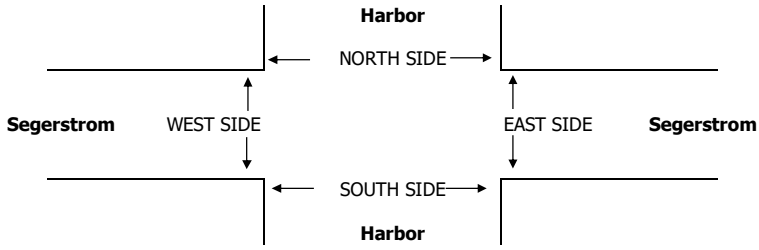
DATE:	LOCATION:	Santa Ana	PROJECT #:	SC0846
Tue, Feb 9, 16	NORTH & SOUTH:	Harbor	LOCATION #:	45
	EAST & WEST:	Segerstrom	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND (Harbor)			SOUTHBOUND (Harbor)			EASTBOUND (Segerstrom)			WESTBOUND (Segerstrom)				U-TURNS				
	LANES:	NL 2	NT 3	NR 0	SL 1	ST 3	SR 0	EL 1	ET 2	ER 0	WL 1	WT 2	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	19	119	13	39	581	13	10	143	46	19	53	13	1,068	0	0	0	0	0
	7:15 AM	21	169	20	29	555	13	21	198	76	15	48	16	1,181	1	0	0	0	1
	7:30 AM	18	180	12	45	588	24	17	193	77	23	65	20	1,262	0	0	0	0	0
	7:45 AM	31	176	11	39	556	20	18	189	66	37	101	25	1,269	4	0	0	0	4
	8:00 AM	35	233	15	38	503	18	30	167	65	32	107	16	1,259	3	1	1	0	5
	8:15 AM	22	171	10	42	562	20	19	159	77	20	74	17	1,193	2	2	0	0	4
	8:30 AM	20	218	15	24	509	16	19	129	76	14	56	10	1,106	1	0	1	0	2
	8:45 AM	19	202	9	28	501	19	24	132	58	16	64	15	1,087	0	0	0	0	0
	VOLUMES	185	1,468	105	284	4,355	143	158	1,310	541	176	568	132	9,425	11	3	2	0	16
	APPROACH %	11%	84%	6%	6%	91%	3%	8%	65%	27%	20%	65%	15%						
	APP/DEPART	1,758	/	1,759	4,782	/	5,083	2,009	/	1,696	876	/	887	0					
	BEGIN PEAK HR		7:30 AM																
	VOLUMES	106	760	48	164	2,209	82	84	708	285	112	347	78	4,983					
	APPROACH %	12%	83%	5%	7%	90%	3%	8%	66%	26%	21%	65%	15%						
	PEAK HR FACTOR		0.807			0.934			0.938			0.824		0.982					
	APP/DEPART	914	/	924	2,455	/	2,615	1,077	/	917	537	/	527	0					
PM	4:00 PM	47	410	21	32	218	18	37	108	46	19	249	74	1,279	1	4	0	0	5
	4:15 PM	50	455	13	19	198	13	27	101	45	22	263	65	1,271	2	0	0	0	2
	4:30 PM	58	456	12	28	230	18	39	112	43	33	237	84	1,350	1	5	0	0	6
	4:45 PM	66	429	10	23	243	18	25	121	50	31	248	87	1,351	2	1	1	0	4
	5:00 PM	67	455	16	21	254	25	40	151	48	25	251	76	1,429	1	1	4	0	6
	5:15 PM	80	454	16	23	262	26	21	144	43	21	271	114	1,475	0	3	1	0	4
	5:30 PM	74	469	15	20	227	18	20	114	32	18	262	97	1,366	1	2	0	0	3
	5:45 PM	52	479	13	15	223	18	18	122	31	16	261	92	1,340	1	0	0	0	1
	VOLUMES	494	3,607	116	181	1,855	154	227	973	338	185	2,042	689	10,861	9	16	6	0	31
	APPROACH %	12%	86%	3%	8%	85%	7%	15%	63%	22%	6%	70%	24%						
	APP/DEPART	4,217	/	4,533	2,190	/	2,387	1,538	/	1,254	2,916	/	2,687	0					
	BEGIN PEAK HR		4:45 PM																
	VOLUMES	287	1,807	57	87	986	87	106	530	173	95	1,032	374	5,621					
	APPROACH %	13%	84%	3%	8%	85%	8%	13%	66%	21%	6%	69%	25%						
	PEAK HR FACTOR		0.964			0.932			0.846			0.924		0.953					
	APP/DEPART	2,151	/	2,288	1,160	/	1,258	809	/	667	1,501	/	1,408	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	1	1	3	5	10	1	0	1	4	6	0	1	2	1	4
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	4	3	2	2	11	1	2	2	1	6	3	1	0	1	5
	7:45 AM	3	1	0	6	10	2	0	0	4	6	1	1	0	2	4
	8:00 AM	0	1	0	2	3	0	1	0	1	2	0	0	0	1	1
	8:15 AM	0	3	0	6	9	0	1	0	5	6	0	2	0	1	3
	8:30 AM	3	4	0	2	9	0	2	0	1	3	3	2	0	1	6
	8:45 AM	2	0	1	2	5	1	0	0	2	3	1	0	1	0	2
	TOTAL	13	13	6	25	57	5	6	3	18	32	8	7	3	7	25
PM	4:00 PM	0	0	3	0	3	0	0	0	0	0	0	0	3	0	3
	4:15 PM	0	0	3	1	4	0	0	2	0	2	0	0	1	1	2
	4:30 PM	2	6	0	4	12	0	1	0	4	5	2	5	0	0	7
	4:45 PM	2	0	1	0	3	2	0	0	0	2	0	0	1	0	1
	5:00 PM	2	3	1	2	8	2	3	0	1	6	0	0	1	1	2
	5:15 PM	2	0	0	0	2	1	0	0	0	1	1	0	0	0	1
	5:30 PM	2	3	1	1	7	1	0	1	1	3	1	3	0	0	4
	5:45 PM	0	0	0	1	1	0	0	0	0	0	0	0	0	1	1
	TOTAL	10	12	9	9	40	6	4	3	6	19	4	8	6	3	21

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

DATE:	LOCATION:	Santa Ana	PROJECT #:	SC0846
Tue, Mar 1, 16	NORTH & SOUTH:	Hyland	LOCATION #:	9
	EAST & WEST:	MacArthur	CONTROL:	SIGNAL

NOTES:

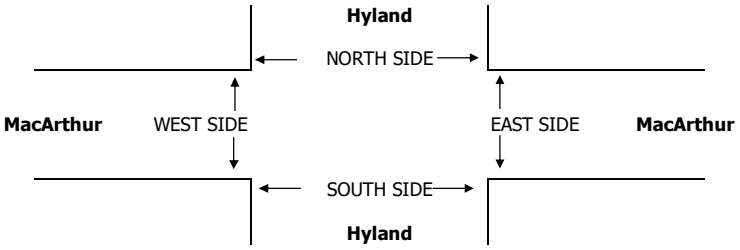
AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Hyland			SOUTHBOUND Hyland			EASTBOUND MacArthur			WESTBOUND MacArthur			
	LANES:	NL 2	NT 0.5	NR 0.5	SL 1	ST 1	SR 1	EL 1	ET 3	ER 1	WL 1	WT 3	WR 1	TOTAL
AM	7:00 AM	10	0	8	2	1	1	1	258	105	12	78	3	479
	7:15 AM	10	0	2	2	0	13	2	434	157	10	88	5	723
	7:30 AM	20	0	6	1	0	2	0	505	178	12	92	6	822
	7:45 AM	10	0	8	1	1	2	3	491	226	22	138	2	904
	8:00 AM	26	2	2	3	0	1	1	446	183	17	152	6	839
	8:15 AM	13	1	5	5	0	0	3	412	154	13	106	4	716
	8:30 AM	9	2	6	1	0	3	5	415	134	20	107	6	708
	8:45 AM	21	4	13	4	1	0	6	319	111	13	127	4	623
	VOLUMES	119	9	50	19	3	22	21	3,280	1,248	119	888	36	5,814
	APPROACH %	67%	5%	28%	43%	7%	50%	0%	72%	27%	11%	85%	3%	
	APP/DEPART	178	/	65	44	/	1,369	4,549	/	3,350	1,043	/	1,030	0
	BEGIN PEAK HR		7:15 AM											
	VOLUMES	66	2	18	7	1	18	6	1,876	744	61	470	19	3,288
	APPROACH %	77%	2%	21%	27%	4%	69%	0%	71%	28%	11%	85%	3%	
	PEAK HR FACTOR		0.717			0.433			0.912			0.786		0.909
	APP/DEPART	86	/	26	26	/	805	2,626	/	1,902	550	/	555	0
PM	4:00 PM	250	2	18	4	1	6	6	192	29	6	476	2	992
	4:15 PM	254	3	10	3	0	8	3	184	34	3	582	3	1,087
	4:30 PM	270	0	15	6	3	2	6	227	45	10	532	5	1,121
	4:45 PM	282	0	14	3	2	1	9	200	45	6	561	1	1,124
	5:00 PM	284	2	24	3	0	2	7	218	32	10	556	1	1,139
	5:15 PM	302	2	18	3	1	7	9	233	38	3	556	5	1,177
	5:30 PM	286	2	11	2	0	3	9	205	38	5	585	4	1,150
	5:45 PM	280	2	6	2	2	4	7	195	44	3	566	5	1,116
	VOLUMES	2,208	13	116	26	9	33	56	1,654	305	46	4,414	26	8,906
	APPROACH %	94%	1%	5%	38%	13%	49%	3%	82%	15%	1%	98%	1%	
	APP/DEPART	2,337	/	70	68	/	358	2,015	/	1,798	4,486	/	6,680	0
	BEGIN PEAK HR		4:45 PM											
	VOLUMES	1,154	6	67	11	3	13	34	856	153	24	2,258	11	4,590
	APPROACH %	94%	0%	5%	41%	11%	48%	3%	82%	15%	1%	98%	0%	
	PEAK HR FACTOR		0.953			0.614			0.931			0.965		0.975
	APP/DEPART	1,227	/	38	27	/	178	1,043	/	936	2,293	/	3,438	0

U-TURNS

NB	SB	EB	WB	TTL
0	0	0	0	
0	0	0	0	0
0	0	0	1	1
0	0	0	0	0
0	0	0	0	0
0	0	0	0	0
0	0	0	0	0
0	0	1	0	1
0	0	0	0	0
0	0	0	0	0
0	0	1	1	2
0	0	3	0	3
0	0	2	0	2
0	0	5	0	5
0	0	4	1	5
0	0	4	0	4
0	0	4	1	5
0	0	1	0	1
0	0	2	0	2
0	0	25	2	27



PEDESTRIAN + BIKE CROSSINGS

	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL
7:00 AM	0	0	0	0	0
7:15 AM	2	2	1	0	5
7:30 AM	0	1	0	0	1
7:45 AM	3	2	0	0	5
8:00 AM	1	0	0	0	1
8:15 AM	5	0	0	0	5
8:30 AM	1	1	0	0	2
8:45 AM	1	0	0	0	1
TOTAL	13	6	1	0	20
4:00 PM	4	2	0	0	6
4:15 PM	4	1	2	0	7
4:30 PM	3	0	2	0	5
4:45 PM	1	5	3	0	9
5:00 PM	2	1	3	0	6
5:15 PM	3	1	1	0	5
5:30 PM	8	0	0	0	8
5:45 PM	1	1	0	0	2
TOTAL	26	11	11	0	48

PEDESTRIAN CROSSINGS

N SIDE	S SIDE	E SIDE	W SIDE	TOTAL
0	0	0	0	0
1	2	0	0	3
0	1	0	0	1
1	2	0	0	3
0	0	0	0	0
5	0	0	0	5
0	0	0	0	0
1	0	0	0	1
8	5	0	0	13
1	2	0	0	3
2	0	1	0	3
0	0	1	0	1
0	3	2	0	5
0	1	2	0	3
1	0	1	0	2
1	0	0	0	1
1	1	0	0	2
6	7	7	0	20

BICYCLE CROSSINGS

NS	SS	ES	WS	TOTAL
0	0	0	0	0
1	0	1	0	2
0	0	0	0	0
2	0	0	0	2
1	0	0	0	1
0	0	0	0	0
1	1	0	0	2
0	0	0	0	0
5	1	1	0	7
3	0	0	0	3
2	1	1	0	4
3	0	1	0	4
1	2	1	0	4
2	0	1	0	3
2	1	0	0	3
7	0	0	0	7
0	0	0	0	0
20	4	4	0	28

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

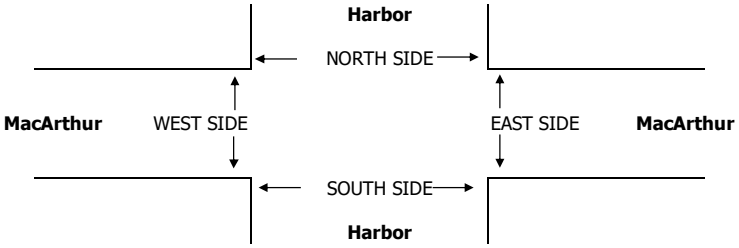
DATE:	LOCATION:	Costa Mesa	PROJECT #:	SC
Tue, May 22, 18	NORTH & SOUTH:	Harbor	LOCATION #:	1
	EAST & WEST:	MacArthur	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Harbor			SOUTHBOUND Harbor			EASTBOUND MacArthur			WESTBOUND MacArthur				U-TURNS				
	LANES:	NL 2	NT 3	NR 1	SL 2	ST 3	SR 1	EL 1	ET 3	ER 1	WL 1	WT 3	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	21	181	7	74	415	39	33	217	36	14	54	16	1,107	0	0	0	3	3
	7:15 AM	18	188	9	70	474	26	37	249	65	16	78	28	1,258	0	2	0	3	5
	7:30 AM	27	197	19	89	468	20	36	322	102	29	93	38	1,440	3	0	0	2	5
	7:45 AM	33	212	24	79	422	34	23	293	74	39	125	29	1,387	0	3	0	10	13
	8:00 AM	41	251	24	59	454	35	32	254	76	33	119	38	1,416	1	1	0	10	12
	8:15 AM	24	220	23	82	440	34	32	249	85	29	97	24	1,339	3	0	0	6	9
	8:30 AM	39	192	21	73	412	40	30	212	68	31	80	26	1,224	3	0	0	9	12
	8:45 AM	36	203	23	75	438	39	26	177	78	28	76	28	1,227	4	0	0	6	10
	VOLUMES	239	1,644	150	601	3,523	267	249	1,973	584	219	722	227	10,398	14	6	0	49	69
	APPROACH %	12%	81%	7%	14%	80%	6%	9%	70%	21%	19%	62%	19%						
	APP/DEPART	2,033	/	2,126	4,391	/	4,291	2,806	/	2,767	1,168	/	1,214	0					
	BEGIN PEAK HR		7:30 AM																
	VOLUMES	125	880	90	309	1,784	123	123	1,118	337	130	434	129	5,582					
	APPROACH %	11%	80%	8%	14%	81%	6%	8%	71%	21%	19%	63%	19%						
	PEAK HR FACTOR		0.866			0.960			0.858			0.898		0.969					
	APP/DEPART	1,095	/	1,136	2,216	/	2,230	1,578	/	1,541	693	/	675	0					
PM	4:00 PM	131	418	18	47	237	38	37	138	45	26	358	73	1,566	4	4	0	7	15
	4:15 PM	138	393	32	43	250	41	23	110	41	22	362	55	1,510	3	1	2	8	14
	4:30 PM	144	377	26	48	291	32	38	125	35	32	340	81	1,569	2	7	1	9	19
	4:45 PM	144	358	23	50	244	47	30	105	47	13	370	53	1,484	0	1	0	5	6
	5:00 PM	141	355	22	64	284	67	37	126	55	18	369	61	1,599	1	2	3	2	8
	5:15 PM	147	399	15	46	232	26	32	123	40	22	359	75	1,516	1	3	2	6	12
	5:30 PM	112	407	13	50	253	29	45	99	43	20	346	86	1,503	2	3	3	6	14
	5:45 PM	130	411	16	51	220	34	29	98	30	14	356	69	1,458	0	3	1	6	10
	VOLUMES	1,087	3,118	165	399	2,011	314	271	924	336	167	2,860	553	12,205	13	24	12	49	98
	APPROACH %	25%	71%	4%	15%	74%	12%	18%	60%	22%	5%	80%	15%						
	APP/DEPART	4,370	/	3,954	2,724	/	2,478	1,531	/	1,513	3,580	/	4,260	0					
	BEGIN PEAK HR		4:30 PM																
	VOLUMES	576	1,489	86	208	1,051	172	137	479	177	85	1,438	270	6,168					
	APPROACH %	27%	69%	4%	15%	73%	12%	17%	60%	22%	5%	80%	15%						
	PEAK HR FACTOR		0.959			0.862			0.909			0.983		0.964					
	APP/DEPART	2,151	/	1,903	1,431	/	1,295	793	/	782	1,793	/	2,188	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:30 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	4:30 PM					0	0	0	0	0					

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

DATE: Tue, Mar 1, 16

LOCATION: Santa Ana
NORTH & SOUTH: Harbor
EAST & WEST: Sunflower

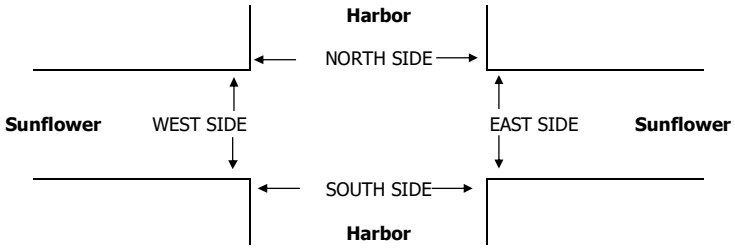
PROJECT #: SC0846
LOCATION #: 11
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — N ▲ / ◄ W / E ► / S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Harbor			SOUTHBOUND Harbor			EASTBOUND Sunflower			WESTBOUND Sunflower				U-TURNS				
	LANES:	NL 2	NT 3	NR 0	SL 2	ST 3	SR 1	EL 1	ET 2	ER 0	WL 1.5	WT 1.5	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	33	170	24	36	445	5	5	22	7	17	31	8	803	0	0	0	0	0
	7:15 AM	48	224	32	58	556	15	0	21	6	16	16	13	1,005	1	0	0	0	1
	7:30 AM	39	272	53	71	616	8	3	32	17	26	34	24	1,195	0	0	0	0	0
	7:45 AM	52	334	70	57	530	16	1	33	8	41	58	19	1,219	2	1	0	0	3
	8:00 AM	69	233	42	50	531	15	4	18	9	41	51	34	1,097	0	0	0	0	0
	8:15 AM	69	246	40	56	453	14	0	17	14	26	34	14	983	1	1	0	0	2
	8:30 AM	54	268	40	62	454	18	2	8	11	13	26	15	971	1	4	0	0	5
	8:45 AM	62	241	27	39	463	13	7	20	8	26	26	15	947	2	1	0	0	3
	VOLUMES	426	1,988	328	429	4,048	104	22	171	80	206	276	142	8,220	7	7	0	0	14
	APPROACH %	16%	73%	12%	9%	88%	2%	8%	63%	29%	33%	44%	23%						
	APP/DEPART	2,742	/	2,159	4,581	/	4,341	273	/	921	624	/	799	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	208	1,063	197	236	2,233	54	8	104	40	124	159	90	4,516					
	APPROACH %	14%	72%	13%	9%	89%	2%	5%	68%	26%	33%	43%	24%						
	PEAK HR FACTOR		0.805			0.908			0.731			0.740		0.926					
	APP/DEPART	1,468	/	1,162	2,523	/	2,400	152	/	536	373	/	418	0					
PM	4:00 PM	39	466	41	31	338	13	12	29	39	65	92	33	1,198	3	3	0	0	6
	4:15 PM	47	493	42	26	300	6	8	21	29	51	110	39	1,172	2	1	0	0	3
	4:30 PM	37	463	58	18	355	11	20	45	57	65	106	44	1,279	2	2	0	0	4
	4:45 PM	48	467	53	30	326	12	9	30	32	55	127	58	1,247	1	2	0	0	3
	5:00 PM	46	358	65	28	330	9	28	56	50	68	132	32	1,202	4	3	0	0	7
	5:15 PM	40	413	76	31	327	8	7	43	42	56	147	57	1,247	1	1	0	0	2
	5:30 PM	41	440	70	35	306	8	8	32	30	53	148	53	1,224	2	3	0	0	5
	5:45 PM	49	485	59	25	266	10	4	31	29	61	151	46	1,216	1	1	0	2	4
	VOLUMES	347	3,585	464	224	2,548	77	96	287	308	474	1,013	362	9,785	16	16	0	2	34
	APPROACH %	8%	82%	11%	8%	89%	3%	14%	42%	45%	26%	55%	20%						
	APP/DEPART	4,396	/	4,059	2,849	/	3,344	691	/	961	1,849	/	1,421	0					
	BEGIN PEAK HR		4:30 PM																
	VOLUMES	171	1,701	252	107	1,338	40	64	174	181	244	512	191	4,975					
	APPROACH %	8%	80%	12%	7%	90%	3%	15%	42%	43%	26%	54%	20%						
	PEAK HR FACTOR		0.935			0.967			0.782			0.911		0.972					
	APP/DEPART	2,124	/	1,964	1,485	/	1,771	419	/	525	947	/	715	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0
	7:15 AM	2	0	0	2	4	2	0	0	0	2	0	0	0	2	2
	7:30 AM	1	1	1	1	4	1	0	1	1	3	0	1	0	0	1
	7:45 AM	4	2	2	2	10	3	0	2	2	7	1	2	0	0	3
	8:00 AM	0	3	0	0	3	0	0	0	0	0	0	3	0	0	3
	8:15 AM	2	1	2	3	8	2	0	2	1	5	0	1	0	2	3
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	4	1	4	3	12	2	0	2	1	5	2	1	2	2	7
	TOTAL	14	8	9	11	42	11	0	7	5	23	3	8	2	6	19
PM	4:00 PM	2	1	4	1	8	1	1	2	0	4	1	0	2	1	4
	4:15 PM	2	0	2	1	5	1	0	1	1	3	1	0	1	0	2
	4:30 PM	3	4	2	2	11	0	0	2	1	3	3	4	0	1	8
	4:45 PM	5	1	3	1	10	3	0	3	1	7	2	1	0	0	3
	5:00 PM	2	1	1	2	6	1	0	0	1	2	1	1	1	1	4
	5:15 PM	2	1	3	1	7	1	0	1	0	2	1	1	2	1	5
	5:30 PM	3	2	2	0	7	2	0	2	0	4	1	2	0	0	3
	5:45 PM	1	1	4	0	6	0	0	4	0	4	1	1	0	0	2
	TOTAL	20	11	21	8	60	9	1	15	4	29	11	10	6	4	31

INTERSECTION TURNING MOVEMENT COUNTS

T319

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Tue, Mar 12, 19

LOCATION: Costa Mesa
NORTH & SOUTH: Harbor
EAST & WEST: NB I-405 Ramps

PROJECT #: CMP2019
LOCATION #: 48
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

Add U-Turns to Left Turns

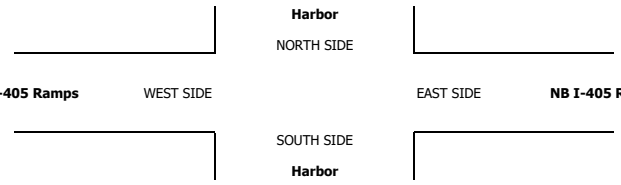
		NORTHBOUND Harbor			SOUTHBOUND Harbor			EASTBOUND NB I-405 Ramps			WESTBOUND NB I-405 Ramps			
	LANES:	NL X	NT 4	NR 2	SL X	ST 5	SR X	EL X	ET X	ER X	WL 1,5	WT X	WR 1,5	TOTAL
AM	6:00 AM	0	118	82	0	193	0	0	0	0	29	0	76	498
	6:15 AM	0	95	79	0	234	0	0	0	0	42	0	87	537
	6:30 AM	0	157	99	0	369	0	0	0	0	51	0	88	764
	6:45 AM	0	177	90	0	441	0	0	0	0	53	0	179	940
	7:00 AM	0	200	119	0	448	0	0	0	0	77	0	156	1,000
	7:15 AM	0	261	133	0	551	0	0	0	0	80	0	169	1,194
	7:30 AM	0	305	132	0	623	0	0	0	0	114	0	172	1,346
	7:45 AM	0	381	161	0	628	0	0	0	0	157	0	230	1,557
	8:00 AM	0	404	130	0	525	0	0	0	0	111	0	204	1,374
	8:15 AM	0	384	135	0	571	0	0	0	0	106	0	211	1,407
	8:30 AM	0	382	144	0	579	0	0	0	0	128	0	240	1,473
	8:45 AM	0	369	112	0	541	0	0	0	0	127	0	218	1,367
	VOLUMES	0	3,233	1,416	0	5,703	0	0	0	0	1,075	0	2,030	13,457
	APPROACH %	0%	70%	30%	0%	100%	0%	0%	0%	0%	35%	0%	65%	
	APP/DEPART	4,649	/	5,263	5,703	/	6,778	0	/	1,416	3,105	/	0	0
	BEGIN PEAK HR		7:45 AM											
	VOLUMES	0	1,551	570	0	2,303	0	0	0	0	502	0	885	5,811
	APPROACH %	0%	73%	27%	0%	100%	0%	0%	0%	0%	36%	0%	64%	
	PEAK HR FACTOR		0.978			0.917			0.000			0.896		0.933
	APP/DEPART	2,121	/	2,436	2,303	/	2,805	0	/	570	1,387	/	0	0
PM	03:00 PM	0	395	118	0	488	0	0	0	0	193	0	240	1,434
	3:15 PM	0	391	135	0	465	0	0	0	0	194	0	233	1,418
	3:30 PM	0	358	113	0	474	0	0	0	0	193	0	291	1,429
	3:45 PM	0	398	105	0	480	0	0	0	0	172	0	268	1,423
	4:00 PM	0	385	137	0	543	0	0	0	0	174	0	280	1,519
	4:15 PM	0	409	151	0	493	0	0	0	0	192	0	267	1,512
	4:30 PM	0	407	156	0	547	0	0	0	0	173	0	287	1,570
	4:45 PM	0	368	112	0	556	0	0	0	0	159	0	301	1,496
	5:00 PM	0	381	188	0	664	0	0	0	0	183	0	265	1,681
	5:15 PM	0	357	148	0	683	0	0	0	0	179	0	267	1,634
	5:30 PM	0	422	117	0	553	0	0	0	0	178	0	241	1,511
	5:45 PM	0	331	139	0	485	0	0	0	0	187	0	234	1,376
	6:00 PM	0	342	155	0	464	0	0	0	0	196	0	242	1,399
	6:15 PM	0	336	124	0	437	0	0	0	0	211	0	260	1,368
	6:30 PM	0	333	79	0	398	0	0	0	0	204	0	233	1,247
	6:45 PM	0	302	89	0	370	0	0	0	0	206	0	180	1,147
	VOLUMES	0	5,915	2,066	0	8,100	0	0	0	0	2,994	0	4,089	23,164
	APPROACH %	0%	74%	26%	0%	100%	0%	0%	0%	0%	42%	0%	58%	
	APP/DEPART	7,981	/	10,004	8,100	/	11,094	0	/	2,066	7,083	/	0	0
	BEGIN PEAK HR		4:30 PM											
	VOLUMES	0	1,513	604	0	2,450	0	0	0	0	694	0	1,120	6,381
	APPROACH %	0%	71%	29%	0%	100%	0%	0%	0%	0%	38%	0%	62%	
	PEAK HR FACTOR		0.930			0.897			0.000			0.986		0.949
	APP/DEPART	2,117	/	2,633	2,450	/	3,144	0	/	604	1,814	/	0	0

U-TURNS

NB	SB	EB	WB	TTL
0	0	0	0	
0	0	0	0	0
0	0	0	0	0
0	0	0	0	0
0	0	0	0	0
0	0	0	0	0
0	0	0	0	0
0	0	0	0	0
0	0	0	0	0
0	0	0	0	0
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0	0	0	0	0
0	0	0	0	0
0	0	0	0	0
0	0	0	0	0
0	0	0	0	0

Right-turn Overlap

NRR X	SRR X	ERR X	WRR X
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0



ALL PED AND BIKE

	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL
6:00 AM	0	0	0	0	0
6:15 AM	0	1	0	0	1
6:30 AM	0	0	1	0	1
6:45 AM	0	1	0	0	1
7:00 AM	0	0	1	0	1
7:15 AM	0	0	2	0	2
7:30 AM	0	0	0	0	0
7:45 AM	0	0	0	0	0
8:00 AM	0	3	0	0	3
8:15 AM	0	1	0	0	1
8:30 AM	0	0	4	0	4
8:45 AM	0	0	2	0	2
TOTAL	0	6	10	0	16
AM BEGIN PEAK HR			7:45 AM		
3:00 PM	0	0	0	0	0
3:15 PM	0	3	1	0	4
3:30 PM	0	0	0	0	0
3:45 PM	0	0	3	0	3
4:00 PM	0	1	1	0	2
4:15 PM	0	0	1	0	1
4:30 PM	0	1	2	0	3
4:45 PM	0	0	1	0	1
5:00 PM	0	2	1	0	3
5:15 PM	0	0	1	0	1
5:30 PM	0	1	0	0	1
5:45 PM	0	2	5	0	7
6:00 PM	0	5	0	0	5
6:15 PM	0	2	0	0	2
6:30 PM	0	1	1	0	2
6:45 PM	0	1	0	0	1
TOTAL	0	19	17	0	36
PM BEGIN PEAK HR			4:30 PM		

PEDESTRIAN CROSSINGS

	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL
6:00 AM	0	0	0	0	0
6:15 AM	0	0	0	0	0
6:30 AM	0	0	1	0	1
6:45 AM	0	0	0	0	0
7:00 AM	0	0	1	0	1
7:15 AM	0	0	2	0	2
7:30 AM	0	0	0	0	0
7:45 AM	0	0	0	0	0
8:00 AM	0	0	0	0	0
8:15 AM	0	0	0	0	0
8:30 AM	0	0	4	0	4
8:45 AM	0	0	2	0	2
TOTAL	0	0	10	0	10
AM BEGIN PEAK HR	0	0	4	0	4
3:00 PM	0	0	0	0	0
3:15 PM	0	0	1	0	1
3:30 PM	0	0	0	0	0
3:45 PM	0	0	3	0	3
4:00 PM	0	0	1	0	1
4:15 PM	0	0	1	0	1
4:30 PM	0	0	2	0	2
4:45 PM	0	0	1	0	1
5:00 PM	0	0	1	0	1
5:15 PM	0	0	1	0	1
5:30 PM	0	0	0	0	0
5:45 PM	0	0	5	0	5
6:00 PM	0	0	0	0	0
6:15 PM	0	0	0	0	0
6:30 PM	0	0	1	0	1
6:45 PM	0	0	0	0	0
TOTAL	0	0	17	0	17
PM BEGIN PEAK HR	0	0	5	0	5

BICYCLE CROSSINGS

	NS	SS	ES	WS	TOTAL
6:00 AM	0	0	0	0	0
6:15 AM	0	1	0	0	1
6:30 AM	0	0	0	0	0
6:45 AM	0	1	0	0	1
7:00 AM	0	0	0	0	0
7:15 AM	0	0	0	0	0
7:30 AM	0	0	0	0	0
7:45 AM	0	0	0	0	0
8:00 AM	0	3	0	0	3
8:15 AM	0	1	0	0	1
8:30 AM	0	0	0	0	0
8:45 AM	0	0	0	0	0
TOTAL	0	6	0	0	6
AM BEGIN PEAK HR	0	4	0	0	4
3:00 PM	0	0	0	0	0
3:15 PM	0	3	0	0	3
3:30 PM	0	0	0	0	0
3:45 PM	0	0	0	0	0
4:00 PM	0	1	0	0	1
4:15 PM	0	0	0	0	0
4:30 PM	0	1	0	0	1
4:45 PM	0	0	0	0	0
5:00 PM	0	2	0	0	2
5:15 PM	0	0	0	0	0
5:30 PM	0	1	0	0	1
5:45 PM	0	2	0	0	2
6:00 PM	0	5	0	0	5
6:15 PM	0	2	0	0	2
6:30 PM	0	1	0	0	1
6:45 PM	0	1	0	0	1
TOTAL	0	19	0	0	19
PM BEGIN PEAK HR	0	3	0	0	3

INTERSECTION TURNING MOVEMENT COUNTS

T319

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Tue, Apr 16, 19

LOCATION: Costa Mesa
NORTH & SOUTH: Harbor
EAST & WEST: SB I-405 Ramps

PROJECT #: CMP2019
LOCATION #: 42
CONTROL: SIGNAL

NOTES:

AM
PM
MD
OTHER
OTHER

N
W
E
S

Add U-Turns to Left Turns

		NORTHBOUND Harbor			SOUTHBOUND Harbor			EASTBOUND SB I-405 Ramps			WESTBOUND SB I-405 Ramps				U-TURNS					Right-turn Overlap			
	LANES:	NL X	NT 3.5	NR 1.5	SL X	ST 4	SR 1	EL 1.5	ET X	ER 1.5	WL X	WT X	WR X	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL	NRR X	SRR X	ERR X	WRR X
AM	6:00 AM	0	161	102	0	99	131	63	0	95	0	0	0	651	0	0	0	0	0	0	0	0	0
	6:15 AM	0	160	146	0	146	177	59	0	71	0	0	0	759	0	0	0	0	0	0	0	0	0
	6:30 AM	0	197	226	0	204	258	49	0	86	0	0	0	1,020	0	0	0	0	0	0	0	0	0
	6:45 AM	0	260	244	0	210	229	52	0	90	0	0	0	1,085	0	0	0	0	0	0	0	0	0
	7:00 AM	0	269	289	0	258	313	42	0	110	0	0	0	1,281	0	0	0	0	0	0	0	0	0
	7:15 AM	0	313	349	0	303	349	74	0	89	0	0	0	1,477	0	0	0	0	0	0	0	0	0
	7:30 AM	0	389	353	0	386	344	105	0	96	0	0	0	1,673	0	0	0	0	0	0	0	0	0
	7:45 AM	0	475	309	0	332	271	80	0	99	0	0	0	1,566	0	0	0	0	0	0	0	0	0
	8:00 AM	0	431	327	0	398	266	104	0	113	0	0	0	1,639	0	0	0	0	0	0	0	0	0
	8:15 AM	0	416	277	0	373	260	92	0	116	0	0	0	1,534	0	0	0	0	0	0	0	0	0
	8:30 AM	0	400	276	0	400	265	109	0	127	0	0	0	1,577	0	0	0	0	0	0	0	0	0
	8:45 AM	0	316	258	0	430	198	112	0	120	0	0	0	1,434	0	0	0	0	0	0	0	0	0
	VOLUMES	0	3,787	3,156	0	3,539	3,061	941	0	1,212	0	0	0	15,696	0	0	0	0	0	0	0	0	0
	APPROACH %	0%	55%	45%	0%	54%	46%	44%	0%	56%	0%	0%	0%										
	APP/DEPART	6,943	/	4,728	6,600	/	4,751	2,153	/	3,156	0	/	3,061	0									
	BEGIN PEAK HR		7:30 AM																				
	VOLUMES	0	1,711	1,266	0	1,489	1,141	381	0	424	0	0	0	6,412						0	0	0	0
	APPROACH %	0%	57%	43%	0%	57%	43%	47%	0%	53%	0%	0%	0%										
	PEAK HR FACTOR		0.949			0.901			0.927			0.000		0.958									
	APP/DEPART	2,977	/	2,092	2,630	/	1,913	805	/	1,266	0	/	1,141	0									
PM	03:00 PM	0	468	180	0	456	213	51	0	139	0	0	0	1,507	0	0	0	0	0	0	0	0	0
	3:15 PM	0	467	199	0	444	191	46	0	139	0	0	0	1,486	0	0	0	0	0	0	0	0	0
	3:30 PM	0	411	169	0	428	203	52	0	180	0	0	0	1,443	0	0	0	0	0	0	0	0	0
	3:45 PM	0	478	205	0	447	205	44	0	163	0	0	0	1,542	0	0	0	0	0	0	0	0	0
	4:00 PM	0	483	188	0	495	250	40	0	167	0	0	0	1,623	0	1	0	0	1	0	0	0	0
	4:15 PM	0	454	201	0	465	222	37	0	161	0	0	0	1,540	0	0	0	0	0	0	0	0	0
	4:30 PM	0	440	156	0	508	241	35	0	178	0	0	0	1,558	0	0	0	0	0	0	0	0	0
	4:45 PM	0	451	171	0	546	256	41	0	165	0	0	0	1,630	0	0	0	0	0	0	0	0	0
	5:00 PM	0	503	170	0	534	352	43	0	172	0	0	0	1,774	0	0	0	0	0	0	0	0	0
	5:15 PM	0	448	170	0	537	299	36	0	199	0	0	0	1,689	0	0	0	0	0	0	0	0	0
	5:30 PM	0	448	163	0	497	241	23	0	170	0	0	0	1,542	0	0	0	0	0	0	0	0	0
	5:45 PM	0	427	182	0	511	211	30	0	184	0	0	0	1,545	0	0	0	0	0	0	0	0	0
	6:00 PM	0	406	178	0	451	222	43	0	158	0	0	0	1,458	0	0	0	0	0	0	0	0	0
	6:15 PM	0	426	200	0	469	175	40	0	169	0	0	0	1,479	0	0	0	0	0	0	0	0	0
	6:30 PM	0	399	180	0	443	148	46	0	160	0	0	0	1,376	0	1	0	0	1	0	0	0	0
	6:45 PM	0	351	141	0	381	133	39	0	138	0	0	0	1,183	0	0	0	0	0	0	0	0	0
	VOLUMES	0	7,060	2,853	0	7,612	3,562	646	0	2,642	0	0	0	24,377	0	2	0	0	2	0	0	0	0
	APPROACH %	0%	71%	29%	0%	68%	32%	20%	0%	80%	0%	0%	0%										
	APP/DEPART	9,913	/	7,708	11,176	/	10,254	3,288	/	2,853	0	/	3,562	0									
	BEGIN PEAK HR		4:30 PM																				
	VOLUMES	0	1,842	667	0	2,125	1,148	155	0	714	0	0	0	6,651						0	0	0	0
	APPROACH %	0%	73%	27%	0%	65%	35%	18%	0%	82%	0%	0%	0%										
	PEAK HR FACTOR		0.932			0.924			0.924			0.000		0.937									
	APP/DEPART	2,509	/	1,997	3,273	/	2,839	869	/	667	0	/	1,148	0									

Harbor

NORTH SIDE

SB I-405 Ramps WEST SIDE EAST SIDE **SB I-405 Ramps**

SOUTH SIDE

Harbor

		ALL PED AND BIKE					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	6:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	6:15 AM	0	0	2	1	3	0	0	0	0	0	0	0	2	1	3
	6:30 AM	0	0	2	0	2	0	0	1	0	1	0	0	1	0	1
	6:45 AM	0	0	2	0	2	0	0	2	0	2	0	0	0	0	0
	7:00 AM	0	0	3	0	3	0	0	2	0	2	0	0	1	0	1
	7:15 AM	0	0	2	0	2	0	0	1	0	1	0	0	1	0	1
	7:30 AM	0	0	2	0	2	0	0	2	0	2	0	0	0	0	0
	7:45 AM	0	0	5	0	5	0	0	4	0	4	0	0	1	0	1
	8:00 AM	0	0	2	0	2	0	0	0	0	0	0	0	2	0	2
	8:15 AM	0	0	3	0	3	0	0	3	0	3	0	0	0	0	0
	8:30 AM	0	0	2	0	2	0	0	1	0	1	0	0	1	0	1
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	25	1	26	0	0	16	0	16	0	0	9	1	10
	AM BEGIN PEAK HR			7:30 AM			0	0	9	0	9	0	0	3	0	3
PM	3:00 PM	0	0	0	1	1	0	0	0	1	1	0	0	0	0	0
	3:15 PM	0	0	2	0	2	0	0	0	0	0	0	0	2	0	2
	3:30 PM	0	0	4	1	5	0	0	2	0	2	0	0	2	1	3
	3:45 PM	0	0	3	0	3	0	0	1	0	1	0	0	2	0	2
	4:00 PM	0	0	3	0	3	0	0	1	0	1	0	0	2	0	2
	4:15 PM	0	0	3	0	3	0	0	2	0	2	0	0	1	0	1
	4:30 PM	0	0	1	0	1	0	0	0	0	0	0	0	1	0	1
	4:45 PM	0	0	1	0	1	0	0	1	0	1	0	0	0	0	0
	5:00 PM	0	0	1	0	1	0	0	1	0	1	0	0	0	0	0
	5:15 PM	0	0	1	0	1	0	0	1	0	1	0	0	0	0	0
	5:30 PM	0	0	3	1	4	0	0	2	0	2	0	0	1	1	2
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	6:00 PM	0	0	4	1	5	0	0	2	1	3	0	0	2	0	2
	6:15 PM	0	0	1	0	1	0	0	0	0	0	0	0	1	0	1
	6:30 PM	0	0	2	0	2	0	0	2	0	2	0	0	0	0	0
	6:45 PM	0	0	3	0	3	0	0	0	0	0	0	0	3	0	3
	TOTAL	0	0	32	4	36	0	0	15	2	17	0	0	17	2	19
	PM BEGIN PEAK HR			4:30 PM			0	0	3	0	3	0	0	1	0	1

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

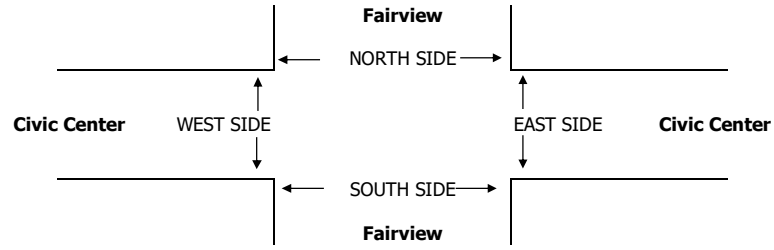
DATE:	LOCATION:	Santa Ana	PROJECT #:	SC0846
Tue, Feb 9, 16	NORTH & SOUTH:	Fairview	LOCATION #:	12
	EAST & WEST:	Civic Center	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Fairview			SOUTHBOUND Fairview			EASTBOUND Civic Center			WESTBOUND Civic Center				U-TURNS				
	LANES:	NL 1	NT 3	NR 0	SL 1	ST 3	SR 0	EL 0.5	ET 1	ER 0.5	WL 1.5	WT 0.5	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	1	313	65	39	371	1	1	5	4	78	3	22	903	0	3	0	0	3
	7:15 AM	0	331	87	57	342	2	2	6	8	82	1	49	967	0	5	0	0	5
	7:30 AM	2	277	122	68	316	1	2	4	3	67	3	45	910	0	1	0	0	1
	7:45 AM	2	307	123	69	318	1	0	7	7	61	1	32	928	0	1	0	0	1
	8:00 AM	3	272	78	49	332	3	2	2	3	63	6	21	834	0	3	0	0	3
	8:15 AM	2	253	64	28	357	1	0	0	3	48	3	39	798	0	1	0	0	1
	8:30 AM	3	247	56	22	373	0	2	1	8	53	2	29	796	0	1	0	0	1
	8:45 AM	2	236	53	19	335	0	1	3	1	61	1	20	732	0	0	0	0	0
	VOLUMES	15	2,236	648	351	2,744	9	10	28	37	513	20	257	6,868	0	15	0	0	15
	APPROACH %	1%	77%	22%	11%	88%	0%	13%	37%	49%	65%	3%	33%						
	APP/DEPART	2,899	/	2,518	3,104	/	3,294	75	/	1,012	790	/	44	0					
	BEGIN PEAK HR		7:00 AM																
	VOLUMES	5	1,228	397	233	1,347	5	5	22	22	288	8	148	3,708					
	APPROACH %	0%	75%	24%	15%	85%	0%	10%	45%	45%	65%	2%	33%						
	PEAK HR FACTOR		0.943			0.964			0.766			0.841		0.959					
	APP/DEPART	1,630	/	1,391	1,585	/	1,657	49	/	642	444	/	18	0					
PM	4:00 PM	1	335	69	36	307	2	4	1	3	74	0	41	873	0	2	0	0	2
	4:15 PM	3	358	78	22	317	0	0	0	3	76	0	41	898	2	2	0	0	4
	4:30 PM	2	360	82	28	291	0	2	1	2	113	1	34	916	1	1	0	0	2
	4:45 PM	0	337	82	26	314	0	1	1	2	111	0	38	912	0	1	0	0	1
	5:00 PM	1	325	93	29	296	0	1	3	2	112	0	50	912	0	5	0	0	5
	5:15 PM	0	340	99	34	330	0	0	1	1	126	0	32	963	0	2	0	0	2
	5:30 PM	0	351	102	32	342	0	0	0	0	88	0	41	956	0	3	0	0	3
	5:45 PM	3	358	92	43	317	0	0	0	0	94	0	32	939	2	6	0	0	8
	VOLUMES	10	2,764	697	250	2,514	2	8	7	13	794	1	309	7,369	5	22	0	0	27
	APPROACH %	0%	80%	20%	9%	91%	0%	29%	25%	46%	72%	0%	28%						
	APP/DEPART	3,471	/	3,103	2,766	/	3,326	28	/	932	1,104	/	8	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	4	1,374	386	138	1,285	0	1	4	3	420	0	155	3,770					
	APPROACH %	0%	78%	22%	10%	90%	0%	13%	50%	38%	73%	0%	27%						
	PEAK HR FACTOR		0.974			0.951			0.333			0.887		0.979					
	APP/DEPART	1,764	/	1,546	1,423	/	1,710	8	/	512	575	/	2	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	30	0	8	16	54	29	0	6	15	50	1	0	2	1	4
	7:15 AM	24	0	0	20	44	23	0	0	18	41	1	0	0	2	3
	7:30 AM	36	0	8	25	69	34	0	4	24	62	2	0	4	1	7
	7:45 AM	33	0	4	25	62	33	0	4	25	62	0	0	0	0	0
	8:00 AM	8	0	1	2	11	6	0	1	1	8	2	0	0	1	3
	8:15 AM	10	0	2	7	19	10	0	2	7	19	0	0	0	0	0
	8:30 AM	4	0	5	2	11	4	0	1	2	7	0	0	4	0	4
	8:45 AM	4	0	2	5	11	3	0	2	4	9	1	0	0	1	2
	TOTAL	149	0	30	102	281	142	0	20	96	258	7	0	10	6	23
PM	4:00 PM	7	0	6	0	13	6	0	4	0	10	1	0	2	0	3
	4:15 PM	10	0	1	5	16	6	0	1	2	9	4	0	0	3	7
	4:30 PM	6	0	2	4	12	6	0	2	3	11	0	0	0	1	1
	4:45 PM	5	0	0	5	10	4	0	0	5	9	1	0	0	0	1
	5:00 PM	5	0	4	5	14	4	0	3	3	10	1	0	1	2	4
	5:15 PM	3	0	4	3	10	1	0	1	0	2	2	0	3	3	8
	5:30 PM	5	0	6	1	12	5	0	3	1	9	0	0	3	0	3
	5:45 PM	2	0	4	2	8	2	0	4	2	8	0	0	0	0	0
	TOTAL	43	0	27	25	95	34	0	18	16	68	9	0	9	9	27

Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: FAIRVIEW STREET
E-W Direction: 1ST STREET

File Name : H1702002
Site Code : 00000000
Start Date : 2/7/2017
Page No : 1

Groups Printed- Turning Movements

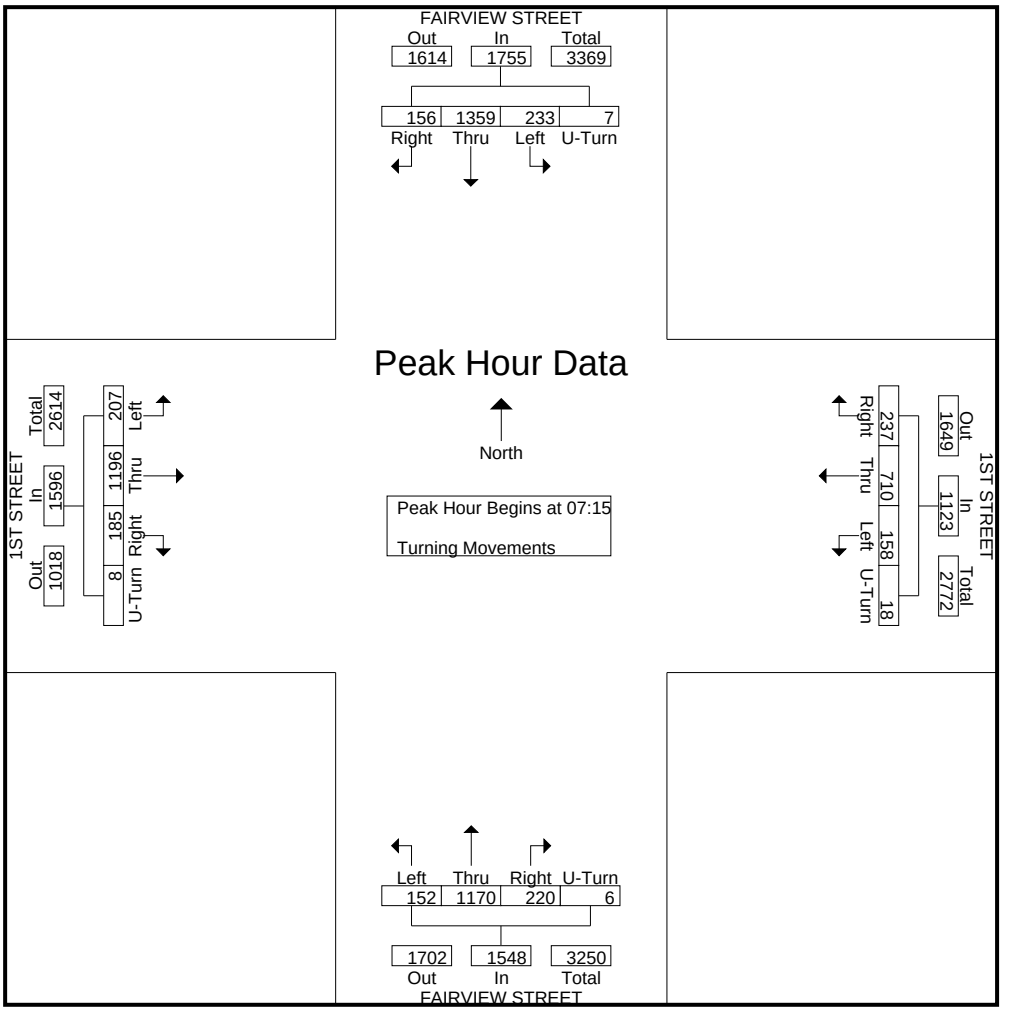
	FAIRVIEW STREET Southbound				1ST STREET Westbound				FAIRVIEW STREET Northbound				1ST STREET Eastbound				
Start Time	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Int. Total
07:00	20	352	41	1	39	136	32	2	45	284	30	0	54	254	59	0	1349
07:15	28	383	54	1	43	148	49	5	45	283	28	4	34	311	59	0	1475
07:30	40	378	65	2	67	177	47	4	44	302	29	2	34	290	48	0	1529
07:45	53	298	60	3	71	203	38	2	69	303	49	0	52	290	53	4	1548
Total	141	1411	220	7	220	664	166	13	203	1172	136	6	174	1145	219	4	5901
08:00	35	300	54	1	56	182	24	7	62	282	46	0	65	305	47	4	1470
08:15	28	338	53	2	32	123	29	5	39	243	34	0	56	261	53	1	1297
08:30	19	363	50	2	42	116	20	5	43	225	20	0	34	230	48	1	1218
08:45	26	293	33	0	53	127	27	7	40	206	26	4	27	230	50	1	1150
Total	108	1294	190	5	183	548	100	24	184	956	126	4	182	1026	198	7	5135
*** BREAK ***																	
16:00	51	246	45	6	45	275	31	7	30	240	43	0	43	218	30	3	1313
16:15	49	268	48	3	60	220	34	7	41	275	25	0	32	181	49	1	1293
16:30	72	277	56	2	49	281	38	13	36	259	32	2	38	173	40	1	1369
16:45	63	264	42	6	51	274	38	10	31	285	45	3	28	200	49	3	1392
Total	235	1055	191	17	205	1050	141	37	138	1059	145	5	141	772	168	8	5367
17:00	75	299	50	5	45	300	38	7	30	277	43	3	34	214	58	2	1480
17:15	57	346	48	2	39	292	39	7	32	273	25	2	30	213	59	0	1464
17:30	63	279	49	3	51	277	35	5	14	268	42	0	36	212	46	3	1383
17:45	48	302	50	2	45	299	42	11	20	243	34	2	46	205	45	3	1397
Total	243	1226	197	12	180	1168	154	30	96	1061	144	7	146	844	208	8	5724
Grand Total	727	4986	798	41	788	3430	561	104	621	4248	551	22	643	3787	793	27	22127
Apprch %	11.1	76.1	12.2	0.6	16.1	70.2	11.5	2.1	11.4	78.1	10.1	0.4	12.2	72.1	15.1	0.5	
Total %	3.3	22.5	3.6	0.2	3.6	15.5	2.5	0.5	2.8	19.2	2.5	0.1	2.9	17.1	3.6	0.1	

Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: FAIRVIEW STREET
E-W Direction: 1ST STREET

File Name : H1702002
Site Code : 00000000
Start Date : 2/7/2017
Page No : 2

	FAIRVIEW STREET Southbound					1ST STREET Westbound					FAIRVIEW STREET Northbound					1ST STREET Eastbound					
Start Time	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 07:15																					
07:15	28	383						49						4	360	34	311	**59**			
07:30	40	378	**65**		**485**	67	177	47	4	295	44	302	29	2	377	34	290	48	0	372	1529
07:45	**53**			**3**		**71**	**203**			**314**	**69**	**303**	**49**		**421**	52	290	53	**4**	399	**1548**
08:00	**35**	**300**	**54**	**1**	**390**	**56**	**182**	**24**	**7**	269	62	282	46	0	390	**65**	305	47	4	**421**	**1470**
Total Volume	156	1359	233	7	1755	237	710	158	18	1123	220	1170	152	6	1548	185	1196	207	8	1596	6022
% App. Total																					
PHF	.736	.887	.896	.583	.905	.835	.874	.806	.643	.894	.797	.965	.776	.375	.919	.712	.961	.877	.500	.948	.973

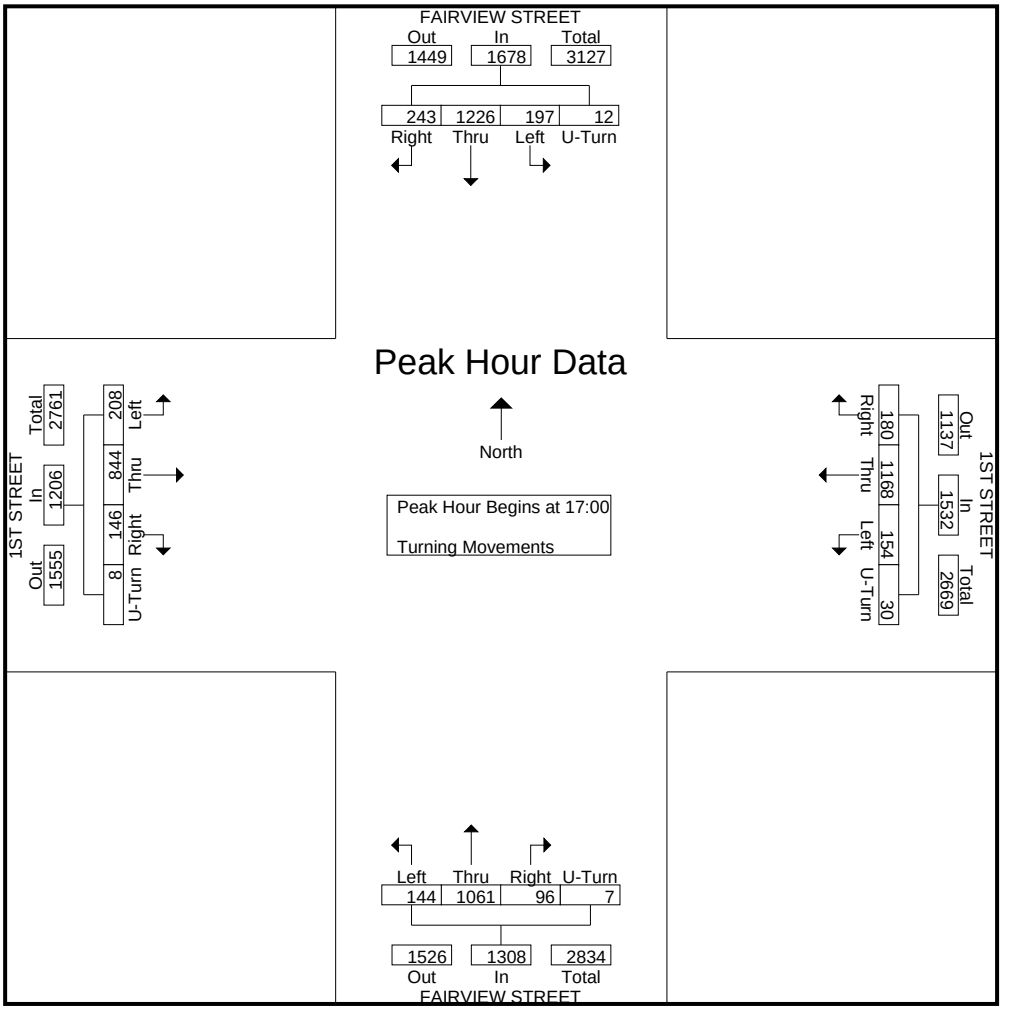


Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: FAIRVIEW STREET
E-W Direction: 1ST STREET

File Name : H1702002
Site Code : 00000000
Start Date : 2/7/2017
Page No : 3

	FAIRVIEW STREET Southbound					1ST STREET Westbound					FAIRVIEW STREET Northbound					1ST STREET Eastbound					
Start Time	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 17:00																					
17:00	75	299	50	5	429	45	300	38	7	390	30	**277**	**43**	**3**	**353**	34	**214**			**308**	**1480**
17:15	**57**	**346**	48	2	**453**	39	292	39	7	377	**32**	**273**	**25**	**2**	**332**	**30**	**213**	**59**	0	302	1464
17:30	63	279	49	3	394	**51**	277	35	5	368	14	268	42	0	324	36	212	46	**3**	297	1383
17:45	48	302	50	2	402	45	299	**42**	**11**	**397**						**46**	205	45	3	299	1397
Total Volume	243	1226	197	12	1678	180	1168	154	30	1532	96	1061	144	7	1308	146	844	208	8	1206	5724
% App. Total	14.5	73.1	11.7	0.7		11.7	76.2	10.1	2		7.3	81.1	11	0.5		12.1	70	17.2	0.7		
PHF	.810	.886	.985	.600	.926	.882	.973	.917	.682	.965	.750	.958	.837	.583	.926	.793	.986	.881	.667	.979	.967



INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

DATE: Tue, Feb 9, 16

LOCATION: Santa Ana
NORTH & SOUTH: Fairview
EAST & WEST: McFadden

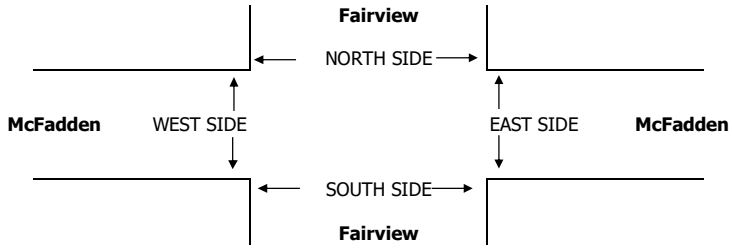
PROJECT #: SC0846
LOCATION #: 33
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Fairview			SOUTHBOUND Fairview			EASTBOUND McFadden			WESTBOUND McFadden				U-TURNS				
		NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL	NB	SB	EB	WB	TTL
	LANES:	1	3	1	1	3	1	1	2	0	1	2	0		0	0	0	0	
AM	7:00 AM	20	257	19	17	393	28	44	97	23	46	79	14	1,037	0	3	0	0	3
	7:15 AM	20	321	30	14	493	25	57	102	30	54	105	13	1,264	4	3	0	0	7
	7:30 AM	29	325	41	41	426	26	54	128	26	56	152	5	1,309	4	6	0	0	10
	7:45 AM	30	326	51	39	395	39	49	140	37	49	114	11	1,280	0	4	0	0	4
	8:00 AM	37	271	44	42	345	30	52	131	22	27	109	9	1,119	3	10	0	0	13
	8:15 AM	29	239	27	41	344	27	54	130	43	30	88	33	1,085	2	13	0	0	15
	8:30 AM	24	194	19	28	415	27	43	109	56	25	102	18	1,060	1	8	0	0	9
	8:45 AM	19	186	17	38	398	26	34	110	43	33	84	21	1,009	3	7	0	0	10
	VOLUMES	208	2,119	248	260	3,209	228	387	947	280	320	833	124	9,163	17	54	0	0	71
	APPROACH %	8%	82%	10%	7%	87%	6%	24%	59%	17%	25%	65%	10%						
	APP/DEPART	2,575	/	2,684	3,697	/	3,826	1,614	/	1,401	1,277	/	1,252	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	116	1,243	166	136	1,659	120	212	501	115	186	480	38	4,972					
	APPROACH %	8%	82%	11%	7%	87%	6%	26%	61%	14%	26%	68%	5%						
	PEAK HR FACTOR		0.937			0.900			0.916			0.826		0.950					
	APP/DEPART	1,525	/	1,516	1,915	/	1,971	828	/	780	704	/	705	0					
PM	4:00 PM	32	372	34	36	235	43	46	153	21	30	117	32	1,151	0	9	0	0	9
	4:15 PM	42	388	35	34	236	38	40	116	37	30	134	21	1,151	1	3	0	0	4
	4:30 PM	35	369	35	35	277	54	37	146	27	29	133	24	1,201	0	4	0	0	4
	4:45 PM	32	367	38	37	284	46	53	152	29	31	124	16	1,209	0	1	0	0	1
	5:00 PM	48	338	29	35	314	49	53	158	31	38	125	37	1,255	0	4	0	0	4
	5:15 PM	26	346	25	30	300	51	55	141	46	39	150	34	1,243	1	5	0	0	6
	5:30 PM	36	361	41	36	293	48	59	165	39	35	118	28	1,259	0	9	0	0	9
	5:45 PM	42	359	27	47	300	40	67	169	40	39	130	29	1,289	1	3	0	0	4
	VOLUMES	293	2,900	264	290	2,239	369	410	1,200	270	271	1,031	221	9,758	3	38	0	0	41
	APPROACH %	8%	84%	8%	10%	77%	13%	22%	64%	14%	18%	68%	15%						
	APP/DEPART	3,457	/	3,569	2,898	/	2,783	1,880	/	1,716	1,523	/	1,690	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	152	1,404	122	148	1,207	188	234	633	156	151	523	128	5,046					
	APPROACH %	9%	84%	7%	10%	78%	12%	23%	62%	15%	19%	65%	16%						
	PEAK HR FACTOR		0.958			0.969			0.927			0.899		0.979					
	APP/DEPART	1,678	/	1,787	1,543	/	1,516	1,023	/	882	802	/	861	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	18	15	27	25	85	14	14	25	21	74	4	1	2	4	11
	7:15 AM	26	46	46	34	152	26	43	44	30	143	0	3	2	4	9
	7:30 AM	120	117	87	110	434	118	110	82	104	414	2	7	5	6	20
	7:45 AM	83	96	44	81	304	81	90	42	76	289	2	6	2	5	15
	8:00 AM	33	27	28	22	110	27	27	26	21	101	6	0	2	1	9
	8:15 AM	8	39	29	5	81	7	36	26	5	74	1	3	3	0	7
	8:30 AM	12	11	11	13	47	9	10	10	10	39	3	1	1	3	8
	8:45 AM	8	5	9	9	31	8	3	6	8	25	0	2	3	1	6
	TOTAL	308	356	281	299	1,244	290	333	261	275	1,159	18	23	20	24	85
PM	4:00 PM	7	25	33	4	69	6	23	31	1	61	1	2	2	3	8
	4:15 PM	52	4	15	13	84	50	2	13	10	75	2	2	2	3	9
	4:30 PM	12	7	18	9	46	6	6	17	7	36	6	1	1	2	10
	4:45 PM	48	18	18	7	91	46	14	16	5	81	2	4	2	2	10
	5:00 PM	4	6	18	4	32	4	5	15	3	27	0	1	3	1	5
	5:15 PM	18	8	21	13	60	11	6	19	8	44	7	2	2	5	16
	5:30 PM	17	10	24	14	65	11	6	22	7	46	6	4	2	7	19
	5:45 PM	17	16	30	13	76	14	15	28	11	68	3	1	2	2	8
	TOTAL	175	94	177	77	523	148	77	161	52	438	27	17	16	25	85

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Thu, May 16, 19

LOCATION: Santa Ana
NORTH & SOUTH: Fairview
EAST & WEST: Edinger

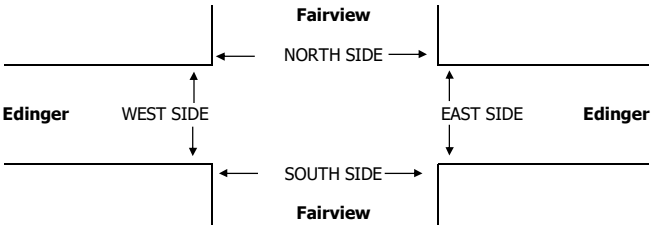
PROJECT #: SC2183
LOCATION #: 10
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

Add U-Turns to Left Turns

		NORTHBOUND (Fairview)			SOUTHBOUND (Fairview)			EASTBOUND (Edinger)			WESTBOUND (Edinger)				U-TURNS				
	LANES:	NL 2	NT 3	NR 0	SL 2	ST 3	SR 0	EL 2	ET 3	ER 0	WL 2	WT 3	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	34	179	28	44	430	28	24	127	34	88	86	29	1,131	2	1	0	0	3
	7:15 AM	40	186	27	63	336	21	58	215	54	110	126	22	1,258	1	0	1	0	2
	7:30 AM	55	226	31	50	363	37	66	154	45	96	164	28	1,315	4	0	0	0	4
	7:45 AM	57	231	28	55	359	39	86	154	41	122	171	29	1,372	3	1	0	0	4
	8:00 AM	31	193	26	38	289	43	67	130	39	31	134	49	1,070	1	3	0	1	5
	8:15 AM	35	149	16	36	298	36	48	160	38	57	137	28	1,038	1	1	0	0	2
	8:30 AM	22	138	22	37	424	32	56	154	31	48	101	21	1,086	0	2	0	0	2
	8:45 AM	27	127	12	38	324	33	28	122	40	30	90	22	893	0	2	0	0	2
	VOLUMES	301	1,429	190	361	2,823	269	433	1,216	322	582	1,009	228	9,163	12	10	1	1	24
	APPROACH %	16%	74%	10%	10%	82%	8%	22%	62%	16%	32%	55%	13%						
	APP/DEPART	1,920	/	2,099	3,453	/	3,738	1,971	/	1,758	1,819	/	1,568	0					
	BEGIN PEAK HR		7:00 AM																
	VOLUMES	186	822	114	212	1,488	125	234	650	174	416	547	108	5,076					
	APPROACH %	17%	73%	10%	12%	82%	7%	22%	61%	16%	39%	51%	10%						
	PEAK HR FACTOR		0.888			0.909			0.809			0.832		0.925					
	APP/DEPART	1,122	/	1,165	1,825	/	2,088	1,058	/	974	1,071	/	849	0					
PM	4:00 PM	25	398	32	46	152	31	25	187	39	96	173	40	1,244	0	2	1	0	3
	4:15 PM	45	411	25	63	155	25	49	199	59	93	178	32	1,334	0	0	0	0	0
	4:30 PM	51	473	34	50	159	37	57	173	46	103	204	39	1,426	2	0	0	0	2
	4:45 PM	59	435	26	52	143	39	55	162	44	89	201	43	1,348	5	0	0	0	5
	5:00 PM	31	456	23	31	152	48	66	162	37	84	198	57	1,345	3	0	2	0	5
	5:15 PM	34	444	22	39	140	41	52	173	41	72	192	36	1,286	2	1	0	0	3
	5:30 PM	23	501	26	37	129	32	60	180	30	66	189	36	1,309	0	1	0	1	2
	5:45 PM	29	477	20	37	153	38	57	172	39	56	181	32	1,291	1	0	0	2	3
	VOLUMES	297	3,595	208	355	1,183	291	421	1,408	335	659	1,516	315	10,583	13	4	3	3	23
	APPROACH %	7%	88%	5%	19%	65%	16%	19%	65%	15%	26%	61%	13%						
	APP/DEPART	4,100	/	4,332	1,829	/	2,187	2,164	/	1,970	2,490	/	2,094	0					
	BEGIN PEAK HR		4:15 PM																
	VOLUMES	186	1,775	108	196	609	149	227	696	186	369	781	171	5,453					
	APPROACH %	9%	86%	5%	21%	64%	16%	20%	63%	17%	28%	59%	13%						
	PEAK HR FACTOR		0.927			0.970			0.903			0.954		0.956					
	APP/DEPART	2,069	/	2,171	954	/	1,174	1,109	/	1,000	1,321	/	1,108	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:00 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	4:15 PM					0	0	0	0	0					

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

DATE: Tue, Feb 9, 16

LOCATION: Santa Ana
NORTH & SOUTH: Fairview
EAST & WEST: Warner

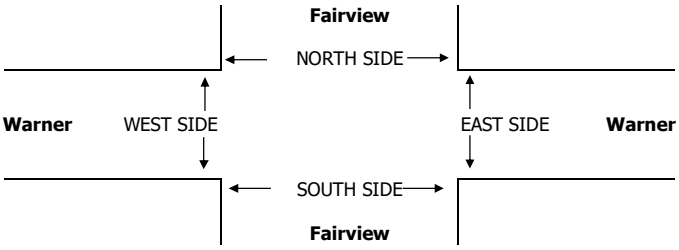
PROJECT #: SC0846
LOCATION #: 42
CONTROL: SIGNAL

NOTES:

AM PM MD OTHER OTHER — ▲ N ◄ W E ► S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Fairview			SOUTHBOUND Fairview			EASTBOUND Warner			WESTBOUND Warner				U-TURNS				
	LANES:	NL 2	NT 3	NR 0	SL 2	ST 3	SR 0	EL 2	ET 3	ER 0	WL 2	WT 3	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	30	152	19	57	397	21	18	173	43	48	131	41	1,130	4	3	0	6	13
	7:15 AM	49	211	34	35	427	28	11	272	78	56	193	49	1,443	1	0	0	8	9
	7:30 AM	40	240	38	64	381	26	15	287	84	95	236	87	1,593	2	0	0	9	11
	7:45 AM	65	236	21	51	351	25	16	183	59	89	247	93	1,436	3	1	0	5	9
	8:00 AM	52	227	27	47	405	35	24	228	68	55	213	59	1,440	5	0	1	3	9
	8:15 AM	42	172	29	56	381	32	31	270	72	56	150	38	1,329	3	2	0	6	11
	8:30 AM	42	136	30	47	419	24	24	277	77	47	184	17	1,324	1	1	0	3	5
	8:45 AM	28	174	24	42	410	24	32	193	46	49	171	24	1,217	4	0	1	5	10
	VOLUMES	348	1,548	222	399	3,171	215	171	1,883	527	495	1,525	408	10,912	23	7	2	45	77
	APPROACH %	16%	73%	10%	11%	84%	6%	7%	73%	20%	20%	63%	17%						
	APP/DEPART	2,118	/	2,132	3,785	/	4,171	2,581	/	2,542	2,428	/	2,067	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	206	914	120	197	1,564	114	66	970	289	295	889	288	5,912					
	APPROACH %	17%	74%	10%	11%	83%	6%	5%	73%	22%	20%	60%	20%						
	PEAK HR FACTOR		0.963			0.957			0.858			0.858		0.928					
	APP/DEPART	1,240	/	1,268	1,875	/	2,134	1,325	/	1,311	1,472	/	1,199	0					
PM	4:00 PM	51	414	39	54	212	23	56	210	35	31	197	26	1,348	3	2	0	5	10
	4:15 PM	53	396	42	42	202	25	39	205	42	41	225	37	1,349	4	3	1	7	15
	4:30 PM	43	395	37	46	201	27	47	228	43	46	241	37	1,391	4	3	0	7	14
	4:45 PM	34	415	38	37	209	25	49	231	45	41	230	53	1,407	1	3	0	8	12
	5:00 PM	49	393	41	40	259	23	65	254	47	41	249	50	1,511	5	0	2	12	19
	5:15 PM	47	432	37	51	202	32	44	240	36	38	278	49	1,486	3	2	2	6	13
	5:30 PM	49	419	43	38	233	24	44	217	30	48	283	65	1,493	4	0	1	12	17
	5:45 PM	63	416	33	51	219	13	36	206	29	53	254	66	1,439	2	4	0	12	18
	VOLUMES	389	3,280	310	359	1,737	192	380	1,791	307	339	1,957	383	11,424	26	17	6	69	118
	APPROACH %	10%	82%	8%	16%	76%	8%	15%	72%	12%	13%	73%	14%						
	APP/DEPART	3,979	/	4,054	2,288	/	2,340	2,478	/	2,512	2,679	/	2,518	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	208	1,660	154	180	913	92	189	917	142	180	1,064	230	5,929					
	APPROACH %	10%	82%	8%	15%	77%	8%	15%	73%	11%	12%	72%	16%						
	PEAK HR FACTOR		0.980			0.920			0.852			0.931		0.981					
	APP/DEPART	2,022	/	2,080	1,185	/	1,207	1,248	/	1,287	1,474	/	1,355	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	1	1	4	0	6	0	1	3	0	4	1	0	1	0	2
	7:15 AM	2	8	6	4	20	0	6	5	4	15	2	2	1	0	5
	7:30 AM	1	3	3	4	11	0	3	3	2	8	1	0	0	2	3
	7:45 AM	2	3	2	4	11	0	3	1	4	8	2	0	1	0	3
	8:00 AM	7	9	6	8	30	2	8	2	8	20	5	1	4	0	10
	8:15 AM	3	5	7	9	24	2	4	7	8	21	1	1	0	1	3
	8:30 AM	1	3	5	2	11	1	1	4	0	6	0	2	1	2	5
	8:45 AM	1	2	4	2	9	1	2	3	1	7	0	0	1	1	2
	TOTAL	18	34	37	33	122	6	28	28	27	89	12	6	9	6	33
PM	4:00 PM	5	16	15	9	45	4	11	9	7	31	1	5	6	2	14
	4:15 PM	6	5	13	2	26	4	3	11	1	19	2	2	2	1	7
	4:30 PM	5	8	7	2	22	4	5	6	1	16	1	3	1	1	6
	4:45 PM	3	6	19	2	30	3	3	13	1	20	0	3	6	1	10
	5:00 PM	1	7	20	3	31	1	4	12	2	19	0	3	8	1	12
	5:15 PM	7	5	9	14	35	3	5	5	9	22	4	0	4	5	13
	5:30 PM	5	0	5	5	15	5	0	4	4	13	0	0	1	1	2
	5:45 PM	1	5	6	3	15	1	2	4	2	9	0	3	2	1	6
	TOTAL	33	52	94	40	219	25	33	64	27	149	8	19	30	13	70

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Thu, May 16, 19

LOCATION: Santa Ana
NORTH & SOUTH: Fairview
EAST & WEST: MacArthur

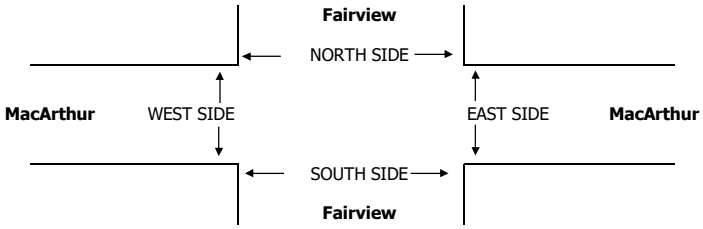
PROJECT #: SC2183
LOCATION #: 11
CONTROL: SIGNAL

NOTES:

AM PM MD OTHER OTHER — ▲ N ◄ W E ► S ▼

Add U-Turns to Left Turns

		NORTHBOUND Fairview			SOUTHBOUND Fairview			EASTBOUND MacArthur			WESTBOUND MacArthur				U-TURNS				
	LANES:	NL 2	NT 3	NR 0	SL 2	ST 3	SR 1	EL 2	ET 3	ER 1	WL 2	WT 3	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	25	129	15	55	313	33	18	188	35	38	81	14	944	5	1	0	3	9
	7:15 AM	48	175	15	63	416	29	20	233	36	51	108	25	1,219	11	1	2	2	16
	7:30 AM	71	199	21	65	381	34	34	248	44	50	113	44	1,304	20	0	0	4	24
	7:45 AM	67	213	22	80	372	54	31	258	37	75	146	37	1,392	24	0	0	2	26
	8:00 AM	59	176	30	95	361	54	38	269	31	56	122	32	1,323	18	0	0	2	20
	8:15 AM	41	193	17	47	303	51	27	230	39	32	131	27	1,138	9	1	0	3	13
	8:30 AM	38	140	10	66	353	22	26	195	40	41	104	21	1,056	4	2	0	1	7
	8:45 AM	30	158	17	54	343	42	26	166	24	40	106	17	1,023	4	0	0	3	7
	VOLUMES	379	1,383	147	525	2,842	319	220	1,787	286	383	911	217	9,399	95	5	2	20	122
	APPROACH %	20%	72%	8%	14%	77%	9%	10%	78%	12%	25%	60%	14%						
	APP/DEPART	1,909	/	1,823	3,686	/	3,586	2,293	/	2,474	1,511	/	1,516	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	245	763	88	303	1,530	171	123	1,008	148	232	489	138	5,238					
	APPROACH %	22%	70%	8%	15%	76%	9%	10%	79%	12%	27%	57%	16%						
	PEAK HR FACTOR		0.907			0.982			0.946			0.832		0.941					
	APP/DEPART	1,096	/	1,023	2,004	/	1,973	1,279	/	1,408	859	/	834	0					
PM	4:00 PM	72	367	32	52	217	34	73	165	59	40	373	47	1,531	3	3	0	5	11
	4:15 PM	66	451	13	38	205	24	65	175	55	36	304	54	1,486	6	6	0	1	13
	4:30 PM	69	424	24	31	235	19	74	196	44	37	322	49	1,524	7	3	1	4	15
	4:45 PM	69	452	27	32	228	23	56	169	49	41	354	67	1,567	4	5	1	9	19
	5:00 PM	47	351	17	64	232	21	80	189	82	47	353	67	1,550	4	7	0	5	16
	5:15 PM	55	428	22	42	250	24	67	169	68	52	322	53	1,552	3	2	0	4	9
	5:30 PM	60	395	32	42	223	26	68	163	44	47	349	64	1,513	8	4	0	6	18
	5:45 PM	71	429	34	39	194	35	46	146	42	47	299	57	1,439	1	5	1	3	10
	VOLUMES	509	3,297	201	340	1,784	206	529	1,372	443	347	2,676	458	12,162	36	35	3	37	111
	APPROACH %	13%	82%	5%	15%	77%	9%	23%	59%	19%	10%	77%	13%						
	APP/DEPART	4,007	/	4,316	2,330	/	2,573	2,344	/	1,915	3,481	/	3,358	0					
	BEGIN PEAK HR		4:30 PM																
	VOLUMES	240	1,655	90	169	945	87	277	723	243	177	1,351	236	6,193					
	APPROACH %	12%	83%	5%	14%	79%	7%	22%	58%	20%	10%	77%	13%						
	PEAK HR FACTOR		0.906			0.947			0.885			0.944		0.988					
	APP/DEPART	1,985	/	2,183	1,201	/	1,361	1,243	/	987	1,764	/	1,662	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:15 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	4:30 PM					0	0	0	0	0					

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

DATE: Tue, Mar 1, 16

LOCATION: Santa Ana
NORTH & SOUTH: Fairview
EAST & WEST: Sunflower

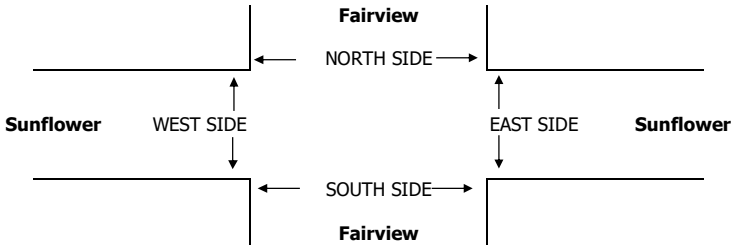
PROJECT #: SC0846
LOCATION #: 12
CONTROL: SIGNAL

NOTES:

AM PM MD OTHER OTHER — ▲ N ◄ W E ► S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Fairview			SOUTHBOUND Fairview			EASTBOUND Sunflower			WESTBOUND Sunflower				U-TURNS				
	LANES:	NL 2	NT 3	NR 1	SL 2	ST 3	SR 0	EL 2	ET 2	ER 0	WL 2	WT 3	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	27	152	19	17	322	12	10	45	17	47	22	14	704	0	2	0	0	2
	7:15 AM	30	254	31	40	416	23	4	64	10	61	22	18	973	0	5	0	0	5
	7:30 AM	47	294	36	38	411	13	12	76	14	88	58	35	1,122	0	1	2	0	3
	7:45 AM	52	255	32	55	419	31	14	91	14	73	102	46	1,184	1	4	10	0	15
	8:00 AM	48	226	57	63	424	39	17	78	24	72	64	27	1,139	0	4	6	0	10
	8:15 AM	32	224	41	56	376	38	6	59	16	61	34	16	959	0	5	1	0	6
	8:30 AM	29	182	33	37	395	39	11	46	9	40	25	10	856	0	3	3	0	6
	8:45 AM	33	210	22	51	379	32	10	48	9	60	34	9	897	2	2	1	0	5
	VOLUMES	298	1,797	271	357	3,142	227	84	507	113	502	361	175	7,834	3	26	23	0	52
	APPROACH %	13%	76%	11%	10%	84%	6%	12%	72%	16%	48%	35%	17%						
	APP/DEPART	2,366	/	2,059	3,726	/	3,760	704	/	1,109	1,038	/	906	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	177	1,029	156	196	1,670	106	47	309	62	294	246	126	4,418					
	APPROACH %	13%	76%	11%	10%	85%	5%	11%	74%	15%	44%	37%	19%						
	PEAK HR FACTOR		0.903			0.937			0.878			0.753		0.933					
	APP/DEPART	1,362	/	1,198	1,972	/	2,027	418	/	647	666	/	546	0					
PM	4:00 PM	29	400	51	31	238	7	24	61	14	53	112	34	1,054	0	7	1	0	8
	4:15 PM	35	472	74	26	257	15	13	60	11	58	137	43	1,201	1	4	3	1	9
	4:30 PM	32	413	80	24	321	22	37	84	21	71	150	40	1,295	0	2	6	0	8
	4:45 PM	32	436	63	30	277	11	51	83	17	69	169	47	1,285	0	4	5	1	10
	5:00 PM	40	448	67	27	305	27	60	106	26	57	150	46	1,359	0	3	1	0	4
	5:15 PM	46	444	98	31	287	12	52	124	23	60	162	36	1,375	1	5	3	0	9
	5:30 PM	58	450	100	39	308	22	44	90	26	55	153	42	1,387	0	5	4	0	9
	5:45 PM	49	393	112	41	249	18	39	93	16	82	173	42	1,307	2	6	4	0	12
	VOLUMES	321	3,456	645	249	2,242	134	320	701	154	505	1,206	330	10,263	4	36	27	2	69
	APPROACH %	7%	78%	15%	9%	85%	5%	27%	60%	13%	25%	59%	16%						
	APP/DEPART	4,422	/	4,115	2,625	/	2,903	1,175	/	1,561	2,041	/	1,684	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	193	1,735	377	138	1,149	79	195	413	91	254	638	166	5,428					
	APPROACH %	8%	75%	16%	10%	84%	6%	28%	59%	13%	24%	60%	16%						
	PEAK HR FACTOR		0.948			0.925			0.878			0.891		0.978					
	APP/DEPART	2,305	/	2,103	1,366	/	1,497	699	/	909	1,058	/	919	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	3	1	1	3	8	0	1	1	2	4	3	0	0	1	4
	7:15 AM	2	1	1	1	5	0	0	1	1	2	2	1	0	0	3
	7:30 AM	2	3	2	3	10	0	1	1	2	4	2	2	1	1	6
	7:45 AM	3	3	1	3	10	1	1	1	3	6	2	2	0	0	4
	8:00 AM	2	2	2	3	9	0	0	2	2	4	2	2	0	1	5
	8:15 AM	2	7	0	3	12	0	4	0	2	6	2	3	0	1	6
	8:30 AM	2	0	1	5	8	0	0	1	4	5	2	0	0	1	3
	8:45 AM	5	1	0	3	9	2	0	0	3	5	3	1	0	0	4
	TOTAL	21	18	8	24	71	3	7	7	19	36	18	11	1	5	35
PM	4:00 PM	2	2	1	0	5	0	0	1	0	1	2	2	0	0	4
	4:15 PM	1	1	1	2	5	0	1	1	1	3	1	0	0	1	2
	4:30 PM	3	3	4	1	11	0	2	3	1	6	3	1	1	0	5
	4:45 PM	6	4	1	0	11	3	3	0	0	6	3	1	1	0	5
	5:00 PM	5	13	2	0	20	0	3	1	0	4	5	10	1	0	16
	5:15 PM	6	2	2	3	13	4	0	0	2	6	2	2	2	1	7
	5:30 PM	6	5	3	1	15	2	2	2	0	6	4	3	1	1	9
	5:45 PM	3	8	0	3	14	1	3	0	3	7	2	5	0	0	7
	TOTAL	32	38	14	10	94	10	14	8	7	39	22	24	6	3	55

Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: GREENVILLE STREET
E-W Direction: EDINGER AVENUE

File Name : H1702003
Site Code : 00000000
Start Date : 2/7/2017
Page No : 1

Groups Printed- Turning Movements

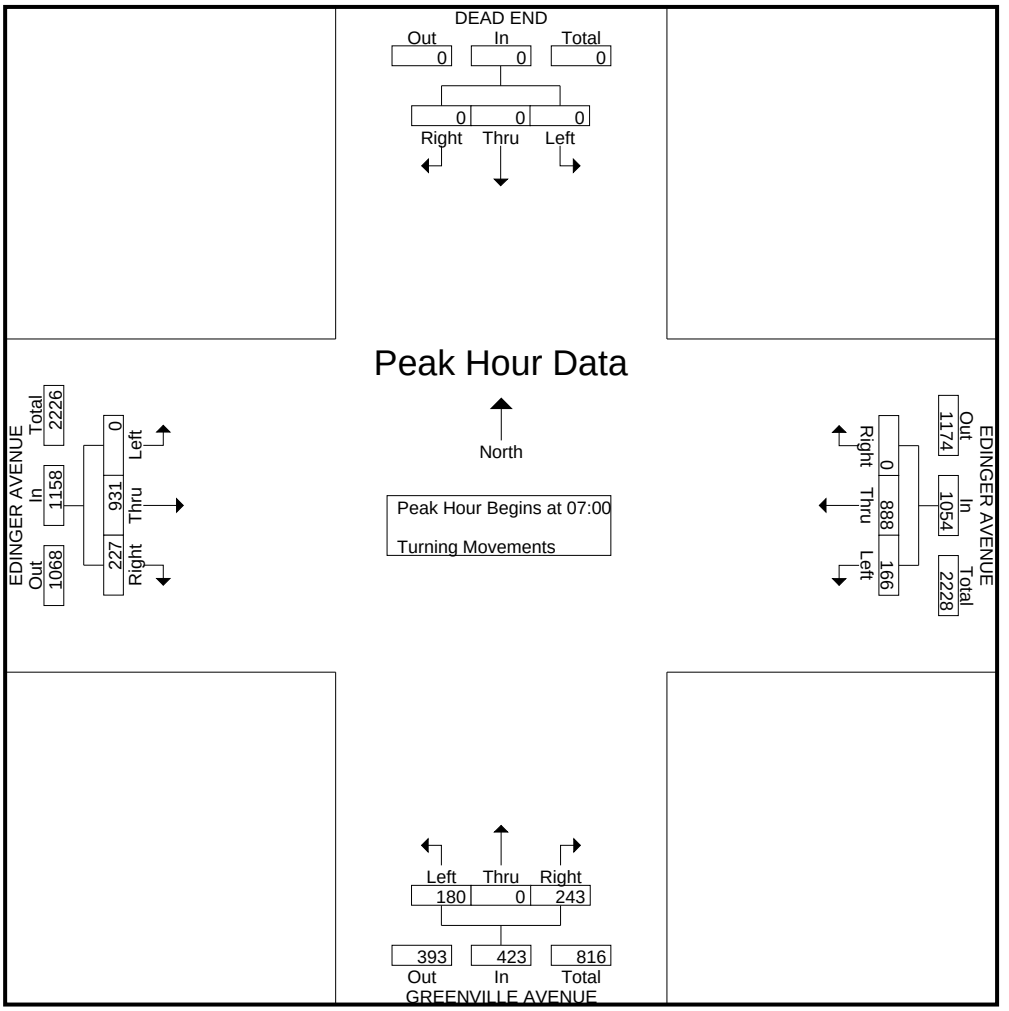
	DEAD END Southbound			EDINGER AVENUE Westbound			GREENVILLE AVENUE Northbound			EDINGER AVENUE Eastbound			
Start Time	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Int. Total
07:00	0	0	0	0	183	30	48	0	32	40	220	0	553
07:15	0	0	0	0	234	36	46	0	38	64	268	0	686
07:30	0	0	0	0	234	59	64	0	48	61	232	0	698
07:45	0	0	0	0	237	41	85	0	62	62	211	0	698
Total	0	0	0	0	888	166	243	0	180	227	931	0	2635
08:00	0	0	0	0	217	22	49	0	31	37	189	0	545
08:15	0	0	0	0	197	8	20	0	20	28	210	0	483
08:30	0	0	0	0	173	11	33	0	24	23	239	0	503
08:45	0	0	0	0	179	21	29	0	23	16	151	0	419
Total	0	0	0	0	766	62	131	0	98	104	789	0	1950
*** BREAK ***													
16:00	0	0	0	0	244	31	51	0	61	50	232	0	669
16:15	0	0	0	0	264	29	54	0	66	44	265	0	722
16:30	0	0	0	0	276	25	52	0	67	41	231	0	692
16:45	0	0	0	0	280	19	55	0	78	49	236	0	717
Total	0	0	0	0	1064	104	212	0	272	184	964	0	2800
17:00	0	0	0	0	270	30	49	0	78	54	245	0	726
17:15	0	0	0	0	274	24	53	0	85	52	208	0	696
17:30	0	0	0	0	270	19	54	0	76	39	264	0	722
17:45	0	0	0	0	275	31	40	0	69	45	199	0	659
Total	0	0	0	0	1089	104	196	0	308	190	916	0	2803
Grand Total	0	0	0	0	3807	436	782	0	858	705	3600	0	10188
Apprch %	0	0	0	0	89.7	10.3	47.7	0	52.3	16.4	83.6	0	
Total %	0	0	0	0	37.4	4.3	7.7	0	8.4	6.9	35.3	0	

Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: GREENVILLE STREET
E-W Direction: EDINGER AVENUE

File Name : H1702003
Site Code : 00000000
Start Date : 2/7/2017
Page No : 2

	DEAD END Southbound				EDINGER AVENUE Westbound				GREENVILLE AVENUE Northbound				EDINGER AVENUE Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:00																	
07:00	0	0	0	0	0	183	30	213	48	0	32	80	40	220	0	260	553
07:15	0	0	0	0	0	234	36	270	46	0	38	84	**64**	**268**	0	**332**	686
07:30	0	0	0	0	0	234	**59**	**293**	64	0	48	112	61	232	0	293	**698**
07:45	0	0	0	0	0	**237**	41	278	**85**	0	**62**	**147**	62	211	0	273	698
Total Volume	0	0	0	0	0	888	166	1054	243	0	180	423	227	931	0	1158	2635
% App. Total	0	0	0		0	84.3	15.7		57.4	0	42.6		19.6	80.4	0		
PHF	.000	.000	.000	.000	.000	.937	.703	.899	.715	.000	.726	.719	.887	.868	.000	.872	.944

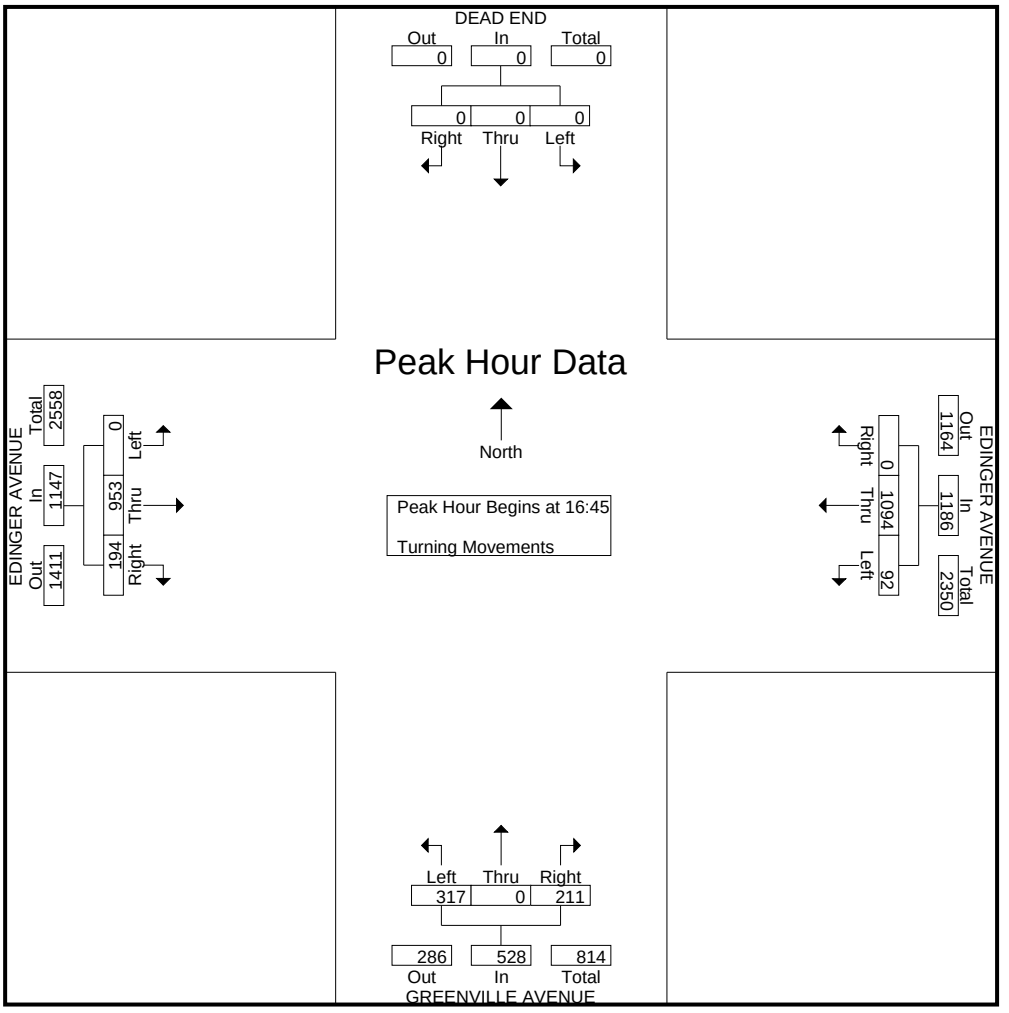


Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: GREENVILLE STREET
E-W Direction: EDINGER AVENUE

File Name : H1702003
Site Code : 00000000
Start Date : 2/7/2017
Page No : 3

	DEAD END Southbound				EDINGER AVENUE Westbound				GREENVILLE AVENUE Northbound				EDINGER AVENUE Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 16:45																	
16:45	0	0	0	0	0	**280**	19	299	**55**	0	78	133	49	236	0	285	717
17:00	0	0	0	0	0	270	**30**	**300**	49	0	78	127	**54**	245	0	299	**726**
17:15	0	0	0	0	0	274	24	298	53	0	**85**	**138**	52	208	0	260	696
17:30	0	0	0	0	0	270	19	289	54	0	76	130	39	**264**	0	**303**	722
Total Volume	0	0	0	0	0	1094	92	1186	211	0	317	528	194	953	0	1147	2861
% App. Total	0	0	0		0	92.2	7.8		40	0	60		16.9	83.1	0		
PHF	.000	.000	.000	.000	.000	.977	.767	.988	.959	.000	.932	.957	.898	.902	.000	.946	.985



Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
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City: SANTA ANA
N-S Direction: GREENVILLE STREET
E-W Direction: SEGERSTROM AVENUE

File Name : H1702004
Site Code : 00000000
Start Date : 2/7/2017
Page No : 1

Groups Printed- Turning Movements

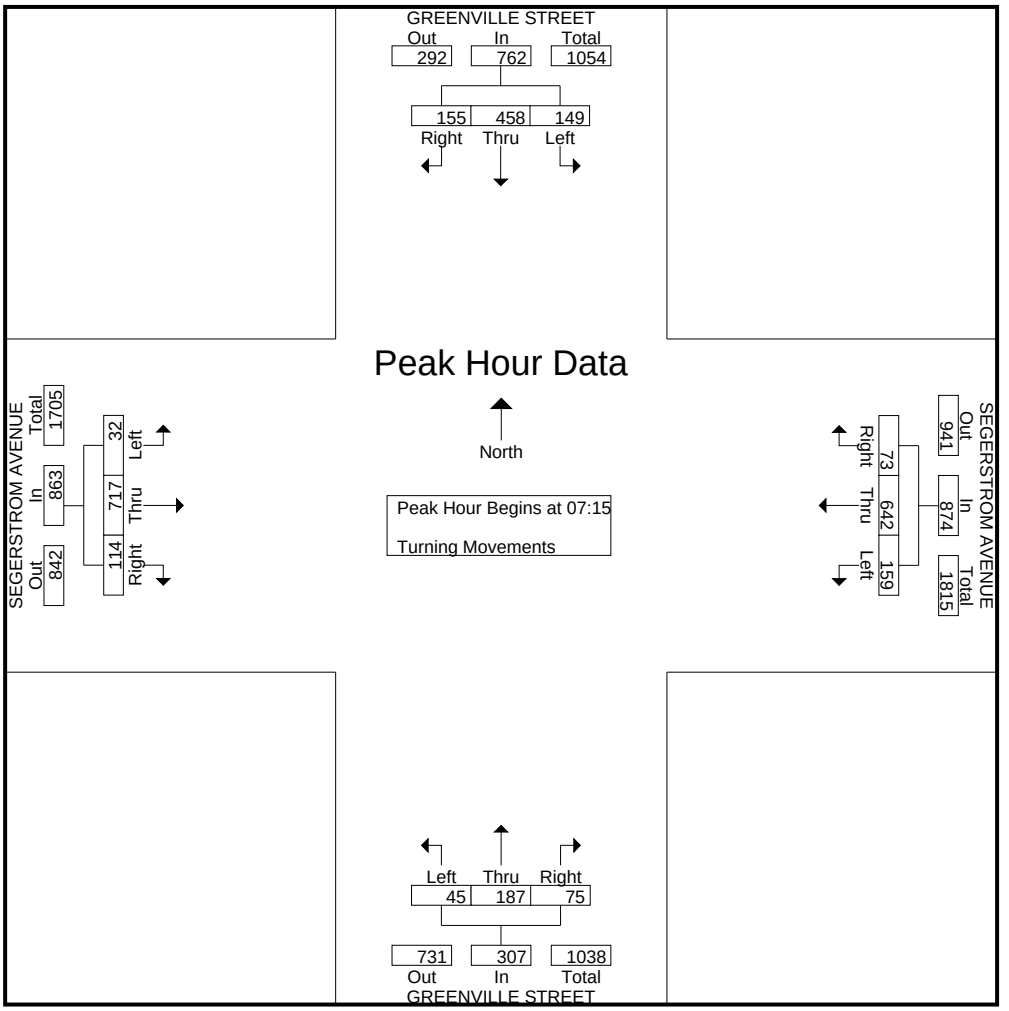
	GREENVILLE STREET Southbound			SEGERSTROM AVENUE Westbound			GREENVILLE STREET Northbound			SEGERSTROM AVENUE Eastbound			
Start Time	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Int. Total
07:00	18	44	21	11	97	9	10	9	6	11	119	5	360
07:15	23	88	27	12	114	24	8	41	6	26	175	10	554
07:30	44	139	49	12	161	40	17	37	9	31	193	10	742
07:45	53	138	45	29	189	57	27	62	12	29	191	5	837
Total	138	409	142	64	561	130	62	149	33	97	678	30	2493
08:00	35	93	28	20	178	38	23	47	18	28	158	7	673
08:15	17	59	32	8	98	6	6	31	10	7	176	9	459
08:30	15	50	14	7	99	1	7	10	5	9	185	7	409
08:45	11	28	8	4	97	6	6	11	3	5	130	6	315
Total	78	230	82	39	472	51	42	99	36	49	649	29	1856
*** BREAK ***													
16:00	8	36	8	25	215	7	4	69	13	7	167	32	591
16:15	12	38	12	25	254	8	4	112	15	12	147	24	663
16:30	14	45	14	27	229	5	6	101	11	5	174	37	668
16:45	6	35	6	24	252	8	9	111	21	8	166	25	671
Total	40	154	40	101	950	28	23	393	60	32	654	118	2593
17:00	13	38	13	35	223	4	7	121	17	12	207	30	720
17:15	14	52	14	37	280	6	12	128	15	10	207	31	806
17:30	15	58	15	38	246	6	5	105	8	11	213	28	748
17:45	17	46	17	33	257	10	10	108	15	12	171	15	711
Total	59	194	59	143	1006	26	34	462	55	45	798	104	2985
Grand Total	315	987	323	347	2989	235	161	1103	184	223	2779	281	9927
Apprch %	19.4	60.7	19.9	9.7	83.7	6.6	11.1	76.2	12.7	6.8	84.6	8.6	
Total %	3.2	9.9	3.3	3.5	30.1	2.4	1.6	11.1	1.9	2.2	28	2.8	

Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: GREENVILLE STREET
E-W Direction: SEGERSTROM AVENUE

File Name : H1702004
Site Code : 00000000
Start Date : 2/7/2017

	GREENVILLE STREET Southbound				SEGERSTROM AVENUE Westbound				GREENVILLE STREET Northbound				SEGERSTROM AVENUE Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:15																	
07:15	23	88	27	138	12	114	24	150	8	41	6	55	26	175	**10**	211	554
07:30	44	**139**	**49**	232	12	161	40	213	17	37	9	63	**31**	**193**	10	**234**	742
07:45	**53**	138	45	**236**	**29**	**189**	**57**	**275**	**27**	**62**	12	**101**	29	191	5	225	**837**
08:00	35	93	28	156	20	178	38	236	23	47	**18**	88	28	158	7	193	673
Total Volume	155	458	149	762	73	642	159	874	75	187	45	307	114	717	32	863	2806
% App. Total	20.3	60.1	19.6		8.4	73.5	18.2		24.4	60.9	14.7		13.2	83.1	3.7		
PHF	.731	.824	.760	.807	.629	.849	.697	.795	.694	.754	.625	.760	.919	.929	.800	.922	.838

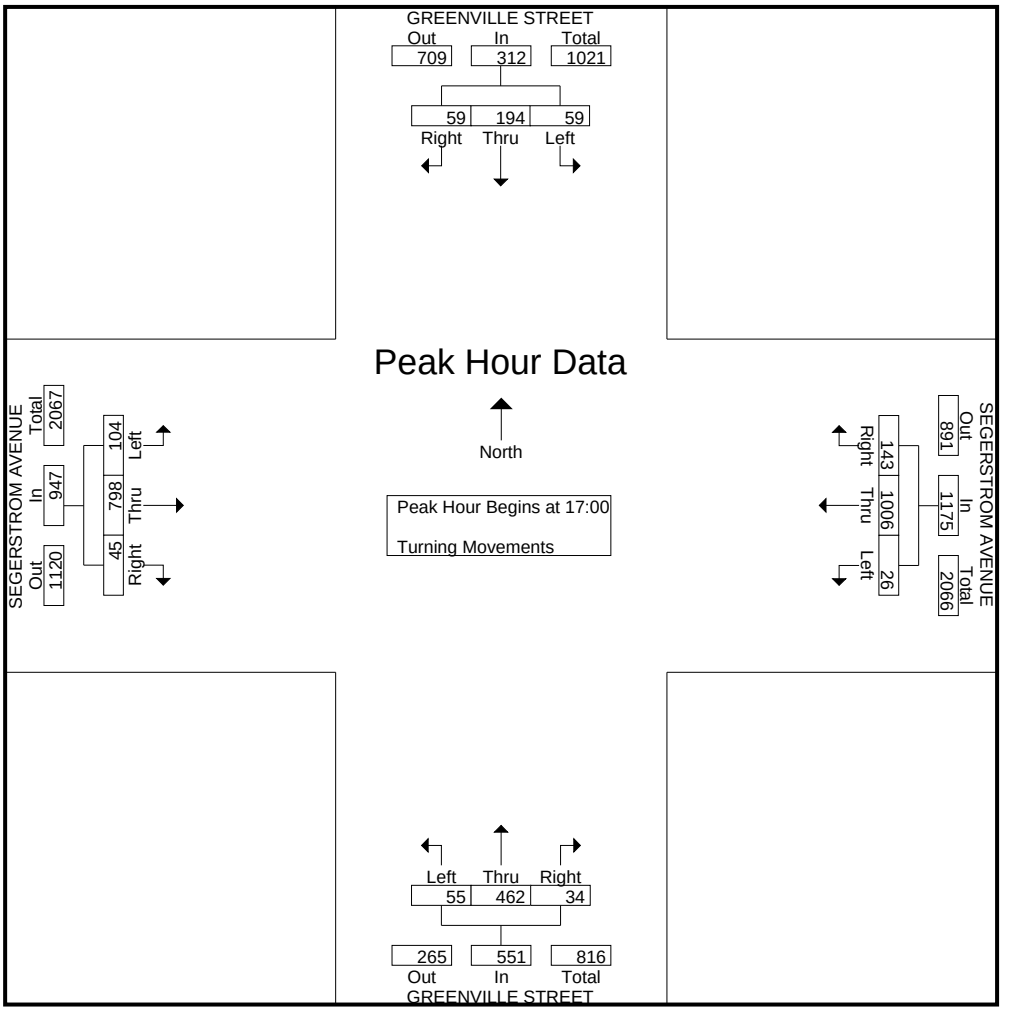


Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: GREENVILLE STREET
E-W Direction: SEGERSTROM AVENUE

File Name : H1702004
Site Code : 00000000
Start Date : 2/7/2017
Page No : 3

	GREENVILLE STREET Southbound				SEGERSTROM AVENUE Westbound				GREENVILLE STREET Northbound				SEGERSTROM AVENUE Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 17:00																	
17:00	13	38	13	64	35	223	4	262	7	121	**17**	145	**12**	207	30	249	720
17:15	14	52	14	80	37	**280**	6	**323**	**12**	**128**	15	**155**	10	207	**31**	248	**806**
17:30	15	**58**	15	**88**	**38**	246	6	290	5	105	8	118	11	**213**	28	**252**	748
17:45	**17**	46	**17**	80	33	257	**10**	300	10	108	15	133	12	171	15	198	711
Total Volume	59	194	59	312	143	1006	26	1175	34	462	55	551	45	798	104	947	2985
% App. Total	18.9	62.2	18.9		12.2	85.6	2.2		6.2	83.8	10		4.8	84.3	11		
PHF	.868	.836	.868	.886	.941	.898	.650	.909	.708	.902	.809	.889	.938	.937	.839	.939	.926



INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

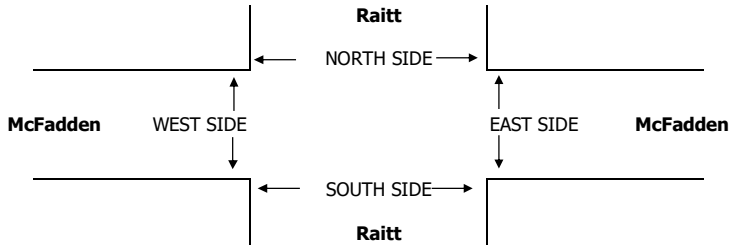
DATE:	LOCATION:	Santa Ana	PROJECT #:	SC0846
Tue, Feb 9, 16	NORTH & SOUTH:	Raitt	LOCATION #:	34
	EAST & WEST:	McFadden	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND (Raitt)			SOUTHBOUND (Raitt)			EASTBOUND (McFadden)			WESTBOUND (McFadden)				U-TURNS				
	LANES:	NL 1	NT 2	NR 0	SL 1	ST 2	SR 0	EL 1	ET 2	ER 0	WL 1	WT 2	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	6	75	22	9	149	13	12	142	21	24	87	10	570	0	0	0	0	0
	7:15 AM	8	109	20	20	231	25	24	130	25	39	124	18	773	0	0	0	0	0
	7:30 AM	8	153	21	27	213	25	24	175	22	54	155	26	903	0	0	0	0	0
	7:45 AM	20	137	18	28	183	27	23	190	25	47	133	22	853	0	0	0	0	0
	8:00 AM	23	148	27	21	105	22	40	181	14	25	108	27	741	0	0	0	0	0
	8:15 AM	9	88	15	20	95	14	24	137	18	13	105	10	548	0	0	0	0	0
	8:30 AM	11	62	16	10	82	14	18	132	11	10	120	14	500	0	0	0	0	0
	8:45 AM	7	63	13	11	58	15	16	123	2	12	109	13	442	0	0	0	0	0
	VOLUMES	92	835	152	146	1,116	155	181	1,210	138	224	941	140	5,330	0	0	0	0	0
	APPROACH %	9%	77%	14%	10%	79%	11%	12%	79%	9%	17%	72%	11%						
	APP/DEPART	1,079	/	1,156	1,417	/	1,478	1,529	/	1,508	1,305	/	1,188	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	59	547	86	96	732	99	111	676	86	165	520	93	3,270					
	APPROACH %	9%	79%	12%	10%	79%	11%	13%	77%	10%	21%	67%	12%						
	PEAK HR FACTOR		0.874			0.840			0.917			0.828		0.905					
	APP/DEPART	692	/	751	927	/	983	873	/	858	778	/	678	0					
PM	4:00 PM	12	184	34	11	91	12	23	157	8	18	114	17	681	0	0	0	0	0
	4:15 PM	13	171	32	8	93	17	15	160	6	31	174	21	741	0	0	0	0	0
	4:30 PM	16	191	29	11	92	17	28	151	11	21	158	22	747	0	0	0	0	0
	4:45 PM	12	198	17	11	109	13	19	160	11	19	184	25	778	0	0	0	0	0
	5:00 PM	18	181	39	10	98	12	24	180	14	24	158	29	787	0	0	0	0	0
	5:15 PM	11	209	31	9	114	21	30	172	10	12	193	29	841	0	0	0	0	0
	5:30 PM	17	210	30	11	121	18	37	190	18	21	166	22	861	0	0	0	0	0
	5:45 PM	9	202	24	13	122	22	30	175	20	27	196	18	858	0	0	0	0	0
	VOLUMES	108	1,546	236	84	840	132	206	1,345	98	173	1,343	183	6,294	0	0	0	0	0
	APPROACH %	6%	82%	12%	8%	80%	13%	12%	82%	6%	10%	79%	11%						
	APP/DEPART	1,890	/	1,935	1,056	/	1,111	1,649	/	1,665	1,699	/	1,583	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	55	802	124	43	455	73	121	717	62	84	713	98	3,347					
	APPROACH %	6%	82%	13%	8%	80%	13%	13%	80%	7%	9%	80%	11%						
	PEAK HR FACTOR		0.954			0.909			0.918			0.928		0.972					
	APP/DEPART	981	/	1,021	571	/	601	900	/	884	895	/	841	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	19	8	1	16	44	17	6	1	14	38	2	2	0	2	6
	7:15 AM	12	8	6	24	50	9	6	3	22	40	3	2	3	2	10
	7:30 AM	23	30	12	36	101	23	28	10	35	96	0	2	2	1	5
	7:45 AM	62	19	13	44	138	60	18	11	42	131	2	1	2	2	7
	8:00 AM	16	15	5	27	63	13	12	1	25	51	3	3	4	2	12
	8:15 AM	6	19	2	12	39	4	18	1	12	35	2	1	1	0	4
	8:30 AM	11	14	0	10	35	10	12	0	10	32	1	2	0	0	3
	8:45 AM	5	8	4	3	20	4	7	4	3	18	1	1	0	0	2
	TOTAL	154	121	43	172	490	140	107	31	163	441	14	14	12	9	49
PM	4:00 PM	13	14	7	17	51	11	13	5	17	46	2	1	2	0	5
	4:15 PM	9	17	4	14	44	6	11	3	12	32	3	6	1	2	12
	4:30 PM	12	5	7	6	30	9	2	7	6	24	3	3	0	0	6
	4:45 PM	7	11	4	12	34	5	7	2	12	26	2	4	2	0	8
	5:00 PM	7	9	3	15	34	3	7	2	15	27	4	2	1	0	7
	5:15 PM	13	16	14	15	58	12	15	13	15	55	1	1	1	0	3
	5:30 PM	6	12	1	20	39	5	7	0	19	31	1	5	1	1	8
	5:45 PM	12	17	5	13	47	7	14	5	12	38	5	3	0	1	9
	TOTAL	79	101	45	112	337	58	76	37	108	279	21	25	8	4	58

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

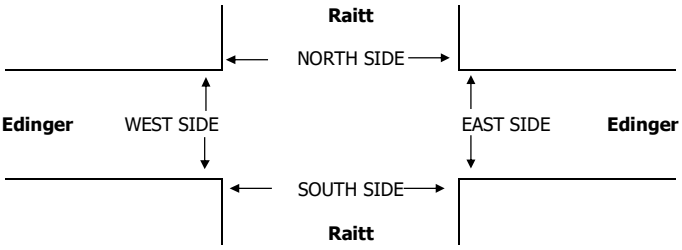
DATE:	LOCATION:	Santa Ana	PROJECT #:	SC0846
Tue, Feb 9, 16	NORTH & SOUTH:	Raitt	LOCATION #:	40
	EAST & WEST:	Edinger	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND (Raitt)			SOUTHBOUND (Raitt)			EASTBOUND (Edinger)			WESTBOUND (Edinger)				U-TURNS				
	LANES:	NL 1	NT 2	NR 0	SL 1	ST 2	SR 0	EL 1	ET 2	ER 0	WL 1	WT 2	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	20	71	19	25	121	29	22	198	28	22	198	7	760	0	0	0	0	0
	7:15 AM	33	93	33	25	173	32	45	240	42	24	250	13	1,003	0	0	0	0	0
	7:30 AM	55	125	42	48	193	14	38	222	37	36	223	11	1,044	0	0	0	0	0
	7:45 AM	45	123	45	36	178	15	41	254	24	32	215	11	1,019	0	0	0	0	0
	8:00 AM	47	140	53	37	144	20	34	252	36	20	192	16	991	0	0	0	0	0
	8:15 AM	28	82	28	32	71	15	11	249	33	14	160	16	739	0	0	0	0	0
	8:30 AM	10	56	22	35	57	8	8	205	9	17	171	11	609	0	0	0	0	0
	8:45 AM	19	44	19	24	61	11	13	205	8	15	151	11	581	0	0	0	0	0
	VOLUMES	257	734	261	262	998	144	212	1,825	217	180	1,560	96	6,746	0	0	0	0	0
	APPROACH %	21%	59%	21%	19%	71%	10%	9%	81%	10%	10%	85%	5%						
	APP/DEPART	1,252	/	1,042	1,404	/	1,395	2,254	/	2,348	1,836	/	1,961	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	180	481	173	146	688	81	158	968	139	112	880	51	4,057					
	APPROACH %	22%	58%	21%	16%	75%	9%	12%	77%	11%	11%	84%	5%						
	PEAK HR FACTOR		0.869			0.897			0.967			0.909		0.972					
	APP/DEPART	834	/	690	915	/	939	1,265	/	1,287	1,043	/	1,141	0					
PM	4:00 PM	32	190	58	16	71	20	24	244	25	14	245	19	958	0	0	0	0	0
	4:15 PM	35	173	61	22	81	21	30	210	30	16	240	28	947	0	0	0	0	0
	4:30 PM	51	169	32	32	64	26	29	222	18	23	231	28	925	0	0	0	0	0
	4:45 PM	50	203	56	24	93	26	34	214	28	17	235	26	1,006	0	0	0	0	0
	5:00 PM	50	198	47	29	76	18	31	227	34	19	257	16	1,002	0	0	0	0	0
	5:15 PM	37	193	63	32	90	25	34	216	31	20	244	20	1,005	0	0	0	0	0
	5:30 PM	36	193	55	17	107	21	30	255	26	21	238	27	1,026	0	0	0	0	0
	5:45 PM	59	200	55	28	96	28	38	225	40	22	195	20	1,006	0	0	0	0	0
	VOLUMES	350	1,519	427	200	678	185	250	1,813	232	152	1,885	184	7,875	0	0	0	0	0
	APPROACH %	15%	66%	19%	19%	64%	17%	11%	79%	10%	7%	85%	8%						
	APP/DEPART	2,296	/	1,953	1,063	/	1,062	2,295	/	2,440	2,221	/	2,420	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	182	784	220	106	369	92	133	923	131	82	934	83	4,039					
	APPROACH %	15%	66%	19%	19%	65%	16%	11%	78%	11%	7%	85%	8%						
	PEAK HR FACTOR		0.944			0.933			0.954			0.941		0.984					
	APP/DEPART	1,186	/	1,000	567	/	582	1,187	/	1,249	1,099	/	1,208	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	12	32	8	17	69	10	26	6	16	58	2	6	2	1	11
	7:15 AM	48	33	7	27	115	46	27	7	26	106	2	6	0	1	9
	7:30 AM	86	52	24	78	240	81	47	24	75	227	5	5	0	3	13
	7:45 AM	91	65	41	70	267	87	60	39	68	254	4	5	2	2	13
	8:00 AM	51	57	29	55	192	48	53	28	52	181	3	4	1	3	11
	8:15 AM	11	29	8	15	63	10	28	7	15	60	1	1	1	0	3
	8:30 AM	7	4	2	2	15	6	2	2	2	12	1	2	0	0	3
	8:45 AM	8	3	2	5	18	8	2	2	5	17	0	1	0	0	1
	TOTAL	314	275	121	269	979	296	245	115	259	915	18	30	6	10	64
PM	4:00 PM	27	27	14	16	84	19	23	13	13	68	8	4	1	3	16
	4:15 PM	12	13	7	3	35	7	11	5	3	26	5	2	2	0	9
	4:30 PM	17	22	5	11	55	7	14	4	6	31	10	8	1	5	24
	4:45 PM	27	31	22	22	102	20	29	19	18	86	7	2	3	4	16
	5:00 PM	11	24	9	21	65	10	17	9	20	56	1	7	0	1	9
	5:15 PM	21	25	8	17	71	15	18	4	15	52	6	7	4	2	19
	5:30 PM	25	16	12	12	65	21	9	7	12	49	4	7	5	0	16
	5:45 PM	7	12	3	11	33	6	3	3	6	18	1	9	0	5	15
	TOTAL	147	170	80	113	510	105	124	64	93	386	42	46	16	20	124

Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: BEAR STREET
E-W Direction: MACARTHUR BOULEVARD

File Name : H1702005
Site Code : 00000000
Start Date : 2/7/2017
Page No : 1

Groups Printed- Turning Movements

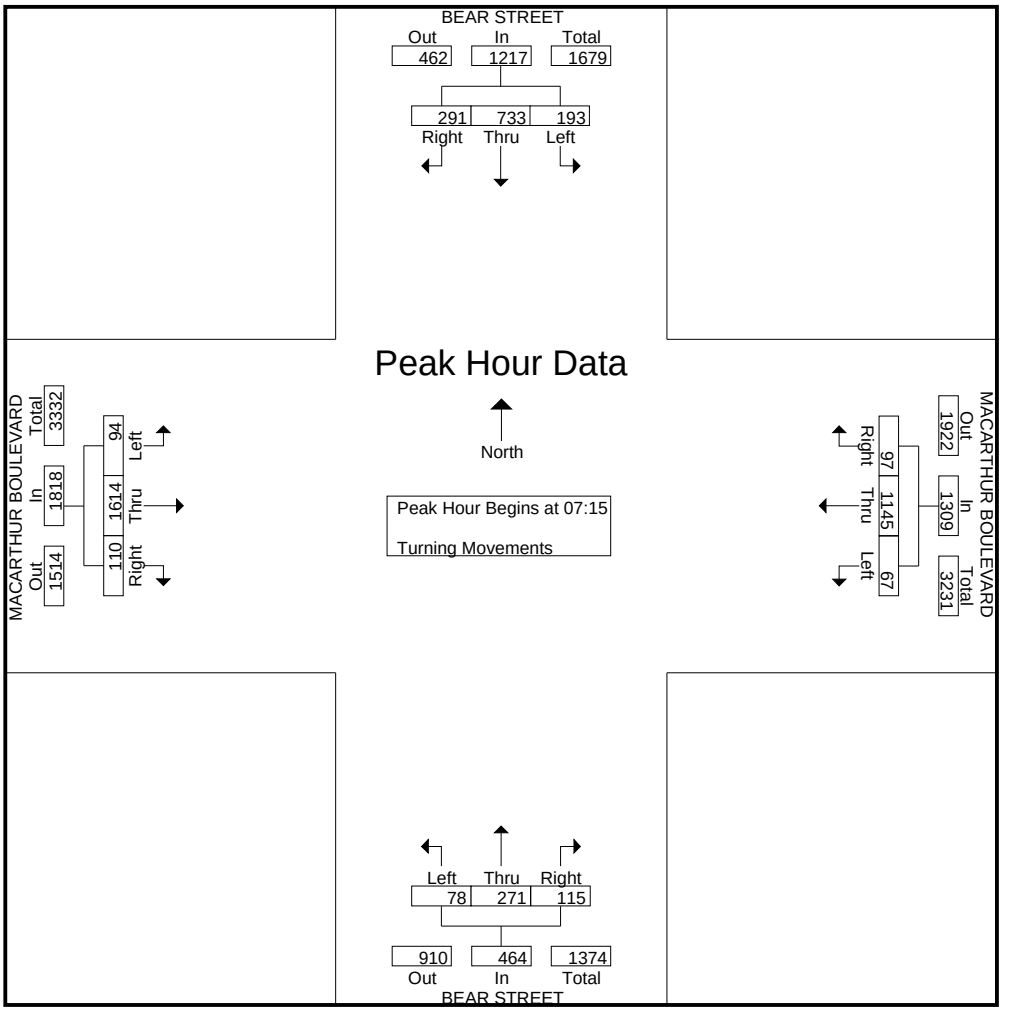
	BEAR STREET Southbound			MACARTHUR BOULEVARD Westbound			BEAR STREET Northbound			MACARTHUR BOULEVARD Eastbound			
Start Time	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Int. Total
07:00	22	100	28	11	160	5	26	63	9	17	271	9	721
07:15	56	154	42	19	294	16	27	66	21	23	382	11	1111
07:30	77	198	53	30	380	13	25	49	24	26	434	19	1328
07:45	111	199	45	31	342	17	30	72	20	36	392	34	1329
Total	266	651	168	91	1176	51	108	250	74	102	1479	73	4489
08:00	47	182	53	17	129	21	33	84	13	25	406	30	1040
08:15	26	155	43	13	136	14	36	85	5	27	346	14	900
08:30	18	127	37	6	109	11	36	69	10	53	386	7	869
08:45	19	95	23	14	110	16	34	46	10	30	276	10	683
Total	110	559	156	50	484	62	139	284	38	135	1414	61	3492
*** BREAK ***													
16:00	16	43	18	53	395	17	67	196	39	25	210	14	1093
16:15	13	74	19	66	388	16	59	210	59	18	214	16	1152
16:30	14	60	27	62	417	25	62	214	46	24	223	15	1189
16:45	17	62	24	65	396	25	59	227	51	20	213	21	1180
Total	60	239	88	246	1596	83	247	847	195	87	860	66	4614
17:00	17	63	25	69	386	18	58	242	46	20	240	29	1213
17:15	21	75	36	66	399	20	68	244	56	20	206	18	1229
17:30	24	63	23	58	442	27	58	220	41	31	224	26	1237
17:45	32	66	28	59	416	29	56	204	48	18	198	22	1176
Total	94	267	112	252	1643	94	240	910	191	89	868	95	4855
Grand Total	530	1716	524	639	4899	290	734	2291	498	413	4621	295	17450
Apprch %	19.1	61.9	18.9	11	84.1	5	20.8	65	14.1	7.8	86.7	5.5	
Total %	3	9.8	3	3.7	28.1	1.7	4.2	13.1	2.9	2.4	26.5	1.7	

Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: BEAR STREET
E-W Direction: MACARTHUR BOULEVARD

File Name : H1702005
Site Code : 00000000
Start Date : 2/7/2017

	BEAR STREET Southbound				MACARTHUR BOULEVARD Westbound				BEAR STREET Northbound				MACARTHUR BOULEVARD Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:15																	
07:15	56	154	42	252	19	294	16	329	27	66	21	114	23	382	11	416	1111
07:30	77	198	**53**	328	30	**380**	13	**423**	25	49	**24**	98	26	**434**	19	**479**	1328
07:45	**111**	**199**	45	**355**	**31**	342	17	390	30	72	20	122	**36**	392	**34**	462	**1329**
08:00	47	182	53	282	17	129	**21**	167	**33**	**84**	13	**130**	25	406	30	461	1040
Total Volume	291	733	193	1217	97	1145	67	1309	115	271	78	464	110	1614	94	1818	4808
% App. Total	23.9	60.2	15.9		7.4	87.5	5.1		24.8	58.4	16.8		6.1	88.8	5.2		
PHF	.655	.921	.910	.857	.782	.753	.798	.774	.871	.807	.813	.892	.764	.930	.691	.949	.904

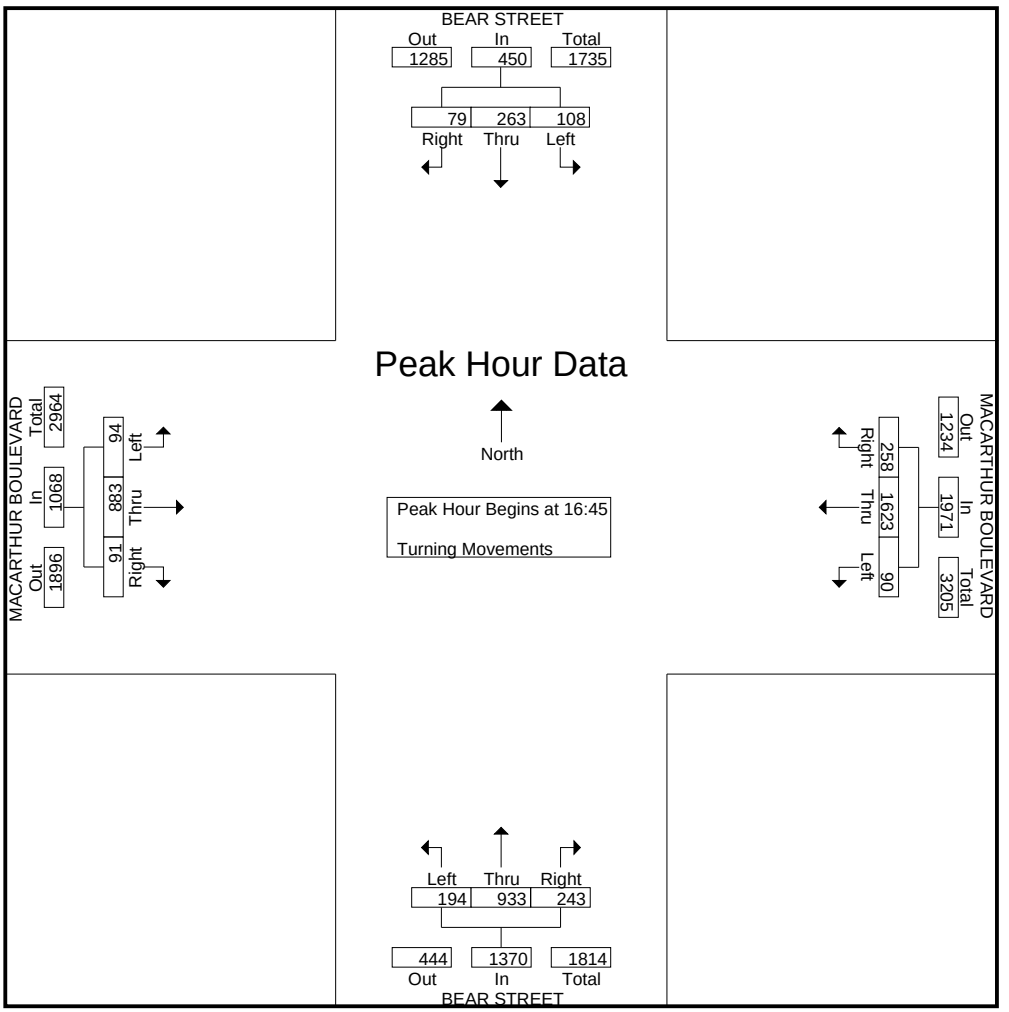


Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: BEAR STREET
E-W Direction: MACARTHUR BOULEVARD

File Name : H1702005
Site Code : 00000000
Start Date : 2/7/2017

	BEAR STREET Southbound				MACARTHUR BOULEVARD Westbound				BEAR STREET Northbound				MACARTHUR BOULEVARD Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 16:45																	
16:45	17	62	24	103	65	396	25	486	59	227	51	337	20	213	21	254	1180
17:00	17	63	25	105	**69**	386	18	473	58	242	46	346	20	**240**	**29**	**289**	1213
17:15	21	**75**	**36**	**132**	66	399	20	485	**68**	**244**	**56**	**368**	20	206	18	244	1229
17:30	**24**	63	23	110	58	**442**	**27**	**527**	58	220	41	319	**31**	224	26	281	**1237**
Total Volume	79	263	108	450	258	1623	90	1971	243	933	194	1370	91	883	94	1068	4859
% App. Total	17.6	58.4	24		13.1	82.3	4.6		17.7	68.1	14.2		8.5	82.7	8.8		
PHF	.823	.877	.750	.852	.935	.918	.833	.935	.893	.956	.866	.931	.734	.920	.810	.924	.982



INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Tue, May 14, 19

LOCATION: Santa Ana
NORTH & SOUTH: Bristol
EAST & WEST: 17th

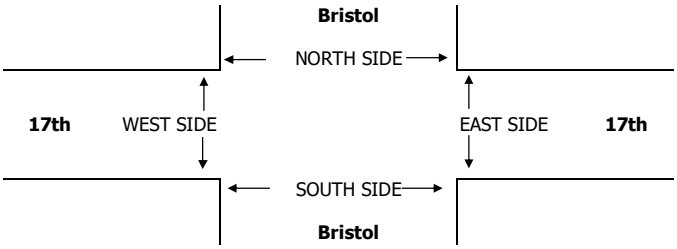
PROJECT #: SC2183
LOCATION #: 12
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Bristol			SOUTHBOUND Bristol			EASTBOUND 17th			WESTBOUND 17th				U-TURNS				
	LANES:	NL 2	NT 3	NR 1	SL 2	ST 2	SR 1	EL 2	ET 3	ER 0	WL 2	WT 3	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	23	254	42	59	328	28	47	230	34	46	116	51	1,258	4	0	0	2	6
	7:15 AM	32	285	28	83	324	26	67	258	35	48	141	69	1,396	3	0	0	3	6
	7:30 AM	45	270	41	102	342	21	78	281	38	76	202	92	1,588	3	0	0	8	11
	7:45 AM	59	234	49	94	311	40	86	250	33	87	225	73	1,541	2	1	1	8	12
	8:00 AM	39	248	39	83	368	46	71	183	42	91	184	51	1,445	4	0	0	11	15
	8:15 AM	47	226	36	52	341	72	85	239	41	101	204	54	1,498	7	0	1	2	10
	8:30 AM	39	256	43	53	361	61	63	203	26	51	129	22	1,307	5	0	0	5	10
	8:45 AM	37	246	38	65	280	63	55	174	37	51	93	31	1,170	5	1	1	6	13
	VOLUMES	321	2,019	316	591	2,655	357	552	1,818	286	551	1,294	443	11,203	33	2	3	45	83
	APPROACH %	12%	76%	12%	16%	74%	10%	21%	68%	11%	24%	57%	19%						
	APP/DEPART	2,656	/	3,013	3,603	/	3,480	2,656	/	2,768	2,288	/	1,942	0					
	BEGIN PEAK HR		7:30 AM																
	VOLUMES	190	978	165	331	1,362	179	320	953	154	355	815	270	6,072					
	APPROACH %	14%	73%	12%	18%	73%	10%	22%	67%	11%	25%	57%	19%						
	PEAK HR FACTOR		0.936			0.942			0.899			0.935		0.956					
	APP/DEPART	1,333	/	1,567	1,872	/	1,858	1,427	/	1,477	1,440	/	1,170	0					
PM	4:00 PM	43	282	37	68	272	60	85	168	25	48	260	76	1,424	2	2	2	7	13
	4:15 PM	45	282	37	66	295	50	61	178	33	63	234	67	1,411	4	1	0	12	17
	4:30 PM	40	277	26	53	286	48	69	193	34	55	308	71	1,460	2	1	0	7	10
	4:45 PM	36	281	40	68	308	45	74	177	28	70	255	87	1,469	3	0	2	6	11
	5:00 PM	61	284	58	50	311	56	85	192	40	57	286	107	1,587	5	0	1	7	13
	5:15 PM	53	264	41	62	317	54	97	223	30	71	247	81	1,540	2	0	2	8	12
	5:30 PM	55	304	42	75	322	57	71	163	37	77	265	93	1,561	2	0	1	8	11
	5:45 PM	64	284	48	53	330	50	79	164	27	76	260	77	1,512	8	0	0	11	19
	VOLUMES	397	2,258	329	495	2,441	420	621	1,458	254	517	2,115	659	11,964	28	4	8	66	106
	APPROACH %	13%	76%	11%	15%	73%	13%	27%	62%	11%	16%	64%	20%						
	APP/DEPART	2,984	/	3,534	3,356	/	3,174	2,333	/	2,344	3,291	/	2,912	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	233	1,136	189	240	1,280	217	332	742	134	281	1,058	358	6,200					
	APPROACH %	15%	73%	12%	14%	74%	12%	27%	61%	11%	17%	62%	21%						
	PEAK HR FACTOR		0.967			0.956			0.863			0.943		0.977					
	APP/DEPART	1,558	/	1,822	1,737	/	1,678	1,208	/	1,205	1,697	/	1,495	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:30 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	5:00 PM					0	0	0	0	0					

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

DATE: Wed, Feb 10, 16

LOCATION: Santa Ana
NORTH & SOUTH: Bristol
EAST & WEST: Civic Center

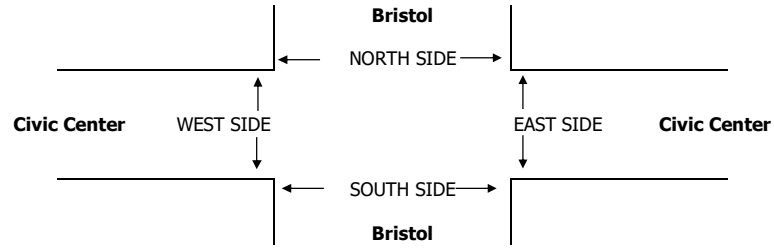
PROJECT #: SC0846
LOCATION #: 13
CONTROL: SIGNAL

NOTES:

AM PM MD OTHER OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Bristol			SOUTHBOUND Bristol			EASTBOUND Civic Center			WESTBOUND Civic Center				U-TURNS				
	LANES:	NL 1	NT 3	NR 0	SL 1	ST 3	SR 0	EL 1	ET 2	ER 0	WL 1	WT 2	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	15	257	12	66	336	19	35	100	24	26	44	8	942	2	1	0	0	3
	7:15 AM	31	265	11	104	263	15	44	153	27	22	61	6	1,002	2	0	0	0	2
	7:30 AM	28	256	15	83	297	22	49	195	28	25	77	14	1,089	1	1	0	0	2
	7:45 AM	39	306	21	87	291	16	37	217	18	21	103	17	1,173	0	4	0	0	4
	8:00 AM	37	240	17	72	288	27	29	182	28	45	136	25	1,126	3	5	0	0	8
	8:15 AM	14	242	39	63	357	11	28	131	15	25	69	23	1,017	0	5	0	6	11
	8:30 AM	18	267	27	70	357	14	24	101	14	15	63	14	984	3	3	0	0	6
	8:45 AM	13	259	19	57	326	17	21	94	20	23	53	13	915	4	2	0	0	6
	VOLUMES	195	2,092	161	602	2,515	141	267	1,173	174	202	606	120	8,248	15	21	0	6	42
	APPROACH %	8%	85%	7%	18%	77%	4%	17%	73%	11%	22%	65%	13%						
	APP/DEPART	2,448	/	2,500	3,258	/	2,900	1,614	/	1,921	928	/	927	0					
	BEGIN PEAK HR		7:30 AM																
	VOLUMES	118	1,044	92	305	1,233	76	143	725	89	116	385	79	4,405					
	APPROACH %	9%	83%	7%	19%	76%	5%	15%	76%	9%	20%	66%	14%						
	PEAK HR FACTOR		0.857			0.936			0.880			0.704		0.939					
	APP/DEPART	1,254	/	1,281	1,614	/	1,436	957	/	1,113	580	/	575	0					
PM	4:00 PM	28	247	15	25	206	20	35	108	16	28	93	32	853	8	7	0	0	15
	4:15 PM	44	335	16	37	311	29	37	101	12	21	86	33	1,062	9	5	0	0	14
	4:30 PM	36	314	26	32	330	28	25	60	13	38	134	31	1,067	5	6	0	0	11
	4:45 PM	41	314	20	30	296	30	56	80	17	41	133	52	1,110	4	3	4	0	11
	5:00 PM	42	341	23	30	280	28	45	92	14	38	178	34	1,145	1	1	0	0	2
	5:15 PM	55	371	21	38	311	30	32	153	13	38	212	17	1,291	6	3	0	0	9
	5:30 PM	43	334	18	33	310	29	44	119	12	37	160	17	1,156	7	7	0	0	14
	5:45 PM	52	298	8	29	300	36	61	149	21	49	144	10	1,157	5	3	0	0	8
	VOLUMES	341	2,554	147	254	2,344	230	335	862	118	290	1,140	226	8,841	45	35	4	0	84
	APPROACH %	11%	84%	5%	9%	83%	8%	25%	66%	9%	18%	69%	14%						
	APP/DEPART	3,042	/	3,146	2,828	/	2,797	1,315	/	1,228	1,656	/	1,670	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	192	1,344	70	130	1,201	123	182	513	60	162	694	78	4,749					
	APPROACH %	12%	84%	4%	9%	83%	8%	24%	68%	8%	17%	74%	8%						
	PEAK HR FACTOR		0.898			0.959			0.817			0.875		0.920					
	APP/DEPART	1,606	/	1,618	1,454	/	1,442	755	/	699	934	/	990	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	2	7	10	5	24	2	7	8	5	22	0	0	2	0	2
	7:15 AM	10	4	8	11	33	8	3	8	7	26	2	1	0	4	7
	7:30 AM	9	12	3	10	34	7	10	3	6	26	2	2	0	4	8
	7:45 AM	10	10	5	12	37	10	9	2	12	33	0	1	3	0	4
	8:00 AM	7	6	2	6	21	5	6	2	5	18	2	0	0	1	3
	8:15 AM	7	9	2	2	20	7	8	0	2	17	0	1	2	0	3
	8:30 AM	1	9	3	5	18	1	6	3	2	12	0	3	0	3	6
	8:45 AM	3	6	2	12	23	2	6	1	10	19	1	0	1	2	4
	TOTAL	49	63	35	63	210	42	55	27	49	173	7	8	8	14	37
PM	4:00 PM	1	6	2	6	15	1	4	1	5	11	0	2	1	1	4
	4:15 PM	1	11	0	5	17	1	8	0	4	13	0	3	0	1	4
	4:30 PM	1	11	1	9	22	0	11	1	6	18	1	0	0	3	4
	4:45 PM	0	11	0	5	16	0	10	0	3	13	0	1	0	2	3
	5:00 PM	1	11	3	9	24	0	7	3	7	17	1	4	0	2	7
	5:15 PM	5	8	2	11	26	4	8	2	9	23	1	0	0	2	3
	5:30 PM	5	11	4	7	27	5	9	4	6	24	0	2	0	1	3
	5:45 PM	7	18	0	7	32	7	17	0	5	29	0	1	0	2	3
	TOTAL	21	87	12	59	179	18	74	11	45	148	3	13	1	14	31

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

DATE: Tue, Feb 9, 16

LOCATION: Santa Ana
NORTH & SOUTH: Bristol
EAST & WEST: Santa Ana

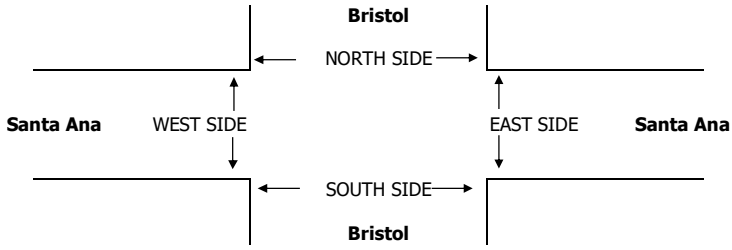
PROJECT #: SC0846
LOCATION #: 21
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Bristol			SOUTHBOUND Bristol			EASTBOUND Santa Ana			WESTBOUND Santa Ana				U-TURNS				
	LANES:	NL 1	NT 3	NR 0	SL 1	ST 3	SR 0	EL 1	ET 2	ER 0	WL 1	WT 2	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	12	299	16	24	375	3	21	53	17	25	26	9	880	0	0	0	0	0
	7:15 AM	16	341	36	33	399	9	13	72	8	24	54	4	1,009	0	3	0	0	3
	7:30 AM	16	360	52	26	369	7	29	110	9	30	64	13	1,085	0	1	0	0	1
	7:45 AM	23	352	55	39	309	12	19	136	12	21	79	8	1,065	2	6	0	0	8
	8:00 AM	7	310	46	33	371	11	18	69	4	17	33	14	933	0	6	0	0	6
	8:15 AM	12	274	25	36	369	7	9	29	3	20	23	12	819	3	2	0	0	5
	8:30 AM	11	237	21	23	307	8	13	25	7	21	15	12	700	1	3	0	0	4
	8:45 AM	5	292	14	31	345	4	8	26	2	10	18	11	766	0	5	0	0	5
	VOLUMES	102	2,465	265	245	2,844	61	130	520	62	168	312	83	7,257	6	26	0	0	32
	APPROACH %	4%	87%	9%	8%	90%	2%	18%	73%	9%	30%	55%	15%						
	APP/DEPART	2,832	/	2,704	3,150	/	3,080	712	/	1,004	563	/	469	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	62	1,363	189	131	1,448	39	79	387	33	92	230	39	4,092					
	APPROACH %	4%	84%	12%	8%	89%	2%	16%	78%	7%	25%	64%	11%						
	PEAK HR FACTOR		0.938			0.917			0.747			0.836		0.943					
	APP/DEPART	1,614	/	1,497	1,618	/	1,575	499	/	691	361	/	329	0					
PM	4:00 PM	5	338	23	30	351	12	19	30	7	43	59	11	928	0	5	0	0	5
	4:15 PM	6	345	21	25	387	13	9	35	4	31	57	18	951	0	8	0	0	8
	4:30 PM	13	345	26	29	353	18	12	42	7	38	64	23	970	3	3	0	0	6
	4:45 PM	10	361	15	18	352	7	11	34	5	42	76	23	954	1	8	0	0	9
	5:00 PM	19	333	16	20	289	14	9	32	9	64	88	29	922	3	6	0	0	9
	5:15 PM	21	386	19	27	357	14	11	41	6	61	96	27	1,066	2	5	0	0	7
	5:30 PM	13	399	21	22	353	14	15	39	14	50	85	22	1,047	1	5	0	0	6
	5:45 PM	19	346	27	31	348	8	21	44	7	48	87	18	1,004	3	7	0	0	10
	VOLUMES	106	2,853	168	202	2,790	100	107	297	59	377	612	171	7,842	13	47	0	0	60
	APPROACH %	3%	91%	5%	7%	90%	3%	23%	64%	13%	33%	53%	15%						
	APP/DEPART	3,127	/	3,178	3,092	/	3,239	463	/	620	1,160	/	805	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	72	1,464	83	100	1,347	50	56	156	36	223	356	96	4,039					
	APPROACH %	4%	90%	5%	7%	90%	3%	23%	63%	15%	33%	53%	14%						
	PEAK HR FACTOR		0.935			0.940			0.861			0.917		0.947					
	APP/DEPART	1,619	/	1,639	1,497	/	1,615	248	/	316	675	/	469	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	13	6	9	9	37	13	4	6	8	31	0	2	3	1	6
	7:15 AM	42	16	13	11	82	40	16	12	7	75	2	0	1	4	7
	7:30 AM	40	26	14	26	106	39	26	12	23	100	1	0	2	3	6
	7:45 AM	43	21	5	17	86	40	20	2	16	78	3	1	3	1	8
	8:00 AM	32	7	10	12	61	31	7	9	10	57	1	0	1	2	4
	8:15 AM	5	3	5	7	20	5	3	4	6	18	0	0	1	1	2
	8:30 AM	16	10	5	6	37	15	5	3	3	26	1	5	2	3	11
	8:45 AM	7	13	8	4	32	5	12	7	2	26	2	1	1	2	6
	TOTAL	198	102	69	92	461	188	93	55	75	411	10	9	14	17	50
PM	4:00 PM	5	10	7	9	31	5	6	4	8	23	0	4	3	1	8
	4:15 PM	8	13	13	10	44	6	11	5	8	30	2	2	8	2	14
	4:30 PM	12	3	6	10	31	10	2	6	9	27	2	1	0	1	4
	4:45 PM	1	3	4	10	18	0	3	4	8	15	1	0	0	2	3
	5:00 PM	6	15	11	10	42	6	10	7	7	30	0	5	4	3	12
	5:15 PM	21	7	18	17	63	20	7	12	16	55	1	0	6	1	8
	5:30 PM	36	18	12	30	96	29	18	9	24	80	7	0	3	6	16
	5:45 PM	26	11	28	11	76	24	11	22	9	66	2	0	6	2	10
	TOTAL	115	80	99	107	401	100	68	69	89	326	15	12	30	18	75

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

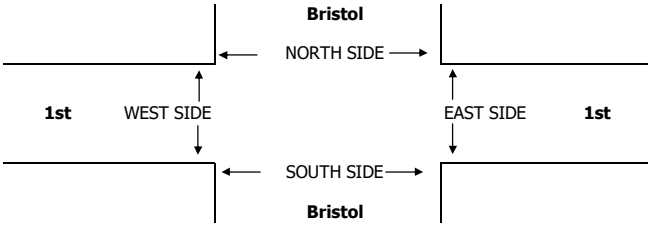
DATE:	LOCATION:	Santa Ana	PROJECT #:	SC2183
Tue, May 14, 19	NORTH & SOUTH:	Bristol	LOCATION #:	13
	EAST & WEST:	1st	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

Add U-Turns to Left Turns

		NORTHBOUND Bristol			SOUTHBOUND Bristol			EASTBOUND 1st			WESTBOUND 1st				U-TURNS				
	LANES:	NL 2	NT 3	NR 1	SL 2	ST 3	SR 1	EL 1	ET 3	ER 0	WL 1	WT 3	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	36	259	34	58	271	47	43	290	22	28	124	17	1,229	2	2	0	0	4
	7:15 AM	39	329	43	65	316	34	36	318	32	23	145	23	1,403	2	1	0	1	4
	7:30 AM	46	328	36	88	284	51	53	332	28	30	204	28	1,508	3	3	0	0	6
	7:45 AM	57	346	48	74	259	37	62	314	30	25	258	36	1,546	2	2	0	0	4
	8:00 AM	39	262	33	45	262	36	52	291	36	28	197	30	1,311	2	3	0	0	5
	8:15 AM	35	266	40	56	275	32	44	293	29	29	179	23	1,301	1	3	0	0	4
	8:30 AM	23	240	58	63	248	34	55	299	24	22	143	41	1,250	3	0	0	0	3
	8:45 AM	24	202	37	57	229	31	36	285	26	17	137	24	1,105	1	0	0	0	1
	VOLUMES	299	2,232	329	506	2,144	302	381	2,422	227	202	1,387	222	10,653	16	14	0	1	31
	APPROACH %	10%	78%	12%	17%	73%	10%	13%	80%	7%	11%	77%	12%						
	APP/DEPART	2,860	/	2,849	2,952	/	2,588	3,030	/	3,244	1,811	/	1,972	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	181	1,265	160	272	1,121	158	203	1,255	126	106	804	117	5,768					
	APPROACH %	11%	79%	10%	18%	72%	10%	13%	79%	8%	10%	78%	11%						
	PEAK HR FACTOR		0.890			0.917			0.959			0.805		0.933					
	APP/DEPART	1,606	/	1,594	1,551	/	1,361	1,584	/	1,679	1,027	/	1,134	0					
PM	4:00 PM	72	301	45	37	155	34	49	244	38	47	256	28	1,306	5	3	0	2	10
	4:15 PM	66	288	50	39	176	39	43	279	36	41	293	38	1,388	5	3	0	1	9
	4:30 PM	63	284	54	51	260	54	58	283	39	58	328	27	1,559	4	5	1	2	12
	4:45 PM	70	290	40	64	266	72	42	255	42	45	339	25	1,550	7	3	1	1	12
	5:00 PM	87	338	47	42	275	59	46	231	32	50	304	33	1,544	10	1	0	2	13
	5:15 PM	63	310	39	46	302	64	41	282	21	61	345	39	1,613	4	3	0	0	7
	5:30 PM	66	317	46	50	279	59	57	261	20	47	307	34	1,543	2	1	1	3	7
	5:45 PM	63	335	53	54	246	63	48	253	33	39	287	30	1,504	3	1	0	2	6
	VOLUMES	550	2,463	374	383	1,959	444	384	2,088	261	388	2,459	254	12,007	40	20	3	13	76
	APPROACH %	16%	73%	11%	14%	70%	16%	14%	76%	10%	13%	79%	8%						
	APP/DEPART	3,387	/	3,118	2,786	/	2,635	2,733	/	2,838	3,101	/	3,416	0					
	BEGIN PEAK HR		4:30 PM																
	VOLUMES	283	1,222	180	203	1,103	249	187	1,051	134	214	1,316	124	6,266					
	APPROACH %	17%	73%	11%	13%	71%	16%	14%	77%	10%	13%	80%	7%						
	PEAK HR FACTOR		0.892			0.944			0.903			0.929		0.971					
	APP/DEPART	1,685	/	1,543	1,555	/	1,471	1,372	/	1,427	1,654	/	1,825	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:15 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	4:30 PM					0	0	0	0	0					

Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: BRISTOL STREET
E-W Direction: MCFADDEN AVENUE

File Name : H1702008
Site Code : 00000000
Start Date : 2/7/2017
Page No : 1

Groups Printed- Turning Movements

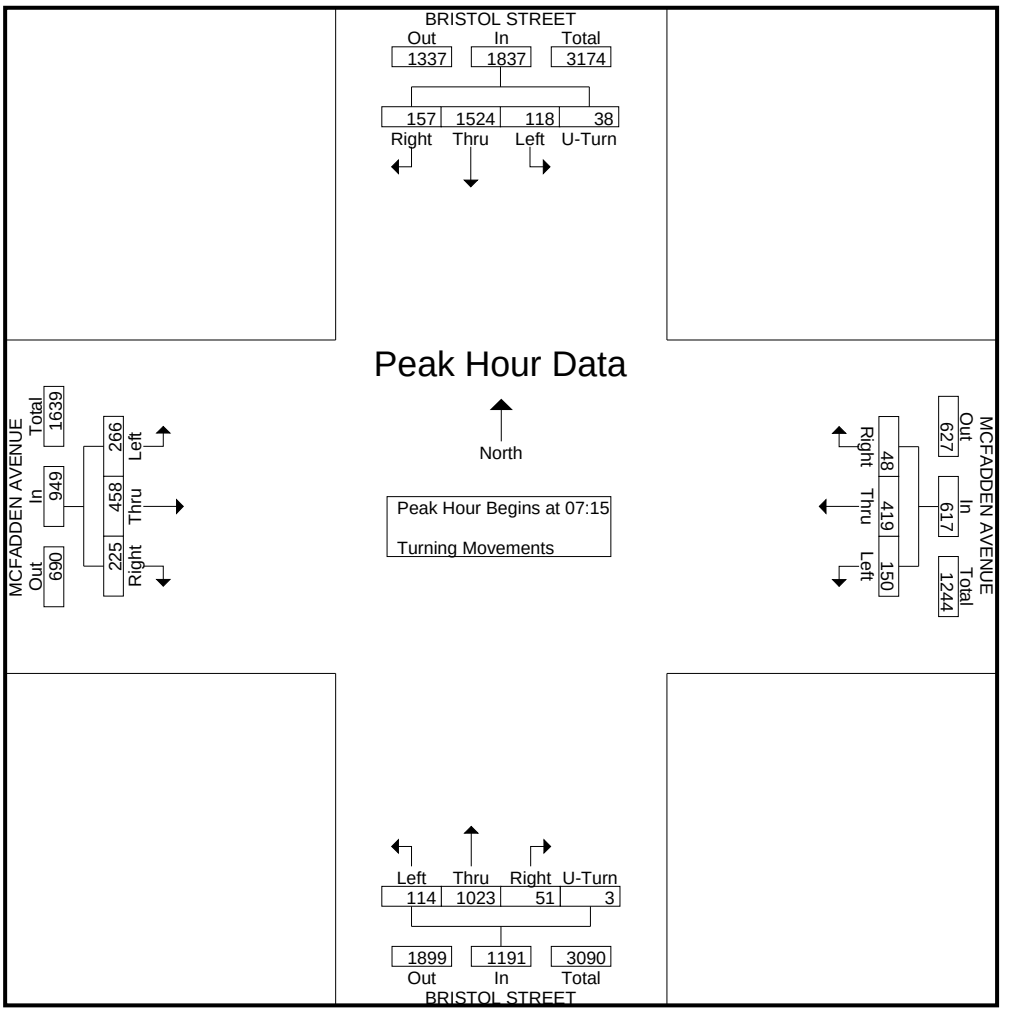
	BRISTOL STREET Southbound				MCFADDEN AVENUE Westbound			BRISTOL STREET Northbound				MCFADDEN AVENUE Eastbound			
Start Time	Right	Thru	Left	U-Turn	Right	Thru	Left	Right	Thru	Left	U-Turn	Right	Thru	Left	Int. Total
07:00	41	347	20	6	18	80	43	9	170	18	0	53	94	41	940
07:15	49	378	27	6	12	103	43	11	246	26	1	53	119	72	1146
07:30	28	388	30	8	9	110	34	18	264	35	0	53	115	80	1172
07:45	52	391	34	11	12	111	33	12	255	35	2	62	110	46	1166
Total	170	1504	111	31	51	404	153	50	935	114	3	221	438	239	4424
08:00	28	367	27	13	15	95	40	10	258	18	0	57	114	68	1110
08:15	25	350	28	5	14	66	23	14	174	19	2	33	76	48	877
08:30	24	323	16	6	10	74	24	10	218	19	0	32	81	47	884
08:45	25	320	23	7	11	54	26	11	193	24	1	22	78	47	842
Total	102	1360	94	31	50	289	113	45	843	80	3	144	349	210	3713
*** BREAK ***															
16:00	47	257	17	15	26	100	34	18	382	48	5	23	111	67	1150
16:15	61	258	27	6	22	102	31	17	368	41	2	24	98	71	1128
16:30	64	246	23	14	14	118	40	24	363	49	5	25	114	68	1167
16:45	62	268	21	14	19	116	32	19	384	46	3	30	122	57	1193
Total	234	1029	88	49	81	436	137	78	1497	184	15	102	445	263	4638
17:00	66	296	19	16	15	127	19	15	376	57	4	23	118	67	1218
17:15	65	298	25	8	12	122	22	30	382	43	2	23	136	63	1231
17:30	69	274	16	8	11	124	21	18	377	60	3	23	120	67	1191
17:45	56	286	19	16	11	121	28	22	320	64	1	28	113	71	1156
Total	256	1154	79	48	49	494	90	85	1455	224	10	97	487	268	4796
Grand Total	762	5047	372	159	231	1623	493	258	4730	602	31	564	1719	980	17571
Apprch %	12	79.6	5.9	2.5	9.8	69.2	21	4.6	84.1	10.7	0.6	17.3	52.7	30	
Total %	4.3	28.7	2.1	0.9	1.3	9.2	2.8	1.5	26.9	3.4	0.2	3.2	9.8	5.6	

Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: BRISTOL STREET
E-W Direction: MCFADDEN AVENUE

File Name : H1702008
Site Code : 00000000
Start Date : 2/7/2017
Page No : 2

	BRISTOL STREET Southbound					MCFADDEN AVENUE Westbound				BRISTOL STREET Northbound					MCFADDEN AVENUE Eastbound				
Start Time	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																			
Peak Hour for Entire Intersection Begins at 07:15																			
07:15	49	378	27	6	460	12	103	43	158	11	246	26	1	284	53	119			
07:30	28	388	30	8	454	9	110	34	153	18	**264**	**35**	0	**317**	53	115	**80**	**248**	**1172**
07:45	**52**	**391**	**34**	11	**488**	12	**111**	33	156	12	255	35	**2**	304	**62**	110	46	218	1166
08:00	28	367	27	**13**	435	**15**													
Total Volume	157	1524	118	38	1837	48	419	150	617	51	1023	114	3	1191	225	458	266	949	4594
% App. Total	8.5	83	6.4	2.1		7.8	67.9	24.3		4.3	85.9	9.6	0.3		23.7	48.3	28		
PHF	.755	.974	.868	.731	.941	.800	.944	.872	.976	.708	.969	.814	.375	.939	.907	.962	.831	.957	.980

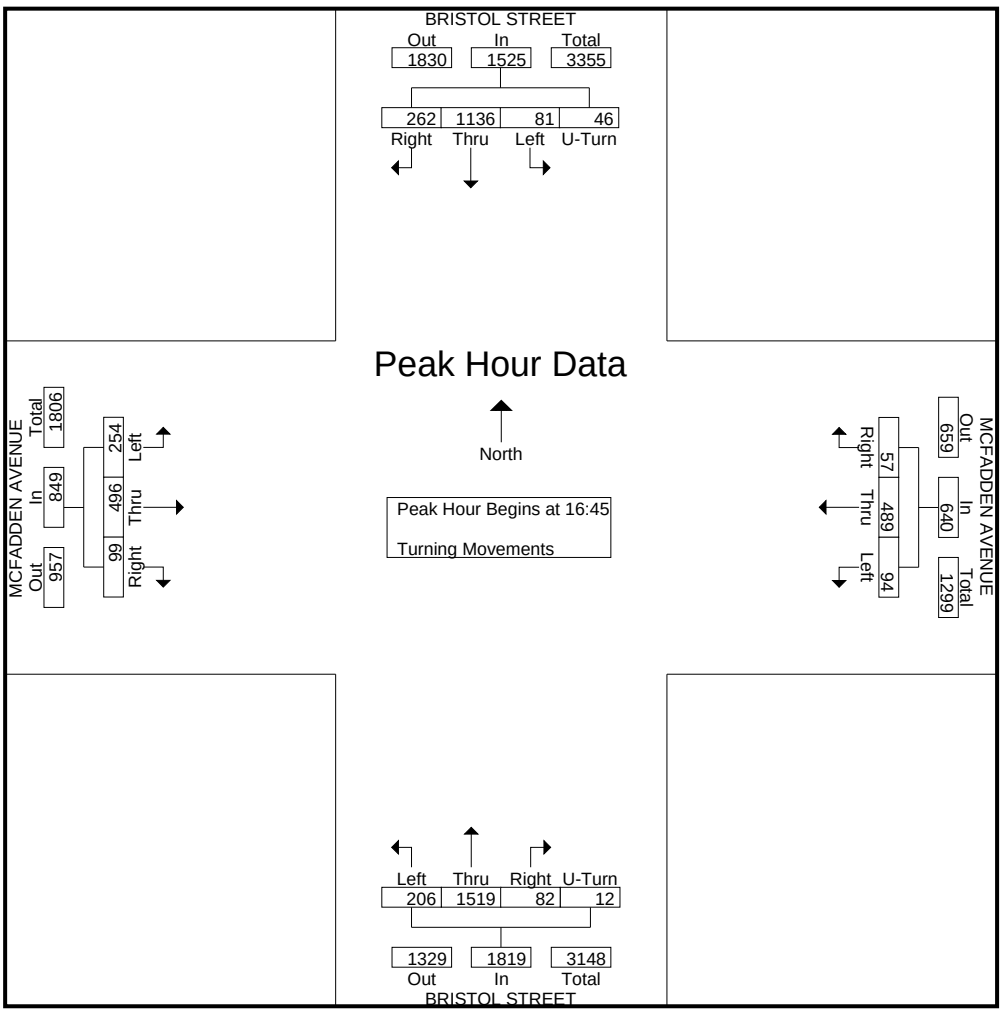


Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: BRISTOL STREET
E-W Direction: MCFADDEN AVENUE

File Name : H1702008
Site Code : 00000000
Start Date : 2/7/2017
Page No : 3

	BRISTOL STREET Southbound					MCFADDEN AVENUE Westbound				BRISTOL STREET Northbound					MCFADDEN AVENUE Eastbound				
Start Time	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1																			
Peak Hour for Entire Intersection Begins at 16:45																			
16:45	62	268	21	14	365	19	116	32	167	19	384				30				
17:00	66	296	19	**16**	**397**	15	**127**	19	161	15	376	57	**4**	452	23	118	**67**	208	1218
17:15	65	**298**	**25**	8	396	12	122	22	156	**30**	382	43	2	457	23	**136**	63	**222**	**1231**
17:30	**69**	274	16	8	367	11	124	21	156	18	377	**60**	3	**458**	23	120	67	210	1191
Total Volume	262	1136	81	46	1525	57	489	94	640	82	1519	206	12	1819	99	496	254	849	4833
% App. Total	17.2	74.5	5.3	3		8.9	76.4	14.7		4.5	83.5	11.3	0.7		11.7	58.4	29.9		
PHF	.949	.953	.810	.719	.960	.750	.963	.734	.958	.683	.989	.858	.750	.993	.825	.912	.948	.956	.982



INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Thu, May 16, 19

LOCATION: Santa Ana
NORTH & SOUTH: Bristol
EAST & WEST: Warner

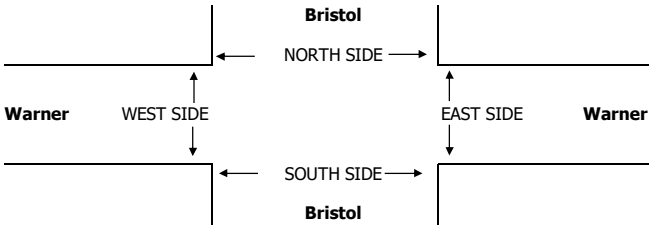
PROJECT #: SC2183
LOCATION #: 15
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND (Bristol)			SOUTHBOUND (Bristol)			EASTBOUND (Warner)			WESTBOUND (Warner)				U-TURNS				
	LANES:	NL 2	NT 2	NR 1	SL 2	ST 3	SR 0	EL 1	ET 3	ER 1	WL 1	WT 3	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	40	125	29	104	291	34	30	297	55	36	143	25	1,209	0	2	0	0	2
	7:15 AM	43	163	24	110	327	37	48	304	71	35	199	25	1,386	2	0	0	1	3
	7:30 AM	49	182	42	79	336	33	52	292	64	54	230	35	1,448	1	1	0	1	3
	7:45 AM	53	250	34	90	316	36	50	266	72	49	243	37	1,496	0	1	0	1	2
	8:00 AM	45	258	57	103	315	40	53	275	76	48	179	47	1,496	0	1	0	0	1
	8:15 AM	36	272	81	84	324	40	53	217	72	35	126	41	1,381	0	0	0	0	0
	8:30 AM	26	205	62	74	322	36	48	221	65	49	127	29	1,264	1	0	0	2	3
	8:45 AM	36	127	35	72	310	35	33	197	72	52	110	30	1,109	0	2	0	0	2
	VOLUMES	328	1,582	364	716	2,541	291	367	2,069	547	358	1,357	269	10,789	4	7	0	5	16
	APPROACH %	14%	70%	16%	20%	72%	8%	12%	69%	18%	18%	68%	14%						
	APP/DEPART	2,274	/	2,225	3,548	/	3,445	2,983	/	3,147	1,984	/	1,972	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	190	853	157	382	1,294	146	203	1,137	283	186	851	144	5,826					
	APPROACH %	16%	71%	13%	21%	71%	8%	13%	70%	17%	16%	72%	12%						
	PEAK HR FACTOR		0.833			0.961			0.959			0.897		0.974					
	APP/DEPART	1,200	/	1,203	1,822	/	1,763	1,623	/	1,676	1,181	/	1,184	0					
PM	4:00 PM	68	304	50	84	211	30	53	206	36	32	219	93	1,386	0	0	1	0	1
	4:15 PM	80	289	54	72	192	43	43	227	36	35	247	79	1,397	1	0	1	1	3
	4:30 PM	73	295	54	44	205	35	44	208	44	45	243	77	1,367	2	0	0	2	4
	4:45 PM	80	284	44	77	195	36	51	193	36	38	255	72	1,361	2	0	0	0	2
	5:00 PM	72	277	56	55	237	47	53	231	35	53	265	68	1,449	2	0	0	1	3
	5:15 PM	59	254	50	81	202	42	51	210	46	36	257	69	1,357	0	2	1	0	3
	5:30 PM	70	291	63	66	176	36	54	207	49	40	260	72	1,384	1	0	2	0	3
	5:45 PM	72	282	50	70	192	47	54	207	30	42	260	67	1,373	0	1	0	0	1
	VOLUMES	574	2,276	421	549	1,610	316	403	1,689	312	321	2,006	597	11,074	8	3	5	4	20
	APPROACH %	18%	70%	13%	22%	65%	13%	17%	70%	13%	11%	69%	20%						
	APP/DEPART	3,271	/	3,274	2,475	/	2,247	2,404	/	2,660	2,924	/	2,893	0					
	BEGIN PEAK HR		4:15 PM																
	VOLUMES	305	1,145	208	248	829	161	191	859	151	171	1,010	296	5,574					
	APPROACH %	18%	69%	13%	20%	67%	13%	16%	72%	13%	12%	68%	20%						
	PEAK HR FACTOR		0.980			0.913			0.941			0.957		0.962					
	APP/DEPART	1,658	/	1,631	1,238	/	1,154	1,201	/	1,319	1,477	/	1,470	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:15 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	4:15 PM					0	0	0	0	0					

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

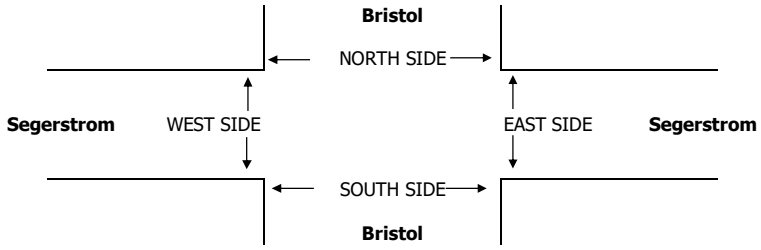
DATE:	LOCATION:	Santa Ana	PROJECT #:	SC0846
Tue, Feb 9, 16	NORTH & SOUTH:	Bristol	LOCATION #:	46
	EAST & WEST:	Segerstrom	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Bristol			SOUTHBOUND Bristol			EASTBOUND Segerstrom			WESTBOUND Segerstrom				U-TURNS				
	LANES:	NL 1	NT 3	NR 0	SL 1	ST 3	SR 0	EL 1	ET 2	ER 0	WL 1	WT 2	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	6	92	12	45	252	33	14	166	16	19	100	11	766	0	0	0	0	0
	7:15 AM	12	143	29	62	244	39	41	232	10	23	122	11	968	0	0	0	0	0
	7:30 AM	17	188	30	80	249	25	26	232	24	25	171	18	1,085	0	0	0	0	0
	7:45 AM	22	176	32	76	234	21	64	220	20	44	151	24	1,084	0	0	0	0	0
	8:00 AM	11	189	39	53	244	29	50	193	26	25	88	15	962	0	0	0	0	0
	8:15 AM	12	179	23	38	250	25	49	166	21	26	62	12	863	1	0	0	0	1
	8:30 AM	4	236	21	27	259	36	56	164	20	18	51	12	904	0	0	0	0	0
	8:45 AM	9	223	13	33	265	36	55	143	18	16	77	22	910	0	0	0	0	0
	VOLUMES	93	1,426	199	414	1,997	244	355	1,516	155	196	822	125	7,542	1	0	0	0	1
	APPROACH %	5%	83%	12%	16%	75%	9%	18%	75%	8%	17%	72%	11%						
	APP/DEPART	1,718	/	1,906	2,655	/	2,349	2,026	/	2,129	1,143	/	1,158	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	62	696	130	271	971	114	181	877	80	117	532	68	4,099					
	APPROACH %	7%	78%	15%	20%	72%	8%	16%	77%	7%	16%	74%	9%						
	PEAK HR FACTOR		0.929			0.958			0.936			0.818		0.944					
	APP/DEPART	888	/	945	1,356	/	1,168	1,138	/	1,278	717	/	708	0					
PM	4:00 PM	33	218	38	20	165	31	43	135	11	32	248	24	998	0	0	0	0	0
	4:15 PM	17	229	32	20	179	43	48	141	15	30	259	16	1,029	0	1	0	0	1
	4:30 PM	21	269	29	21	188	27	53	152	11	26	249	13	1,059	0	0	0	0	0
	4:45 PM	30	328	43	25	183	35	45	139	6	37	243	20	1,134	0	0	0	0	0
	5:00 PM	38	320	50	26	195	39	47	154	11	34	272	10	1,196	1	0	0	0	1
	5:15 PM	38	319	69	26	185	40	47	191	13	29	258	15	1,230	0	0	0	0	0
	5:30 PM	46	321	49	16	200	48	46	189	13	19	250	18	1,215	0	0	0	0	0
	5:45 PM	50	310	55	20	230	42	52	154	17	32	252	8	1,222	1	0	0	0	1
	VOLUMES	273	2,314	365	174	1,525	305	381	1,255	97	239	2,031	124	9,083	2	1	0	0	3
	APPROACH %	9%	78%	12%	9%	76%	15%	22%	72%	6%	10%	85%	5%						
	APP/DEPART	2,952	/	2,820	2,004	/	1,863	1,733	/	1,793	2,394	/	2,607	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	172	1,270	223	88	810	169	192	688	54	114	1,032	51	4,863					
	APPROACH %	10%	76%	13%	8%	76%	16%	21%	74%	6%	10%	86%	4%						
	PEAK HR FACTOR		0.977			0.914			0.930			0.947		0.988					
	APP/DEPART	1,665	/	1,513	1,067	/	980	934	/	999	1,197	/	1,371	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	8	6	4	3	21	1	3	4	1	9	7	3	0	2	12
	7:15 AM	3	16	0	3	22	2	7	0	1	10	1	9	0	2	12
	7:30 AM	11	7	8	6	32	1	2	7	5	15	10	5	1	1	17
	7:45 AM	4	7	2	4	17	0	0	2	3	5	4	7	0	1	12
	8:00 AM	1	2	5	3	11	0	0	5	2	7	1	2	0	1	4
	8:15 AM	2	0	3	2	7	1	0	3	1	5	1	0	0	1	2
	8:30 AM	4	3	1	2	10	2	0	0	1	3	2	3	1	1	7
	8:45 AM	2	2	5	3	12	2	1	4	3	10	0	1	1	0	2
	TOTAL	35	43	28	26	132	9	13	25	17	64	26	30	3	9	68
PM	4:00 PM	16	3	10	8	37	3	1	6	4	14	13	2	4	4	23
	4:15 PM	10	6	3	5	24	0	1	2	1	4	10	5	1	4	20
	4:30 PM	16	5	8	7	36	1	3	4	3	11	15	2	4	4	25
	4:45 PM	6	5	6	6	23	1	0	4	3	8	5	5	2	3	15
	5:00 PM	9	2	6	6	23	0	2	5	4	11	9	0	1	2	12
	5:15 PM	13	1	4	7	25	1	1	3	6	11	12	0	1	1	14
	5:30 PM	8	3	17	2	30	3	1	16	1	21	5	2	1	1	9
	5:45 PM	6	0	3	4	13	1	0	1	3	5	5	0	2	1	8
	TOTAL	84	25	57	45	211	10	9	41	25	85	74	16	16	20	126

Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: BRISTOL STREET
E-W Direction: ALTON AVENUE

File Name : H1702006
Site Code : 00000000
Start Date : 2/15/2017
Page No : 1

Groups Printed- Turning Movements

	BRISTOL STREET Southbound				ALTON AVENUE Westbound			BRISTOL STREET Northbound				ALTON AVENUE Eastbound				
Start Time	Right	Thru	Left	U-Turn	Right	Thru	Left	Right	Thru	Left	U-Turn	Right	Thru	Left	Peds	Int. Total
07:00	30	305	9	0	7	6	2	2	122	7	1	31	9	28	0	559
07:15	47	369	4	0	7	10	11	3	137	15	0	34	19	36	0	692
07:30	82	373	8	0	14	31	6	1	166	12	0	55	42	50	0	840
07:45	64	388	11	0	9	33	6	1	148	10	2	52	28	74	0	826
Total	223	1435	32	0	37	80	25	7	573	44	3	172	98	188	0	2917
08:00	27	394	11	0	9	10	7	1	158	8	0	43	24	76	0	768
08:15	21	343	7	0	10	11	4	2	143	6	0	35	15	75	0	672
08:30	19	312	6	0	11	7	3	0	142	6	2	20	16	36	0	580
08:45	18	306	13	1	4	12	4	3	126	7	0	13	12	18	0	537
Total	85	1355	37	1	34	40	18	6	569	27	2	111	67	205	0	2557
*** BREAK ***																
16:00	21	204	10	0	28	17	7	8	448	37	1	12	19	41	0	853
16:15	24	200	11	2	15	23	8	3	381	33	4	17	17	39	0	777
16:30	24	214	12	0	27	20	10	5	406	30	7	14	26	43	0	838
16:45	28	219	16	1	21	34	7	8	417	36	4	12	29	48	0	880
Total	97	837	49	3	91	94	32	24	1652	136	16	55	91	171	0	3348
17:00	31	204	9	1	26	26	7	5	383	35	3	13	26	45	0	814
17:15	27	212	17	1	25	51	7	5	405	48	6	23	22	46	0	895
17:30	25	234	12	1	16	34	7	7	426	34	3	24	24	50	0	897
17:45	25	220	19	0	29	34	7	9	381	37	3	22	23	35	0	844
Total	108	870	57	3	96	145	28	26	1595	154	15	82	95	176	0	3450
Grand Total	513	4497	175	7	258	359	103	63	4389	361	36	420	351	740	0	12272
Apprch %	9.9	86.6	3.4	0.1	35.8	49.9	14.3	1.3	90.5	7.4	0.7	27.8	23.2	49	0	
Total %	4.2	36.6	1.4	0.1	2.1	2.9	0.8	0.5	35.8	2.9	0.3	3.4	2.9	6	0	

Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: BRISTOL STREET
E-W Direction: MACARTHUR BOULEVARD

File Name : H1702007
Site Code : 00000000
Start Date : 2/7/2017
Page No : 1

Groups Printed- Turning Movements

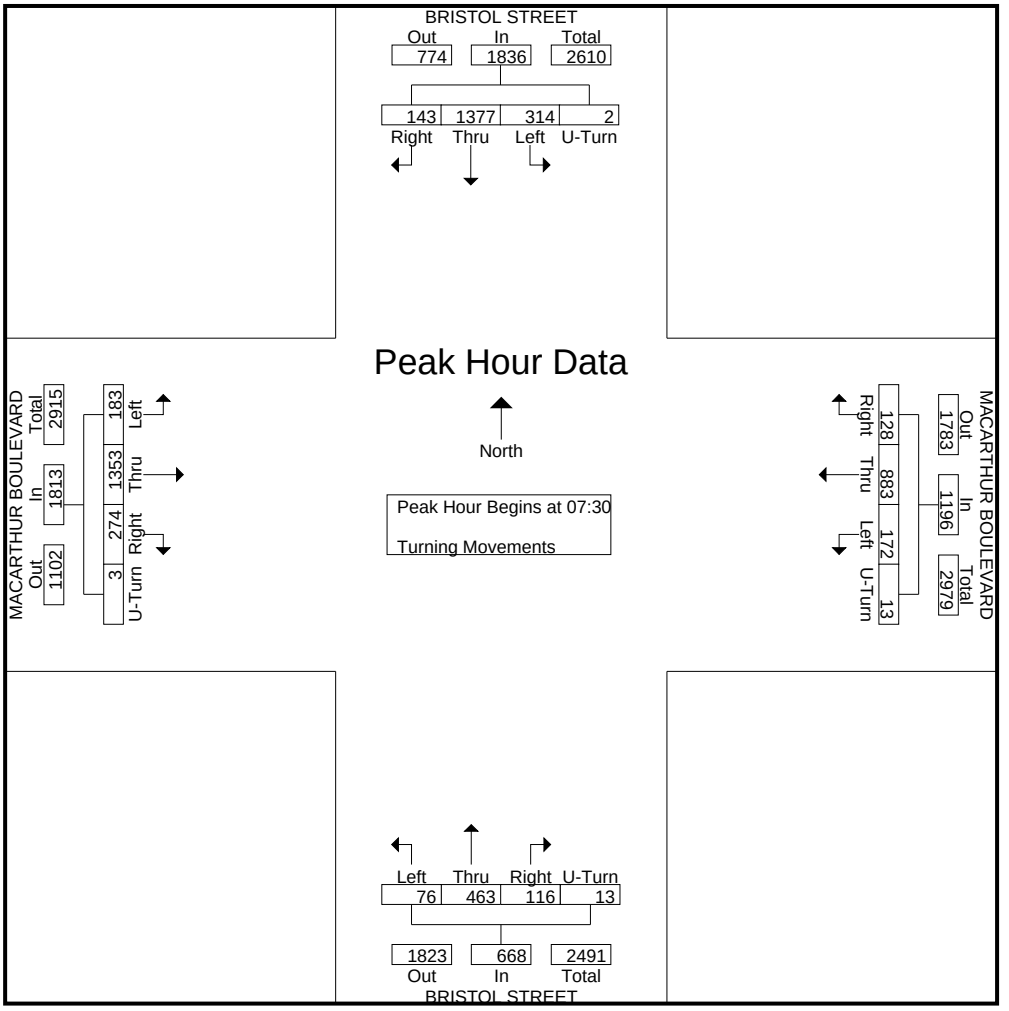
	BRISTOL STREET Southbound				MACARTHUR BOULEVARD Westbound				BRISTOL STREET Northbound				MACARTHUR BOULEVARD Eastbound				
Start Time	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Int. Total
07:00	20	276	34	0	26	139	44	0	21	78	15	4	23	172	23	0	875
07:15	37	323	48	1	30	241	41	4	16	86	10	2	19	291	37	1	1187
07:30	45	328	70	0	39	360	38	1	24	98	22	3	66	384	35	0	1513
07:45	34	336	83	1	33	292	49	2	28	121	30	5	69	324	51	1	1459
Total	136	1263	235	2	128	1032	172	7	89	383	77	14	177	1171	146	2	5034
08:00	32	379	86	1	31	122	39	4	26	119	11	2	64	351	50	1	1318
08:15	32	334	75	0	25	109	46	6	38	125	13	3	75	294	47	1	1223
08:30	20	274	44	0	32	92	51	8	34	107	11	1	70	332	62	0	1138
08:45	27	262	35	3	14	113	59	8	25	113	17	7	55	258	37	0	1033
Total	111	1249	240	4	102	436	195	26	123	464	52	13	264	1235	196	2	4712
*** BREAK ***																	
16:00	23	170	42	8	55	344	68	2	61	313	73	4	27	176	87	1	1454
16:15	30	174	39	6	59	370	83	0	56	350	62	11	31	166	92	1	1530
16:30	30	198	48	6	57	378	70	0	58	340	59	14	32	197	82	0	1569
16:45	27	183	57	5	74	355	61	0	72	346	67	13	46	195	87	0	1588
Total	110	725	186	25	245	1447	282	2	247	1349	261	42	136	734	348	2	6141
17:00	26	191	39	8	59	366	76	0	73	353	56	8	39	209	89	1	1593
17:15	31	210	45	11	74	385	63	0	58	350	67	13	45	190	80	2	1624
17:30	41	212	44	7	74	384	57	0	49	337	86	15	34	178	86	1	1605
17:45	34	213	57	7	92	339	61	0	51	295	91	16	45	183	89	1	1574
Total	132	826	185	33	299	1474	257	0	231	1335	300	52	163	760	344	5	6396
Grand Total	489	4063	846	64	774	4389	906	35	690	3531	690	121	740	3900	1034	11	22283
Apprch %	9	74.4	15.5	1.2	12.7	71.9	14.8	0.6	13.7	70.2	13.7	2.4	13	68.6	18.2	0.2	
Total %	2.2	18.2	3.8	0.3	3.5	19.7	4.1	0.2	3.1	15.8	3.1	0.5	3.3	17.5	4.6	0	

Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: BRISTOL STREET
E-W Direction: MACARTHUR BOULEVARD

File Name : H1702007
Site Code : 00000000
Start Date : 2/7/2017
Page No : 2

	BRISTOL STREET Southbound					MACARTHUR BOULEVARD Westbound					BRISTOL STREET Northbound					MACARTHUR BOULEVARD Eastbound					
Start Time	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 07:30																					
07:30	45	328	70	0	443	39	360	38	1	438	24	98	22	3	147	66	**384**	**35**	**0**	**485**	**1513**
07:45	**34**	**336**	**83**	**1**	**454**	**33**	**292**	**49**	2	376	28	121	**30**	**5**	**184**	69	324	**51**	**1**	445	1459
08:00	32	**379**	**86**		**498**	31	122	39	4	196	26	119	11	2	158	64	351	50	1	466	1318
08:15	32	334	75	0	441	25	109	46	**6**	186	**38**	**125**	13	3	179	**75**	294	47	1	417	1223
Total Volume	143	1377	314	2	1836	128	883	172	13	1196	116	463	76	13	668	274	1353	183	3	1813	5513
% App. Total																					
PHF	.794	.908	.913	.500	.922	.821	.613	.878	.542	.683	.763	.926	.633	.650	.908	.913	.881	.897	.750	.935	.911

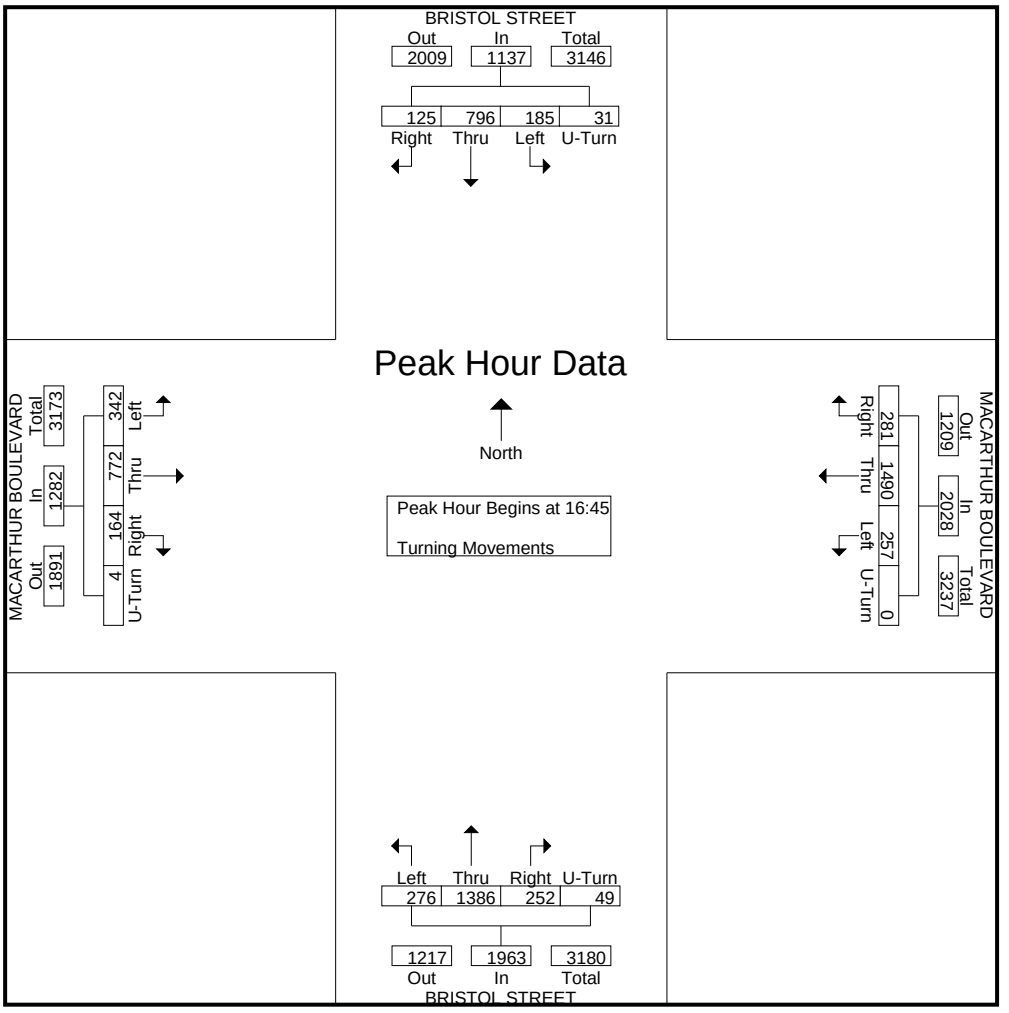


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City: SANTA ANA
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Start Date : 2/7/2017
Page No : 3

	BRISTOL STREET Southbound					MACARTHUR BOULEVARD Westbound					BRISTOL STREET Northbound					MACARTHUR BOULEVARD Eastbound					
Start Time	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1																					
Peak Hour for Entire Intersection Begins at 16:45																					
16:45	27	183	57	5	272	74									498	46					
17:00	26	191	39	8	264	59	366	**76**	0	501	**73**	**353**	56	8	490	39	**209**	**89**	1	**338**	
17:15	**31**	**210**	**45**	**11**	**297**	**74**	**385**	**63**	**0**	**522**	**58**	**350**	**67**	**13**	**488**	**45**	**190**	**80**	**2**	317	**1624**
17:30	**41**	**212**	44	7	**304**	74	384	57	0	515	49	337	**86**	**15**	**487**	**34**	**178**	**86**	**1**	**299**	**1605**
Total Volume	125	796	185	31	1137	281	1490	257	0	2028	252	1386	276	49	1963	164	772	342	4	1282	6410
% App. Total	11	70	16.3	2.7		13.9	73.5	12.7	0		12.8	70.6	14.1	2.5		12.8	60.2	26.7	0.3		
PHF	.762	.939	.811	.705	.935	.949	.968	.845	.000	.971	.863	.982	.802	.817	.985	.891	.923	.961	.500	.948	.987



INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

DATE: Tue, Mar 1, 16

LOCATION: Santa Ana
NORTH & SOUTH: Bristol
EAST & WEST: Sunflower

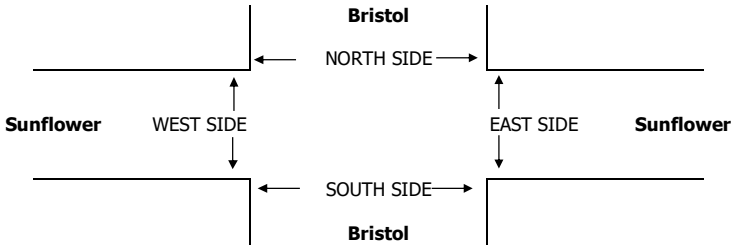
PROJECT #: SC0846
LOCATION #: 14
CONTROL: SIGNAL

NOTES:

AM PM MD OTHER OTHER — ▲ N ◄ W E ► S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Bristol			SOUTHBOUND Bristol			EASTBOUND Sunflower			WESTBOUND Sunflower				U-TURNS				
	LANES:	NL 2	NT 2.5	NR 1.5	SL 2	ST 3	SR 1	EL 2	ET 2.5	ER 1.5	WL 2	WT 3	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	20	112	12	30	301	14	15	94	61	59	55	32	805	0	2	0	4	6
	7:15 AM	16	121	19	41	356	9	16	115	79	54	68	25	919	0	0	0	2	2
	7:30 AM	20	97	26	43	351	18	34	205	85	74	108	34	1,095	0	2	0	3	5
	7:45 AM	32	104	35	84	339	29	15	265	111	49	140	34	1,237	2	0	0	0	2
	8:00 AM	23	103	26	47	284	29	27	259	101	55	76	35	1,065	2	0	0	4	6
	8:15 AM	15	139	43	61	359	23	33	176	69	58	60	29	1,065	3	1	0	5	9
	8:30 AM	17	117	28	47	254	27	23	134	69	69	51	38	874	1	1	0	7	9
	8:45 AM	20	114	28	56	285	26	22	98	63	49	82	33	876	0	1	0	7	8
	VOLUMES	163	907	217	409	2,529	175	185	1,346	638	467	640	260	7,936	8	7	0	32	47
	APPROACH %	13%	70%	17%	13%	81%	6%	9%	62%	29%	34%	47%	19%						
	APP/DEPART	1,287	/	1,359	3,113	/	3,610	2,169	/	1,997	1,367	/	970	0					
	BEGIN PEAK HR		7:30 AM																
	VOLUMES	90	443	130	235	1,333	99	109	905	366	236	384	132	4,462					
	APPROACH %	14%	67%	20%	14%	80%	6%	8%	66%	27%	31%	51%	18%						
	PEAK HR FACTOR		0.841			0.922			0.882			0.843		0.902					
	APP/DEPART	663	/	687	1,667	/	1,930	1,380	/	1,279	752	/	566	0					
PM	4:00 PM	139	353	49	33	143	36	63	104	49	42	162	65	1,238	3	1	4	3	11
	4:15 PM	152	356	47	28	158	46	81	125	47	52	168	63	1,323	5	2	6	4	17
	4:30 PM	148	341	54	44	185	41	64	102	39	56	218	83	1,375	6	5	3	7	21
	4:45 PM	150	347	44	43	152	38	67	111	53	65	253	70	1,393	4	2	7	8	21
	5:00 PM	138	339	67	48	167	44	68	122	47	52	228	80	1,400	6	2	3	5	16
	5:15 PM	165	318	46	40	173	50	68	131	53	62	246	87	1,439	4	4	3	6	17
	5:30 PM	155	331	48	57	188	54	73	149	54	53	259	64	1,485	0	4	5	6	15
	5:45 PM	161	321	50	48	192	58	72	148	59	85	239	78	1,511	2	2	4	6	14
	VOLUMES	1,208	2,706	405	341	1,358	367	556	992	401	467	1,773	590	11,164	30	22	35	45	132
	APPROACH %	28%	63%	9%	17%	66%	18%	29%	51%	21%	17%	63%	21%						
	APP/DEPART	4,319	/	3,839	2,066	/	2,211	1,949	/	1,761	2,830	/	3,353	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	619	1,309	211	193	720	206	281	550	213	252	972	309	5,835					
	APPROACH %	29%	61%	10%	17%	64%	18%	27%	53%	20%	16%	63%	20%						
	PEAK HR FACTOR		0.983			0.936			0.935			0.953		0.965					
	APP/DEPART	2,139	/	1,896	1,119	/	1,174	1,044	/	965	1,533	/	1,800	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	13	5	4	4	26	13	2	4	4	23	0	3	0	0	3
	7:15 AM	3	1	5	0	9	3	0	3	0	6	0	1	2	0	3
	7:30 AM	3	3	1	5	12	3	3	1	4	11	0	0	0	1	1
	7:45 AM	3	4	0	2	9	3	2	0	2	7	0	2	0	0	2
	8:00 AM	5	3	0	2	10	4	3	0	2	9	1	0	0	0	1
	8:15 AM	5	2	2	3	12	5	1	1	2	9	0	1	1	1	3
	8:30 AM	13	6	6	1	26	11	5	6	1	23	2	1	0	0	3
	8:45 AM	6	3	2	6	17	5	3	2	5	15	1	0	0	1	2
	TOTAL	51	27	20	23	121	47	19	17	20	103	4	8	3	3	18
PM	4:00 PM	13	4	7	8	32	12	4	6	8	30	1	0	1	0	2
	4:15 PM	9	5	5	1	20	7	5	5	1	18	2	0	0	0	2
	4:30 PM	21	13	5	11	50	17	11	5	9	42	4	2	0	2	8
	4:45 PM	18	9	3	11	41	15	8	3	11	37	3	1	0	0	4
	5:00 PM	11	4	13	10	38	11	3	9	10	33	0	1	4	0	5
	5:15 PM	8	4	7	6	25	8	4	6	6	24	0	0	1	0	1
	5:30 PM	13	5	5	6	29	12	4	4	5	25	1	1	1	1	4
	5:45 PM	16	5	7	5	33	16	4	6	5	31	0	1	1	0	2
	TOTAL	109	49	52	58	268	98	43	44	55	240	11	6	8	3	28

Prepared by National Data & Surveying Services

Bristol St & I-405 NB Ramps

Peak Hour Turning Movement Count

ID: Historical
City: Costa Mesa

Day: Thursday
Date: 10/11/2018

PEAK HOURS

AM: 07:45 AM - 08:45 AM
NOON: NONE
PM: 04:45 PM - 05:45 PM

COUNT PERIODS

AM: 07:00 AM - 09:00 AM
NOON: NONE
PM: 04:00 PM - 06:00 PM

Bristol St — SOUTHBOUND

AM	7	1928	0	0	2110	AM
NOON	0	0	0	0	0	NOON
PM	19	2105	0	0	3212	PM

I-405 NB Ramps — EASTBOUND

AM	NOON	PM	
78	0	318	
0	0	0	0
0	0	0	0
0	0	0	0
33	0	190	2
AM	NOON	PM	

I-405 NB Ramps — WESTBOUND

PM	NOON	AM	
1160	0	651	2
299	0	71	1.5
349	0	118	1.5
0	0	0	0
179	0	187	
PM	NOON	AM	

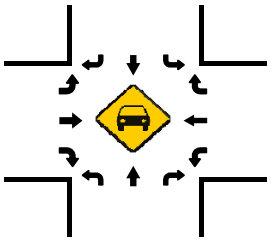
0	5	0	0	
0	0	4	1	

CONTROL			
Signalized			
TEV	4454	0	6353
	AM	NOON	PM
PHF	0.97		0.97

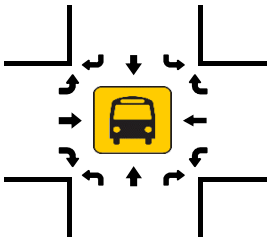
Bristol St — NORTHBOUND

PM	2644	0	0	2052	179	PM
NOON	0	0	0	0	0	NOON
AM	2079	0	0	1459	187	AM

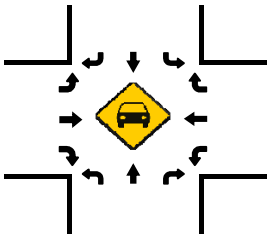
Total Vehicles (AM)



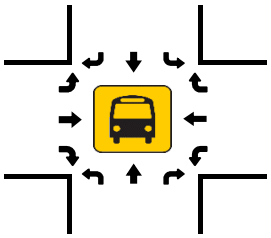
Total Vehicles (AM)



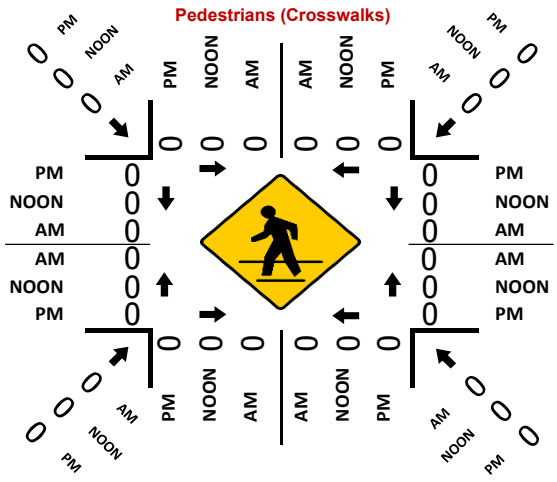
Total Vehicles (NOON)



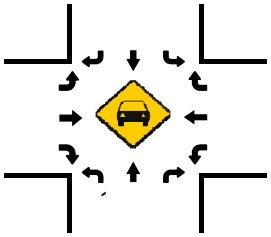
Total Vehicles (NOON)



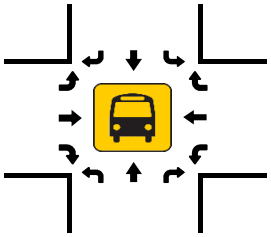
Pedestrians (Crosswalks)



Total Vehicles (PM)



Total Vehicles (PM)



Prepared by National Data & Surveying Services

Bristol St & I-405 SB Ramps

Peak Hour Turning Movement Count

ID: Historical
City: Costa Mesa

Day: Thursday
Date: 10/11/2018

PEAK HOURS

AM: 07:45 AM - 08:45 AM
NOON: NONE
PM: 05:00 PM - 06:00 PM

COUNT PERIODS

AM: 07:00 AM - 09:00 AM
NOON: NONE
PM: 04:00 PM - 06:00 PM

Bristol St — SOUTHBOUND

AM	830	965	0	0	1647	AM
NOON	0	0	0	0	0	NOON
PM	862	1476	0	3	2252	PM
	2	3	0	0		

I-405 SB Ramps — EASTBOUND

AM	NOON	PM	
940	0	986	
0	0	0	0
561	0	725	3
0	0	0	0
513	0	324	1
AM	NOON	PM	

I-405 SB Ramps — WESTBOUND

PM	NOON	AM	
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	0
0	0	0	
PM	NOON	AM	

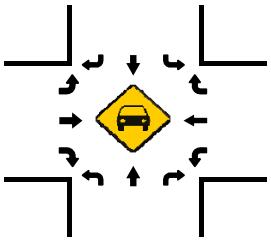
CONTROL: Signalized

TEV	4065	0	5038
	AM	NOON	PM
PHF	0.96		0.94

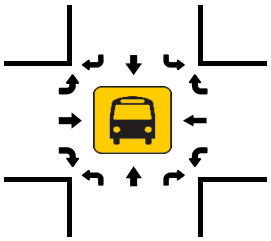
Bristol St — NORTHBOUND

PM	1800	0	124	1524	0	PM
NOON	0	0	0	0	0	NOON
AM	1478	0	110	1086	0	AM
		0	1	4	0	

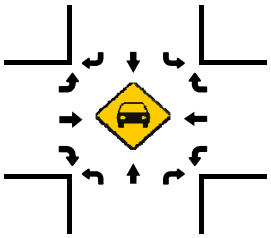
Total Vehicles (AM)



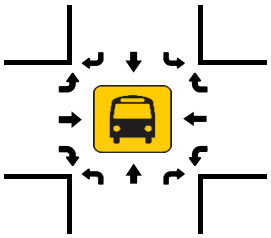
Total Vehicles (AM)



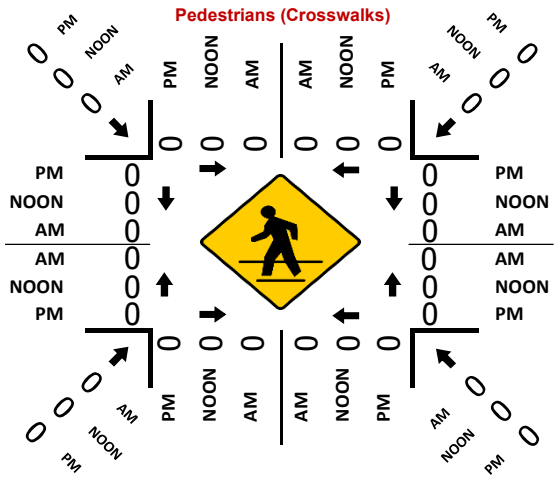
Total Vehicles (NOON)



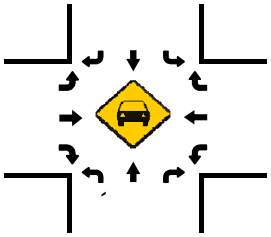
Total Vehicles (NOON)



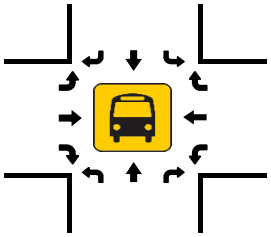
Pedestrians (Crosswalks)



Total Vehicles (PM)



Total Vehicles (PM)



INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

DATE: Thu, Feb 11, 16

LOCATION: Santa Ana
NORTH & SOUTH: Flower
EAST & WEST: Santa Ana

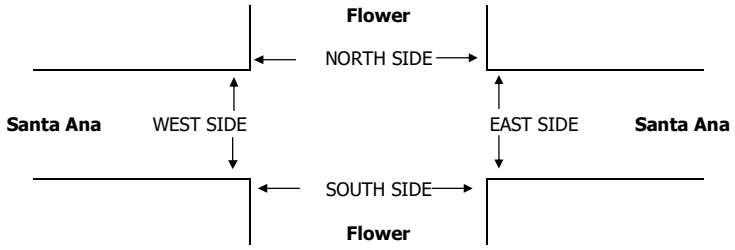
PROJECT #: SC0846
LOCATION #: 23
CONTROL: SIGNAL

NOTES:

AM PM MD OTHER OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Flower			SOUTHBOUND Flower			EASTBOUND Santa Ana			WESTBOUND Santa Ana				U-TURNS				
	LANES:	NL 1	NT 2	NR 1	SL 1	ST 2	SR 1	EL 1	ET 3	ER 1	WL 1	WT 2	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	11	120	17	30	104	24	11	85	18	16	63	26	525	0	0	0	0	0
	7:15 AM	14	199	19	36	131	20	24	146	24	22	78	25	738	0	0	0	0	0
	7:30 AM	17	234	36	38	156	17	26	202	20	35	92	33	906	0	0	0	0	0
	7:45 AM	26	194	35	50	156	25	33	193	23	18	73	22	848	0	0	0	0	0
	8:00 AM	26	152	23	37	102	11	26	141	10	22	67	49	666	1	0	0	0	1
	8:15 AM	10	95	28	47	85	20	20	104	14	10	51	35	519	0	0	0	0	0
	8:30 AM	13	128	19	26	86	9	6	50	11	14	42	25	429	1	0	0	0	1
	8:45 AM	20	105	13	24	63	4	15	52	8	18	36	26	384	2	0	0	0	2
	VOLUMES	137	1,227	190	288	883	130	161	973	128	155	502	241	5,015	4	0	0	0	4
	APPROACH %	9%	79%	12%	22%	68%	10%	13%	77%	10%	17%	56%	27%						
	APP/DEPART	1,554	/	1,629	1,301	/	1,170	1,262	/	1,451	898	/	765	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	83	779	113	161	545	73	109	682	77	97	310	129	3,158					
	APPROACH %	9%	80%	12%	21%	70%	9%	13%	79%	9%	18%	58%	24%						
	PEAK HR FACTOR		0.849			0.843			0.871			0.838		0.871					
	APP/DEPART	975	/	1,017	779	/	720	868	/	956	536	/	465	0					
PM	4:00 PM	19	161	17	28	111	14	19	84	11	26	82	20	592	0	0	0	0	0
	4:15 PM	25	148	23	23	94	7	13	105	17	37	71	30	593	0	0	0	0	0
	4:30 PM	18	152	13	30	123	11	42	123	13	34	98	33	690	0	0	0	0	0
	4:45 PM	31	171	18	39	133	10	26	82	20	25	97	25	677	0	0	0	0	0
	5:00 PM	26	152	16	24	125	9	47	103	15	43	149	56	765	1	0	0	0	1
	5:15 PM	33	210	12	19	120	14	22	90	13	46	125	29	733	0	0	0	0	0
	5:30 PM	29	191	13	17	131	13	33	95	12	26	119	35	714	1	0	0	0	1
	5:45 PM	24	206	14	9	90	7	15	69	11	30	98	28	601	0	0	0	0	0
	VOLUMES	205	1,391	126	189	927	85	217	751	112	267	839	256	5,365	2	0	0	0	2
	APPROACH %	12%	81%	7%	16%	77%	7%	20%	70%	10%	20%	62%	19%						
	APP/DEPART	1,722	/	1,864	1,201	/	1,308	1,080	/	1,066	1,362	/	1,127	0					
	BEGIN PEAK HR		4:45 PM																
	VOLUMES	119	724	59	99	509	46	128	370	60	140	490	145	2,889					
	APPROACH %	13%	80%	7%	15%	78%	7%	23%	66%	11%	18%	63%	19%						
	PEAK HR FACTOR		0.884			0.898			0.845			0.781		0.944					
	APP/DEPART	902	/	997	654	/	711	558	/	528	775	/	653	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	7	10	13	18	48	1	1	9	14	25	6	9	4	4	23
	7:15 AM	7	9	27	23	66	3	0	26	16	45	4	9	1	7	21
	7:30 AM	6	8	23	20	57	1	1	16	17	35	5	7	7	3	22
	7:45 AM	2	4	55	25	86	0	0	53	22	75	2	4	2	3	11
	8:00 AM	3	8	17	12	40	2	0	17	9	28	1	8	0	3	12
	8:15 AM	5	9	17	6	37	0	4	16	3	23	5	5	1	3	14
	8:30 AM	10	15	15	9	49	0	0	11	9	20	10	15	4	0	29
	8:45 AM	11	24	9	26	70	4	1	8	26	39	7	23	1	0	31
	TOTAL	51	87	176	139	453	11	7	156	116	290	40	80	20	23	163
PM	4:00 PM	2	7	7	22	38	0	2	5	17	24	2	5	2	5	14
	4:15 PM	8	14	9	26	57	4	0	8	24	36	4	14	1	2	21
	4:30 PM	10	5	8	10	33	1	1	6	9	17	9	4	2	1	16
	4:45 PM	2	3	10	13	28	0	1	8	9	18	2	2	2	4	10
	5:00 PM	1	9	6	12	28	0	1	5	12	18	1	8	1	0	10
	5:15 PM	5	5	6	4	20	1	3	3	3	10	4	2	3	1	10
	5:30 PM	1	3	5	8	17	1	2	4	8	15	0	1	1	0	2
	5:45 PM	4	3	5	10	22	1	0	2	9	12	3	3	3	1	10
	TOTAL	33	49	56	105	243	8	10	41	91	150	25	39	15	14	93

Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: FLOWER STREET
E-W Direction: 1ST STREET

File Name : H1702009
Site Code : 00000000
Start Date : 2/9/2017
Page No : 1

Groups Printed- Turning Movements

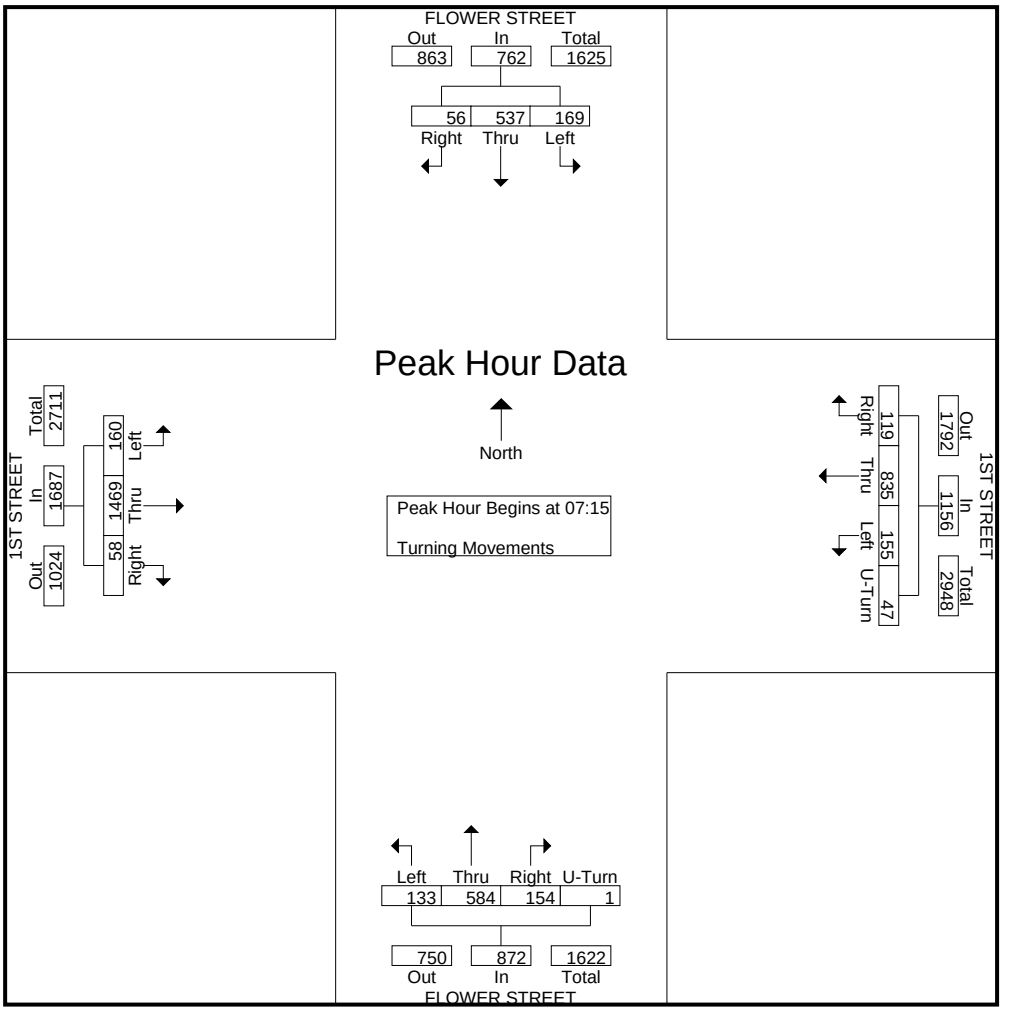
	FLOWER STREET Southbound			1ST STREET Westbound				FLOWER STREET Northbound				1ST STREET Eastbound			
Start Time	Right	Thru	Left	Right	Thru	Left	U-Turn	Right	Thru	Left	U-Turn	Right	Thru	Left	Int. Total
07:00	13	106	37	21	158	32	2	37	110	22	1	18	324	25	906
07:15	12	113	35	35	201	42	10	44	143	39	0	15	383	39	1111
07:30	16	132	55	21	205	39	18	32	155	32	1	18	364	35	1123
07:45	13	162	46	40	235	45	12	41	175	40	0	16	363	39	1227
Total	54	513	173	117	799	158	42	154	583	133	2	67	1434	138	4367
08:00	15	130	33	23	194	29	7	37	111	22	0	9	359	47	1016
08:15	17	58	32	23	168	39	6	32	78	21	1	15	329	37	856
08:30	15	61	35	20	145	28	0	28	69	16	2	9	319	33	780
08:45	24	60	39	16	139	17	1	29	61	18	0	17	293	25	739
Total	71	309	139	82	646	113	14	126	319	77	3	50	1300	142	3391
*** BREAK ***															
16:00	30	82	49	28	305	52	3	28	97	34	0	21	288	35	1052
16:15	34	94	46	29	358	41	2	28	142	35	2	16	271	44	1142
16:30	22	140	37	20	336	40	8	29	122	35	0	16	264	31	1100
16:45	26	114	51	31	360	53	3	33	143	25	0	25	291	34	1189
Total	112	430	183	108	1359	186	16	118	504	129	2	78	1114	144	4483
17:00	37	115	47	36	327	53	3	39	159	34	3	18	312	35	1218
17:15	34	110	40	34	364	45	2	35	150	35	0	25	293	29	1196
17:30	34	113	27	41	322	39	5	37	154	29	2	30	285	36	1154
17:45	25	118	36	47	317	43	8	27	141	34	1	23	303	32	1155
Total	130	456	150	158	1330	180	18	138	604	132	6	96	1193	132	4723
Grand Total	367	1708	645	465	4134	637	90	536	2010	471	13	291	5041	556	16964
Apprch %	13.5	62.8	23.7	8.7	77.6	12	1.7	17.7	66.3	15.5	0.4	4.9	85.6	9.4	
Total %	2.2	10.1	3.8	2.7	24.4	3.8	0.5	3.2	11.8	2.8	0.1	1.7	29.7	3.3	

Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: FLOWER STREET
E-W Direction: 1ST STREET

File Name : H1702009
Site Code : 00000000
Start Date : 2/9/2017
Page No : 2

	FLOWER STREET Southbound				1ST STREET Westbound					FLOWER STREET Northbound					1ST STREET Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																			
Peak Hour for Entire Intersection Begins at 07:15																			
07:15	12	113	35	160	35	201	42	10	288	44								383	437	1111
07:30	16	132	**55**	203	21	205	39	**18**					**1**	220	**18**	364	35	417	1123
07:45	13	**162**	46	**221**	**40**	**235**	**45**	12	**332**		**175**	**40**	0	**256**	16	363	39	418	**1227**
08:00	**15**	**130**	**33**	**178**	**23**	**194**	**29**	**7**	**253**	**37**	**111**	**22**	**0**	**170**	**9**	**359**	**47**	415	1016
Total Volume	56	537	169	762	119	835	155	47	1156	154	584	133	1	872	58	1469	160	1687	4477
% App. Total	7.3	70.5	22.2		10.3	72.2	13.4	4.1		17.7	67	15.3	0.1		3.4	87.1	9.5		
PHF	.875	.829	.768	.862	.744	.888	.861	.653	.870	.875	.834	.831	.250	.852	.806	.959	.851	.965	.912

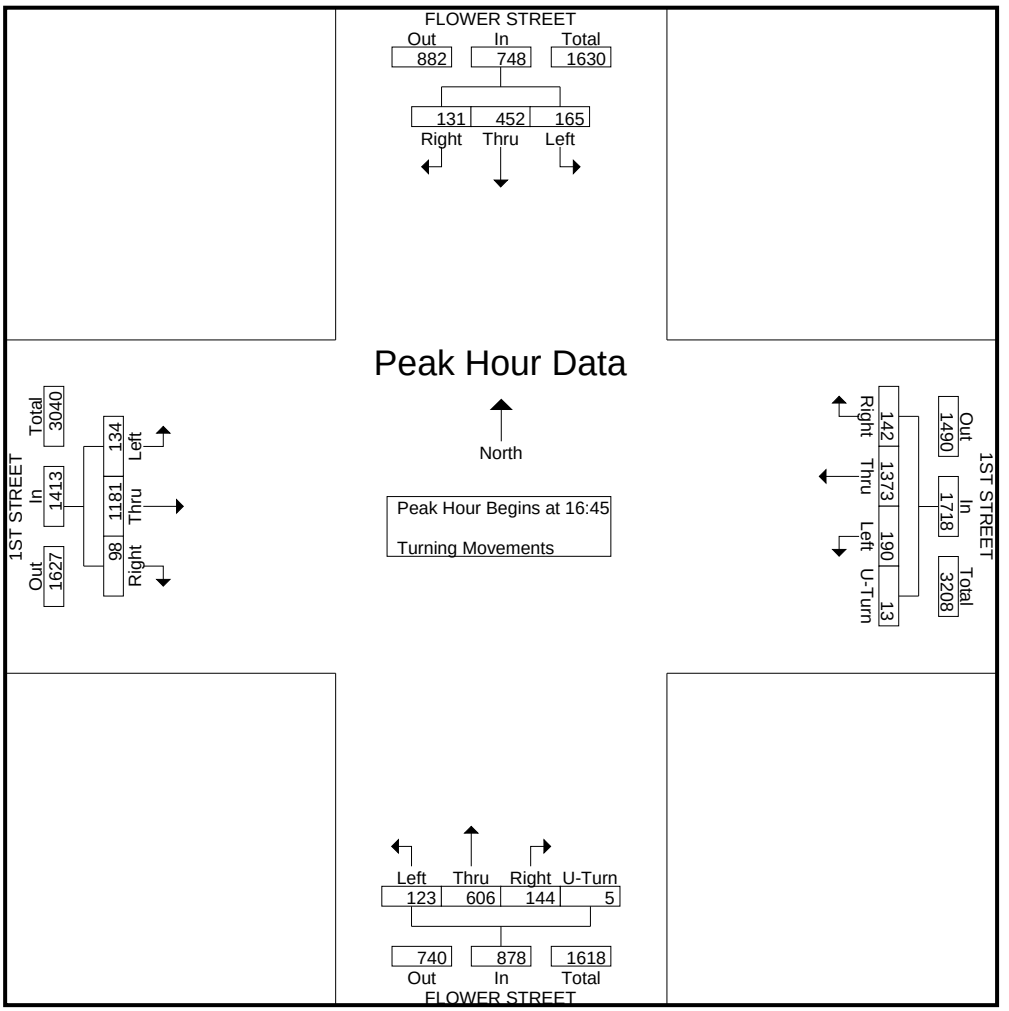


Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: FLOWER STREET
E-W Direction: 1ST STREET

File Name : H1702009
Site Code : 00000000
Start Date : 2/9/2017
Page No : 3

	FLOWER STREET Southbound				1ST STREET Westbound					FLOWER STREET Northbound					1ST STREET Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	U-Turn	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1																			
Peak Hour for Entire Intersection Begins at 16:45																			
16:45	26	114	51				53		447	33	143	25	0	201	25	291	34	350	1189
17:00	37	**115**	47	**199**						**39**	**159**	34	**3**	**235**	18	**312**	35	**365**	**1218**
17:15	**34**	**110**	**40**	**184**	**34**	**364**	**45**	**2**	**445**	**35**	**150**	**35**	0	220	25	293	29	347	1196
17:30	34	113	27	174	**41**	322	39	**5**							**30**	285	**36**	351	1154
Total Volume	131	452	165	748	142	1373	190	13	1718	144	606	123	5	878	98	1181	134	1413	4757
% App. Total	17.5	60.4	22.1		8.3	79.9	11.1	0.8		16.4	69	14	0.6		6.9	83.6	9.5		
PHF	.885	.983	.809	.940	.866	.943	.896	.650	.961	.923	.953	.879	.417	.934	.817	.946	.931	.968	.976



INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

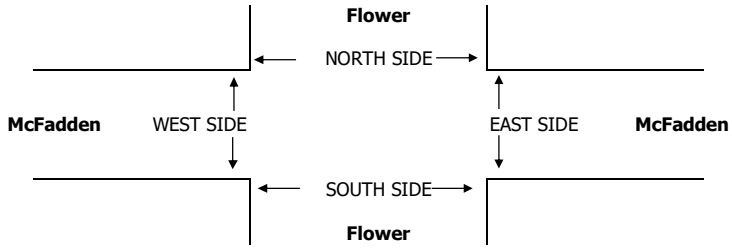
DATE:	LOCATION:	Santa Ana	PROJECT #:	SC0846
Thu, Feb 11, 16	NORTH & SOUTH:	Flower	LOCATION #:	36
	EAST & WEST:	McFadden	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Flower			SOUTHBOUND Flower			EASTBOUND McFadden			WESTBOUND McFadden				U-TURNS				
	LANES:	NL 1	NT 1	NR 0	SL 1	ST 1	SR 0	EL 1	ET 1	ER 0	WL 1	WT 1	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	8	75	5	14	75	8	12	86	10	8	79	5	385	0	0	0	0	0
	7:15 AM	8	102	8	10	89	10	13	134	12	12	92	15	505	0	0	0	0	0
	7:30 AM	13	105	10	14	85	13	13	139	19	16	120	15	562	0	0	0	0	0
	7:45 AM	15	111	8	18	90	15	11	145	5	22	112	13	565	0	0	0	0	0
	8:00 AM	21	102	10	21	88	12	12	140	6	11	91	12	526	0	0	0	0	0
	8:15 AM	9	82	8	15	66	7	13	89	6	13	66	4	378	0	0	0	0	0
	8:30 AM	5	64	10	10	79	8	10	81	4	9	59	8	347	0	0	0	0	0
	8:45 AM	7	63	6	9	54	10	9	90	4	11	76	3	342	0	0	0	0	0
	VOLUMES	86	704	65	111	626	83	93	904	66	102	695	75	3,610	0	0	0	0	0
	APPROACH %	10%	82%	8%	14%	76%	10%	9%	85%	6%	12%	80%	9%						
	APP/DEPART	855	/	872	820	/	794	1,063	/	1,080	872	/	864	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	57	420	36	63	352	50	49	558	42	61	415	55	2,158					
	APPROACH %	11%	82%	7%	14%	76%	11%	8%	86%	6%	11%	78%	10%						
	PEAK HR FACTOR		0.957			0.945			0.949			0.879		0.955					
	APP/DEPART	513	/	524	465	/	455	649	/	657	531	/	522	0					
PM	4:00 PM	11	106	6	6	65	9	9	121	9	9	126	18	495	0	0	0	0	0
	4:15 PM	10	137	5	8	93	13	8	106	4	12	136	15	547	0	0	0	0	0
	4:30 PM	12	119	5	14	64	18	12	100	2	15	126	13	500	0	0	0	0	0
	4:45 PM	8	135	7	8	75	17	10	110	4	9	148	7	538	0	0	0	0	0
	5:00 PM	10	135	9	14	60	9	15	109	4	6	131	17	519	0	0	0	0	0
	5:15 PM	9	143	10	10	95	16	15	100	3	13	145	21	580	0	0	0	0	0
	5:30 PM	10	139	7	10	105	10	15	127	5	10	142	21	601	0	0	0	0	0
	5:45 PM	11	127	7	10	78	19	16	123	6	17	153	22	589	0	0	0	0	0
	VOLUMES	81	1,041	56	80	635	111	100	896	37	91	1,107	134	4,369	0	0	0	0	0
	APPROACH %	7%	88%	5%	10%	77%	13%	10%	87%	4%	7%	83%	10%						
	APP/DEPART	1,178	/	1,275	826	/	763	1,033	/	1,032	1,332	/	1,299	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	40	544	33	44	338	54	61	459	18	46	571	81	2,289					
	APPROACH %	6%	88%	5%	10%	78%	12%	11%	85%	3%	7%	82%	12%						
	PEAK HR FACTOR		0.952			0.872			0.915			0.909		0.952					
	APP/DEPART	617	/	686	436	/	402	538	/	536	698	/	665	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	8	9	9	5	31	2	1	7	5	15	6	8	2	0	16
	7:15 AM	7	2	3	4	16	1	0	2	4	7	6	2	1	0	9
	7:30 AM	15	9	7	14	45	1	0	7	12	20	14	9	0	2	25
	7:45 AM	8	19	9	15	51	3	0	8	15	26	5	19	1	0	25
	8:00 AM	4	5	3	9	21	2	0	3	9	14	2	5	0	0	7
	8:15 AM	3	7	5	11	26	3	2	4	9	18	0	5	1	2	8
	8:30 AM	2	2	2	6	12	1	1	1	4	7	1	1	1	2	5
	8:45 AM	2	2	0	4	8	0	0	0	4	4	2	2	0	0	4
	TOTAL	49	55	38	68	210	13	4	32	62	111	36	51	6	6	99
PM	4:00 PM	10	7	13	10	40	3	0	11	9	23	7	7	2	1	17
	4:15 PM	7	8	7	5	27	0	1	7	5	13	7	7	0	0	14
	4:30 PM	4	3	3	3	13	3	0	2	2	7	1	3	1	1	6
	4:45 PM	3	7	6	6	22	2	0	6	6	14	1	7	0	0	8
	5:00 PM	8	3	0	8	19	2	0	0	6	8	6	3	0	2	11
	5:15 PM	4	6	7	7	24	1	1	6	6	14	3	5	1	1	10
	5:30 PM	11	8	8	7	34	1	0	8	7	16	10	8	0	0	18
	5:45 PM	6	11	4	4	25	1	4	4	2	11	5	7	0	2	14
	TOTAL	53	53	48	50	204	13	6	44	43	106	40	47	4	7	98

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

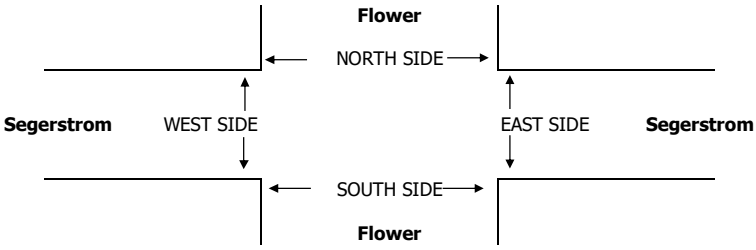
DATE:	LOCATION:	Santa Ana	PROJECT #:	SC0846
Tue, Feb 9, 16	NORTH & SOUTH:	Flower	LOCATION #:	47
	EAST & WEST:	Segerstrom	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Flower			SOUTHBOUND Flower			EASTBOUND Segerstrom			WESTBOUND Segerstrom				U-TURNS				
	LANES:	NL 1	NT 2	NR 0	SL 1	ST 2	SR 0	EL 1	ET 2	ER 0	WL 1	WT 2	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	16	50	12	28	108	31	33	200	68	18	96	10	670	0	0	0	0	0
	7:15 AM	16	84	14	25	149	16	24	252	65	18	114	11	788	0	0	0	0	0
	7:30 AM	14	96	27	21	155	27	37	272	91	20	181	17	958	0	0	0	0	0
	7:45 AM	14	88	17	20	110	41	54	299	61	15	133	19	871	0	0	0	0	0
	8:00 AM	10	52	9	32	95	18	29	287	44	9	77	5	667	0	0	0	0	0
	8:15 AM	11	42	10	26	99	9	10	204	16	8	82	6	523	0	0	0	0	0
	8:30 AM	14	51	9	40	82	5	4	190	20	7	63	2	487	0	0	0	0	0
	8:45 AM	10	32	11	24	80	5	5	179	21	2	83	9	461	0	0	0	0	0
	VOLUMES	105	495	109	216	878	152	196	1,883	386	97	829	79	5,425	0	0	0	0	0
	APPROACH %	15%	70%	15%	17%	70%	12%	8%	76%	16%	10%	82%	8%						
	APP/DEPART	709	/	770	1,246	/	1,361	2,465	/	2,208	1,005	/	1,086	0					
	BEGIN PEAK HR		7:00 AM																
	VOLUMES	60	318	70	94	522	115	148	1,023	285	71	524	57	3,287					
	APPROACH %	13%	71%	16%	13%	71%	16%	10%	70%	20%	11%	80%	9%						
	PEAK HR FACTOR		0.818			0.900			0.879			0.748		0.858					
	APP/DEPART	448	/	523	731	/	878	1,456	/	1,187	652	/	699	0					
PM	4:00 PM	35	162	18	21	71	15	38	138	19	16	281	32	846	0	0	0	0	0
	4:15 PM	26	157	17	13	69	18	28	156	13	13	277	23	810	0	0	0	0	0
	4:30 PM	43	185	23	13	83	10	29	152	29	20	287	18	892	0	0	0	0	0
	4:45 PM	26	194	22	20	72	21	31	153	19	20	296	30	904	0	0	0	0	0
	5:00 PM	29	201	24	15	81	28	52	170	13	20	274	19	926	0	0	0	0	0
	5:15 PM	31	219	20	17	80	22	44	211	23	16	317	19	1,019	0	0	0	0	0
	5:30 PM	30	208	16	15	97	22	33	199	23	19	315	17	994	0	0	0	0	0
	5:45 PM	25	182	16	16	96	29	22	175	20	17	302	22	922	0	0	0	0	0
	VOLUMES	245	1,508	156	130	649	165	277	1,354	159	141	2,349	180	7,313	0	0	0	0	0
	APPROACH %	13%	79%	8%	14%	69%	17%	15%	76%	9%	5%	88%	7%						
	APP/DEPART	1,909	/	1,965	944	/	949	1,790	/	1,640	2,670	/	2,759	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	115	810	76	63	354	101	151	755	79	72	1,208	77	3,861					
	APPROACH %	11%	81%	8%	12%	68%	19%	15%	77%	8%	5%	89%	6%						
	PEAK HR FACTOR		0.927			0.918			0.886			0.964		0.947					
	APP/DEPART	1,001	/	1,038	518	/	505	985	/	894	1,357	/	1,424	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	5	5	1	8	19	3	4	1	8	16	2	1	0	0	3
	7:15 AM	8	17	6	28	59	5	14	5	26	50	3	3	1	2	9
	7:30 AM	27	34	9	54	124	26	32	9	53	120	1	2	0	1	4
	7:45 AM	27	56	20	132	235	27	55	19	132	233	0	1	1	0	2
	8:00 AM	1	2	0	11	14	1	2	0	11	14	0	0	0	0	0
	8:15 AM	2	2	2	8	14	2	2	0	6	10	0	0	2	2	4
	8:30 AM	1	1	2	2	6	0	1	1	1	3	1	0	1	1	3
	8:45 AM	1	1	0	1	3	0	0	0	1	1	1	1	0	0	2
	TOTAL	72	118	40	244	474	64	110	35	238	447	8	8	5	6	27
PM	4:00 PM	1	11	3	11	26	0	11	3	11	25	1	0	0	0	1
	4:15 PM	2	9	4	2	17	1	6	2	2	11	1	3	2	0	6
	4:30 PM	3	7	2	8	20	1	6	2	6	15	2	1	0	2	5
	4:45 PM	5	7	1	12	25	4	5	1	11	21	1	2	0	1	4
	5:00 PM	3	5	4	14	26	1	4	1	10	16	2	1	3	4	10
	5:15 PM	1	5	8	5	19	0	2	7	5	14	1	3	1	0	5
	5:30 PM	4	6	1	4	15	2	6	1	4	13	2	0	0	0	2
	5:45 PM	6	7	0	4	17	3	7	0	2	12	3	0	0	2	5
	TOTAL	25	57	23	60	165	12	47	17	51	127	13	10	6	9	38

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

DATE: Tue, Feb 9, 16

LOCATION: Santa Ana
NORTH & SOUTH: Flower
EAST & WEST: MacArthur

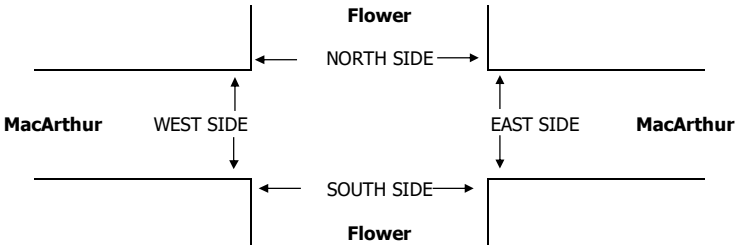
PROJECT #: SC0846
LOCATION #: 53
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Flower			SOUTHBOUND Flower			EASTBOUND MacArthur			WESTBOUND MacArthur				U-TURNS				
	LANES:	NL 1	NT 2	NR 0	SL 1	ST 2	SR 0	EL 1	ET 3	ER 0	WL 1	WT 3	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	9	26	20	36	60	51	28	315	14	6	142	9	716	0	0	0	0	0
	7:15 AM	5	32	17	40	70	54	54	424	15	11	218	17	957	0	0	0	1	1
	7:30 AM	6	44	25	35	88	100	45	478	16	16	297	16	1,166	0	0	1	0	1
	7:45 AM	4	35	13	45	104	67	30	460	27	12	197	15	1,009	0	0	1	1	2
	8:00 AM	6	25	15	36	76	32	29	443	18	12	161	7	860	0	0	1	0	1
	8:15 AM	9	32	15	47	82	21	24	438	33	10	171	13	895	0	0	0	1	1
	8:30 AM	10	27	16	36	47	20	23	368	21	17	148	17	750	0	0	0	0	0
	8:45 AM	10	21	12	30	65	24	13	329	16	8	128	11	667	0	0	0	0	0
	VOLUMES	59	242	133	305	592	369	246	3,255	160	92	1,462	105	7,020	0	0	3	3	6
	APPROACH %	14%	56%	31%	24%	47%	29%	7%	89%	4%	6%	88%	6%						
	APP/DEPART	434	/	590	1,266	/	841	3,661	/	3,696	1,659	/	1,893	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	21	136	70	156	338	253	158	1,805	76	51	873	55	3,992					
	APPROACH %	9%	60%	31%	21%	45%	34%	8%	89%	4%	5%	89%	6%						
	PEAK HR FACTOR		0.757			0.837			0.946			0.744		0.856					
	APP/DEPART	227	/	346	747	/	463	2,039	/	2,033	979	/	1,150	0					
PM	4:00 PM	17	121	16	25	44	36	46	240	14	10	454	35	1,058	0	0	0	0	0
	4:15 PM	22	134	7	18	39	31	45	217	10	15	476	38	1,052	0	0	1	1	2
	4:30 PM	23	124	15	15	51	49	45	249	11	15	477	33	1,107	0	0	0	3	3
	4:45 PM	22	144	17	15	39	39	56	244	10	8	451	38	1,083	0	0	1	0	1
	5:00 PM	35	167	13	19	44	45	49	242	15	12	504	44	1,189	0	0	0	0	0
	5:15 PM	33	190	17	17	52	41	49	221	21	16	464	45	1,166	0	0	1	0	1
	5:30 PM	38	140	28	24	49	54	47	245	10	15	463	35	1,148	0	0	0	0	0
	5:45 PM	21	112	13	21	52	49	55	247	14	16	480	46	1,126	0	0	0	0	0
	VOLUMES	211	1,132	126	154	370	344	392	1,905	105	107	3,769	314	8,929	0	0	3	4	7
	APPROACH %	14%	77%	9%	18%	43%	40%	16%	79%	4%	3%	90%	7%						
	APP/DEPART	1,469	/	1,835	868	/	578	2,402	/	2,189	4,190	/	4,327	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	127	609	71	81	197	189	200	955	60	59	1,911	170	4,629					
	APPROACH %	16%	75%	9%	17%	42%	40%	16%	79%	5%	3%	89%	8%						
	PEAK HR FACTOR		0.841			0.919			0.961			0.955		0.973					
	APP/DEPART	807	/	978	467	/	316	1,215	/	1,107	2,140	/	2,228	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	1	3	5	4	13	1	3	4	3	11	0	0	1	1	2
	7:15 AM	2	6	3	14	25	2	6	3	14	25	0	0	0	0	0
	7:30 AM	4	6	8	3	21	3	6	8	3	20	1	0	0	0	1
	7:45 AM	2	12	5	6	25	2	12	5	6	25	0	0	0	0	0
	8:00 AM	1	8	3	1	13	1	8	3	1	13	0	0	0	0	0
	8:15 AM	0	20	3	7	30	0	20	2	5	27	0	0	1	2	3
	8:30 AM	1	11	7	3	22	0	10	5	2	17	1	1	2	1	5
	8:45 AM	1	2	3	1	7	1	2	2	1	6	0	0	1	0	1
	TOTAL	12	68	37	39	156	10	67	32	35	144	2	1	5	4	12
PM	4:00 PM	3	10	2	4	19	2	9	1	4	16	1	1	1	0	3
	4:15 PM	5	3	1	2	11	5	3	1	1	10	0	0	0	1	1
	4:30 PM	4	4	0	0	8	1	3	0	0	4	3	1	0	0	4
	4:45 PM	2	2	2	2	8	1	1	0	2	4	1	1	2	0	4
	5:00 PM	1	4	5	7	17	1	4	2	7	14	0	0	3	0	3
	5:15 PM	4	2	5	0	11	3	2	5	0	10	1	0	0	0	1
	5:30 PM	0	2	3	3	8	0	1	3	1	5	0	1	0	2	3
	5:45 PM	5	4	10	2	21	3	4	9	2	18	2	0	1	0	3
	TOTAL	24	31	28	20	103	16	27	21	17	81	8	4	7	3	22

Prepared by National Data & Surveying Services

Main St & La Veta Ave

Peak Hour Turning Movement Count

ID: Historical
City: Orange

Day: Tuesday
Date: 08/28/2018

PEAK HOURS

AM	07:30 AM - 08:30 AM
NOON	NONE
PM	05:00 PM - 06:00 PM

COUNT PERIODS

AM	07:00 AM - 09:00 AM
NOON	NONE
PM	04:00 PM - 06:00 PM

Main St — SOUTHBOUND

AM	169	1013	194	0	1135	AM
NOON	0	0	0	0	0	NOON
PM	230	893	142	0	1410	PM
	0	3	2	0		

La Veta Ave — EASTBOUND

AM	NOON	PM	
521	0	1032	
0	0	0	0
343	0	290	2
485	0	413	3
210	0	185	0
AM	NOON	PM	

La Veta Ave — WESTBOUND

PM	NOON	AM	
201	0	225	1
502	0	270	2
251	0	309	2
0	0	0	0
985	0	973	
PM	NOON	AM	

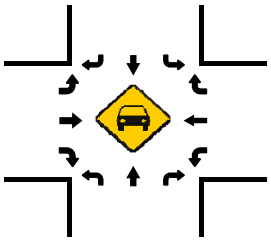
CONTROL: Signalized

TEV	4161	0	4756
	AM	NOON	PM
PHF	0.95		0.90

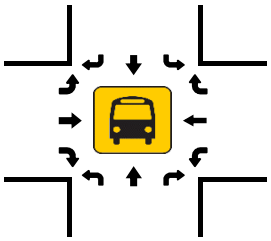
Main St — NORTHBOUND

PM	1329	0	300	919	430	PM
NOON	0	0	0	0	0	NOON
AM	1532	0	82	567	294	AM
		0	2	3	1	

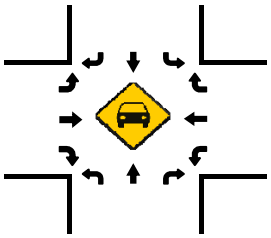
Total Vehicles (AM)



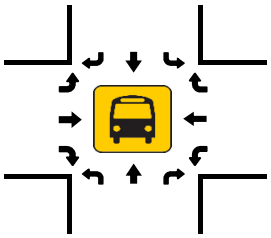
Total Vehicles (AM)



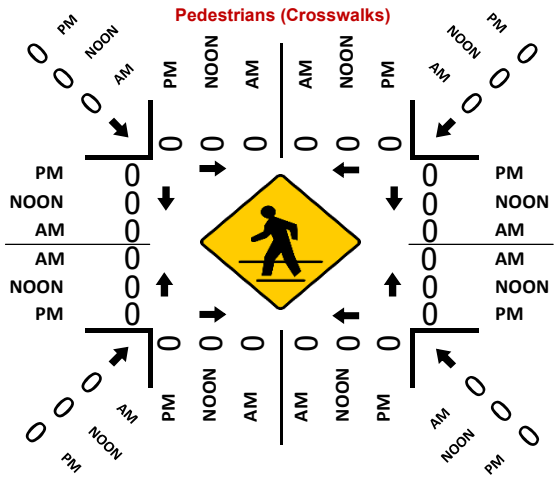
Total Vehicles (NOON)



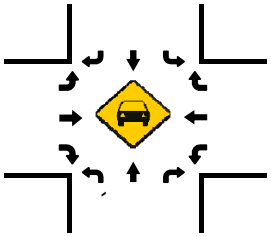
Total Vehicles (NOON)



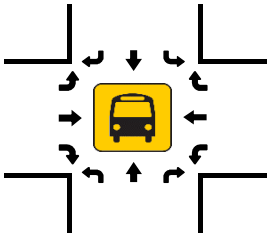
Pedestrians (Crosswalks)



Total Vehicles (PM)



Total Vehicles (PM)



INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Tue, May 14, 19

LOCATION: Santa Ana
NORTH & SOUTH: Main
EAST & WEST: Mainplace

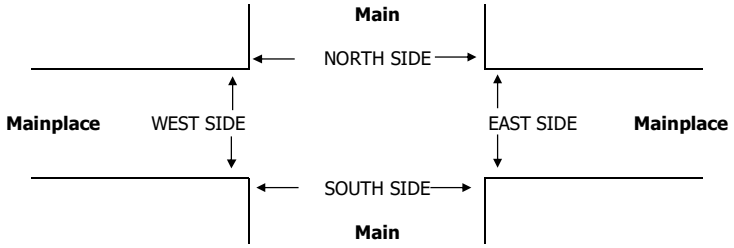
PROJECT #: SC2183
LOCATION #: 16
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N ◄ W E ► S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Main			SOUTHBOUND Main			EASTBOUND Mainplace			WESTBOUND Mainplace				U-TURNS				
	LANES:	NL 2	NT 3	NR 0	SL 2	ST 3	SR 0	EL 2	ET 2.5	ER 0.5	WL 2	WT 2	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	3	121	34	5	255	43	16	23	5	27	46	1	579	1	2	0	0	3
	7:15 AM	8	140	43	4	355	62	31	31	7	31	37	6	755	0	0	0	0	0
	7:30 AM	0	165	72	9	328	66	39	67	3	62	58	2	871	0	2	0	0	2
	7:45 AM	8	237	70	10	306	60	35	65	5	50	73	3	922	1	1	0	1	3
	8:00 AM	6	177	52	8	307	53	38	63	4	51	47	4	810	1	1	0	1	3
	8:15 AM	13	177	53	4	290	78	34	35	3	33	45	5	770	1	1	0	0	2
	8:30 AM	6	165	38	6	219	61	36	37	12	35	36	5	656	0	0	0	0	0
	8:45 AM	12	160	46	9	233	46	20	19	10	29	36	4	624	1	4	0	1	6
	VOLUMES	56	1,342	408	55	2,293	469	249	340	49	318	378	30	5,987	5	11	0	3	19
	APPROACH %	3%	74%	23%	2%	81%	17%	39%	53%	8%	44%	52%	4%						
	APP/DEPART	1,806	/	1,632	2,817	/	2,662	638	/	795	726	/	898	0					
	BEGIN PEAK HR		7:30 AM																
	VOLUMES	27	756	247	31	1,231	257	146	230	15	196	223	14	3,373					
	APPROACH %	3%	73%	24%	2%	81%	17%	37%	59%	4%	45%	52%	3%						
	PEAK HR FACTOR		0.817			0.942			0.897			0.859		0.915					
	APP/DEPART	1,030	/	921	1,519	/	1,443	391	/	505	433	/	504	0					
PM	4:00 PM	9	201	37	15	241	40	46	54	28	27	39	14	751	0	6	0	0	6
	4:15 PM	17	194	35	16	277	41	53	65	21	48	29	8	804	1	2	0	2	5
	4:30 PM	21	218	26	9	267	36	60	61	33	36	41	8	816	2	3	0	1	6
	4:45 PM	23	213	37	12	257	27	66	80	13	46	42	16	832	1	5	0	0	6
	5:00 PM	22	247	57	19	243	43	69	79	25	51	34	24	913	2	6	0	0	8
	5:15 PM	25	246	43	8	282	33	83	92	26	50	42	18	948	1	5	0	0	6
	5:30 PM	19	229	40	11	262	34	72	70	22	36	31	14	840	0	4	0	1	5
	5:45 PM	28	208	39	12	187	30	61	62	15	28	29	12	711	0	6	0	3	9
	VOLUMES	164	1,756	314	102	2,016	284	510	563	183	322	287	114	6,615	7	37	0	7	51
	APPROACH %	7%	79%	14%	4%	84%	12%	41%	45%	15%	45%	40%	16%						
	APP/DEPART	2,234	/	2,417	2,402	/	2,521	1,256	/	949	723	/	728	0					
	BEGIN PEAK HR		4:45 PM																
	VOLUMES	89	935	177	50	1,044	137	290	321	86	183	149	72	3,533					
	APPROACH %	7%	78%	15%	4%	85%	11%	42%	46%	12%	45%	37%	18%						
	PEAK HR FACTOR		0.921			0.953			0.867			0.918		0.932					
	APP/DEPART	1,201	/	1,317	1,231	/	1,316	697	/	529	404	/	371	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:30 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	4:45 PM					0	0	0	0	0					

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Tue, May 14, 19

LOCATION: Santa Ana
NORTH & SOUTH: Main
EAST & WEST: 17th

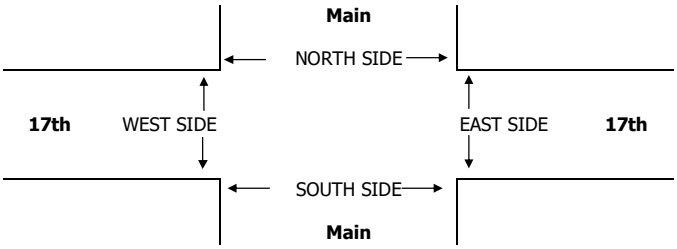
PROJECT #: SC2183
LOCATION #: 17
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N ◄ W E ► S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Main			SOUTHBOUND Main			EASTBOUND 17th			WESTBOUND 17th				U-TURNS				
	LANES:	NL 2	NT 2	NR 1	SL 2	ST 2	SR 0	EL 2	ET 3	ER 0	WL 2	WT 3	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	19	107	16	23	233	12	27	300	16	64	222	6	1,045	0	0	1	3	4
	7:15 AM	27	164	20	37	287	13	26	301	22	64	294	5	1,260	0	0	0	1	1
	7:30 AM	33	190	37	48	288	14	20	302	14	88	337	4	1,375	0	0	0	2	2
	7:45 AM	36	218	62	61	268	23	29	304	14	100	347	4	1,466	0	0	1	1	2
	8:00 AM	30	180	42	47	232	19	39	305	10	69	309	5	1,287	0	0	1	3	4
	8:15 AM	29	130	15	43	220	13	30	287	23	48	255	15	1,108	0	0	2	2	4
	8:30 AM	16	125	35	34	201	17	26	265	24	39	220	11	1,013	0	0	1	3	4
	8:45 AM	22	138	14	30	195	15	29	218	17	32	192	18	920	0	0	1	2	3
	VOLUMES	212	1,252	241	323	1,924	126	226	2,282	140	504	2,176	68	9,474	0	0	7	17	24
	APPROACH %	12%	73%	14%	14%	81%	5%	9%	86%	5%	18%	79%	2%						
	APP/DEPART	1,705	/	1,539	2,373	/	2,551	2,648	/	2,863	2,748	/	2,521	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	126	752	161	193	1,075	69	114	1,212	60	321	1,287	18	5,388					
	APPROACH %	12%	72%	15%	14%	80%	5%	8%	87%	4%	20%	79%	1%						
	PEAK HR FACTOR		0.822			0.950			0.979			0.901		0.919					
	APP/DEPART	1,039	/	882	1,337	/	1,449	1,386	/	1,573	1,626	/	1,484	0					
PM	4:00 PM	39	236	54	33	164	28	43	273	20	47	263	17	1,217	0	1	2	3	6
	4:15 PM	56	253	49	40	188	30	48	300	26	37	259	20	1,306	0	0	2	1	3
	4:30 PM	47	243	49	44	196	26	47	283	15	50	279	24	1,303	0	1	2	2	5
	4:45 PM	45	255	63	42	193	22	56	313	18	55	291	16	1,369	0	0	1	2	3
	5:00 PM	47	257	58	45	172	37	67	328	13	34	249	11	1,318	0	0	8	0	8
	5:15 PM	52	254	36	46	175	41	49	325	22	32	228	16	1,276	0	0	2	1	3
	5:30 PM	46	242	41	51	176	31	46	291	18	37	299	13	1,291	0	0	2	0	2
	5:45 PM	38	263	40	50	158	19	59	276	22	41	280	7	1,253	0	0	3	2	5
	VOLUMES	370	2,003	390	351	1,422	234	415	2,389	154	333	2,148	124	10,333	0	2	22	11	35
	APPROACH %	13%	72%	14%	17%	71%	12%	14%	81%	5%	13%	82%	5%						
	APP/DEPART	2,763	/	2,522	2,007	/	1,898	2,958	/	3,139	2,605	/	2,774	0					
	BEGIN PEAK HR		4:15 PM																
	VOLUMES	195	1,008	219	171	749	115	218	1,224	72	176	1,078	71	5,296					
	APPROACH %	14%	71%	15%	17%	72%	11%	14%	81%	5%	13%	81%	5%						
	PEAK HR FACTOR		0.979			0.973			0.928			0.915		0.967					
	APP/DEPART	1,422	/	1,285	1,035	/	992	1,514	/	1,618	1,325	/	1,401	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:15 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	4:15 PM					0	0	0	0	0					

1822

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

DATE: Thu, Feb 11, 16

LOCATION: Santa Ana
NORTH & SOUTH: Main
EAST & WEST: Civic Center

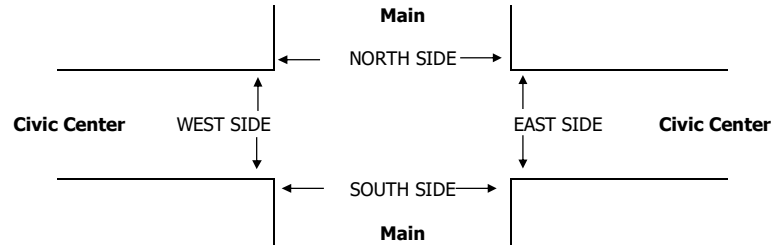
PROJECT #: SC0846
LOCATION #: 14
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Main			SOUTHBOUND Main			EASTBOUND Civic Center			WESTBOUND Civic Center				U-TURNS				
	LANES:	NL 1	NT 2	NR 0	SL 1	ST 2	SR 0	EL 1	ET 2	ER 0	WL 1	WT 2	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	19	158	9	12	248	29	8	80	16	6	65	3	653	0	0	0	0	0
	7:15 AM	35	205	12	13	242	21	19	106	27	8	125	12	825	0	0	0	0	0
	7:30 AM	35	219	13	15	251	47	20	112	16	9	137	5	879	0	0	0	0	0
	7:45 AM	30	234	25	16	257	47	20	159	25	14	166	10	1,003	0	0	0	0	0
	8:00 AM	32	207	25	12	251	37	27	146	27	10	115	5	894	0	0	0	0	0
	8:15 AM	32	188	17	17	254	42	19	64	24	6	80	14	757	0	0	0	0	0
	8:30 AM	31	189	10	13	251	22	15	63	39	9	68	5	715	0	0	0	0	0
	8:45 AM	20	175	8	8	203	22	17	53	32	3	49	5	595	0	0	0	0	0
	VOLUMES	234	1,575	119	106	1,957	267	145	783	206	65	805	59	6,321	0	0	0	0	0
	APPROACH %	12%	82%	6%	5%	84%	11%	13%	69%	18%	7%	87%	6%						
	APP/DEPART	1,928	/	1,779	2,330	/	2,228	1,134	/	1,008	929	/	1,306	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	132	865	75	56	1,001	152	86	523	95	41	543	32	3,601					
	APPROACH %	12%	81%	7%	5%	83%	13%	12%	74%	13%	7%	88%	5%						
	PEAK HR FACTOR		0.927			0.945			0.863			0.811		0.898					
	APP/DEPART	1,072	/	983	1,209	/	1,137	704	/	654	616	/	827	0					
PM	4:00 PM	17	275	24	10	199	10	30	126	29	14	59	5	798	0	0	0	0	0
	4:15 PM	17	238	13	9	201	19	34	118	22	7	57	12	747	0	0	0	0	0
	4:30 PM	27	291	12	6	227	10	39	187	24	6	77	14	920	1	0	0	0	1
	4:45 PM	17	278	25	18	240	21	51	228	22	6	87	9	1,002	0	0	0	0	0
	5:00 PM	18	245	24	23	248	18	42	241	36	16	119	4	1,034	0	0	0	0	0
	5:15 PM	23	268	29	11	241	11	20	190	18	8	86	10	915	0	0	0	0	0
	5:30 PM	24	258	16	8	210	21	14	120	30	17	76	17	811	0	0	0	0	0
	5:45 PM	23	271	16	13	226	15	11	97	15	9	76	9	781	0	0	0	0	0
	VOLUMES	166	2,124	159	98	1,792	125	241	1,307	196	83	637	80	7,008	1	0	0	0	1
	APPROACH %	7%	87%	6%	5%	89%	6%	14%	75%	11%	10%	80%	10%						
	APP/DEPART	2,449	/	2,445	2,015	/	2,072	1,744	/	1,564	800	/	927	0					
	BEGIN PEAK HR		4:30 PM																
	VOLUMES	85	1,082	90	58	956	60	152	846	100	36	369	37	3,871					
	APPROACH %	7%	86%	7%	5%	89%	6%	14%	77%	9%	8%	83%	8%						
	PEAK HR FACTOR		0.952			0.929			0.861			0.795		0.936					
	APP/DEPART	1,257	/	1,271	1,074	/	1,093	1,098	/	994	442	/	513	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	9	1	2	3	15	9	0	2	3	14	0	1	0	0	1
	7:15 AM	1	1	15	3	20	1	1	14	3	19	0	0	1	0	1
	7:30 AM	1	7	6	2	16	0	6	4	2	12	1	1	2	0	4
	7:45 AM	0	6	6	6	18	0	4	6	4	14	0	2	0	2	4
	8:00 AM	8	5	8	1	22	8	2	8	1	19	0	3	0	0	3
	8:15 AM	18	1	8	8	35	17	1	6	7	31	1	0	2	1	4
	8:30 AM	13	6	7	7	33	13	6	5	7	31	0	0	2	0	2
	8:45 AM	12	0	10	5	27	12	0	7	4	23	0	0	3	1	4
	TOTAL	62	27	62	35	186	60	20	52	31	163	2	7	10	4	23
PM	4:00 PM	6	6	8	7	27	6	5	5	7	23	0	1	3	0	4
	4:15 PM	7	4	7	19	37	7	4	6	15	32	0	0	1	4	5
	4:30 PM	10	7	15	6	38	10	6	14	5	35	0	1	1	1	3
	4:45 PM	6	4	11	3	24	6	3	9	3	21	0	1	2	0	3
	5:00 PM	48	9	16	1	74	48	5	14	1	68	0	4	2	0	6
	5:15 PM	5	5	7	1	18	4	2	7	1	14	1	3	0	0	4
	5:30 PM	8	3	6	2	19	8	3	6	2	19	0	0	0	0	0
	5:45 PM	6	3	15	1	25	6	0	11	1	18	0	3	4	0	7
	TOTAL	96	41	85	40	262	95	28	72	35	230	1	13	13	5	32

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

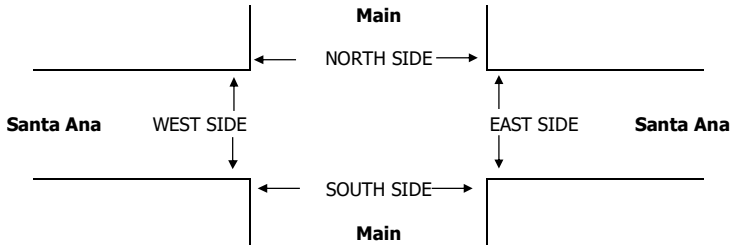
DATE:	LOCATION:	Santa Ana	PROJECT #:	SC0846
Thu, Feb 11, 16	NORTH & SOUTH:	Main	LOCATION #:	26
	EAST & WEST:	Santa Ana	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Main			SOUTHBOUND Main			EASTBOUND Santa Ana			WESTBOUND Santa Ana				U-TURNS				
	LANES:	NL 1	NT 2	NR X	SL X	ST 2	SR 0	EL X	ET X	ER X	WL 0.5	WT 2	WR 0.5	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	10	149	0	0	240	25	0	0	0	14	152	6	596	0	0	0	0	0
	7:15 AM	16	220	0	0	273	22	0	0	0	19	153	14	717	0	0	0	0	0
	7:30 AM	16	241	0	0	270	26	0	0	0	15	233	15	816	0	0	0	0	0
	7:45 AM	15	334	0	0	258	17	0	0	0	8	203	15	850	0	0	0	0	0
	8:00 AM	18	245	0	0	262	29	0	0	0	16	225	18	813	0	0	0	0	0
	8:15 AM	11	207	0	0	235	42	0	0	0	17	148	15	675	0	0	0	0	0
	8:30 AM	15	217	0	0	288	28	0	0	0	15	144	9	716	0	0	0	0	0
	8:45 AM	15	167	0	0	203	26	0	0	0	13	116	14	554	0	0	0	0	0
	VOLUMES	116	1,780	0	0	2,029	215	0	0	0	117	1,374	106	5,737	0	0	0	0	0
	APPROACH %	6%	94%	0%	0%	90%	10%	0%	0%	0%	7%	86%	7%						
	APP/DEPART	1,896	/	1,886	2,244	/	2,146	0	/	0	1,597	/	1,705	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	65	1,040	0	0	1,063	94	0	0	0	58	814	62	3,196					
	APPROACH %	6%	94%	0%	0%	92%	8%	0%	0%	0%	6%	87%	7%						
	PEAK HR FACTOR		0.792			0.977			0.000			0.888		0.940					
	APP/DEPART	1,105	/	1,102	1,157	/	1,121	0	/	0	934	/	973	0					
PM	4:00 PM	12	261	0	0	219	9	0	0	0	17	146	18	682	0	0	0	0	0
	4:15 PM	10	251	0	0	218	12	0	0	0	2	113	13	619	0	0	0	0	0
	4:30 PM	12	302	0	0	217	7	0	0	0	15	122	17	692	0	0	0	0	0
	4:45 PM	10	330	0	0	247	12	0	0	0	15	157	19	790	0	0	0	0	0
	5:00 PM	16	250	0	0	300	15	0	0	0	18	127	25	751	0	0	0	0	0
	5:15 PM	14	319	0	0	263	12	0	0	0	15	177	20	820	0	0	0	0	0
	5:30 PM	11	282	0	0	230	11	0	0	0	14	169	17	734	0	0	0	0	0
	5:45 PM	6	254	0	0	252	10	0	0	0	15	147	20	704	0	0	0	0	0
	VOLUMES	91	2,249	0	0	1,946	88	0	0	0	111	1,158	149	5,792	0	0	0	0	0
	APPROACH %	4%	96%	0%	0%	96%	4%	0%	0%	0%	8%	82%	11%						
	APP/DEPART	2,340	/	2,398	2,034	/	2,057	0	/	0	1,418	/	1,337	0					
	BEGIN PEAK HR		4:45 PM																
	VOLUMES	51	1,181	0	0	1,040	50	0	0	0	62	630	81	3,095					
	APPROACH %	4%	96%	0%	0%	95%	5%	0%	0%	0%	8%	82%	10%						
	PEAK HR FACTOR		0.906			0.865			0.000			0.912		0.944					
	APP/DEPART	1,232	/	1,262	1,090	/	1,102	0	/	0	773	/	731	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	7	7	2	4	20	4	5	2	4	15	3	2	0	0	5
	7:15 AM	11	13	17	7	48	11	11	14	7	43	0	2	3	0	5
	7:30 AM	7	20	22	6	55	6	18	21	6	51	1	2	1	0	4
	7:45 AM	6	5	6	6	23	6	5	6	5	22	0	0	0	1	1
	8:00 AM	8	5	8	4	25	6	4	7	3	20	2	1	1	1	5
	8:15 AM	4	3	5	6	18	3	2	5	4	14	1	1	0	2	4
	8:30 AM	4	2	7	8	21	2	2	6	7	17	2	0	1	1	4
	8:45 AM	1	0	6	2	9	1	0	3	2	6	0	0	3	0	3
	TOTAL	48	55	73	43	219	39	47	64	38	188	9	8	9	5	31
PM	4:00 PM	7	5	6	8	26	5	4	5	6	20	2	1	1	2	6
	4:15 PM	4	4	7	14	29	4	4	7	14	29	0	0	0	0	0
	4:30 PM	11	8	10	15	44	8	7	9	13	37	3	1	1	2	7
	4:45 PM	12	2	11	9	34	11	2	10	6	29	1	0	1	3	5
	5:00 PM	9	8	6	21	44	8	6	5	20	39	1	2	1	1	5
	5:15 PM	8	18	6	21	53	8	18	6	21	53	0	0	0	0	0
	5:30 PM	12	2	10	9	33	10	1	9	7	27	2	1	1	2	6
	5:45 PM	3	3	10	11	27	3	2	8	9	22	0	1	2	2	5
	TOTAL	66	50	66	108	290	57	44	59	96	256	9	6	7	12	34

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

DATE: Thu, Feb 11, 16

LOCATION: Santa Ana
NORTH & SOUTH: Main
EAST & WEST: 4th

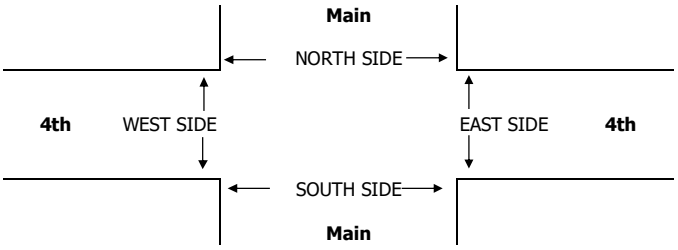
PROJECT #: SC0846
LOCATION #: 25
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Main			SOUTHBOUND Main			EASTBOUND 4th			WESTBOUND 4th				U-TURNS				
		NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL	NB	SB	EB	WB	TTL
	LANES:	0	2	0	0	2	0	0	1	0	0	1	0		0	0	0	0	
AM	7:00 AM	0	169	4	0	244	8	0	12	5	0	10	6	458	0	0	0	0	0
	7:15 AM	0	202	5	0	276	4	0	21	2	0	18	6	534	0	0	0	0	0
	7:30 AM	0	251	4	0	258	7	0	25	2	0	27	9	583	0	0	0	0	0
	7:45 AM	0	271	9	0	261	3	0	25	3	0	35	5	612	0	0	0	0	0
	8:00 AM	0	214	4	0	260	11	0	18	6	0	29	6	548	0	0	0	0	0
	8:15 AM	0	184	8	2	239	11	0	16	7	0	48	7	522	0	0	0	0	0
	8:30 AM	0	208	7	0	272	8	1	14	3	1	43	11	568	0	0	0	0	0
	8:45 AM	0	168	5	0	216	5	0	13	5	1	29	13	455	0	0	0	0	0
	VOLUMES	0	1,667	46	2	2,026	57	1	144	33	2	239	63	4,280	0	0	0	0	0
	APPROACH %	0%	97%	3%	0%	97%	3%	1%	81%	19%	1%	79%	21%						
	APP/DEPART	1,713	/	1,731	2,085	/	2,061	178	/	192	304	/	296	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	0	938	22	0	1,055	25	0	89	13	0	109	26	2,277					
	APPROACH %	0%	98%	2%	0%	98%	2%	0%	87%	13%	0%	81%	19%						
	PEAK HR FACTOR		0.857			0.964			0.911			0.844		0.930					
	APP/DEPART	960	/	964	1,080	/	1,068	102	/	111	135	/	134	0					
PM	4:00 PM	0	254	12	0	206	7	0	32	12	0	37	19	579	0	0	0	0	0
	4:15 PM	2	239	9	0	195	7	1	27	6	1	41	8	536	0	0	0	0	0
	4:30 PM	0	261	20	0	224	8	1	24	10	2	40	10	600	0	0	0	0	0
	4:45 PM	0	270	16	0	220	13	0	27	8	1	43	14	612	0	0	0	0	0
	5:00 PM	0	235	8	0	274	12	2	17	17	3	38	14	620	0	0	0	0	0
	5:15 PM	0	259	7	0	257	10	1	27	14	0	45	8	628	0	0	0	0	0
	5:30 PM	0	249	17	0	245	10	1	23	13	0	47	8	613	0	0	0	0	0
	5:45 PM	0	247	10	0	220	14	0	17	22	2	38	22	592	0	0	0	0	0
	VOLUMES	2	2,014	99	0	1,841	81	6	194	102	9	329	103	4,780	0	0	0	0	0
	APPROACH %	0%	95%	5%	0%	96%	4%	2%	64%	34%	2%	75%	23%						
	APP/DEPART	2,115	/	2,123	1,922	/	1,952	302	/	293	441	/	412	0					
	BEGIN PEAK HR		4:45 PM																
	VOLUMES	0	1,013	48	0	996	45	4	94	52	4	173	44	2,473					
	APPROACH %	0%	95%	5%	0%	96%	4%	3%	63%	35%	2%	78%	20%						
	PEAK HR FACTOR		0.927			0.910			0.893			0.953		0.984					
	APP/DEPART	1,061	/	1,061	1,041	/	1,052	150	/	142	221	/	218	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	4	6	1	3	14	3	5	1	3	12	1	1	0	0	2
	7:15 AM	8	0	9	3	20	7	0	9	3	19	1	0	0	0	1
	7:30 AM	4	6	5	4	19	4	5	1	4	14	0	1	4	0	5
	7:45 AM	3	12	9	5	29	3	11	8	4	26	0	1	1	1	3
	8:00 AM	6	14	3	4	27	6	14	3	4	27	0	0	0	0	0
	8:15 AM	10	17	2	7	36	10	13	1	7	31	0	4	1	0	5
	8:30 AM	15	12	13	12	52	13	11	12	12	48	2	1	1	0	4
	8:45 AM	10	12	7	8	37	8	9	4	8	29	2	3	3	0	8
	TOTAL	60	79	49	46	234	54	68	39	45	206	6	11	10	1	28
PM	4:00 PM	32	44	24	26	126	32	44	23	24	123	0	0	1	2	3
	4:15 PM	48	61	28	31	168	46	60	27	31	164	2	1	1	0	4
	4:30 PM	39	76	31	39	185	37	71	27	34	169	2	5	4	5	16
	4:45 PM	32	66	18	23	139	31	66	16	23	136	1	0	2	0	3
	5:00 PM	51	42	13	28	134	51	40	12	26	129	0	2	1	2	5
	5:15 PM	29	45	16	24	114	28	45	16	22	111	1	0	0	2	3
	5:30 PM	39	71	22	33	165	35	71	21	29	156	4	0	1	4	9
	5:45 PM	34	41	20	21	116	34	40	17	20	111	0	1	3	1	5
	TOTAL	304	446	172	225	1,147	294	437	159	209	1,099	10	9	13	16	48

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Tue, May 14, 19

LOCATION: Santa Ana
NORTH & SOUTH: Main
EAST & WEST: 1st

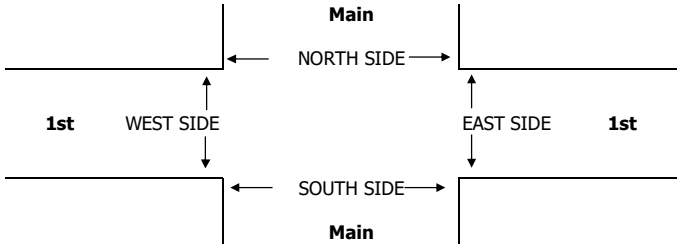
PROJECT #: SC2183
LOCATION #: 18
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Main			SOUTHBOUND Main			EASTBOUND 1st			WESTBOUND 1st				U-TURNS				
	LANES:	NL 1	NT 2	NR 0	SL 1	ST 2	SR 1	EL 1	ET 3	ER 0	WL 1	WT 3	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	21	110	8	17	205	15	24	398	30	14	167	9	1,018	0	0	0	0	0
	7:15 AM	23	143	14	23	226	13	26	298	29	25	251	9	1,080	0	0	0	2	2
	7:30 AM	37	166	8	23	213	23	28	362	44	21	285	13	1,223	0	0	0	0	0
	7:45 AM	27	168	13	23	232	17	30	362	42	21	311	22	1,268	0	0	0	0	0
	8:00 AM	27	113	24	18	202	11	33	357	30	20	239	16	1,090	0	0	0	0	0
	8:15 AM	35	119	15	24	198	12	21	326	40	15	240	19	1,064	0	0	0	0	0
	8:30 AM	34	132	20	18	225	12	31	318	36	26	214	14	1,080	0	0	0	0	0
	8:45 AM	14	128	16	22	193	16	35	276	45	23	197	19	984	0	0	0	0	0
	VOLUMES	218	1,079	118	168	1,694	119	228	2,697	296	165	1,904	121	8,807	0	0	0	2	2
	APPROACH %	15%	76%	8%	8%	86%	6%	7%	84%	9%	8%	87%	6%						
	APP/DEPART	1,415	/	1,428	1,981	/	2,153	3,221	/	2,985	2,190	/	2,241	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	114	590	59	87	873	64	117	1,379	145	87	1,086	60	4,661					
	APPROACH %	15%	77%	8%	8%	85%	6%	7%	84%	9%	7%	88%	5%						
	PEAK HR FACTOR		0.904			0.941			0.945			0.871		0.919					
	APP/DEPART	763	/	767	1,024	/	1,103	1,641	/	1,527	1,233	/	1,264	0					
PM	4:00 PM	34	204	15	34	135	30	34	233	26	32	287	17	1,081	0	0	1	1	2
	4:15 PM	38	219	24	35	171	34	37	272	19	26	306	10	1,191	0	0	0	2	2
	4:30 PM	33	214	20	35	160	32	35	285	25	26	294	19	1,178	0	0	0	1	1
	4:45 PM	43	191	20	48	150	32	39	312	18	18	306	24	1,201	0	0	0	0	0
	5:00 PM	40	206	15	40	173	30	21	272	26	31	316	20	1,190	0	0	0	0	0
	5:15 PM	43	197	21	43	169	38	30	315	18	32	316	23	1,245	0	0	1	0	1
	5:30 PM	41	223	15	42	172	41	28	302	27	31	324	21	1,267	0	0	0	0	0
	5:45 PM	39	194	17	43	160	41	22	291	25	26	302	30	1,190	0	0	0	0	0
	VOLUMES	311	1,648	147	320	1,290	278	246	2,282	184	222	2,451	164	9,543	0	0	2	4	6
	APPROACH %	15%	78%	7%	17%	68%	15%	9%	84%	7%	8%	86%	6%						
	APP/DEPART	2,106	/	2,056	1,888	/	1,692	2,712	/	2,753	2,837	/	3,042	0					
	BEGIN PEAK HR		4:45 PM																
	VOLUMES	167	817	71	173	664	141	118	1,201	89	112	1,262	88	4,903					
	APPROACH %	16%	77%	7%	18%	68%	14%	8%	85%	6%	8%	86%	6%						
	PEAK HR FACTOR		0.945			0.959			0.954			0.972		0.967					
	APP/DEPART	1,055	/	1,022	978	/	865	1,408	/	1,445	1,462	/	1,571	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:15 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	4:45 PM					0	0	0	0	0					

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

DATE: Thu, Feb 11, 16

LOCATION: Santa Ana
NORTH & SOUTH: Main
EAST & WEST: McFadden

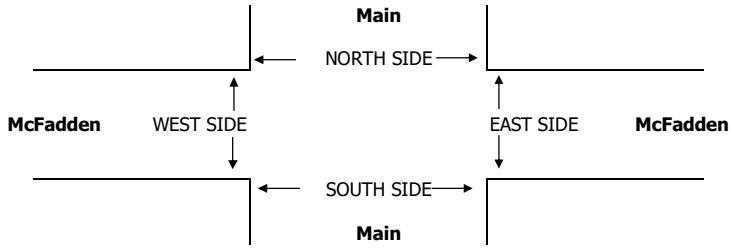
PROJECT #: SC0846
LOCATION #: 37
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Main			SOUTHBOUND Main			EASTBOUND McFadden			WESTBOUND McFadden				U-TURNS				
		NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL	NB	SB	EB	WB	TTL
	LANES:	1	2	0	1	2	0	1	1	0	1	1	1		0	0	0	0	
AM	7:00 AM	14	149	6	40	242	6	5	93	5	18	70	21	669	0	0	0	0	0
	7:15 AM	15	179	10	47	309	3	8	97	12	30	99	31	840	0	0	0	0	0
	7:30 AM	26	234	5	36	337	8	12	93	11	40	96	22	920	0	0	0	0	0
	7:45 AM	27	216	15	46	343	5	12	98	6	32	87	24	911	0	0	0	0	0
	8:00 AM	17	176	10	39	321	9	8	92	12	32	76	25	817	0	0	0	0	0
	8:15 AM	17	153	14	42	318	4	8	84	10	8	51	11	720	0	0	0	0	0
	8:30 AM	22	173	11	25	247	14	14	68	11	11	49	28	673	0	0	0	0	0
	8:45 AM	19	148	7	30	302	8	9	68	15	14	79	8	707	0	0	0	0	0
	VOLUMES	157	1,428	78	305	2,419	57	76	693	82	185	607	170	6,257	0	0	0	0	0
	APPROACH %	9%	86%	5%	11%	87%	2%	9%	81%	10%	19%	63%	18%						
	APP/DEPART	1,663	/	1,674	2,781	/	2,686	851	/	1,076	962	/	821	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	85	805	40	168	1,310	25	40	380	41	134	358	102	3,488					
	APPROACH %	9%	87%	4%	11%	87%	2%	9%	82%	9%	23%	60%	17%						
	PEAK HR FACTOR		0.877			0.954			0.985			0.928		0.948					
	APP/DEPART	930	/	947	1,503	/	1,485	461	/	588	594	/	468	0					
PM	4:00 PM	35	268	13	30	175	13	18	80	13	17	112	31	805	0	0	0	0	0
	4:15 PM	37	291	8	38	174	14	12	77	15	22	123	57	868	0	0	0	0	0
	4:30 PM	31	283	17	33	195	19	18	77	7	20	121	49	870	0	0	0	0	0
	4:45 PM	34	283	11	27	189	10	21	83	10	21	121	40	850	0	0	0	0	0
	5:00 PM	39	286	12	33	198	7	17	82	9	10	117	60	870	0	0	0	0	0
	5:15 PM	32	256	17	29	229	21	18	84	11	23	115	41	876	0	0	0	1	1
	5:30 PM	35	292	28	36	276	23	20	68	10	27	96	49	960	0	0	0	0	0
	5:45 PM	34	268	11	30	169	12	19	97	10	21	133	57	861	0	0	0	0	0
	VOLUMES	277	2,227	117	256	1,605	119	143	648	85	161	938	384	6,960	0	0	0	1	1
	APPROACH %	11%	85%	4%	13%	81%	6%	16%	74%	10%	11%	63%	26%						
	APP/DEPART	2,621	/	2,754	1,980	/	1,850	876	/	1,022	1,483	/	1,334	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	140	1,102	68	128	872	63	74	331	40	81	461	207	3,567					
	APPROACH %	11%	84%	5%	12%	82%	6%	17%	74%	9%	11%	62%	28%						
	PEAK HR FACTOR		0.923			0.793			0.883			0.887		0.929					
	APP/DEPART	1,310	/	1,383	1,063	/	992	445	/	528	749	/	664	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	6	2	5	4	17	4	2	4	4	14	2	0	1	0	3
	7:15 AM	4	5	11	5	25	4	5	11	5	25	0	0	0	0	0
	7:30 AM	9	16	20	20	65	8	16	18	18	60	1	0	2	2	5
	7:45 AM	9	9	30	32	80	8	9	29	29	75	1	0	1	3	5
	8:00 AM	7	14	12	25	58	6	13	12	25	56	1	1	0	0	2
	8:15 AM	6	5	8	13	32	5	5	8	11	29	1	0	0	2	3
	8:30 AM	7	6	9	4	26	4	3	8	3	18	3	3	1	1	8
	8:45 AM	4	2	13	19	38	2	2	12	14	30	2	0	1	5	8
	TOTAL	52	59	108	122	341	41	55	102	109	307	11	4	6	13	34
PM	4:00 PM	10	11	20	22	63	8	10	18	17	53	2	1	2	5	10
	4:15 PM	5	8	13	17	43	4	6	9	17	36	1	2	4	0	7
	4:30 PM	14	13	26	22	75	12	8	22	20	62	2	5	4	2	13
	4:45 PM	21	30	29	15	95	18	25	29	13	85	3	5	0	2	10
	5:00 PM	11	3	12	9	35	10	2	8	4	24	1	1	4	5	11
	5:15 PM	13	13	19	18	63	12	12	18	16	58	1	1	1	2	5
	5:30 PM	9	10	15	12	46	8	9	14	12	43	1	1	1	0	3
	5:45 PM	12	13	8	27	60	10	10	8	25	53	2	3	0	2	7
	TOTAL	95	101	142	142	480	82	82	126	124	414	13	19	16	18	66

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Thu, May 16, 19

LOCATION: Santa Ana
NORTH & SOUTH: Main
EAST & WEST: Edinger

PROJECT #: SC2183
LOCATION #: 19
CONTROL: SIGNAL

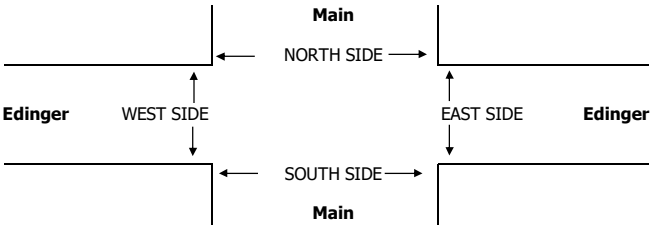
NOTES:

AM / PM / MD / OTHER / OTHER

N ▲ S ▼ W ◄ E ►

☑ Add U-Turns to Left Turns

		NORTHBOUND Main			SOUTHBOUND Main			EASTBOUND Edinger			WESTBOUND Edinger				U-TURNS				
	LANES:	NL 1	NT 2	NR 0	SL 1	ST 2	SR 0	EL 1	ET 2	ER 1	WL 1	WT 3	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	18	115	19	34	247	18	14	241	38	20	149	15	928	0	0	0	0	0
	7:15 AM	21	158	17	47	248	14	15	243	35	28	184	26	1,036	0	0	2	3	5
	7:30 AM	27	171	18	61	265	17	9	246	38	31	159	21	1,063	0	0	0	2	2
	7:45 AM	28	128	13	59	244	12	23	250	28	38	165	24	1,012	0	0	1	2	3
	8:00 AM	31	153	18	58	284	11	27	222	28	23	193	15	1,063	0	0	0	1	1
	8:15 AM	23	126	19	47	265	9	19	212	21	17	194	22	974	0	0	0	1	1
	8:30 AM	21	102	21	34	233	13	32	220	37	24	141	16	894	0	0	0	3	3
	8:45 AM	10	104	21	22	272	18	18	183	25	27	100	20	820	0	0	1	2	3
	VOLUMES	179	1,057	146	362	2,058	112	157	1,817	250	208	1,285	159	7,790	0	0	4	14	18
	APPROACH %	13%	76%	11%	14%	81%	4%	7%	82%	11%	13%	78%	10%						
	APP/DEPART	1,382	/	1,369	2,532	/	2,502	2,224	/	2,339	1,652	/	1,580	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	107	610	66	225	1,041	54	74	961	129	120	701	86	4,174					
	APPROACH %	14%	78%	8%	17%	79%	4%	6%	83%	11%	13%	77%	9%						
	PEAK HR FACTOR		0.906			0.935			0.967			0.953		0.982					
	APP/DEPART	783	/	767	1,320	/	1,282	1,164	/	1,260	907	/	865	0					
PM	4:00 PM	21	237	17	32	121	22	41	171	26	20	297	39	1,044	0	0	0	0	0
	4:15 PM	27	226	11	34	133	34	38	180	26	28	316	31	1,084	0	0	1	1	2
	4:30 PM	23	253	14	21	139	9	21	195	17	18	328	27	1,065	0	0	1	2	3
	4:45 PM	14	272	13	20	146	30	29	160	15	13	297	30	1,039	0	0	0	0	0
	5:00 PM	29	250	12	39	137	13	34	164	23	23	330	29	1,083	0	0	2	2	4
	5:15 PM	33	243	15	33	148	20	36	203	23	27	325	32	1,138	0	0	0	2	2
	5:30 PM	18	259	9	28	180	26	22	194	24	23	320	33	1,136	0	0	1	1	2
	5:45 PM	18	287	11	20	137	23	33	183	13	23	308	38	1,094	0	0	0	2	2
	VOLUMES	183	2,027	102	227	1,141	177	254	1,450	167	175	2,521	259	8,683	0	0	5	10	15
	APPROACH %	8%	88%	4%	15%	74%	11%	14%	77%	9%	6%	85%	9%						
	APP/DEPART	2,312	/	2,535	1,545	/	1,473	1,871	/	1,789	2,955	/	2,886	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	98	1,039	47	120	602	82	125	744	83	96	1,283	132	4,451					
	APPROACH %	8%	88%	4%	15%	75%	10%	13%	78%	9%	6%	85%	9%						
	PEAK HR FACTOR		0.937			0.859			0.908			0.984		0.978					
	APP/DEPART	1,184	/	1,293	804	/	774	952	/	918	1,511	/	1,466	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:15 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	5:00 PM					0	0	0	0	0					

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Tue, May 14, 19

LOCATION: Santa Ana
NORTH & SOUTH: Main
EAST & WEST: MacArthur

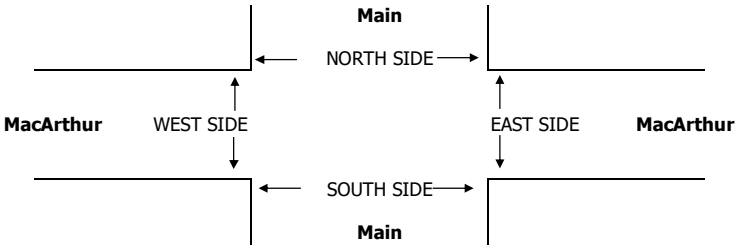
PROJECT #: SC2183
LOCATION #: 20
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N ◄ W E ► S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Main			SOUTHBOUND Main			EASTBOUND MacArthur			WESTBOUND MacArthur				U-TURNS				
	LANES:	NL 2	NT 3	NR 1	SL 2	ST 3	SR 1	EL 2	ET 3	ER 1	WL 2	WT 3	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	11	59	34	159	178	36	35	308	35	43	109	54	1,061	3	8	7	0	18
	7:15 AM	15	53	64	156	193	54	32	358	58	31	106	66	1,186	3	5	1	0	9
	7:30 AM	12	84	67	188	221	66	60	372	61	42	123	40	1,336	2	16	2	1	21
	7:45 AM	13	88	72	157	233	40	84	349	72	36	109	46	1,299	4	8	2	1	15
	8:00 AM	13	82	58	137	192	30	61	300	58	35	92	63	1,121	3	7	1	1	12
	8:15 AM	12	63	43	146	217	43	50	304	73	41	95	47	1,134	1	6	4	1	12
	8:30 AM	13	53	63	131	216	31	53	339	72	36	92	42	1,141	4	3	2	4	13
	8:45 AM	13	49	65	129	198	37	44	301	75	40	88	40	1,079	4	6	2	3	15
	VOLUMES	102	531	466	1,203	1,648	337	419	2,631	504	304	814	398	9,357	24	59	21	11	115
	APPROACH %	9%	48%	42%	38%	52%	11%	12%	74%	14%	20%	54%	26%						
	APP/DEPART	1,099	/	1,386	3,188	/	2,469	3,554	/	4,252	1,516	/	1,250	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	53	307	261	638	839	190	237	1,379	249	144	430	215	4,942					
	APPROACH %	9%	49%	42%	38%	50%	11%	13%	74%	13%	18%	54%	27%						
	PEAK HR FACTOR		0.897			0.877			0.923			0.962		0.925					
	APP/DEPART	621	/	789	1,667	/	1,241	1,865	/	2,245	789	/	667	0					
PM	4:00 PM	97	244	95	68	71	61	51	132	18	34	362	125	1,358	5	2	0	3	10
	4:15 PM	88	277	79	69	85	55	49	145	17	36	405	118	1,423	3	6	3	2	14
	4:30 PM	113	256	78	67	85	67	65	131	14	43	373	107	1,399	7	2	3	2	14
	4:45 PM	109	215	85	75	87	52	72	150	16	43	384	133	1,421	6	8	11	2	27
	5:00 PM	118	285	75	58	81	65	66	153	10	43	408	97	1,459	8	4	3	2	17
	5:15 PM	123	333	89	76	82	80	70	180	15	51	397	107	1,603	3	8	4	1	16
	5:30 PM	125	272	77	72	98	63	79	143	16	49	381	123	1,498	6	10	4	3	23
	5:45 PM	117	267	76	74	103	67	80	130	17	53	368	124	1,476	4	6	5	2	17
	VOLUMES	890	2,149	654	559	692	510	532	1,164	123	352	3,078	934	11,637	42	46	33	17	138
	APPROACH %	24%	58%	18%	32%	39%	29%	29%	64%	7%	8%	71%	21%						
	APP/DEPART	3,693	/	3,628	1,761	/	1,192	1,819	/	2,348	4,364	/	4,469	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	483	1,157	317	280	364	275	295	606	58	196	1,554	451	6,036					
	APPROACH %	25%	59%	16%	30%	40%	30%	31%	63%	6%	9%	71%	20%						
	PEAK HR FACTOR		0.898			0.942			0.905			0.991		0.941					
	APP/DEPART	1,957	/	1,915	919	/	631	959	/	1,183	2,201	/	2,307	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:15 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	5:00 PM					0	0	0	0	0					

Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: PENN WAY
E-W Direction: 17TH STREET

File Name : H1702011
Site Code : 00000000
Start Date : 2/9/2017
Page No : 1

Groups Printed- Turning Movements

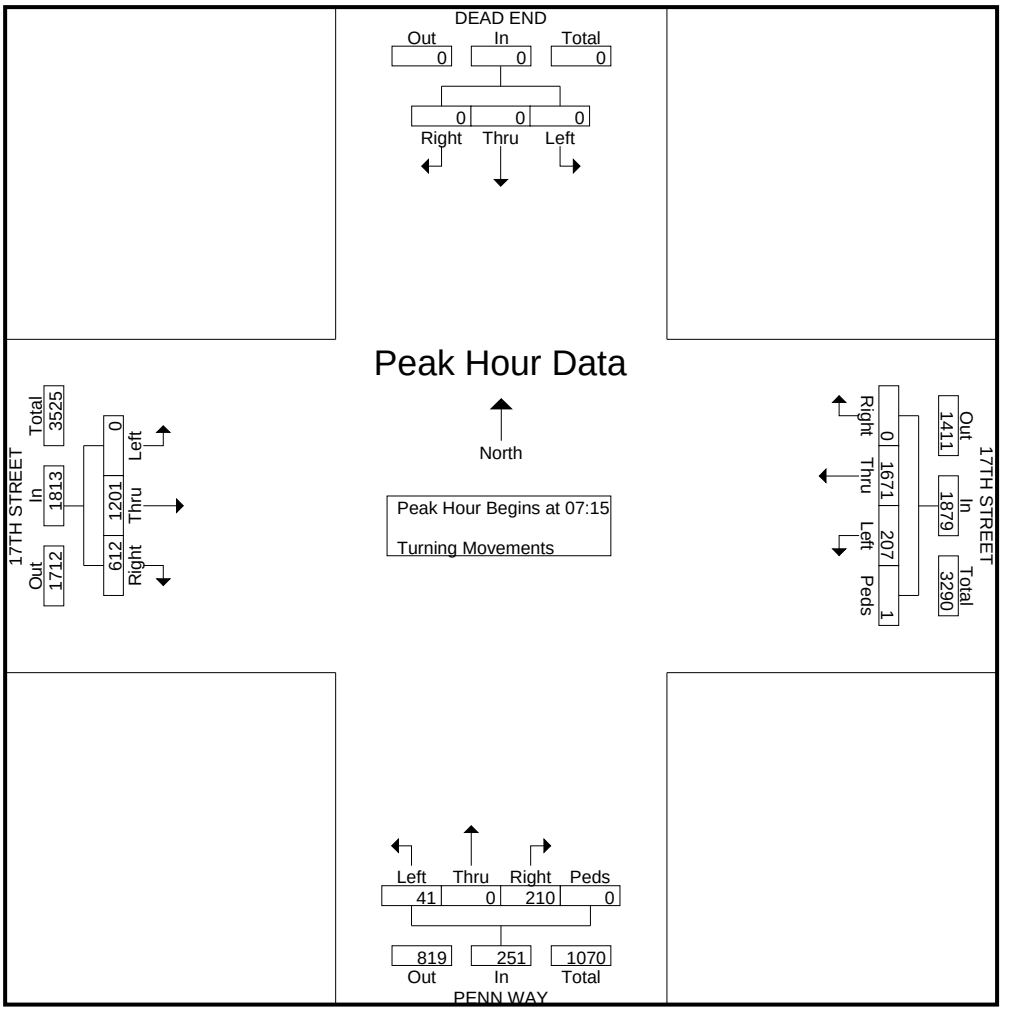
	DEAD END Southbound			17TH STREET Westbound				PENN WAY Northbound				17TH STREET Eastbound			
Start Time	Right	Thru	Left	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Int. Total
07:00	0	0	0	0	298	46	0	44	0	7	0	164	280	0	839
07:15	0	0	0	0	394	55	0	70	0	10	0	163	260	0	952
07:30	0	0	0	0	471	47	0	53	0	6	0	165	321	0	1063
07:45	0	0	0	0	461	68	1	50	0	14	0	162	288	0	1044
Total	0	0	0	0	1624	216	1	217	0	37	0	654	1149	0	3898
08:00	0	0	0	0	345	37	0	37	0	11	0	122	332	0	884
08:15	0	0	0	0	280	37	0	38	0	7	0	137	280	0	779
08:30	0	0	0	0	299	32	0	41	0	7	1	116	255	0	751
08:45	0	0	0	0	307	25	0	52	0	14	0	99	231	0	728
Total	0	0	0	0	1231	131	0	168	0	39	1	474	1098	0	3142
*** BREAK ***															
16:00	0	0	0	0	327	28	1	98	0	23	1	102	326	0	906
16:15	0	0	0	0	405	26	0	78	0	21	0	106	304	0	940
16:30	0	0	0	0	350	40	1	89	0	18	0	111	335	0	944
16:45	0	0	0	0	360	30	0	98	0	19	1	113	329	0	950
Total	0	0	0	0	1442	124	2	363	0	81	2	432	1294	0	3740
17:00	0	0	0	0	335	30	1	108	0	24	0	149	413	0	1060
17:15	0	0	0	0	394	27	0	99	0	27	0	118	388	0	1053
17:30	0	0	0	0	337	35	1	110	0	15	1	122	339	0	960
17:45	0	0	0	0	331	27	0	84	0	17	1	108	403	0	971
Total	0	0	0	0	1397	119	2	401	0	83	2	497	1543	0	4044
Grand Total	0	0	0	0	5694	590	5	1149	0	240	5	2057	5084	0	14824
Apprch %	0	0	0	0	90.5	9.4	0.1	82.4	0	17.2	0.4	28.8	71.2	0	
Total %	0	0	0	0	38.4	4	0	7.8	0	1.6	0	13.9	34.3	0	

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Site Code : 00000000
Start Date : 2/9/2017
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	DEAD END Southbound				17TH STREET Westbound					PENN WAY Northbound					17TH STREET Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																			
Peak Hour for Entire Intersection Begins at 07:15																			
07:15	0	0	0	0	0	394	55	0	449	70				80					
07:30	0	0	0	0	0	471	47	0	518	53	0	6	0	59	165	321	0	**486**	**1063**
07:45	**0**	**0**	**0**	**0**	**0**	**461**	**68**	**1**	**530**	**50**	**0**	**14**	0	64	162	288	0	450	1044
08:00	0	0	0	0	0	345	37	0	382	37	0	11	0	48	122	**332**	0	454	884
Total Volume	0	0	0	0	0	1671	207	1	1879	210	0	41	0	251	612	1201	0	1813	3943
% App. Total	0	0	0		0	88.9	11	0.1		83.7	0	16.3	0		33.8	66.2	0		
PHF	.000	.000	.000	.000	.000	.887	.761	.250	.886	.750	.000	.732	.000	.784	.927	.904	.000	.933	.927

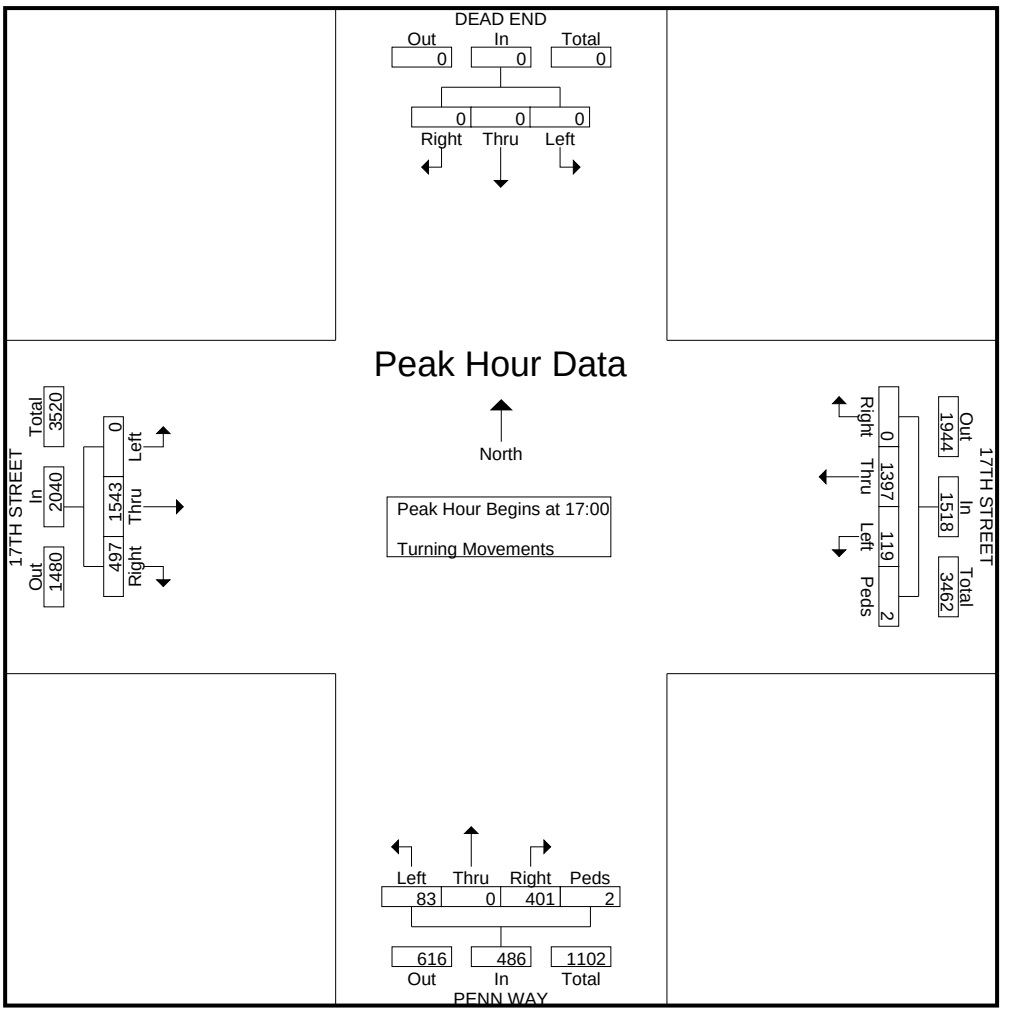


Transportation Studies, Inc.
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Tustin, CA. 92780

City: SANTA ANA
N-S Direction: PENN WAY
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File Name : H1702011
Site Code : 00000000
Start Date : 2/9/2017
Page No : 3

	DEAD END Southbound				17TH STREET Westbound					PENN WAY Northbound					17TH STREET Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1																			
Peak Hour for Entire Intersection Begins at 17:00																			
17:00	0	0	0	0	0	335	30	1	366	108	0	24	0	132	149	413	0	**562**	**1060**
17:15	**0**	**0**	**0**	**0**	**0**	**394**	**27**	**0**	**421**	**99**	**0**	**27**	0	126	118	388	0	506	1053
17:30	0	0	0	0	0	337	**35**	1	373	**110**	0	15	**1**	126	122	339	0	461	960
17:45	0	0	0	0	0	331	27	0	358	84	0	17	1	102	108	403	0	511	971
Total Volume	0	0	0	0	0	1397	119	2	1518	401	0	83	2	486	497	1543	0	2040	4044
% App. Total	0	0	0		0	92	7.8	0.1		82.5	0	17.1	0.4		24.4	75.6	0		
PHF	.000	.000	.000	.000	.000	.886	.850	.500	.901	.911	.000	.769	.500	.920	.834	.934	.000	.907	.954



INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Tue, May 14, 19

LOCATION: Santa Ana
NORTH & SOUTH: I-5 NB Ramps
EAST & WEST: 17th

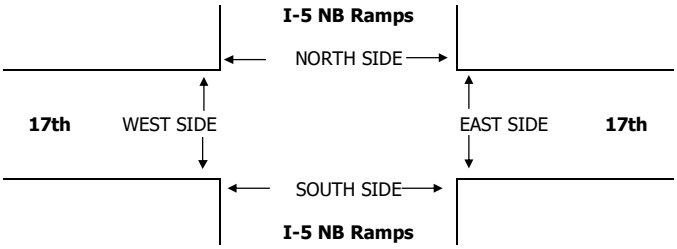
PROJECT #: SC2183
LOCATION #: 30
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND I-5 NB Ramps			SOUTHBOUND I-5 NB Ramps			EASTBOUND 17th			WESTBOUND 17th				U-TURNS				
	LANES:	NL 1.5	NT 0.5	NR 1	SL 1	ST X	SR 1	EL 1	ET 3	ER 1	WL X	WT 3	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	129	11	9	7	0	51	21	188	65	0	200	1	682	0	0	3	0	3
	7:15 AM	180	16	1	15	0	54	15	255	65	0	259	4	864	0	0	5	0	5
	7:30 AM	220	5	10	14	0	69	21	266	91	0	304	7	1,007	0	0	4	0	4
	7:45 AM	201	8	6	15	0	100	29	276	108	0	365	4	1,112	0	0	7	0	7
	8:00 AM	159	5	4	12	0	69	33	232	108	0	304	4	930	0	0	11	0	11
	8:15 AM	151	5	11	9	0	48	20	227	52	0	206	6	735	0	0	2	0	2
	8:30 AM	130	12	11	5	0	45	23	223	60	0	201	8	718	0	0	4	0	4
	8:45 AM	110	8	3	12	0	35	16	192	67	0	202	9	654	0	0	5	0	5
	VOLUMES	1,280	70	55	89	0	471	178	1,859	616	0	2,041	43	6,702	0	0	41	0	41
	APPROACH %	91%	5%	4%	16%	0%	84%	7%	70%	23%	0%	98%	2%						
	APP/DEPART	1,405	/	250	560	/	616	2,653	/	2,003	2,084	/	3,833	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	760	34	21	56	0	292	98	1,029	372	0	1,232	19	3,913					
	APPROACH %	93%	4%	3%	16%	0%	84%	7%	69%	25%	0%	98%	2%						
	PEAK HR FACTOR		0.867			0.757			0.907			0.848		0.880					
	APP/DEPART	815	/	124	348	/	372	1,499	/	1,106	1,251	/	2,311	0					
PM	4:00 PM	81	9	5	5	0	24	26	240	157	0	353	10	910	0	0	8	0	8
	4:15 PM	88	14	4	15	0	33	28	251	128	0	374	9	944	0	0	5	0	5
	4:30 PM	86	8	3	6	0	33	27	228	202	0	367	13	973	0	0	3	0	3
	4:45 PM	83	12	6	9	0	32	29	286	176	0	377	7	1,017	0	0	8	0	8
	5:00 PM	84	11	4	20	0	33	34	323	208	0	391	11	1,119	0	0	6	0	6
	5:15 PM	80	23	4	9	0	42	29	262	141	0	303	9	902	0	0	4	0	4
	5:30 PM	71	14	6	11	0	39	31	241	113	0	361	14	901	0	0	4	0	4
	5:45 PM	94	12	7	17	0	34	29	273	93	0	341	5	905	0	0	10	0	10
	VOLUMES	667	103	39	92	0	270	233	2,104	1,218	0	2,867	78	7,671	0	0	48	0	48
	APPROACH %	82%	13%	5%	25%	0%	75%	7%	59%	34%	0%	97%	3%						
	APP/DEPART	809	/	366	362	/	1,218	3,555	/	2,235	2,945	/	3,852	0					
	BEGIN PEAK HR		4:15 PM																
	VOLUMES	341	45	17	50	0	131	118	1,088	714	0	1,509	40	4,053					
	APPROACH %	85%	11%	4%	28%	0%	72%	6%	57%	37%	0%	97%	3%						
	PEAK HR FACTOR		0.950			0.854			0.850			0.963		0.905					
	APP/DEPART	403	/	181	181	/	714	1,920	/	1,155	1,549	/	2,003	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:15 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	4:15 PM					0	0	0	0	0					

Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
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City: SANTA ANA
N-S Direction: PENN WAY
E-W Direction: I-5 SB RAMPS

File Name : H1702012
Site Code : 00000000
Start Date : 2/9/2017
Page No : 1

Groups Printed- Turning Movements

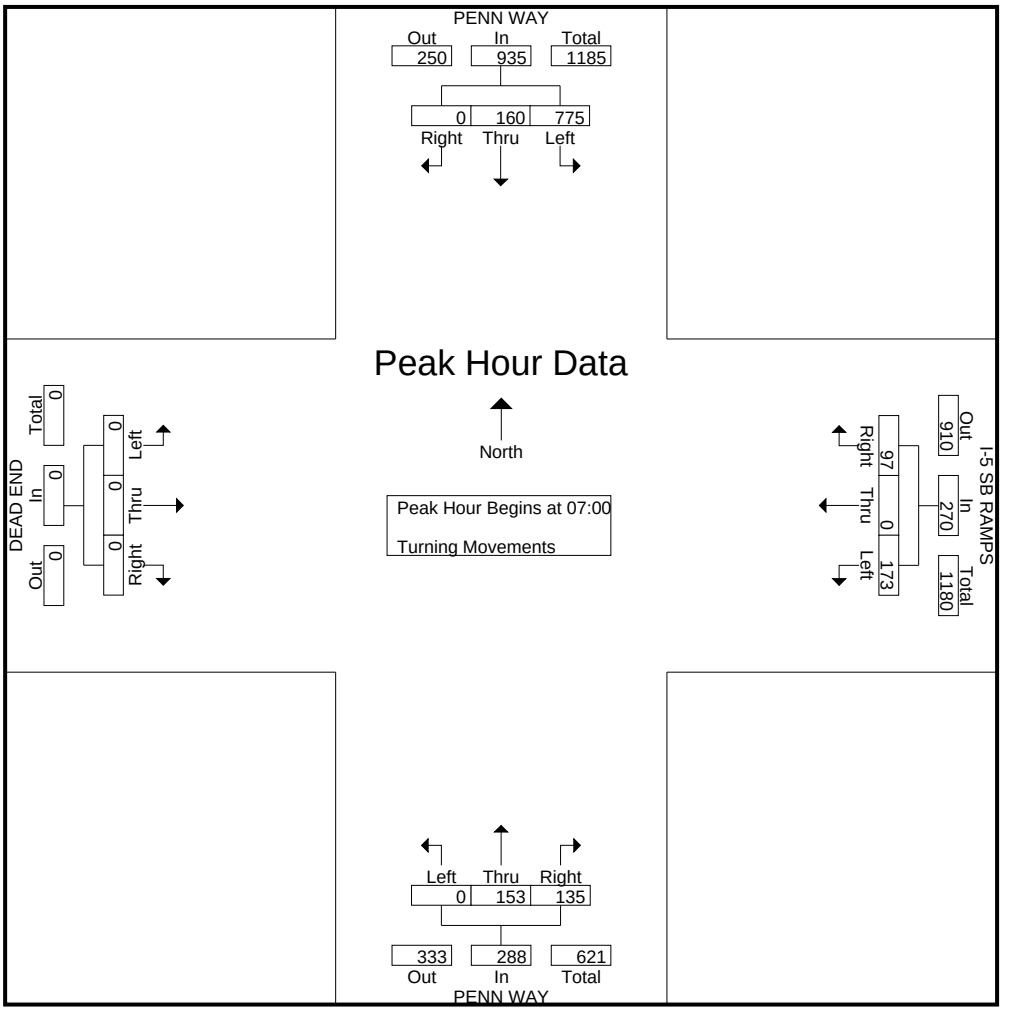
	PENN WAY Southbound			I-5 SB RAMPS Westbound			PENN WAY Northbound			DEAD END Eastbound			
Start Time	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Int. Total
07:00	0	31	198	22	0	44	18	29	0	0	0	0	342
07:15	0	32	196	36	0	39	32	41	0	0	0	0	376
07:30	0	37	191	19	0	38	35	40	0	0	0	0	360
07:45	0	60	190	20	0	52	50	43	0	0	0	0	415
Total	0	160	775	97	0	173	135	153	0	0	0	0	1493
08:00	0	30	144	21	0	48	37	27	0	0	0	0	307
08:15	0	28	153	20	0	17	32	28	0	0	0	0	278
08:30	0	20	139	34	0	21	19	17	0	0	0	0	250
08:45	0	18	113	31	0	23	18	33	0	0	0	0	236
Total	0	96	549	106	0	109	106	105	0	0	0	0	1071
*** BREAK ***													
16:00	0	25	111	57	0	40	44	64	0	0	0	0	341
16:15	0	22	114	45	0	51	37	59	0	0	0	0	328
16:30	0	33	105	31	0	46	69	74	0	0	0	0	358
16:45	0	30	130	43	0	52	51	70	0	0	0	0	376
Total	0	110	460	176	0	189	201	267	0	0	0	0	1403
17:00	0	32	152	40	0	41	77	94	0	0	0	0	436
17:15	0	30	119	34	0	37	63	92	0	0	0	0	375
17:30	0	31	132	28	0	39	40	96	0	0	0	0	366
17:45	0	39	112	27	0	29	22	74	0	0	0	0	303
Total	0	132	515	129	0	146	202	356	0	0	0	0	1480
Grand Total	0	498	2299	508	0	617	644	881	0	0	0	0	5447
Apprch %	0	17.8	82.2	45.2	0	54.8	42.2	57.8	0	0	0	0	
Total %	0	9.1	42.2	9.3	0	11.3	11.8	16.2	0	0	0	0	

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2640 Walnut Avenue, Suite L
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City: SANTA ANA
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File Name : H1702012
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Start Date : 2/9/2017
Page No : 2

	PENN WAY Southbound				I-5 SB RAMPS Westbound				PENN WAY Northbound				DEAD END Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:00																	
07:00	0	31	**198**	229	22	0	44	66	18	29	0	47	0	0	0	0	342
07:15	0	32	196	228	**36**	0	39	**75**	32	41	0	73	0	0	0	0	376
07:30	0	37	191	228	19	0	38	57	35	40	0	75	0	0	0	0	360
07:45	0	**60**	190	**250**	20	0	**52**	72	**50**	**43**	0	**93**	0	0	0	0	**415**
Total Volume	0	160	775	935	97	0	173	270	135	153	0	288	0	0	0	0	1493
% App. Total	0	17.1	82.9		35.9	0	64.1		46.9	53.1	0		0	0	0		
PHF	.000	.667	.979	.935	.674	.000	.832	.900	.675	.890	.000	.774	.000	.000	.000	.000	.899

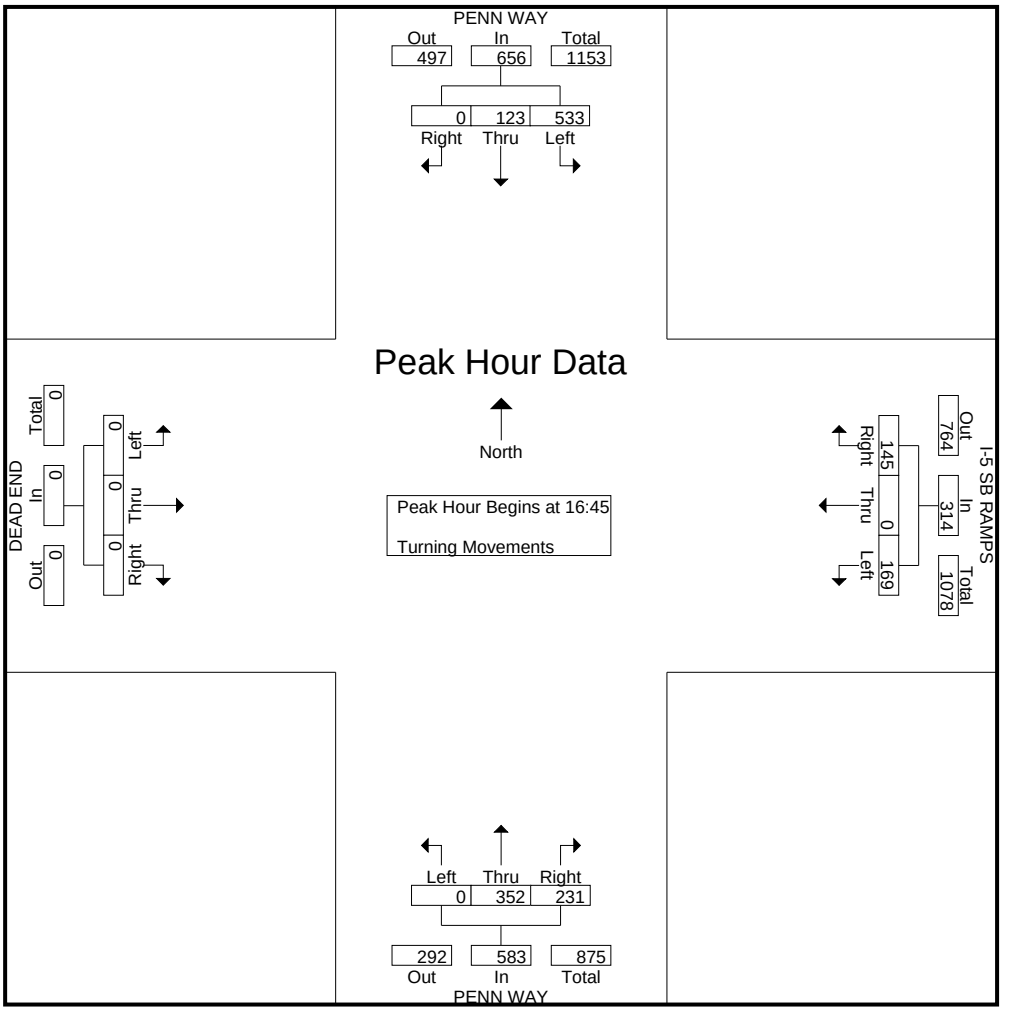


Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: PENN WAY
E-W Direction: I-5 SB RAMPS

File Name : H1702012
Site Code : 00000000
Start Date : 2/9/2017
Page No : 3

	PENN WAY Southbound				I-5 SB RAMPS Westbound				PENN WAY Northbound				DEAD END Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 16:45																	
16:45	0	30	130	160	**43**	0	**52**	**95**	51	70	0	121	0	0	0	0	376
17:00	0	**32**	**152**	**184**	40	0	41	81	**77**	94	0	**171**	0	0	0	0	**436**
17:15	0	30	119	149	34	0	37	71	63	92	0	155	0	0	0	0	375
17:30	0	31	132	163	28	0	39	67	40	**96**	0	136	0	0	0	0	366
Total Volume	0	123	533	656	145	0	169	314	231	352	0	583	0	0	0	0	1553
% App. Total	0	18.8	81.2		46.2	0	53.8		39.6	60.4	0		0	0	0		
PHF	.000	.961	.877	.891	.843	.000	.813	.826	.750	.917	.000	.852	.000	.000	.000	.000	.890



Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: SANTIAGO STREET
E-W Direction: CIVIC CENTER DRIVE

File Name : H1702013
Site Code : 00000000
Start Date : 2/9/2017
Page No : 1

Groups Printed- Turning Movements

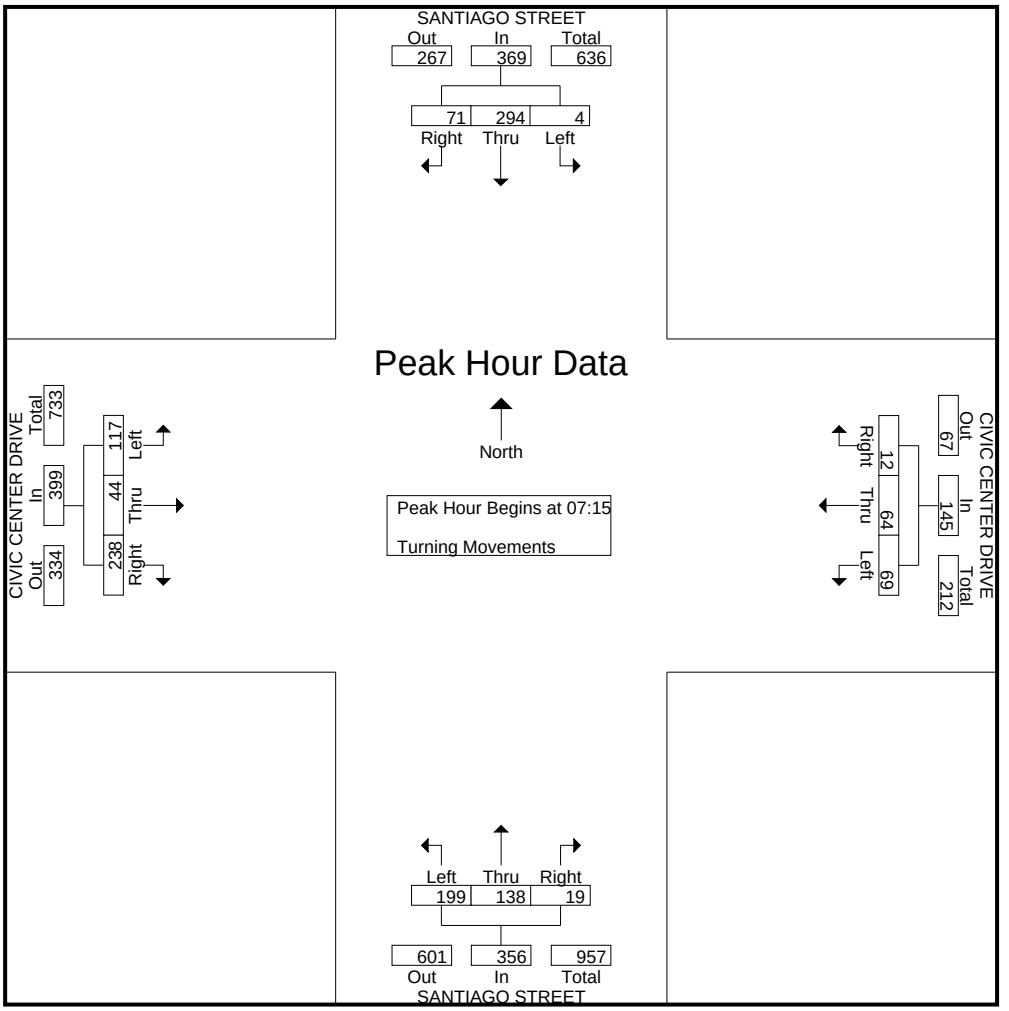
	SANTIAGO STREET Southbound			CIVIC CENTER DRIVE Westbound			SANTIAGO STREET Northbound			CIVIC CENTER DRIVE Eastbound			
Start Time	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Int. Total
07:00	12	60	0	1	6	8	1	22	15	33	4	16	178
07:15	15	73	0	3	12	14	1	20	37	47	9	30	261
07:30	22	82	0	4	14	14	6	41	55	67	11	31	347
07:45	16	70	3	2	23	25	6	44	69	63	13	29	363
Total	65	285	3	10	55	61	14	127	176	210	37	106	1149
08:00	18	69	1	3	15	16	6	33	38	61	11	27	298
08:15	9	35	1	1	1	7	5	19	29	27	8	12	154
08:30	5	44	1	3	1	6	3	8	18	26	9	9	133
08:45	5	43	0	0	1	4	3	18	25	20	6	15	140
Total	37	191	3	7	18	33	17	78	110	134	34	63	725
*** BREAK ***													
16:00	5	52	0	1	4	3	3	46	18	45	6	37	220
16:15	10	41	2	2	3	4	4	59	15	32	8	33	213
16:30	7	48	1	2	3	1	2	48	26	44	12	64	258
16:45	14	47	2	5	3	3	12	59	27	68	20	67	327
Total	36	188	5	10	13	11	21	212	86	189	46	201	1018
17:00	16	38	3	6	8	2	8	70	25	90	20	77	363
17:15	6	47	3	4	4	2	5	76	24	54	16	66	307
17:30	10	49	4	2	3	6	4	85	18	49	17	48	295
17:45	10	51	1	1	6	3	6	65	22	37	11	50	263
Total	42	185	11	13	21	13	23	296	89	230	64	241	1228
Grand Total	180	849	22	40	107	118	75	713	461	763	181	611	4120
Apprch %	17.1	80.8	2.1	15.1	40.4	44.5	6	57.1	36.9	49.1	11.6	39.3	
Total %	4.4	20.6	0.5	1	2.6	2.9	1.8	17.3	11.2	18.5	4.4	14.8	

Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: SANTIAGO STREET
E-W Direction: CIVIC CENTER DRIVE

File Name : H1702013
Site Code : 00000000
Start Date : 2/9/2017
Page No : 2

	SANTIAGO STREET Southbound				CIVIC CENTER DRIVE Westbound				SANTIAGO STREET Northbound				CIVIC CENTER DRIVE Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:15																	
07:15	15	73	0	88	3	12	14	29	1	20	37	58	47	9	30	86	261
07:30	**22**	**82**	0	**104**	**4**	14	14	32	**6**	41	55	102	**67**	11	**31**	**109**	347
07:45	16	70	**3**	89	2	**23**	**25**	**50**	6	**44**	**69**	**119**	63	**13**	29	105	**363**
08:00	18	69	1	88	3	15	16	34	6	33	38	77	61	11	27	99	298
Total Volume	71	294	4	369	12	64	69	145	19	138	199	356	238	44	117	399	1269
% App. Total	19.2	79.7	1.1		8.3	44.1	47.6		5.3	38.8	55.9		59.6	11	29.3		
PHF	.807	.896	.333	.887	.750	.696	.690	.725	.792	.784	.721	.748	.888	.846	.944	.915	.874

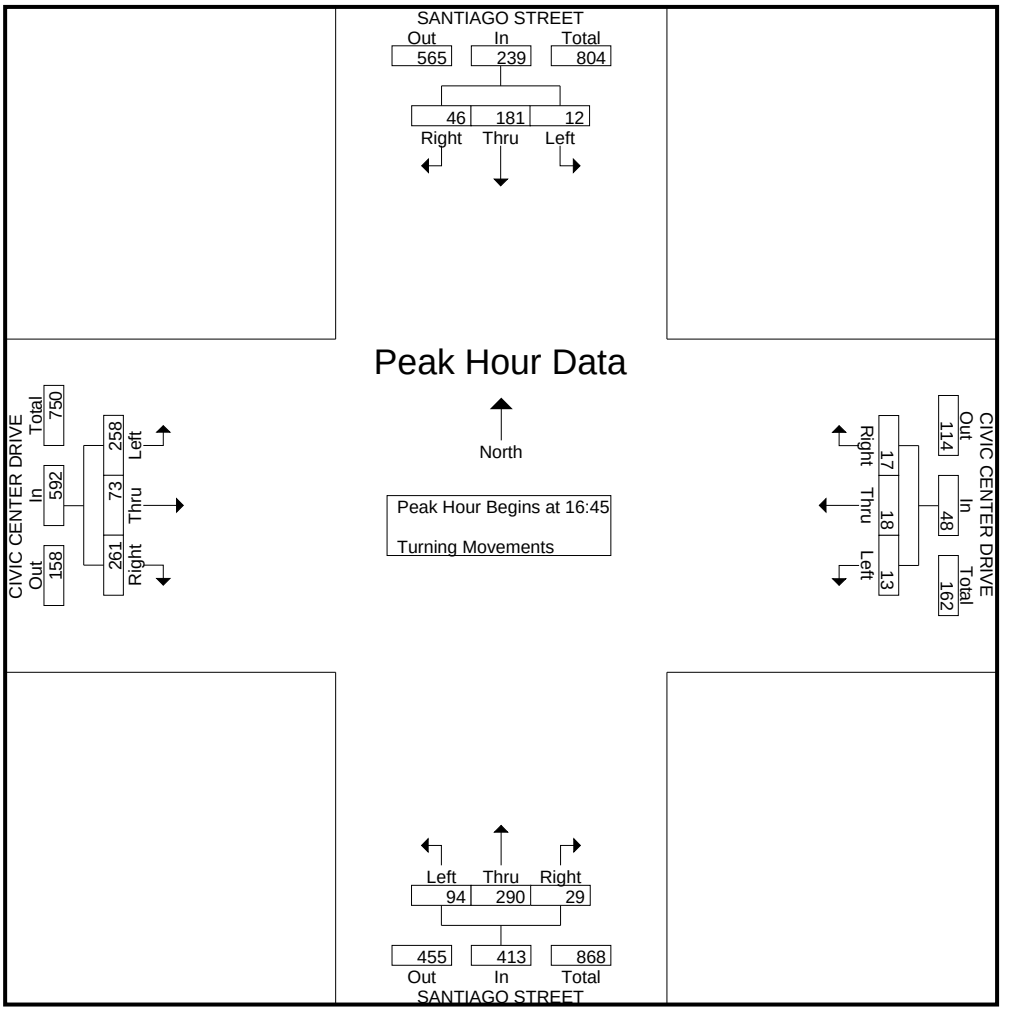


Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: SANTIAGO STREET
E-W Direction: CIVIC CENTER DRIVE

File Name : H1702013
Site Code : 00000000
Start Date : 2/9/2017
Page No : 3

	SANTIAGO STREET Southbound				CIVIC CENTER DRIVE Westbound				SANTIAGO STREET Northbound				CIVIC CENTER DRIVE Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 16:45																	
16:45	14	47	2	**63**	5	3	3	11	**12**	59	**27**	98	68	**20**	67	155	327
17:00	**16**	38	3	57	**6**	**8**	2	**16**	8	70	25	103	**90**	20	**77**	**187**	**363**
17:15	6	47	3	56	4	4	2	10	5	76	24	105	54	16	66	136	307
17:30	10	**49**	**4**	63	2	3	**6**	11	4	**85**	18	**107**	49	17	48	114	295
Total Volume	46	181	12	239	17	18	13	48	29	290	94	413	261	73	258	592	1292
% App. Total	19.2	75.7	5		35.4	37.5	27.1		7	70.2	22.8		44.1	12.3	43.6		
PHF	.719	.923	.750	.948	.708	.563	.542	.750	.604	.853	.870	.965	.725	.913	.838	.791	.890



INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

DATE: Thu, Feb 11, 16

LOCATION: Santa Ana
NORTH & SOUTH: Santiago
EAST & WEST: Santa Ana

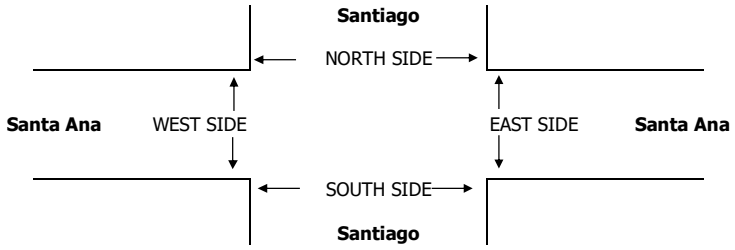
PROJECT #: SC0846
LOCATION #: 15
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Santiago			SOUTHBOUND Santiago			EASTBOUND Santa Ana			WESTBOUND Santa Ana				U-TURNS				
	LANES:	NL 1	NT 1	NR 1	SL 1	ST 1	SR 1	EL 1	ET 2	ER 0	WL 1	WT 2	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	6	12	16	52	27	20	6	77	5	32	137	34	424	0	0	0	9	9
	7:15 AM	7	11	18	69	31	24	3	128	1	17	162	41	512	0	0	0	6	6
	7:30 AM	5	17	9	85	31	36	11	139	4	31	212	67	647	0	0	0	12	12
	7:45 AM	2	16	11	62	62	32	24	104	1	33	250	102	699	0	0	0	5	5
	8:00 AM	9	24	15	82	30	35	8	84	7	22	151	50	517	0	0	0	6	6
	8:15 AM	4	14	19	46	21	16	9	68	4	16	148	35	400	0	0	0	1	1
	8:30 AM	2	12	17	39	15	18	1	69	2	20	139	27	361	0	0	0	2	2
	8:45 AM	2	7	19	40	22	15	1	48	9	17	123	25	328	0	0	0	2	2
	VOLUMES	37	113	124	475	239	196	63	717	33	188	1,322	381	3,888	0	0	0	43	43
	APPROACH %	14%	41%	45%	52%	26%	22%	8%	88%	4%	10%	70%	20%						
	APP/DEPART	274	/	557	910	/	417	813	/	1,359	1,891	/	1,555	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	23	68	53	298	154	127	46	455	13	103	775	260	2,375					
	APPROACH %	16%	47%	37%	51%	27%	22%	9%	89%	3%	9%	68%	23%						
	PEAK HR FACTOR		0.750			0.928			0.834			0.739		0.849					
	APP/DEPART	144	/	374	579	/	241	514	/	835	1,138	/	925	0					
PM	4:00 PM	3	28	16	60	21	15	7	109	12	22	90	32	415	0	0	0	6	6
	4:15 PM	3	21	14	45	24	25	6	127	1	17	77	29	389	0	0	0	4	4
	4:30 PM	8	20	16	61	23	15	21	147	2	21	123	55	512	0	0	0	3	3
	4:45 PM	5	30	14	77	29	14	17	159	8	16	132	46	547	0	0	0	5	5
	5:00 PM	8	38	24	70	28	10	16	175	4	17	102	34	526	0	0	0	8	8
	5:15 PM	13	39	26	77	35	19	14	142	5	16	115	44	545	0	0	0	2	2
	5:30 PM	6	22	19	55	31	22	13	93	6	18	111	26	422	0	0	0	8	8
	5:45 PM	11	42	17	62	29	24	9	90	3	23	101	51	462	0	0	0	10	10
	VOLUMES	57	240	146	507	220	144	103	1,042	41	150	851	317	3,818	0	0	0	46	46
	APPROACH %	13%	54%	33%	58%	25%	17%	9%	88%	3%	11%	65%	24%						
	APP/DEPART	443	/	660	871	/	365	1,186	/	1,741	1,318	/	1,052	0					
	BEGIN PEAK HR		4:30 PM																
	VOLUMES	34	127	80	285	115	58	68	623	19	70	472	179	2,130					
	APPROACH %	14%	53%	33%	62%	25%	13%	10%	88%	3%	10%	65%	25%						
	PEAK HR FACTOR		0.772			0.874			0.910			0.906		0.973					
	APP/DEPART	241	/	374	458	/	186	710	/	1,006	721	/	564	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	9	18	12	8	47	9	16	9	8	42	0	2	3	0	5
	7:15 AM	14	6	8	6	34	11	4	7	6	28	3	2	1	0	6
	7:30 AM	9	4	8	7	28	9	4	6	7	26	0	0	2	0	2
	7:45 AM	1	4	5	2	12	1	4	5	2	12	0	0	0	0	0
	8:00 AM	1	1	6	1	9	1	0	5	1	7	0	1	1	0	2
	8:15 AM	0	1	2	1	4	0	0	0	1	1	0	1	2	0	3
	8:30 AM	2	1	2	2	7	1	1	2	2	6	1	0	0	0	1
	8:45 AM	0	6	3	3	12	0	5	1	3	9	0	1	2	0	3
	TOTAL	36	41	46	30	153	32	34	35	30	131	4	7	11	0	22
PM	4:00 PM	4	6	5	5	20	0	5	4	3	12	4	1	1	2	8
	4:15 PM	0	4	4	1	9	0	4	3	1	8	0	0	1	0	1
	4:30 PM	4	2	5	1	12	4	2	5	1	12	0	0	0	0	0
	4:45 PM	2	2	5	0	9	0	2	3	0	5	2	0	2	0	4
	5:00 PM	3	15	8	4	30	2	13	8	2	25	1	2	0	2	5
	5:15 PM	3	10	7	10	30	3	9	7	9	28	0	1	0	1	2
	5:30 PM	2	8	9	7	26	1	7	7	6	21	1	1	2	1	5
	5:45 PM	1	0	1	6	8	1	0	1	5	7	0	0	0	1	1
	TOTAL	19	47	44	34	144	11	42	38	27	118	8	5	6	7	26

Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: STANDARD AVENUE
E-W Direction: 1ST STREET

File Name : H1702014
Site Code : 00000000
Start Date : 2/9/2017
Page No : 1

Groups Printed- Turning Movements

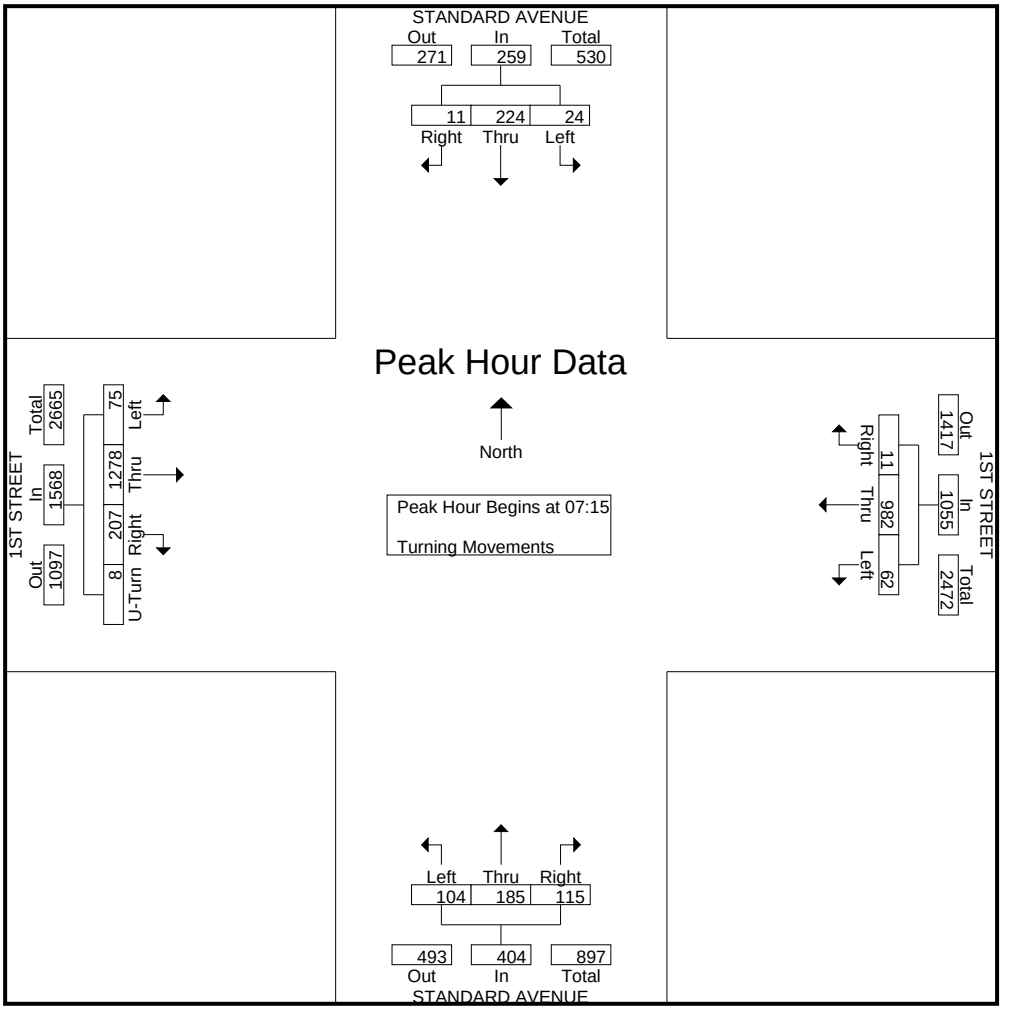
	STANDARD AVENUE Southbound			1ST STREET Westbound			STANDARD AVENUE Northbound			1ST STREET Eastbound				
Start Time	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	U-Turn	Int. Total
07:00	0	41	6	1	179	10	28	43	17	50	282	10	2	669
07:15	3	49	7	1	228	10	19	42	23	39	376	21	2	820
07:30	4	61	3	5	255	14	43	55	28	60	324	20	2	874
07:45	3	65	8	2	282	19	27	47	36	70	291	19	3	872
Total	10	216	24	9	944	53	117	187	104	219	1273	70	9	3235
08:00	1	49	6	3	217	19	26	41	17	38	287	15	1	720
08:15	4	47	3	4	204	13	19	41	20	52	333	8	4	752
08:30	7	19	4	2	177	5	14	32	21	37	284	8	1	611
08:45	2	29	2	5	171	19	22	31	17	22	304	9	3	636
Total	14	144	15	14	769	56	81	145	75	149	1208	40	9	2719
*** BREAK ***														
16:00	5	41	5	3	251	17	19	78	53	31	272	22	2	799
16:15	4	39	2	4	251	23	34	98	43	36	294	19	2	849
16:30	2	37	5	4	243	17	30	79	44	28	311	20	2	822
16:45	2	33	1	6	265	29	23	79	45	26	297	26	4	836
Total	13	150	13	17	1010	86	106	334	185	121	1174	87	10	3306
17:00	8	45	9	6	246	24	51	93	35	30	311	24	6	888
17:15	2	48	2	4	297	21	23	81	39	34	300	37	2	890
17:30	5	50	3	7	281	24	13	98	49	37	290	42	15	914
17:45	7	40	4	3	290	19	28	84	56	24	281	33	12	881
Total	22	183	18	20	1114	88	115	356	179	125	1182	136	35	3573
Grand Total	59	693	70	60	3837	283	419	1022	543	614	4837	333	63	12833
Apprch %	7.2	84.3	8.5	1.4	91.8	6.8	21.1	51.5	27.4	10.5	82.7	5.7	1.1	
Total %	0.5	5.4	0.5	0.5	29.9	2.2	3.3	8	4.2	4.8	37.7	2.6	0.5	

Transportation Studies, Inc.
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City: SANTA ANA
N-S Direction: STANDARD AVENUE
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File Name : H1702014
Site Code : 00000000
Start Date : 2/9/2017
Page No : 2

	STANDARD AVENUE Southbound				1ST STREET Westbound				STANDARD AVENUE Northbound				1ST STREET Eastbound					
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																		
Peak Hour for Entire Intersection Begins at 07:15																		
07:15	3	49	7	59	1	228	10	239	19	42	23	84	39	376	21	2	438	820
07:30	4	61	3	68	**5**	255	14	274	**43**	**55**	28	**126**	60	324	20	2	406	**874**
07:45	**3**	**65**	**8**	**76**	**2**	**282**	**19**	**303**	**27**	**47**	**36**	110	**70**	291	19	**3**	383	872
08:00	1	49	6	56	3	217	19	239	26	41	17	84	38	287	15	1	341	720
Total Volume	11	224	24	259	11	982	62	1055	115	185	104	404	207	1278	75	8	1568	3286
% App. Total	4.2	86.5	9.3		1	93.1	5.9		28.5	45.8	25.7		13.2	81.5	4.8	0.5		
PHF	.688	.862	.750	.852	.550	.871	.816	.870	.669	.841	.722	.802	.739	.850	.893	.667	.895	.940

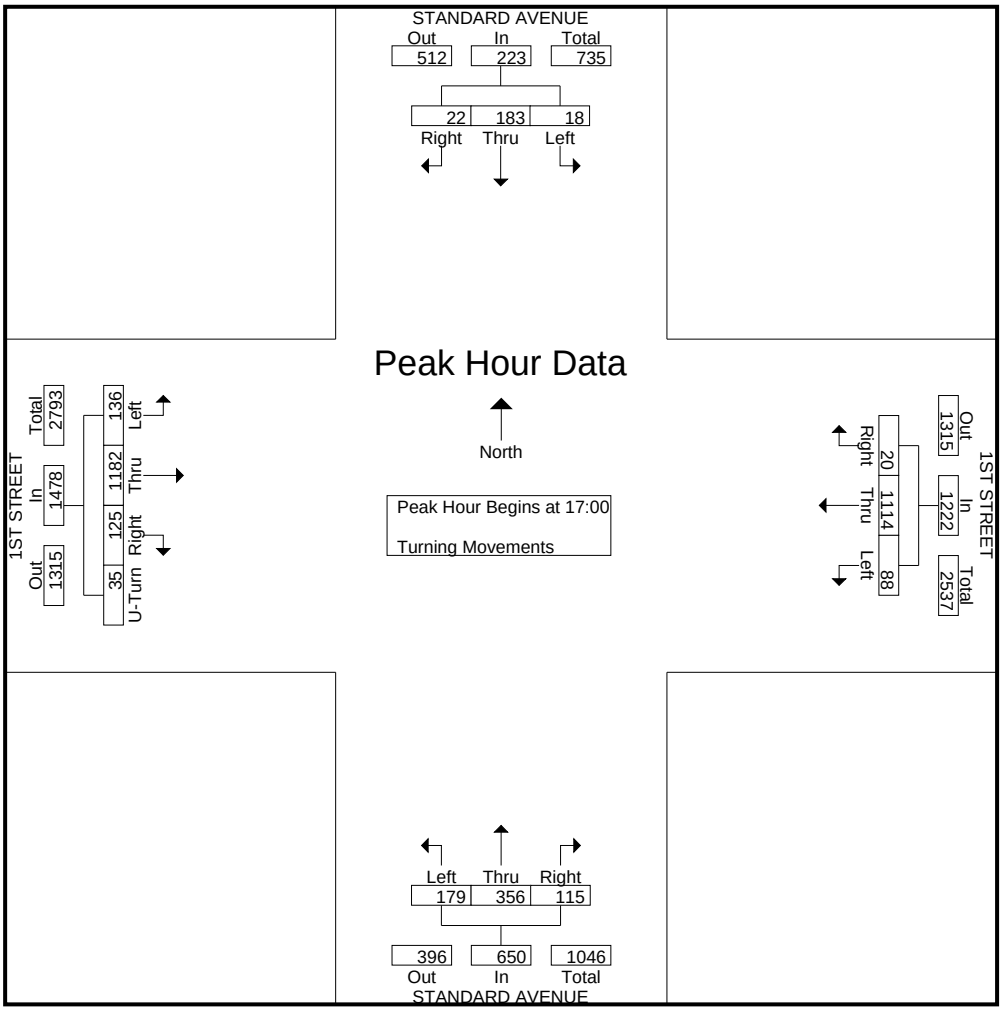


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City: SANTA ANA
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File Name : H1702014
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	STANDARD AVENUE Southbound				1ST STREET Westbound				STANDARD AVENUE Northbound				1ST STREET Eastbound					
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	U-Turn	App. Total	Int. Total
Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1																		
Peak Hour for Entire Intersection Begins at 17:00																		
17:00	8	45	9	62	6	246	24	276	**51**	93	35	**179**	30	**311**	24	6	371	888
17:15	2	48	2	52	4	**297**	21	**322**										
17:30	**5**	**50**	3	58	**7**	281	24	312	13	**98**	49	160	**37**	290	**42**	**15**	**384**	**914**
17:45	**7**	**40**	**4**	**51**	**3**	**290**	**19**	**312**	**28**	**84**	**56**	168	24	281	33	12	350	881
Total Volume	22	183	18	223	20	1114	88	1222	115	356	179	650	125	1182	136	35	1478	3573
% App. Total	9.9	82.1	8.1		1.6	91.2	7.2		17.7	54.8	27.5		8.5	80	9.2	2.4		
PHF	.688	.915	.500	.899	.714	.938	.917	.949	.564	.908	.799	.908	.845	.950	.810	.583	.962	.977



INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Tue, May 15, 18

LOCATION: Santa Ana Zed
NORTH & SOUTH: Standard
EAST & WEST: McFadden

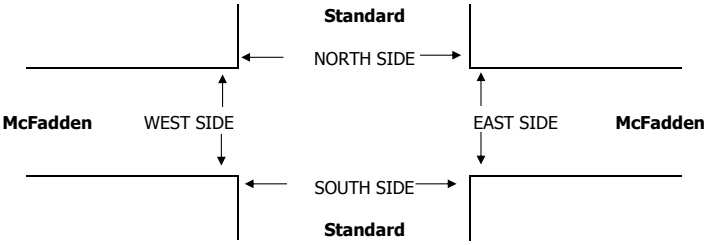
PROJECT #: SC1791
LOCATION #: 63
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

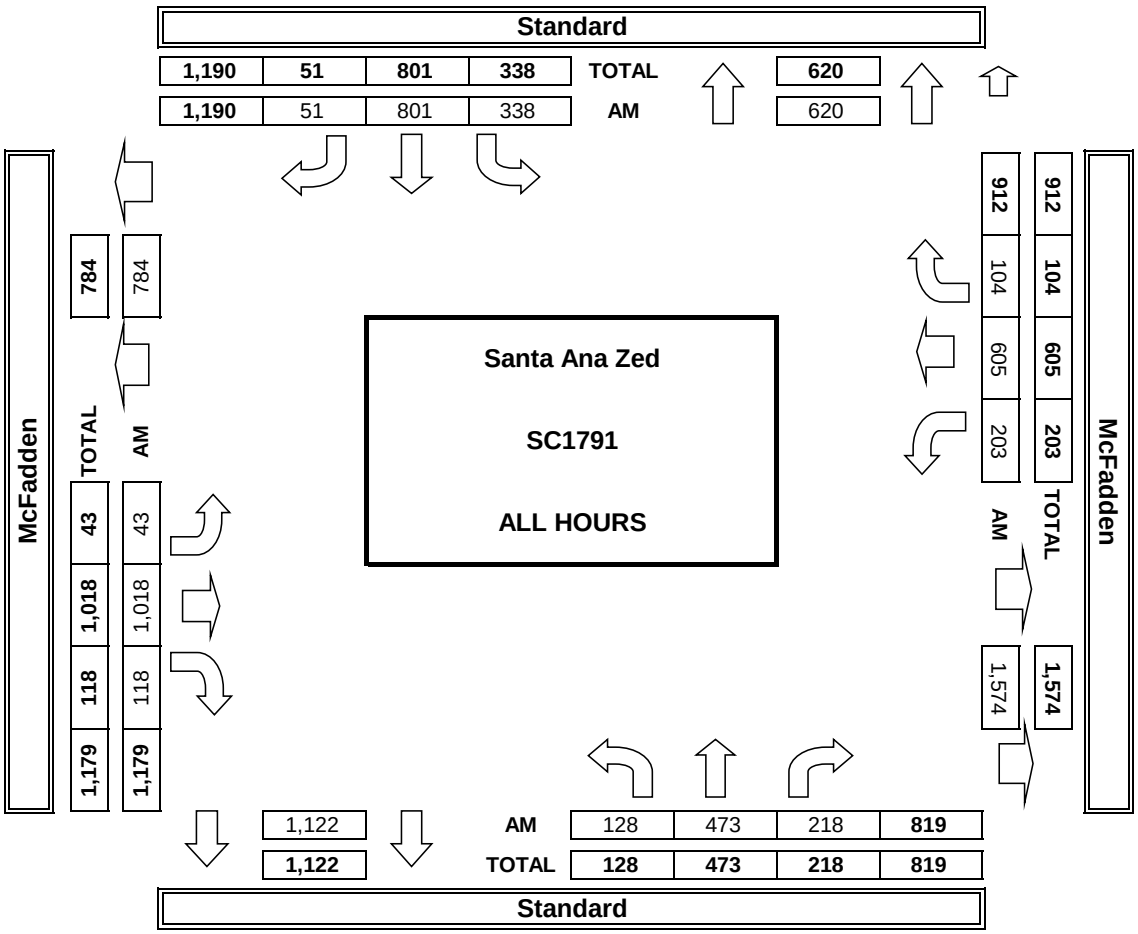
		NORTHBOUND Standard			SOUTHBOUND Standard			EASTBOUND McFadden			WESTBOUND McFadden				U-TURNS				
	LANES:	NL 1	NT 2	NR 0	SL 1	ST 2	SR 0	EL 1	ET 2	ER 0	WL 1	WT 2	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	9	50	30	49	91	6	5	131	15	18	62	9	475	0	0	0	0	0
	7:15 AM	16	62	34	45	84	13	5	146	17	27	76	12	537	0	0	0	0	0
	7:30 AM	18	88	35	58	153	10	3	162	21	25	80	19	672	0	0	0	0	0
	7:45 AM	16	68	27	67	161	7	5	140	15	47	102	23	678	0	0	0	0	0
	8:00 AM	22	60	29	45	122	6	3	129	16	36	100	26	594	0	0	0	0	0
	8:15 AM	17	53	25	26	75	5	11	135	10	23	75	7	462	0	0	0	0	0
	8:30 AM	16	48	18	23	57	3	7	84	12	16	51	2	337	0	0	0	0	0
	8:45 AM	14	44	20	25	58	1	4	91	12	11	59	6	345	0	0	0	0	0
	VOLUMES	128	473	218	338	801	51	43	1,018	118	203	605	104	4,100	0	0	0	0	0
	APPROACH %	16%	58%	27%	28%	67%	4%	4%	86%	10%	22%	66%	11%						
	APP/DEPART	819	/	620	1,190	/	1,122	1,179	/	1,574	912	/	784	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	72	278	125	215	520	36	16	577	69	135	358	80	2,481					
	APPROACH %	15%	59%	26%	28%	67%	5%	2%	87%	10%	24%	62%	14%						
	PEAK HR FACTOR		0.842			0.820			0.890			0.833		0.915					
	APP/DEPART	475	/	374	771	/	724	662	/	917	573	/	466	0					

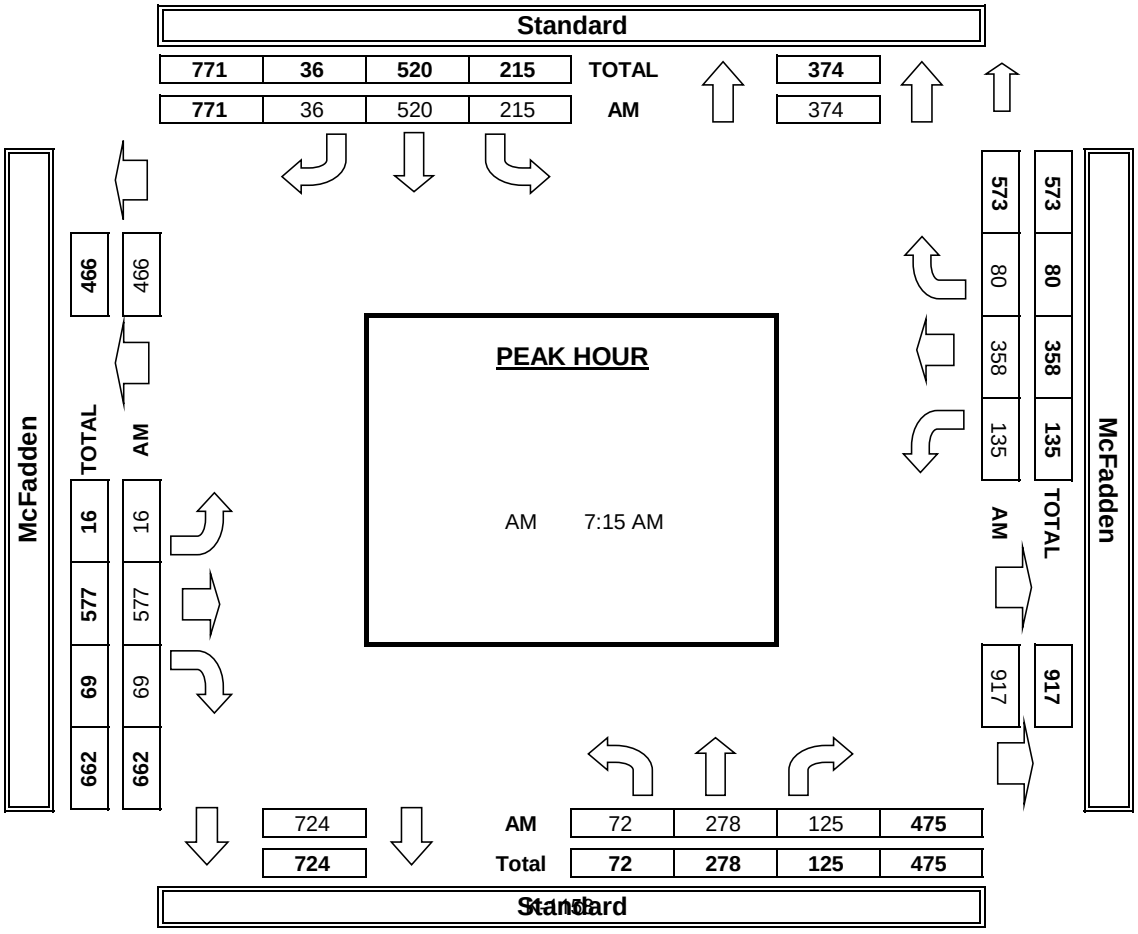


		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	9	22	18	13	62	9	19	17	9	54	0	3	1	4	8
	7:15 AM	20	26	39	10	95	19	24	37	8	88	1	2	2	2	7
	7:30 AM	19	32	25	27	103	16	31	21	23	91	3	1	4	4	12
	7:45 AM	12	43	55	29	139	10	42	51	28	131	2	1	4	1	8
	8:00 AM	8	27	24	19	78	7	26	21	19	73	1	1	3	0	5
	8:15 AM	17	19	16	13	65	16	19	15	10	60	1	0	1	3	5
	8:30 AM	7	20	13	13	53	6	19	12	11	48	1	1	1	2	5
	8:45 AM	6	13	4	8	31	5	12	3	8	28	1	1	1	0	3
	TOTAL	98	202	194	132	626	88	192	177	116	573	10	10	17	16	53
	AM BEGIN PEAK HR			7:15 AM			52	123	130	78	383					

AimTD LLC

TURNING MOVEMENT COUNTS





INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Tue, May 15, 18

LOCATION: Santa Ana Zed
NORTH & SOUTH: Standard
EAST & WEST: McFadden

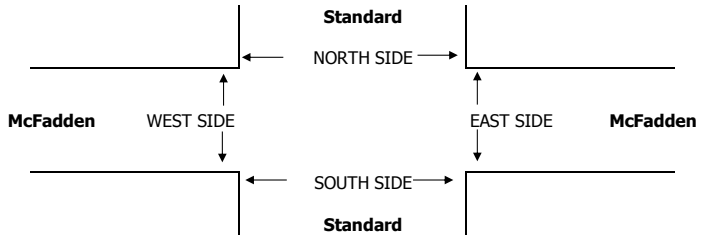
PROJECT #: SC1791
LOCATION #: 63
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

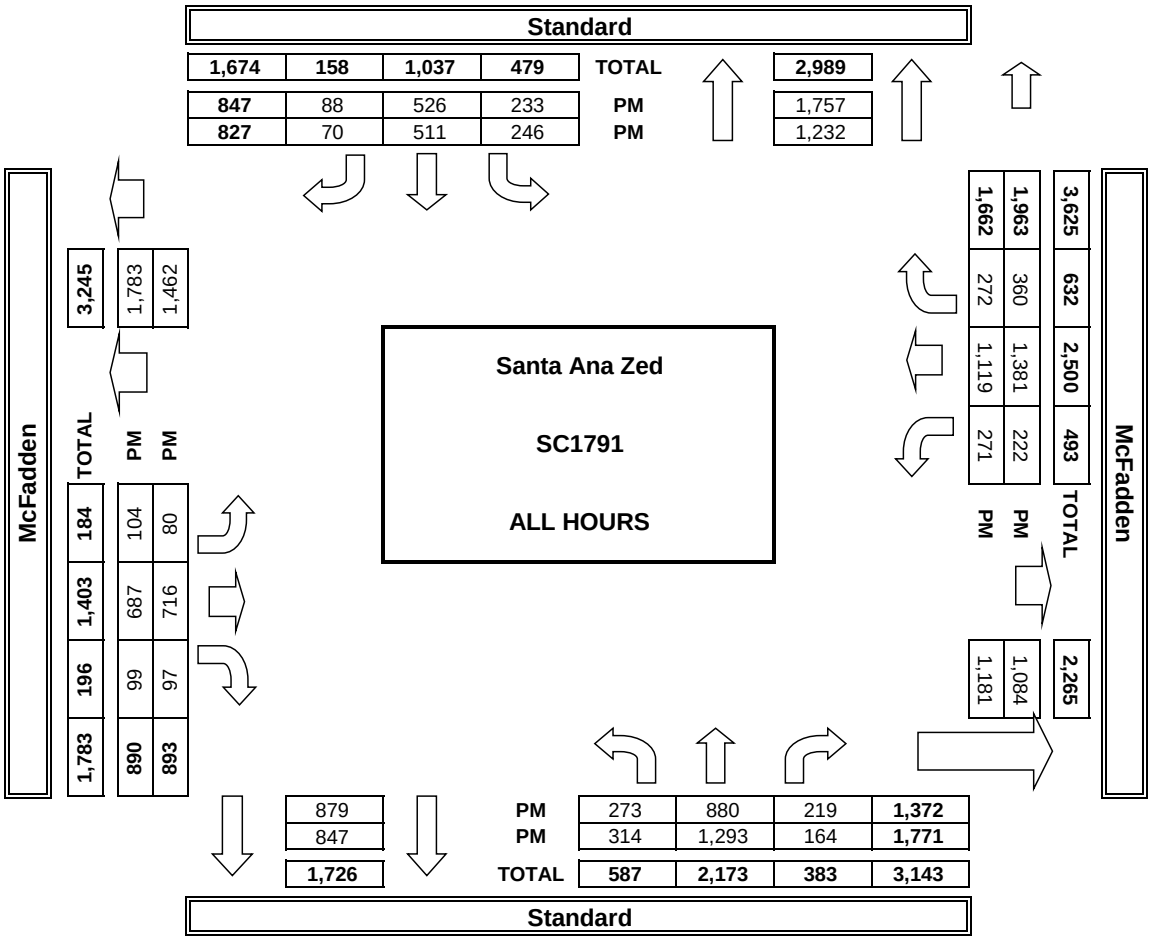
		NORTHBOUND Standard			SOUTHBOUND Standard			EASTBOUND McFadden			WESTBOUND McFadden				U-TURNS				
	LANES:	NL 1	NT 2	NR 0	SL 1	ST 2	SR 0	EL 1	ET 2	ER 0	WL 1	WT 2	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
PM	2:00 PM	28	66	29	29	42	11	6	71	13	17	110	21	443	0	0	0	0	0
	2:15 PM	19	79	21	28	57	12	9	102	16	26	100	19	488	0	0	0	0	0
	2:30 PM	39	86	34	23	66	9	12	89	10	33	147	26	574	0	0	0	0	0
	2:45 PM	42	111	27	33	65	12	10	99	10	43	146	31	629	0	0	0	0	0
	3:00 PM	35	131	33	31	63	7	16	94	15	46	149	55	675	0	0	0	0	0
	3:15 PM	38	125	28	28	53	7	5	96	14	34	124	34	586	0	0	0	0	0
	3:30 PM	29	136	23	36	89	6	11	82	13	36	167	40	668	0	0	0	0	0
	3:45 PM	43	146	24	38	76	6	11	83	6	36	176	46	691	0	0	0	0	0
	VOLUMES	273	880	219	246	511	70	80	716	97	271	1,119	272	4,754	0	0	0	0	0
	APPROACH %	20%	64%	16%	30%	62%	8%	9%	80%	11%	16%	67%	16%						
	APP/DEPART	1,372	/	1,232	827	/	879	893	/	1,181	1,662	/	1,462	0					
	BEGIN PEAK HR		3:00 PM																
	VOLUMES	145	538	108	133	281	26	43	355	48	152	616	175	2,620					
	APPROACH %	18%	68%	14%	30%	64%	6%	10%	80%	11%	16%	65%	19%						
	PEAK HR FACTOR		0.928			0.840			0.892			0.914		0.948					
	APP/DEPART	791	/	756	440	/	481	446	/	596	943	/	787	0					
PM	4:00 PM	45	158	25	24	71	8	16	90	10	27	167	45	686	0	0	0	0	0
	4:15 PM	37	137	22	26	59	6	9	82	11	28	173	23	613	0	0	0	0	0
	4:30 PM	40	171	21	28	51	10	10	87	7	27	183	44	679	0	0	0	0	0
	4:45 PM	27	174	16	21	51	14	18	84	13	26	161	47	652	0	0	0	0	0
	5:00 PM	41	159	25	28	60	7	17	81	13	28	170	46	675	0	0	0	0	0
	5:15 PM	51	182	21	36	70	12	7	92	15	29	196	57	768	0	0	0	0	0
	5:30 PM	37	167	19	33	82	19	17	81	17	29	176	47	724	0	0	0	0	0
	5:45 PM	36	145	15	37	82	12	10	90	13	28	155	51	674	0	0	0	0	0
	VOLUMES	314	1,293	164	233	526	88	104	687	99	222	1,381	360	5,471	0	0	0	0	0
	APPROACH %	18%	73%	9%	28%	62%	10%	12%	77%	11%	11%	70%	18%						
	APP/DEPART	1,771	/	1,757	847	/	847	890	/	1,084	1,963	/	1,783	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	165	653	80	134	294	50	51	344	58	114	697	201	2,841					
	APPROACH %	18%	73%	9%	28%	62%	10%	11%	76%	13%	11%	69%	20%						
	PEAK HR FACTOR		0.884			0.892			0.985			0.897		0.925					
	APP/DEPART	898	/	905	478	/	466	453	/	558	1,012	/	912	0					

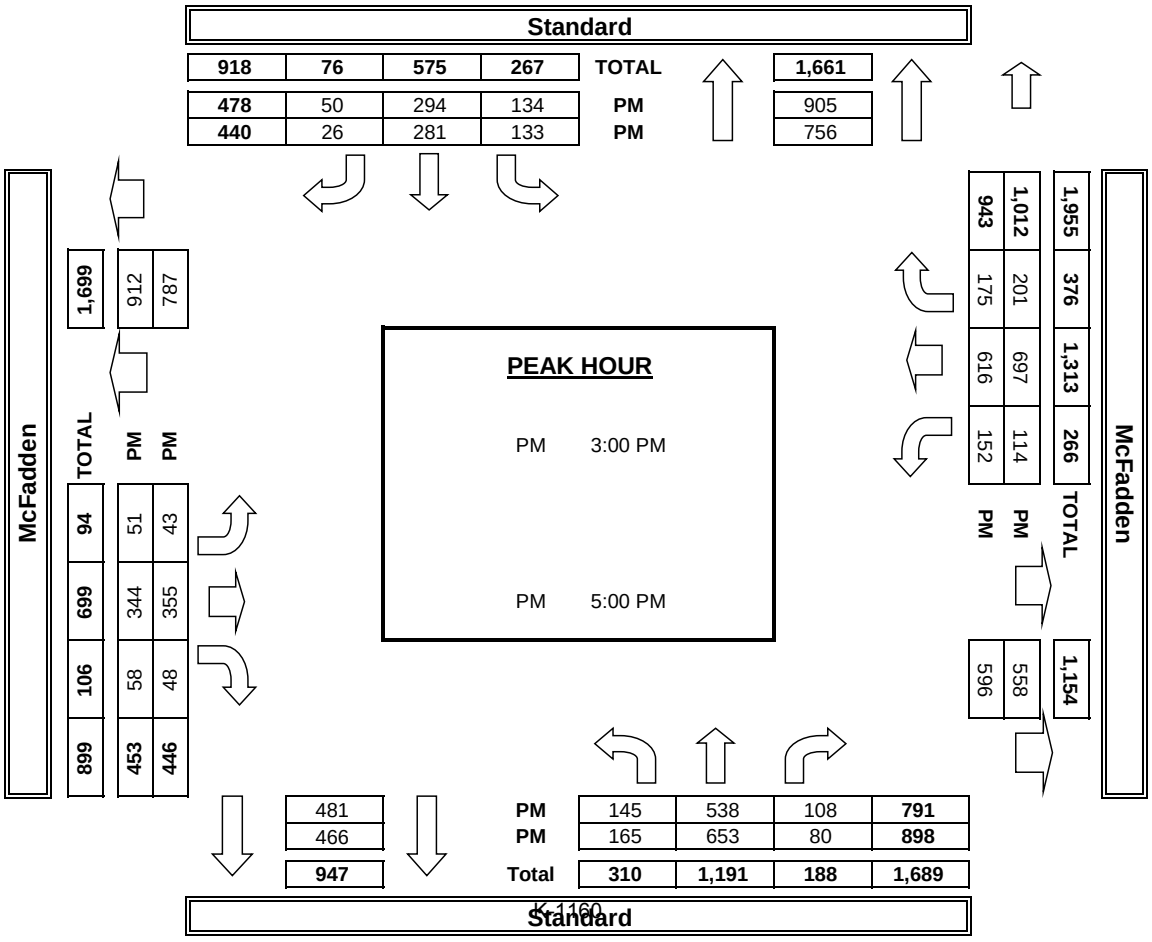


		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
PM	2:00 PM	17	16	28	14	75	12	15	27	14	68	5	1	1	0	7
	2:15 PM	26	20	29	16	91	26	20	29	16	91	0	0	0	0	0
	2:30 PM	29	25	34	28	116	25	24	31	25	105	4	1	3	3	11
	2:45 PM	31	47	45	21	144	27	45	42	19	133	4	2	3	2	11
	3:00 PM	62	47	47	39	195	54	45	44	38	181	8	2	3	1	14
	3:15 PM	43	46	49	28	166	40	46	49	28	163	3	0	0	0	3
	3:30 PM	23	29	41	23	116	17	29	38	22	106	6	0	3	1	10
	3:45 PM	19	17	28	8	72	15	15	27	7	64	4	2	1	1	8
	TOTAL	250	247	301	177	975	216	239	287	169	911	34	8	14	8	64
	AM BEGIN PEAK HR			3:00 PM			126	135	158	95	514					
PM	4:00 PM	10	27	24	21	82	10	26	24	19	79	0	1	0	2	3
	4:15 PM	20	30	18	22	90	15	29	17	18	79	5	1	1	4	11
	4:30 PM	12	30	15	21	78	10	29	15	18	72	2	1	0	3	6
	4:45 PM	21	14	18	23	76	16	14	13	20	63	5	0	5	3	13
	5:00 PM	15	19	22	18	74	12	18	22	17	69	3	1	0	1	5
	5:15 PM	33	23	33	26	115	29	22	30	23	104	4	1	3	3	11
	5:30 PM	17	21	16	28	82	15	16	15	21	67	2	5	1	7	15
	5:45 PM	17	24	21	32	94	15	16	20	29	80	2	8	1	3	14
	TOTAL	145	188	167	191	691	122	170	156	165	613	23	18	11	26	78
	PM BEGIN PEAK HR			5:00 PM			71	72	87	90	320					

AimTD LLC

TURNING MOVEMENT COUNTS





INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

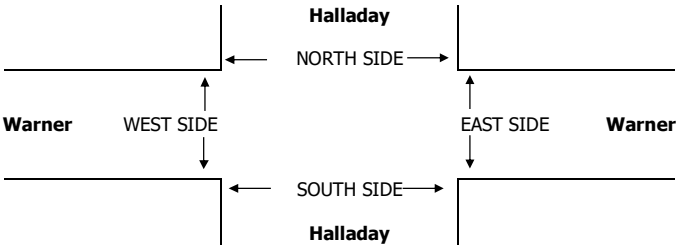
DATE:	LOCATION:	Santa Ana	PROJECT #:	SC0846
Tue, Feb 9, 16	NORTH & SOUTH:	Halladay	LOCATION #:	44
	EAST & WEST:	Warner	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND (Halladay)			SOUTHBOUND (Halladay)			EASTBOUND (Warner)			WESTBOUND (Warner)				U-TURNS				
	LANES:	NL 1	NT X	NR 1	SL X	ST X	SR X	EL X	ET 2	ER 0	WL 0	WT 2	WR X	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	13	0	6	0	0	0	0	247	29	18	151	0	464	0	0	0	0	0
	7:15 AM	10	0	24	0	0	0	0	293	28	36	139	0	530	0	0	0	0	0
	7:30 AM	14	0	24	0	0	0	0	308	46	39	181	0	612	0	0	0	0	0
	7:45 AM	10	0	22	0	0	0	0	323	42	45	146	0	588	0	0	0	0	0
	8:00 AM	7	0	26	0	0	0	0	308	23	33	117	0	514	0	0	0	0	0
	8:15 AM	7	0	21	0	0	0	0	298	17	24	114	0	481	0	0	0	0	0
	8:30 AM	12	0	24	0	0	0	0	245	22	16	134	0	453	0	0	0	0	0
	8:45 AM	14	0	26	0	0	0	0	188	14	24	132	0	398	0	0	0	0	0
	VOLUMES	87	0	173	0	0	0	0	2,210	221	235	1,114	0	4,040	0	0	0	0	0
	APPROACH %	33%	0%	67%	0%	0%	0%	0%	91%	9%	17%	83%	0%						
	APP/DEPART	260	/	0	0	/	456	2,431	/	2,383	1,349	/	1,201	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	41	0	96	0	0	0	0	1,232	139	153	583	0	2,244					
	APPROACH %	30%	0%	70%	0%	0%	0%	0%	90%	10%	21%	79%	0%						
	PEAK HR FACTOR		0.901			0.000			0.939			0.836		0.917					
	APP/DEPART	137	/	0	0	/	292	1,371	/	1,328	736	/	624	0					
PM	4:00 PM	39	0	75	0	0	0	0	197	12	26	236	0	585	0	0	0	0	0
	4:15 PM	29	0	80	0	0	0	0	204	17	22	211	0	563	0	0	0	0	0
	4:30 PM	35	0	81	0	0	0	0	219	22	26	270	0	653	0	0	0	0	0
	4:45 PM	32	0	70	0	0	0	1	205	11	32	244	0	595	0	0	1	0	1
	5:00 PM	44	0	84	0	0	0	0	210	20	30	295	0	683	0	0	0	0	0
	5:15 PM	51	0	103	0	0	0	0	255	36	47	268	0	760	0	0	0	0	0
	5:30 PM	38	0	83	0	0	0	0	204	25	41	231	0	622	0	0	0	0	0
	5:45 PM	30	0	85	0	0	0	0	202	24	37	217	0	595	0	0	0	0	0
	VOLUMES	298	0	661	0	0	0	1	1,696	167	261	1,972	0	5,056	0	0	1	0	1
	APPROACH %	31%	0%	69%	0%	0%	0%	0%	91%	9%	12%	88%	0%						
	APP/DEPART	959	/	0	0	/	428	1,864	/	2,357	2,233	/	2,271	0					
	BEGIN PEAK HR		4:30 PM																
	VOLUMES	162	0	338	0	0	0	1	889	89	135	1,077	0	2,691					
	APPROACH %	32%	0%	68%	0%	0%	0%	0%	91%	9%	11%	89%	0%						
	PEAK HR FACTOR		0.812			0.000			0.841			0.932		0.885					
	APP/DEPART	500	/	0	0	/	224	979	/	1,227	1,212	/	1,240	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	8	8	0	0	0	8	8	0	0	0	0	0
	7:15 AM	0	1	0	2	3	0	1	0	2	3	0	0	0	0	0
	7:30 AM	1	1	0	1	3	0	1	0	1	2	1	0	0	0	1
	7:45 AM	0	0	0	2	2	0	0	0	2	2	0	0	0	0	0
	8:00 AM	0	3	0	2	5	0	2	0	2	4	0	1	0	0	1
	8:15 AM	0	0	0	5	5	0	0	0	4	4	0	0	0	1	1
	8:30 AM	0	1	0	1	2	0	0	0	1	1	0	1	0	0	1
	8:45 AM	0	0	0	7	7	0	0	0	7	7	0	0	0	0	0
	TOTAL	1	6	0	28	35	0	4	0	27	31	1	2	0	1	4
PM	4:00 PM	0	1	0	5	6	0	0	0	5	5	0	1	0	0	1
	4:15 PM	0	1	0	4	5	0	1	0	3	4	0	0	0	1	1
	4:30 PM	0	0	0	7	7	0	0	0	6	6	0	0	0	1	1
	4:45 PM	1	0	0	6	7	0	0	0	2	2	1	0	0	4	5
	5:00 PM	0	2	0	9	11	0	2	0	8	10	0	0	0	1	1
	5:15 PM	0	1	0	16	17	0	0	0	15	15	0	1	0	1	2
	5:30 PM	0	0	0	5	5	0	0	0	4	4	0	0	0	1	1
	5:45 PM	0	2	0	2	4	0	1	0	2	3	0	1	0	0	1
	TOTAL	1	7	0	54	62	0	4	0	45	49	1	3	0	9	13

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Tue, May 14, 19

LOCATION: Santa Ana
NORTH & SOUTH: Halladay
EAST & WEST: Dyer

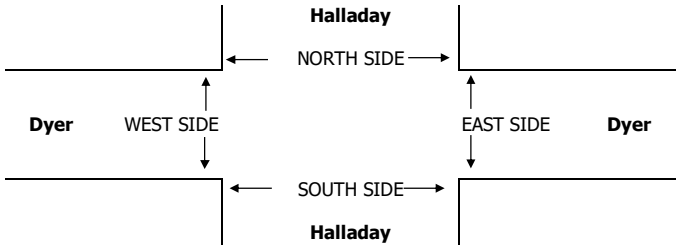
PROJECT #: SC2183
LOCATION #: 21
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — N / W / E / S

Add U-Turns to Left Turns

		NORTHBOUND Halladay			SOUTHBOUND Halladay			EASTBOUND Dyer			WESTBOUND Dyer				U-TURNS				
	LANES:	NL 1	NT 1	NR 0	SL 1	ST 0	SR 1	EL 1	ET 3	ER 0	WL 1	WT 3	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	1	4	14	37	8	9	2	266	1	25	152	33	552	0	0	0	0	0
	7:15 AM	1	3	14	44	5	8	9	334	5	33	147	33	636	0	0	0	0	0
	7:30 AM	1	4	8	51	8	13	11	363	0	20	175	26	680	0	0	0	0	0
	7:45 AM	4	5	10	43	9	7	9	358	2	20	161	21	649	0	0	0	0	0
	8:00 AM	4	5	16	43	14	10	11	306	2	40	136	20	607	0	0	0	0	0
	8:15 AM	2	6	12	52	14	8	5	290	3	41	149	19	601	0	0	0	1	1
	8:30 AM	3	5	8	46	8	6	4	288	7	34	140	18	567	0	0	0	1	1
	8:45 AM	0	3	12	35	14	9	6	235	6	27	148	18	513	0	0	0	0	0
	VOLUMES	16	35	94	351	80	70	57	2,440	26	240	1,208	188	4,805	0	0	0	2	2
	APPROACH %	11%	24%	65%	70%	16%	14%	2%	97%	1%	15%	74%	11%						
	APP/DEPART	145	/	280	501	/	344	2,523	/	2,887	1,636	/	1,294	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	10	17	48	181	36	38	40	1,361	9	113	619	100	2,572					
	APPROACH %	13%	23%	64%	71%	14%	15%	3%	97%	1%	14%	74%	12%						
	PEAK HR FACTOR		0.750			0.885			0.943			0.941		0.946					
	APP/DEPART	75	/	157	255	/	158	1,410	/	1,590	832	/	667	0					
PM	4:00 PM	5	43	50	37	3	12	21	287	0	14	280	21	773	0	0	0	1	1
	4:15 PM	7	37	56	33	6	10	31	281	4	19	312	25	821	0	0	0	1	1
	4:30 PM	15	36	56	35	3	10	43	335	1	12	362	26	934	0	0	0	0	0
	4:45 PM	8	32	51	26	3	4	30	308	1	11	331	34	839	0	0	0	0	0
	5:00 PM	15	61	56	28	5	11	34	338	4	14	333	30	929	0	0	0	1	1
	5:15 PM	2	49	42	23	3	6	41	304	2	7	410	24	913	0	0	0	0	0
	5:30 PM	7	44	46	27	8	3	37	337	2	15	345	40	911	0	0	0	0	0
	5:45 PM	3	48	45	20	9	10	35	320	1	15	298	32	836	0	0	0	1	1
	VOLUMES	62	350	402	229	40	66	272	2,510	15	107	2,671	232	6,956	0	0	0	4	4
	APPROACH %	8%	43%	49%	68%	12%	20%	10%	90%	1%	4%	89%	8%						
	APP/DEPART	814	/	854	335	/	158	2,797	/	3,145	3,010	/	2,799	0					
	BEGIN PEAK HR		4:30 PM																
	VOLUMES	40	178	205	112	14	31	148	1,285	8	44	1,436	114	3,615					
	APPROACH %	9%	42%	48%	71%	9%	20%	10%	89%	1%	3%	90%	7%						
	PEAK HR FACTOR		0.801			0.818			0.951			0.904		0.968					
	APP/DEPART	423	/	440	157	/	65	1,441	/	1,603	1,594	/	1,507	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:15 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	4:30 PM					0	0	0	0	0					

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Tue, May 14, 19

LOCATION: Santa Ana
NORTH & SOUTH: SR-55 SB Ramps
EAST & WEST: MacArthur

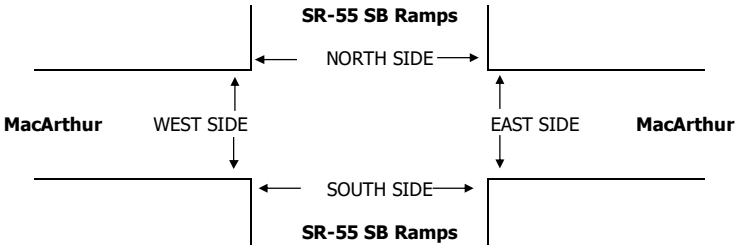
PROJECT #: SC2183
LOCATION #: 23
CONTROL: SIGNAL

NOTES: PM WB queue

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND SR-55 SB Ramps			SOUTHBOUND SR-55 SB Ramps			EASTBOUND MacArthur			WESTBOUND MacArthur				U-TURNS				
	LANES:	NL X	NT X	NR X	SL 2	ST X	SR 2	EL X	ET 3	ER 2	WL X	WT 3	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	0	0	0	236	0	222	0	299	232	0	239	22	1,250	0	0	0	0	0
	7:15 AM	0	0	0	233	0	229	0	324	267	0	246	33	1,332	0	0	0	0	0
	7:30 AM	0	0	0	208	0	215	0	397	280	0	305	31	1,436	0	0	0	0	0
	7:45 AM	0	0	0	256	0	229	0	378	246	0	318	38	1,465	0	0	0	0	0
	8:00 AM	0	0	0	242	0	209	0	342	238	0	314	39	1,384	0	0	0	0	0
	8:15 AM	0	0	0	245	0	208	0	309	252	0	325	35	1,374	0	0	0	0	0
	8:30 AM	0	0	0	204	0	144	0	339	250	0	277	55	1,269	0	0	0	0	0
	8:45 AM	0	0	0	237	0	165	0	287	197	0	248	64	1,198	0	0	0	0	0
	VOLUMES	0	0	0	1,861	0	1,621	0	2,675	1,962	0	2,272	317	10,708	0	0	0	0	0
	APPROACH %	0%	0%	0%	53%	0%	47%	0%	58%	42%	0%	88%	12%						
	APP/DEPART	0	/	317	3,482	/	1,962	4,637	/	4,536	2,589	/	3,893	0					
	BEGIN PEAK HR		7:30 AM																
	VOLUMES	0	0	0	951	0	861	0	1,426	1,016	0	1,262	143	5,659					
	APPROACH %	0%	0%	0%	52%	0%	48%	0%	58%	42%	0%	90%	10%						
	PEAK HR FACTOR		0.000			0.934			0.902			0.976		0.966					
	APP/DEPART	0	/	143	1,812	/	1,016	2,442	/	2,377	1,405	/	2,123	0					
PM	4:00 PM	0	0	0	62	0	132	0	326	263	0	389	160	1,332	0	0	0	0	0
	4:15 PM	0	0	0	71	0	187	0	275	234	0	357	148	1,272	0	0	0	0	0
	4:30 PM	0	0	0	74	0	170	0	277	269	0	415	153	1,358	0	0	0	0	0
	4:45 PM	0	0	0	69	0	176	0	274	239	0	366	133	1,257	0	0	0	0	0
	5:00 PM	0	0	0	63	0	166	0	319	282	0	370	178	1,378	0	0	0	0	0
	5:15 PM	0	0	0	79	0	178	0	285	239	0	365	132	1,278	0	0	0	0	0
	5:30 PM	0	0	0	61	0	180	0	291	206	0	370	125	1,233	0	0	0	0	0
	5:45 PM	0	0	0	93	0	171	0	245	187	0	350	105	1,151	0	0	0	0	0
	VOLUMES	0	0	0	572	0	1,360	0	2,292	1,919	0	2,982	1,134	10,259	0	0	0	0	0
	APPROACH %	0%	0%	0%	30%	0%	70%	0%	54%	46%	0%	72%	28%						
	APP/DEPART	0	/	1,134	1,932	/	1,919	4,211	/	2,864	4,116	/	4,342	0					
	BEGIN PEAK HR		4:30 PM																
	VOLUMES	0	0	0	285	0	690	0	1,155	1,029	0	1,516	596	5,271					
	APPROACH %	0%	0%	0%	29%	0%	71%	0%	53%	47%	0%	72%	28%						
	PEAK HR FACTOR		0.000			0.948			0.908			0.930		0.956					
	APP/DEPART	0	/	596	975	/	1,029	2,184	/	1,440	2,112	/	2,206	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:30 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	4:30 PM					0	0	0	0	0					

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Tue, May 14, 19

LOCATION: Santa Ana
NORTH & SOUTH: SR-55 NB Ramps
EAST & WEST: MacArthur

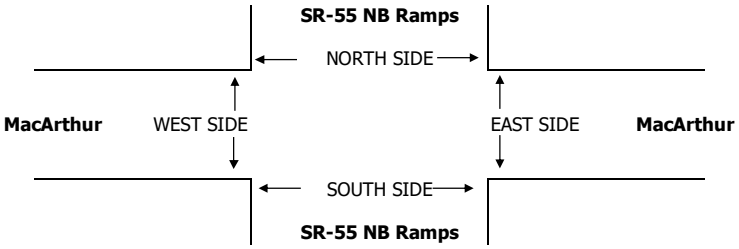
PROJECT #: SC2183
LOCATION #: 22
CONTROL: SIGNAL

NOTES: PM WB queue

AM / PM / MD / OTHER / OTHER — N / W / E / S

☐ Add U-Turns to Left Turns

		NORTHBOUND SR-55 NB Ramps			SOUTHBOUND SR-55 NB Ramps			EASTBOUND MacArthur			WESTBOUND MacArthur				U-TURNS				
	LANES:	NL 2	NT X	NR 1	SL X	ST X	SR X	EL X	ET 2	ER 2	WL X	WT 3	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	183	0	210	0	0	0	0	388	146	0	75	55	1,057	0	0	0	0	0
	7:15 AM	188	0	207	0	0	0	0	365	193	0	88	48	1,089	0	0	0	0	0
	7:30 AM	232	0	201	0	0	0	0	374	237	0	106	48	1,198	0	0	0	0	0
	7:45 AM	215	0	264	0	0	0	0	423	219	0	142	62	1,325	0	0	0	0	0
	8:00 AM	233	0	239	0	0	0	0	399	183	0	118	68	1,240	0	0	0	0	0
	8:15 AM	219	0	262	0	0	0	0	397	149	0	145	63	1,235	0	0	0	0	0
	8:30 AM	209	0	203	0	0	0	0	366	182	0	130	66	1,156	0	0	0	0	0
	8:45 AM	176	0	247	0	0	0	0	374	146	0	134	74	1,151	0	0	0	0	0
	VOLUMES	1,655	0	1,833	0	0	0	0	3,086	1,455	0	938	484	9,451	0	0	0	0	0
	APPROACH %	47%	0%	53%	0%	0%	0%	0%	68%	32%	0%	66%	34%						
	APP/DEPART	3,488	/	484	0	/	1,455	4,541	/	4,919	1,422	/	2,593	0					
	BEGIN PEAK HR		7:30 AM																
	VOLUMES	899	0	966	0	0	0	0	1,593	788	0	511	241	4,998					
	APPROACH %	48%	0%	52%	0%	0%	0%	0%	67%	33%	0%	68%	32%						
	PEAK HR FACTOR		0.969			0.000			0.927			0.904		0.943					
	APP/DEPART	1,865	/	241	0	/	788	2,381	/	2,559	752	/	1,410	0					
PM	4:00 PM	164	0	132	0	0	0	0	178	215	0	391	248	1,328	0	0	0	0	0
	4:15 PM	144	0	122	0	0	0	0	183	160	0	355	271	1,235	0	0	1	0	1
	4:30 PM	167	0	77	0	0	0	0	178	171	0	404	267	1,264	0	0	0	0	0
	4:45 PM	159	0	90	0	0	0	0	164	177	0	336	284	1,210	0	0	0	0	0
	5:00 PM	131	0	67	0	0	0	0	211	175	0	419	273	1,276	0	0	0	0	0
	5:15 PM	160	0	70	0	0	0	0	206	160	0	336	227	1,159	0	0	0	0	0
	5:30 PM	158	0	110	0	0	0	0	235	121	0	338	214	1,176	0	0	0	0	0
	5:45 PM	152	0	93	0	0	0	0	181	151	0	297	218	1,092	0	0	0	0	0
	VOLUMES	1,235	0	761	0	0	0	0	1,536	1,330	0	2,876	2,002	9,741	0	0	1	0	1
	APPROACH %	62%	0%	38%	0%	0%	0%	0%	54%	46%	0%	59%	41%						
	APP/DEPART	1,996	/	2,002	0	/	1,330	2,867	/	2,297	4,878	/	4,112	0					
	BEGIN PEAK HR		4:00 PM																
	VOLUMES	634	0	421	0	0	0	0	703	723	0	1,486	1,070	5,038					
	APPROACH %	60%	0%	40%	0%	0%	0%	0%	49%	51%	0%	58%	42%						
	PEAK HR FACTOR		0.891			0.000			0.908			0.952		0.948					
	APP/DEPART	1,055	/	1,070	0	/	723	1,427	/	1,124	2,556	/	2,121	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:30 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	4:00 PM					0	0	0	0	0					

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Thu, May 16, 19

LOCATION: Santa Ana
NORTH & SOUTH: SR-55 SB Ramps
EAST & WEST: Dyer

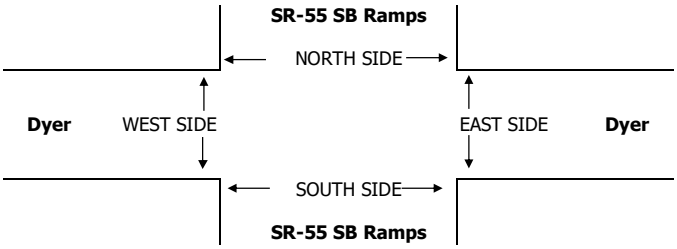
PROJECT #: SC2183
LOCATION #: 28
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N ◄ W E ► S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND SR-55 SB Ramps			SOUTHBOUND SR-55 SB Ramps			EASTBOUND Dyer			WESTBOUND Dyer				U-TURNS				
	LANES:	NL 1.5	NT 0.5	NR 2	SL 0.5	ST 1.5	SR 1	EL 1	ET 3	ER 1	WL 2	WT 3	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	73	5	119	7	46	12	3	254	66	76	129	23	813	0	0	0	0	0
	7:15 AM	56	2	140	5	53	16	7	320	68	79	161	18	925	0	0	0	1	1
	7:30 AM	64	7	127	12	72	16	2	344	81	95	153	21	994	0	0	0	0	0
	7:45 AM	51	6	125	5	68	16	14	331	64	101	166	38	985	0	0	0	0	0
	8:00 AM	65	6	120	14	47	11	6	314	76	90	167	27	943	0	0	0	1	1
	8:15 AM	38	6	128	18	52	16	12	335	61	117	154	20	957	0	0	0	0	0
	8:30 AM	45	3	88	18	41	18	7	243	70	131	137	18	819	0	0	0	0	0
	8:45 AM	46	0	73	18	37	12	12	235	61	92	170	19	775	0	0	0	0	0
	VOLUMES	438	35	920	97	416	117	63	2,376	547	781	1,237	184	7,211	0	0	0	2	2
	APPROACH %	31%	3%	66%	15%	66%	19%	2%	80%	18%	35%	56%	8%						
	APP/DEPART	1,393	/	282	630	/	1,742	2,986	/	3,395	2,202	/	1,792	0					
	BEGIN PEAK HR		7:30 AM																
	VOLUMES	218	25	500	49	239	59	34	1,324	282	403	640	106	3,879					
	APPROACH %	29%	3%	67%	14%	69%	17%	2%	81%	17%	35%	56%	9%						
	PEAK HR FACTOR		0.938			0.868			0.960			0.942		0.976					
	APP/DEPART	743	/	165	347	/	923	1,640	/	1,874	1,149	/	917	0					
PM	4:00 PM	74	1	43	30	57	19	18	308	62	113	219	14	958	0	0	0	0	0
	4:15 PM	74	0	56	21	34	14	16	336	42	107	258	11	969	0	0	0	0	0
	4:30 PM	64	2	55	26	55	24	11	383	51	138	238	9	1,056	0	1	0	1	2
	4:45 PM	84	2	63	15	39	22	15	370	48	127	233	23	1,041	0	0	1	0	1
	5:00 PM	74	4	49	40	61	36	19	372	57	138	257	15	1,122	0	0	0	0	0
	5:15 PM	93	1	70	20	68	26	19	382	48	147	253	16	1,143	0	0	0	1	1
	5:30 PM	59	1	56	23	40	16	23	341	32	97	243	18	949	0	0	1	0	1
	5:45 PM	83	2	68	17	47	22	31	346	40	95	196	15	962	0	0	0	0	0
	VOLUMES	605	13	460	192	401	179	152	2,838	380	962	1,897	121	8,200	0	1	2	2	5
	APPROACH %	56%	1%	43%	25%	52%	23%	5%	84%	11%	32%	64%	4%						
	APP/DEPART	1,078	/	285	772	/	1,741	3,370	/	3,491	2,980	/	2,683	0					
	BEGIN PEAK HR		4:30 PM																
	VOLUMES	315	9	237	101	223	108	64	1,507	204	550	981	63	4,362					
	APPROACH %	56%	2%	42%	23%	52%	25%	4%	85%	11%	35%	62%	4%						
	PEAK HR FACTOR		0.855			0.788			0.988			0.958		0.954					
	APP/DEPART	561	/	136	432	/	975	1,775	/	1,846	1,594	/	1,405	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:30 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	4:30 PM					0	0	0	0	0					

1852

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

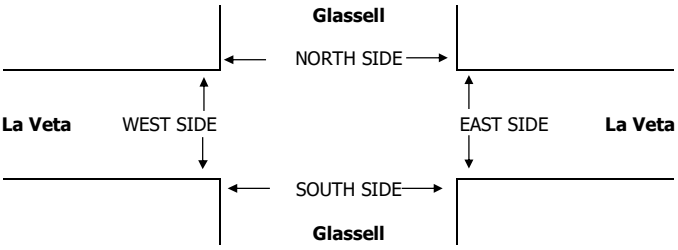
DATE:	LOCATION:	Santa Ana	PROJECT #:	SC0846
Tue, Mar 1, 16	NORTH & SOUTH:	Glassell	LOCATION #:	5
	EAST & WEST:	La Veta	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Glassell			SOUTHBOUND Glassell			EASTBOUND La Veta			WESTBOUND La Veta				U-TURNS				
	LANES:	NL 2	NT 1	NR 1	SL 1	ST 2	SR 0	EL 1	ET 1	ER 1	WL 1	WT 2	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	23	68	11	1	70	14	8	28	80	44	33	1	381	0	0	0	0	0
	7:15 AM	61	73	17	3	94	16	8	37	85	57	39	1	491	0	0	0	0	0
	7:30 AM	58	85	22	1	104	17	6	54	100	65	58	0	570	0	0	0	0	0
	7:45 AM	70	93	27	3	72	17	15	61	107	53	65	1	584	0	0	0	0	0
	8:00 AM	76	121	43	9	74	19	10	45	82	52	61	1	593	0	0	0	0	0
	8:15 AM	48	96	28	3	94	22	5	27	71	56	55	3	508	0	0	0	0	0
	8:30 AM	47	94	16	1	47	9	6	36	82	35	30	0	403	0	0	0	0	0
	8:45 AM	44	112	29	0	30	12	14	34	67	32	43	0	417	0	0	0	0	0
	VOLUMES	427	742	193	21	585	126	72	322	674	394	384	7	3,947	0	0	0	0	0
	APPROACH %	31%	54%	14%	3%	80%	17%	7%	30%	63%	50%	49%	1%						
	APP/DEPART	1,362	/	821	732	/	1,653	1,068	/	536	785	/	937	0					
	BEGIN PEAK HR		7:30 AM																
	VOLUMES	252	395	120	16	344	75	36	187	360	226	239	5	2,255					
	APPROACH %	33%	51%	16%	4%	79%	17%	6%	32%	62%	48%	51%	1%						
	PEAK HR FACTOR		0.799			0.891			0.796			0.955		0.951					
	APP/DEPART	767	/	436	435	/	930	583	/	323	470	/	566	0					
PM	4:00 PM	62	122	38	2	107	5	19	69	92	35	54	3	608	0	0	0	0	0
	4:15 PM	72	111	60	5	103	11	16	53	100	31	33	2	597	0	0	0	0	0
	4:30 PM	89	104	42	3	114	9	16	34	79	29	36	1	556	0	0	0	0	0
	4:45 PM	91	114	40	1	118	14	12	45	79	23	38	2	577	0	0	0	0	0
	5:00 PM	106	131	45	3	105	7	20	58	128	33	49	2	687	0	0	0	0	0
	5:15 PM	109	132	38	5	125	11	24	82	112	33	61	3	735	0	0	0	0	0
	5:30 PM	122	117	36	3	120	8	13	61	95	40	58	2	675	0	0	0	0	0
	5:45 PM	116	90	34	3	129	9	21	56	108	42	101	3	712	0	0	0	0	0
	VOLUMES	767	921	333	25	921	74	141	458	793	266	430	18	5,147	0	0	0	0	0
	APPROACH %	38%	46%	16%	2%	90%	7%	10%	33%	57%	37%	60%	3%						
	APP/DEPART	2,021	/	1,080	1,020	/	1,980	1,392	/	816	714	/	1,271	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	453	470	153	14	479	35	78	257	443	148	269	10	2,809					
	APPROACH %	42%	44%	14%	3%	91%	7%	10%	33%	57%	35%	63%	2%						
	PEAK HR FACTOR		0.954			0.936			0.892			0.731		0.955					
	APP/DEPART	1,076	/	558	528	/	1,070	778	/	424	427	/	757	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	1	3	0	1	5	0	2	0	1	3	1	1	0	0	2
	7:15 AM	2	0	2	2	6	2	0	1	2	5	0	0	1	0	1
	7:30 AM	3	1	1	1	6	3	1	1	0	5	0	0	0	1	1
	7:45 AM	2	3	3	0	8	1	3	2	0	6	1	0	1	0	2
	8:00 AM	0	2	2	2	6	0	2	1	2	5	0	0	1	0	1
	8:15 AM	2	2	2	1	7	1	2	1	0	4	1	0	1	1	3
	8:30 AM	6	1	0	3	10	6	1	0	2	9	0	0	0	1	1
	8:45 AM	4	0	3	3	10	4	0	2	2	8	0	0	1	1	2
	TOTAL	20	12	13	13	58	17	11	8	9	45	3	1	5	4	13
PM	4:00 PM	1	5	2	2	10	1	5	2	0	8	0	0	0	2	2
	4:15 PM	2	3	1	5	11	2	3	1	2	8	0	0	0	3	3
	4:30 PM	2	1	0	5	8	2	1	0	3	6	0	0	0	2	2
	4:45 PM	5	2	0	5	12	4	1	0	3	8	1	1	0	2	4
	5:00 PM	8	0	6	2	16	7	0	6	1	14	1	0	0	1	2
	5:15 PM	5	1	3	2	11	4	1	3	2	10	1	0	0	0	1
	5:30 PM	5	3	3	0	11	4	3	3	0	10	1	0	0	0	1
	5:45 PM	4	0	3	5	12	4	0	1	5	10	0	0	2	0	2
	TOTAL	32	15	18	26	91	28	14	16	16	74	4	1	2	10	17

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

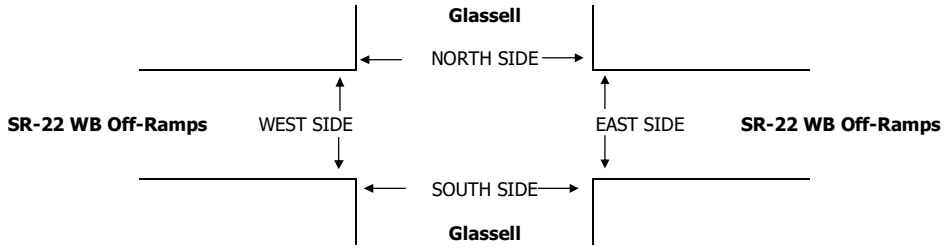
DATE:	LOCATION:	Santa Ana	PROJECT #:	SC0846
Tue, Mar 1, 16	NORTH & SOUTH:	Glassell	LOCATION #:	1
	EAST & WEST:	SR-22 WB Off-Ramps	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Glassell			SOUTHBOUND Glassell			EASTBOUND SR-22 WB Off-Ramps			WESTBOUND SR-22 WB Off-Ramps				U-TURNS				
	LANES:	NL 1	NT 2	NR X	SL X	ST 2	SR 1	EL X	ET X	ER X	WL 1.5	WT 0	WR 1.5	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	62	71	0	0	133	56	0	0	0	61	0	44	427	0	0	0	0	0
	7:15 AM	107	93	0	0	177	63	0	0	0	79	4	89	612	0	0	0	0	0
	7:30 AM	109	124	0	0	174	76	0	0	0	67	0	101	651	0	0	0	0	0
	7:45 AM	139	164	0	0	207	102	0	0	0	60	0	113	785	0	0	0	0	0
	8:00 AM	125	158	0	0	156	91	0	0	0	67	0	106	703	0	0	0	0	0
	8:15 AM	103	127	0	0	166	67	0	0	0	67	0	70	600	0	0	0	0	0
	8:30 AM	99	98	0	0	142	44	0	0	0	92	0	70	545	0	0	0	0	0
	8:45 AM	94	129	0	0	103	50	0	0	0	69	0	76	521	0	0	0	0	0
	VOLUMES	838	964	0	0	1,258	549	0	0	0	562	4	669	4,844	0	0	0	0	0
	APPROACH %	47%	53%	0%	0%	70%	30%	0%	0%	0%	46%	0%	54%						
	APP/DEPART	1,802	/	1,633	1,807	/	1,820	0	/	0	1,235	/	1,391	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	480	539	0	0	714	332	0	0	0	273	4	409	2,751					
	APPROACH %	47%	53%	0%	0%	68%	32%	0%	0%	0%	40%	1%	60%						
	PEAK HR FACTOR		0.841			0.846			0.000			0.991		0.876					
	APP/DEPART	1,019	/	948	1,046	/	987	0	/	0	686	/	816	0					
PM	4:00 PM	99	134	0	0	171	45	0	0	0	31	1	52	533	0	0	0	0	0
	4:15 PM	126	219	0	0	221	53	0	0	0	54	0	58	731	0	0	0	0	0
	4:30 PM	123	188	0	0	182	55	0	0	0	53	0	62	663	0	0	0	0	0
	4:45 PM	129	207	0	0	198	40	0	0	0	51	0	72	697	0	0	0	0	0
	5:00 PM	138	239	0	0	198	68	0	0	0	39	1	88	771	0	0	0	0	0
	5:15 PM	113	210	0	0	230	69	0	0	0	43	1	86	752	0	0	0	0	0
	5:30 PM	144	225	0	0	200	47	0	0	0	55	2	66	739	0	0	0	0	0
	5:45 PM	140	200	0	0	215	61	0	0	0	56	2	103	777	1	0	0	0	1
	VOLUMES	1,012	1,622	0	0	1,615	438	0	0	0	382	7	587	5,663	1	0	0	0	1
	APPROACH %	38%	62%	0%	0%	79%	21%	0%	0%	0%	39%	1%	60%						
	APP/DEPART	2,634	/	2,209	2,053	/	1,998	0	/	0	976	/	1,456	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	535	874	0	0	843	245	0	0	0	193	6	343	3,039					
	APPROACH %	38%	62%	0%	0%	77%	23%	0%	0%	0%	36%	1%	63%						
	PEAK HR FACTOR		0.934			0.910			0.000			0.842		0.978					
	APP/DEPART	1,409	/	1,217	1,088	/	1,037	0	/	0	542	/	785	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	3	1	4	0	0	1	0	1	0	0	2	1	3
	7:15 AM	0	0	3	1	4	0	0	2	0	2	0	0	1	1	2
	7:30 AM	0	0	3	1	4	0	0	2	0	2	0	0	1	1	2
	7:45 AM	0	0	1	1	2	0	0	1	0	1	0	0	0	1	1
	8:00 AM	0	0	4	3	7	0	0	2	3	5	0	0	2	0	2
	8:15 AM	0	0	3	1	4	0	0	1	0	1	0	0	2	1	3
	8:30 AM	0	0	2	3	5	0	0	1	2	3	0	0	1	1	2
	8:45 AM	0	0	2	1	3	0	0	1	0	1	0	0	1	1	2
	TOTAL	0	0	21	12	33	0	0	11	5	16	0	0	10	7	17
PM	4:00 PM	0	0	4	5	9	0	0	2	4	6	0	0	2	1	3
	4:15 PM	0	0	8	1	9	0	0	4	1	5	0	0	4	0	4
	4:30 PM	0	0	3	2	5	0	0	2	0	2	0	0	1	2	3
	4:45 PM	0	0	5	6	11	0	0	2	3	5	0	0	3	3	6
	5:00 PM	0	0	6	2	8	0	0	4	0	4	0	0	2	2	4
	5:15 PM	0	0	5	0	5	0	0	2	0	2	0	0	3	0	3
	5:30 PM	0	0	6	0	6	0	0	3	0	3	0	0	3	0	3
	5:45 PM	0	0	6	2	8	0	0	4	0	4	0	0	2	2	4
	TOTAL	0	0	43	18	61	0	0	23	8	31	0	0	20	10	30

INTERSECTION TURNING MOVEMENT COUNTS

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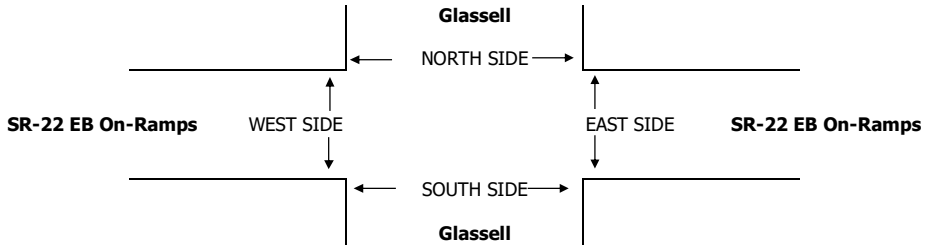
DATE:	LOCATION:	Santa Ana	PROJECT #:	SC0846
Tue, Mar 1, 16	NORTH & SOUTH:	Glassell	LOCATION #:	2
	EAST & WEST:	SR-22 EB On-Ramps	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Glassell			SOUTHBOUND Glassell			EASTBOUND SR-22 EB On-Ramps			WESTBOUND SR-22 EB On-Ramps				U-TURNS				
	LANES:	NL X	NT 2	NR 1	SL 1	ST 2	SR X	EL 1.3	ET 0.3	ER 1.3	WL X	WT X	WR X	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	0	113	51	79	158	0	35	0	209	0	0	0	645	0	0	0	0	0
	7:15 AM	0	167	51	58	192	0	46	0	239	0	0	0	753	0	0	0	0	0
	7:30 AM	0	185	66	81	159	0	57	0	236	0	0	0	784	0	0	0	0	0
	7:45 AM	0	222	50	71	182	0	84	0	241	0	0	0	850	0	0	0	0	0
	8:00 AM	0	212	42	59	171	0	60	0	256	0	0	0	800	0	0	0	0	0
	8:15 AM	0	147	40	70	162	0	53	1	271	0	0	0	744	0	0	0	0	0
	8:30 AM	0	157	41	43	159	0	60	1	233	0	0	0	694	0	0	0	0	0
	8:45 AM	0	145	36	53	128	0	71	0	208	0	0	0	641	0	0	0	0	0
	VOLUMES	0	1,348	377	514	1,311	0	466	2	1,893	0	0	0	5,911	0	0	0	0	0
	APPROACH %	0%	78%	22%	28%	72%	0%	20%	0%	80%	0%	0%	0%						
	APP/DEPART	1,725	/	1,814	1,825	/	3,204	2,361	/	893	0	/	0	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	0	786	209	269	704	0	247	0	972	0	0	0	3,187					
	APPROACH %	0%	79%	21%	28%	72%	0%	20%	0%	80%	0%	0%	0%						
	PEAK HR FACTOR		0.915			0.961			0.938			0.000		0.937					
	APP/DEPART	995	/	1,033	973	/	1,676	1,219	/	478	0	/	0	0					
PM	4:00 PM	0	237	78	85	155	0	64	0	116	0	0	0	735	0	0	0	0	0
	4:15 PM	0	226	64	92	164	0	86	0	126	0	0	0	758	0	1	0	0	1
	4:30 PM	0	253	63	77	146	0	59	0	121	0	0	0	719	0	0	0	0	0
	4:45 PM	0	265	58	91	138	0	77	0	129	0	0	0	758	0	0	0	0	0
	5:00 PM	0	276	69	100	160	0	67	0	147	0	0	0	819	0	0	0	0	0
	5:15 PM	0	277	85	87	157	0	53	2	119	0	0	0	780	0	0	0	0	0
	5:30 PM	0	310	71	87	165	0	44	0	111	0	0	0	788	0	0	0	0	0
	5:45 PM	0	292	70	92	172	0	56	1	106	0	0	0	789	0	0	0	0	0
	VOLUMES	0	2,136	558	711	1,257	0	506	3	975	0	0	0	6,146	0	1	0	0	1
	APPROACH %	0%	79%	21%	36%	64%	0%	34%	0%	66%	0%	0%	0%						
	APP/DEPART	2,694	/	2,643	1,968	/	2,232	1,484	/	1,271	0	/	0	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	0	1,155	295	366	654	0	220	3	483	0	0	0	3,176					
	APPROACH %	0%	80%	20%	36%	64%	0%	31%	0%	68%	0%	0%	0%						
	PEAK HR FACTOR		0.951			0.966			0.825			0.000		0.969					
	APP/DEPART	1,450	/	1,375	1,020	/	1,137	706	/	664	0	/	0	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	3	1	4	0	0	1	0	1	0	0	2	1	3
	7:15 AM	0	0	4	1	5	0	0	2	0	2	0	0	2	1	3
	7:30 AM	0	0	1	1	2	0	0	1	0	1	0	0	0	1	1
	7:45 AM	0	0	3	1	4	0	0	2	1	3	0	0	1	0	1
	8:00 AM	0	0	4	2	6	0	0	2	2	4	0	0	2	0	2
	8:15 AM	0	0	2	1	3	0	0	1	0	1	0	0	1	1	2
	8:30 AM	0	0	2	4	6	0	0	1	2	3	0	0	1	2	3
	8:45 AM	0	1	2	0	3	0	1	1	0	2	0	0	1	0	1
	TOTAL	0	1	21	11	33	0	1	11	5	17	0	0	10	6	16
PM	4:00 PM	0	0	6	5	11	0	0	2	4	6	0	0	4	1	5
	4:15 PM	0	0	7	2	9	0	0	5	1	6	0	0	2	1	3
	4:30 PM	0	0	4	3	7	0	0	2	1	3	0	0	2	2	4
	4:45 PM	0	0	5	5	10	0	0	2	2	4	0	0	3	3	6
	5:00 PM	0	0	7	1	8	0	0	4	0	4	0	0	3	1	4
	5:15 PM	0	2	5	0	7	0	2	3	0	5	0	0	2	0	2
	5:30 PM	0	0	6	0	6	0	0	3	0	3	0	0	3	0	3
	5:45 PM	0	0	11	3	14	0	0	8	0	8	0	0	3	3	6
	TOTAL	0	2	51	19	72	0	2	29	8	39	0	0	22	11	33

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

DATE: Thu, Feb 11, 16

LOCATION: Santa Ana
NORTH & SOUTH: Grand
EAST & WEST: Fairhaven

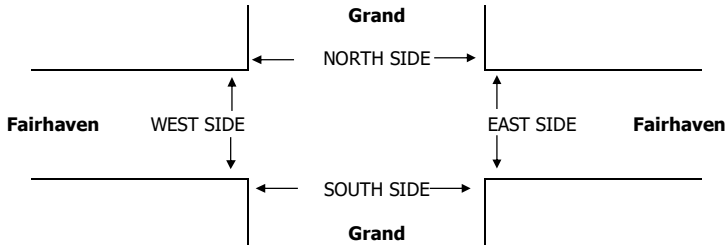
PROJECT #: SC0846
LOCATION #: 2
CONTROL: SIGNAL

NOTES:

AM PM MD OTHER OTHER — ▲ N ◄ W E ► S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Grand			SOUTHBOUND Grand			EASTBOUND Fairhaven			WESTBOUND Fairhaven				U-TURNS				
	LANES:	NL 1	NT 3	NR 0	SL 1	ST 2	SR 1	EL 1	ET 2	ER 0	WL 1	WT 1	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	4	146	15	32	350	6	19	6	11	38	3	30	660	1	0	0	0	1
	7:15 AM	3	169	23	27	367	7	18	15	13	55	3	55	755	3	0	0	0	3
	7:30 AM	6	198	30	45	341	12	26	13	20	56	2	57	806	1	0	0	0	1
	7:45 AM	3	183	31	50	333	8	29	22	13	63	9	52	796	1	1	0	0	2
	8:00 AM	5	158	17	45	346	3	22	7	10	70	12	56	751	2	0	0	0	2
	8:15 AM	9	152	27	28	387	3	19	5	11	66	5	35	747	2	1	0	0	3
	8:30 AM	6	154	7	36	342	16	26	6	8	62	4	42	709	2	0	0	0	2
	8:45 AM	2	164	22	31	335	11	15	6	9	47	1	41	684	1	0	0	0	1
	VOLUMES	38	1,324	172	294	2,801	66	174	80	95	457	39	368	5,908	13	2	0	0	15
	APPROACH %	2%	86%	11%	9%	89%	2%	50%	23%	27%	53%	5%	43%						
	APP/DEPART	1,534	/	1,868	3,161	/	3,366	349	/	544	864	/	130	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	17	708	101	167	1,387	30	95	57	56	244	26	220	3,108					
	APPROACH %	2%	86%	12%	11%	88%	2%	46%	27%	27%	50%	5%	45%						
	PEAK HR FACTOR		0.882			0.988			0.813			0.888		0.964					
	APP/DEPART	826	/	1,024	1,584	/	1,694	208	/	324	490	/	66	0					
PM	4:00 PM	11	271	36	31	181	12	14	11	3	40	6	31	647	4	2	0	0	6
	4:15 PM	17	254	36	36	206	18	21	6	4	44	7	29	678	3	0	0	0	3
	4:30 PM	15	306	34	45	187	14	12	4	4	44	9	24	698	5	3	0	0	8
	4:45 PM	9	313	47	45	203	21	13	7	7	44	7	36	752	3	1	0	0	4
	5:00 PM	9	303	53	54	164	23	25	12	7	54	4	38	746	3	4	0	0	7
	5:15 PM	11	293	62	58	238	16	12	7	4	56	9	47	813	1	4	0	0	5
	5:30 PM	14	297	60	58	218	17	23	13	11	61	18	45	835	3	1	0	0	4
	5:45 PM	10	273	58	43	239	22	15	11	9	74	15	43	812	1	1	0	0	2
	VOLUMES	96	2,310	386	370	1,636	143	135	71	49	417	75	293	5,981	23	16	0	0	39
	APPROACH %	3%	83%	14%	17%	76%	7%	53%	28%	19%	53%	10%	37%						
	APP/DEPART	2,792	/	2,754	2,149	/	2,125	255	/	811	785	/	291	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	44	1,166	233	213	859	78	75	43	31	245	46	173	3,206					
	APPROACH %	3%	81%	16%	19%	75%	7%	50%	29%	21%	53%	10%	37%						
	PEAK HR FACTOR		0.972			0.921			0.793			0.879		0.960					
	APP/DEPART	1,443	/	1,424	1,150	/	1,143	149	/	479	464	/	160	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	6	3	1	10	0	6	1	1	8	0	0	2	0	2
	7:15 AM	0	2	2	0	4	0	2	2	0	4	0	0	0	0	0
	7:30 AM	2	18	9	3	32	2	18	9	1	30	0	0	0	2	2
	7:45 AM	18	22	18	8	66	18	22	16	8	64	0	0	2	0	2
	8:00 AM	6	13	1	3	23	6	13	1	2	22	0	0	0	1	1
	8:15 AM	1	3	1	1	6	1	3	1	1	6	0	0	0	0	0
	8:30 AM	2	3	3	2	10	2	3	1	1	7	0	0	2	1	3
	8:45 AM	2	1	4	0	7	2	0	3	0	5	0	1	1	0	2
	TOTAL	31	68	41	18	158	31	67	34	14	146	0	1	7	4	12
PM	4:00 PM	3	4	1	1	9	3	4	1	0	8	0	0	0	1	1
	4:15 PM	4	2	1	1	8	4	1	1	1	7	0	1	0	0	1
	4:30 PM	2	3	4	2	11	2	2	3	0	7	0	1	1	2	4
	4:45 PM	1	6	2	3	12	1	6	2	3	12	0	0	0	0	0
	5:00 PM	6	8	4	0	18	5	8	3	0	16	1	0	1	0	2
	5:15 PM	3	4	9	3	19	2	3	5	0	10	1	1	4	3	9
	5:30 PM	8	6	2	5	21	8	6	2	2	18	0	0	0	3	3
	5:45 PM	0	10	14	1	25	0	10	8	1	19	0	0	6	0	6
	TOTAL	27	43	37	16	123	25	40	25	7	97	2	3	12	9	26

Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: GRAND AVENUE
E-W Direction: SANTA CLARA AVENUE

File Name : H1702015
Site Code :
Start Date : 2/9/2017
Page No : 1

Groups Printed- Turning Movements

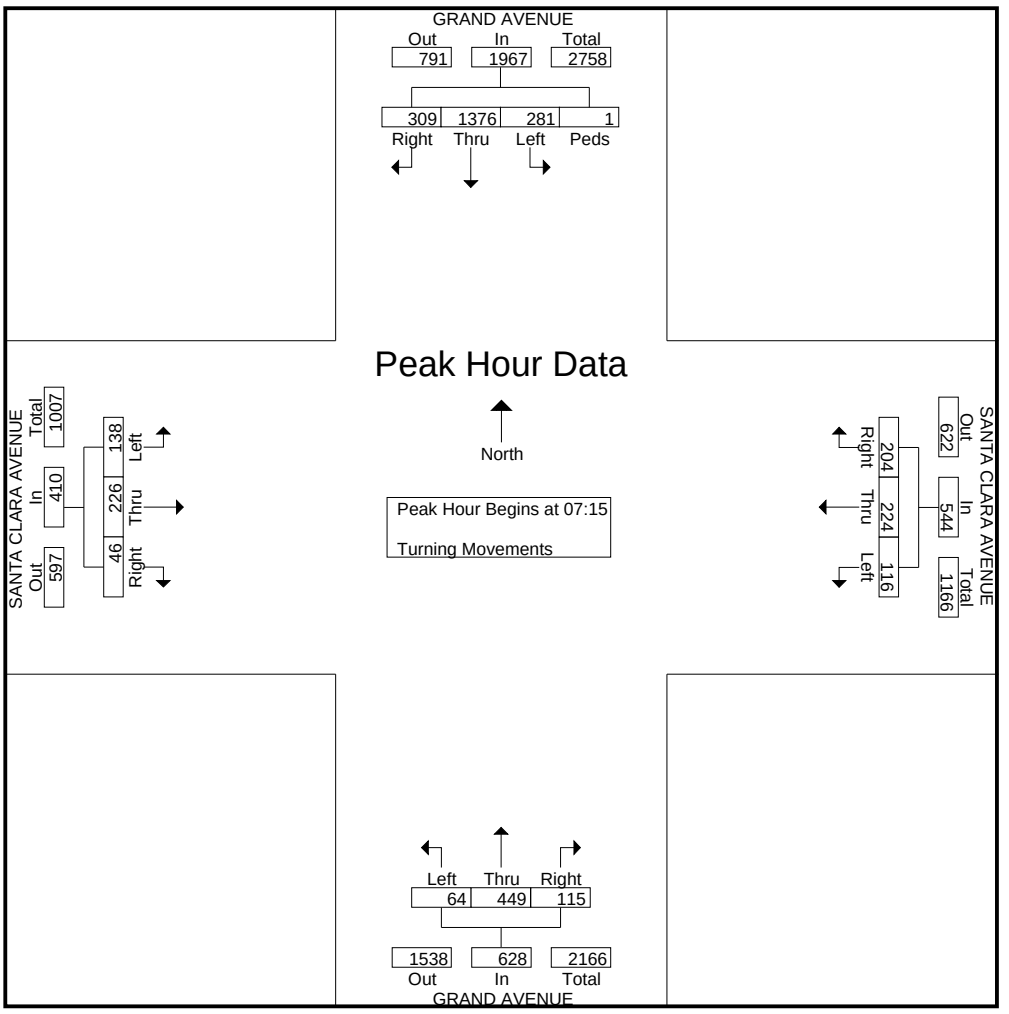
	GRAND AVENUE Southbound				SANTA CLARA AVENUE Westbound			GRAND AVENUE Northbound			SANTA CLARA AVENUE Eastbound			
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Int. Total
07:00	47	415	37	1	39	31	17	17	88	7	10	31	26	766
07:15	78	364	64	0	47	54	19	28	83	13	11	56	35	852
07:30	84	333	80	1	67	58	33	31	131	15	13	65	26	937
07:45	76	317	83	0	52	51	36	37	133	29	16	58	32	920
Total	285	1429	264	2	205	194	105	113	435	64	50	210	119	3475
08:00	71	362	54	0	38	61	28	19	102	7	6	47	45	840
08:15	70	442	35	0	32	47	30	24	91	5	6	32	26	840
08:30	46	429	54	0	29	18	22	19	118	10	15	20	20	800
08:45	45	338	48	0	30	17	25	20	104	11	12	25	21	696
Total	232	1571	191	0	129	143	105	82	415	33	39	124	112	3176
*** BREAK ***														
16:00	45	163	42	1	44	56	35	42	249	22	5	32	33	769
16:15	47	168	38	1	25	46	38	62	268	18	10	31	37	789
16:30	26	195	31	0	43	45	36	55	280	18	11	36	36	812
16:45	43	170	48	1	49	57	25	47	276	17	14	34	36	817
Total	161	696	159	3	161	204	134	206	1073	75	40	133	142	3187
17:00	61	201	40	2	87	68	32	64	297	16	7	56	43	974
17:15	60	234	60	2	52	63	29	59	286	28	7	39	46	965
17:30	48	179	56	0	59	55	37	50	287	26	8	43	27	875
17:45	44	228	57	1	54	50	34	65	280	15	12	50	38	928
Total	213	842	213	5	252	236	132	238	1150	85	34	188	154	3742
Grand Total	891	4538	827	10	747	777	476	639	3073	257	163	655	527	13580
Apprch %	14.2	72.4	13.2	0.2	37.3	38.8	23.8	16.1	77.4	6.5	12.1	48.7	39.2	
Total %	6.6	33.4	6.1	0.1	5.5	5.7	3.5	4.7	22.6	1.9	1.2	4.8	3.9	

Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: GRAND AVENUE
E-W Direction: SANTA CLARA AVENUE

File Name : H1702015
Site Code :
Start Date : 2/9/2017
Page No : 2

	GRAND AVENUE Southbound					SANTA CLARA AVENUE Westbound				GRAND AVENUE Northbound				SANTA CLARA AVENUE Eastbound				
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 07:00 to 08:45 - Peak 1 of 1																		
Peak Hour for Entire Intersection Begins at 07:15																		
07:15	78	**364**	64	0	**506**	47	54	19	120	28	83	13	124	11	56	35	102	852
07:30	**84**	333	80	**1**	498	**67**	58	33	**158**	31	131	15	177	13	**65**	26	104	**937**
07:45	76	317	**83**	0	476	52	51	**36**	139	**37**	**133**	**29**	**199**	**16**	58	32	**106**	920
08:00	71	362	54	0	487	38	**61**	28	127	19	102	7	128	6	47	**45**	98	840
Total Volume	309	1376	281	1	1967	204	224	116	544	115	449	64	628	46	226	138	410	3549
% App. Total	15.7	70	14.3	0.1		37.5	41.2	21.3		18.3	71.5	10.2		11.2	55.1	33.7		
PHF	.920	.945	.846	.250	.972	.761	.918	.806	.861	.777	.844	.552	.789	.719	.869	.767	.967	.947

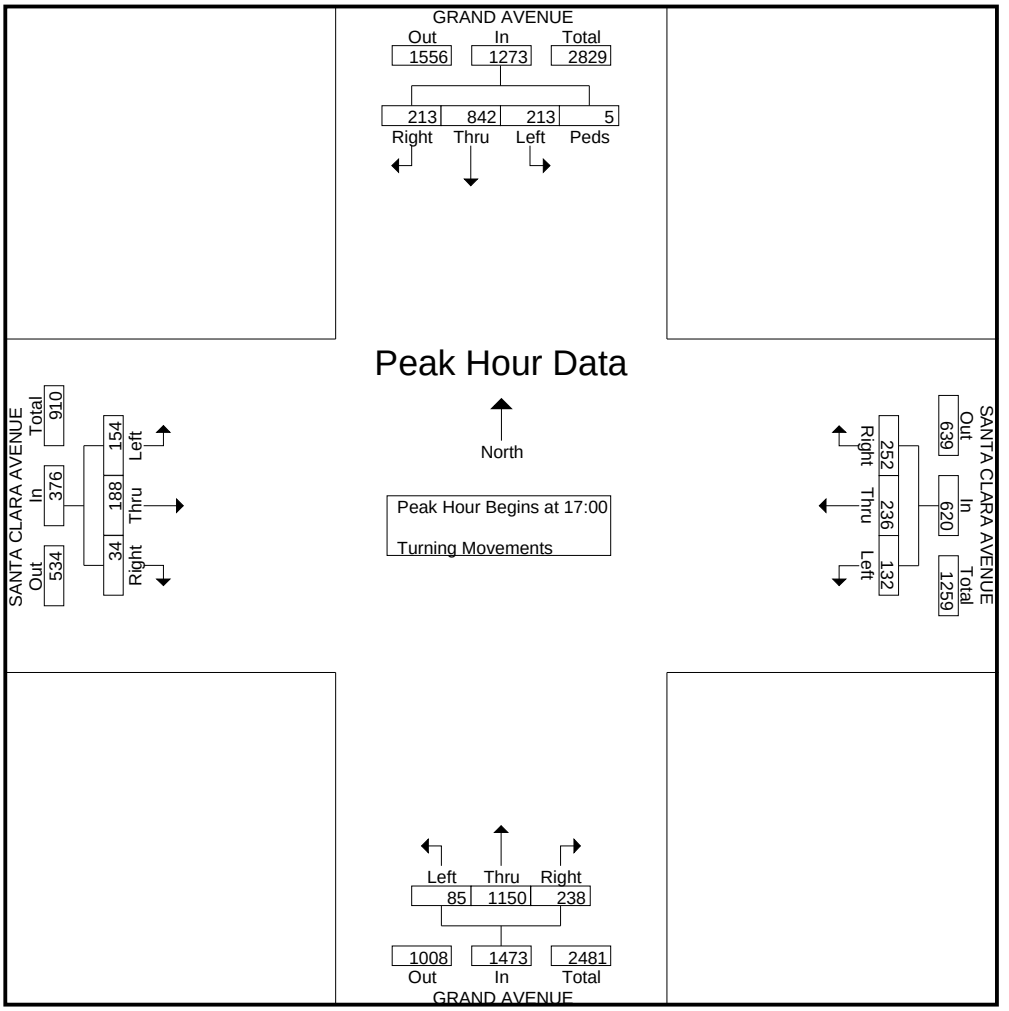


Transportation Studies, Inc.
2640 Walnut Avenue, Suite L
Tustin, CA. 92780

City: SANTA ANA
N-S Direction: GRAND AVENUE
E-W Direction: SANTA CLARA AVENUE

File Name : H1702015
Site Code :
Start Date : 2/9/2017
Page No : 3

	GRAND AVENUE Southbound					SANTA CLARA AVENUE Westbound				GRAND AVENUE Northbound				SANTA CLARA AVENUE Eastbound				
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 16:00 to 17:45 - Peak 1 of 1																		
Peak Hour for Entire Intersection Begins at 17:00																		
17:00	**61**	201	40	**2**	304	**87**	**68**	32	**187**	64	**297**	16	**377**	7	**56**	43	**106**	**974**
17:15	60	**234**	**60**	2	**356**	52	63	29	144	59	286	**28**	373	7	39	**46**	92	965
17:30	48	179	56	0	283	59	55	**37**	151	50	287	26	363	8	43	27	78	875
17:45	44	228	57	1	330	54	50	34	138	**65**	280	15	360	**12**	50	38	100	928
Total Volume	213	842	213	5	1273	252	236	132	620	238	1150	85	1473	34	188	154	376	3742
% App. Total	16.7	66.1	16.7	0.4		40.6	38.1	21.3		16.2	78.1	5.8		9	50	41		
PHF	.873	.900	.888	.625	.894	.724	.868	.892	.829	.915	.968	.759	.977	.708	.839	.837	.887	.960



INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Tue, May 14, 19

LOCATION: Santa Ana
NORTH & SOUTH: Grand
EAST & WEST: 17th

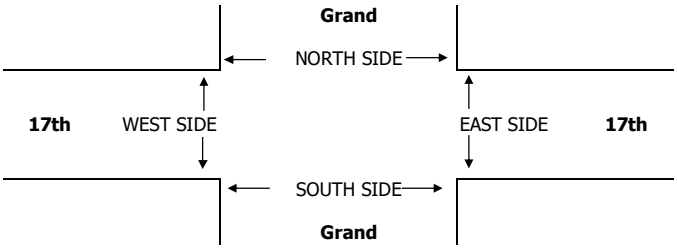
PROJECT #: SC2183
LOCATION #: 24
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Grand			SOUTHBOUND Grand			EASTBOUND 17th			WESTBOUND 17th				U-TURNS				
	LANES:	NL 1	NT 3	NR 0	SL 1	ST 2	SR 1	EL 2	ET 3	ER 0	WL 2	WT 3	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	24	85	43	45	293	34	38	137	41	46	115	10	911	1	0	5	0	6
	7:15 AM	32	100	80	51	301	46	46	183	31	67	176	18	1,131	3	0	5	0	8
	7:30 AM	49	146	87	60	309	60	64	216	31	89	210	26	1,347	1	0	6	0	7
	7:45 AM	43	137	97	77	289	73	53	217	17	84	236	24	1,347	2	0	3	0	5
	8:00 AM	37	88	59	95	287	53	58	210	25	65	177	19	1,173	0	0	4	0	4
	8:15 AM	36	70	56	67	314	39	35	169	34	59	132	15	1,026	3	0	5	0	8
	8:30 AM	44	93	50	62	305	35	33	137	35	48	114	26	982	1	0	12	0	13
	8:45 AM	41	77	50	80	293	45	30	144	30	46	113	13	962	1	0	2	0	3
	VOLUMES	306	796	522	537	2,391	385	357	1,413	244	504	1,273	151	8,879	12	0	42	0	54
	APPROACH %	19%	49%	32%	16%	72%	12%	18%	70%	12%	26%	66%	8%						
	APP/DEPART	1,624	/	1,262	3,313	/	3,151	2,014	/	2,472	1,928	/	1,994	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	161	471	323	283	1,186	232	221	826	104	305	799	87	4,998					
	APPROACH %	17%	49%	34%	17%	70%	14%	19%	72%	9%	26%	67%	7%						
	PEAK HR FACTOR		0.847			0.969			0.925			0.866		0.928					
	APP/DEPART	955	/	761	1,701	/	1,601	1,151	/	1,432	1,191	/	1,204	0					
PM	4:00 PM	76	196	78	49	111	33	54	171	24	60	246	58	1,156	0	0	5	0	5
	4:15 PM	64	193	55	55	132	51	62	204	29	59	255	50	1,209	0	0	8	0	8
	4:30 PM	64	222	62	45	105	28	69	185	25	79	251	45	1,180	0	0	13	0	13
	4:45 PM	68	211	58	59	111	24	72	196	39	74	292	51	1,255	2	0	9	0	11
	5:00 PM	61	186	47	72	154	31	111	232	42	80	262	36	1,314	1	0	13	0	14
	5:15 PM	52	212	49	77	157	29	82	242	39	68	287	46	1,340	1	0	6	0	7
	5:30 PM	62	259	64	59	154	42	77	207	27	75	255	66	1,347	1	0	11	0	12
	5:45 PM	68	249	48	66	126	52	75	168	40	61	213	49	1,215	3	0	14	0	17
	VOLUMES	515	1,728	461	482	1,050	290	602	1,605	265	556	2,061	401	10,016	8	0	79	0	87
	APPROACH %	19%	64%	17%	26%	58%	16%	24%	65%	11%	18%	68%	13%						
	APP/DEPART	2,704	/	2,652	1,822	/	1,879	2,472	/	2,548	3,018	/	2,937	0					
	BEGIN PEAK HR		4:45 PM																
	VOLUMES	243	868	218	267	576	126	342	877	147	297	1,096	199	5,256					
	APPROACH %	18%	65%	16%	28%	59%	13%	25%	64%	11%	19%	69%	13%						
	PEAK HR FACTOR		0.863			0.921			0.887			0.954		0.976					
	APP/DEPART	1,329	/	1,370	969	/	1,025	1,366	/	1,362	1,592	/	1,499	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:15 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	4:45 PM					0	0	0	0	0					

INTERSECTION TURNING MOVEMENT COUNTS

T816

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Thu, Feb 16, 17

LOCATION: Santa Ana
NORTH & SOUTH: Grand
EAST & WEST: I-5 NB Ramps

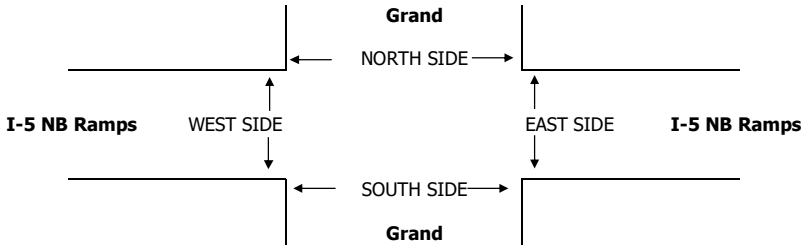
PROJECT #: SC1220
LOCATION #: 3
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N ◄ W E ► S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Grand			SOUTHBOUND Grand			EASTBOUND I-5 NB Ramps			WESTBOUND I-5 NB Ramps				U-TURNS				
	LANES:	NL X	NT 2	NR 1	SL 1	ST 3	SR X	EL X	ET X	ER X	WL 2	WT X	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	0	123	106	19	411	0	0	0	0	104	0	31	794	0	3	0	0	3
	7:15 AM	0	184	108	8	495	0	0	0	0	97	0	15	907	0	3	0	0	3
	7:30 AM	0	226	137	12	482	0	0	0	0	94	0	31	982	0	1	0	0	1
	7:45 AM	0	229	117	20	425	0	0	0	0	131	0	43	965	0	7	0	0	7
	8:00 AM	0	167	103	16	412	0	0	0	0	120	0	33	851	0	6	0	0	6
	8:15 AM	0	147	103	12	389	0	0	0	0	97	0	41	789	0	5	0	0	5
	8:30 AM	0	153	104	9	380	0	0	0	0	93	0	33	772	0	4	0	0	4
	8:45 AM	0	151	96	6	354	0	0	0	0	115	0	37	759	0	0	0	0	0
	VOLUMES	0	1,380	874	102	3,348	0	0	0	0	851	0	264	6,819	0	29	0	0	29
	APPROACH %	0%	61%	39%	3%	97%	0%	0%	0%	0%	76%	0%	24%						
	APP/DEPART	2,254	/	1,673	3,450	/	4,199	0	/	947	1,115	/	0	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	0	806	465	56	1,814	0	0	0	0	442	0	122	3,705					
	APPROACH %	0%	63%	37%	3%	97%	0%	0%	0%	0%	78%	0%	22%						
	PEAK HR FACTOR		0.875			0.929			0.000			0.810		0.943					
	APP/DEPART	1,271	/	945	1,870	/	2,256	0	/	504	564	/	0	0					
PM	4:00 PM	0	292	260	12	217	0	0	0	0	38	0	38	857	0	8	0	0	8
	4:15 PM	0	286	262	14	210	0	0	0	0	43	0	46	861	0	7	0	0	7
	4:30 PM	0	313	244	14	223	0	0	0	0	50	0	46	890	0	7	0	0	7
	4:45 PM	0	337	221	8	262	0	0	0	0	37	0	36	901	0	7	0	0	7
	5:00 PM	0	307	246	12	280	0	0	0	0	22	0	42	909	0	6	0	0	6
	5:15 PM	0	335	273	12	273	0	0	0	0	42	0	28	963	0	6	0	0	6
	5:30 PM	0	286	233	12	285	0	0	0	0	30	0	51	897	0	6	0	0	6
	5:45 PM	0	330	219	10	291	0	0	0	0	44	0	45	939	0	5	0	0	5
	VOLUMES	0	2,486	1,958	94	2,041	0	0	0	0	306	0	332	7,217	0	52	0	0	52
	APPROACH %	0%	56%	44%	4%	96%	0%	0%	0%	0%	48%	0%	52%						
	APP/DEPART	4,444	/	2,870	2,135	/	2,347	0	/	2,000	638	/	0	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	0	1,258	971	46	1,129	0	0	0	0	138	0	166	3,708					
	APPROACH %	0%	56%	44%	4%	96%	0%	0%	0%	0%	45%	0%	55%						
	PEAK HR FACTOR		0.917			0.976			0.000			0.854		0.963					
	APP/DEPART	2,229	/	1,447	1,175	/	1,267	0	/	994	304	/	0	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

INTERSECTION TURNING MOVEMENT COUNTS

T816

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Thu, Feb 16, 17

LOCATION: Santa Ana
NORTH & SOUTH: Grand
EAST & WEST: Santa Ana

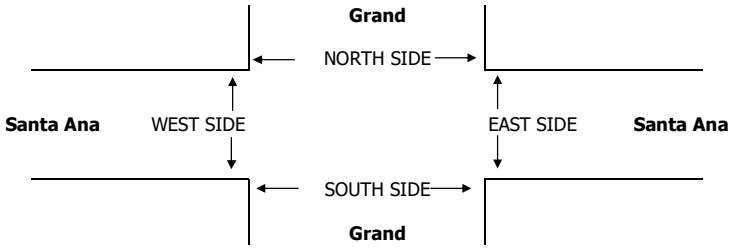
PROJECT #: SC1220
LOCATION #: 2
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — N / W / E / S

☑ Add U-Turns to Left Turns

		NORTHBOUND Grand			SOUTHBOUND Grand			EASTBOUND Santa Ana			WESTBOUND Santa Ana				U-TURNS				
	LANES:	NL 1	NT 2.5	NR 0.5	SL 1	ST 2	SR 2	EL 2	ET 1	ER 2	WL 0.5	WT 1	WR 0.5	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	14	210	14	25	256	235	30	35	126	2	25	8	980	0	0	0	0	0
	7:15 AM	12	194	11	49	263	241	62	49	90	1	51	8	1,031	1	0	0	0	1
	7:30 AM	18	292	18	37	308	253	60	61	99	2	123	9	1,280	2	0	0	0	2
	7:45 AM	18	273	13	22	307	239	75	51	97	0	141	9	1,245	0	0	0	0	0
	8:00 AM	20	214	7	28	253	265	58	49	129	1	47	7	1,078	0	1	0	0	1
	8:15 AM	19	214	9	10	271	219	45	33	98	3	27	3	951	4	0	0	0	4
	8:30 AM	22	198	10	15	275	186	46	25	99	3	32	3	914	2	0	0	0	2
	8:45 AM	17	186	5	13	286	181	49	22	95	5	24	11	894	1	0	0	0	1
	VOLUMES	140	1,781	87	199	2,219	1,819	425	325	833	17	470	58	8,373	10	1	0	0	11
	APPROACH %	7%	89%	4%	5%	52%	43%	27%	21%	53%	3%	86%	11%						
	APP/DEPART	2,008	/	2,265	4,237	/	3,079	1,583	/	610	545	/	2,419	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	68	973	49	136	1,131	998	255	210	415	4	362	33	4,634					
	APPROACH %	6%	89%	4%	6%	50%	44%	29%	24%	47%	1%	91%	8%						
	PEAK HR FACTOR		0.831			0.947			0.932			0.665		0.905					
	APP/DEPART	1,090	/	1,262	2,265	/	1,553	880	/	394	399	/	1,425	0					
PM	4:00 PM	31	435	3	5	131	103	63	18	68	4	19	12	892	0	1	0	0	1
	4:15 PM	33	417	3	12	160	98	87	22	91	4	26	23	976	2	1	0	0	3
	4:30 PM	50	461	3	5	126	122	95	24	88	6	45	21	1,046	1	1	0	0	2
	4:45 PM	40	441	4	9	167	118	87	23	105	3	41	18	1,056	0	0	0	0	0
	5:00 PM	48	427	5	5	190	120	85	91	77	3	34	22	1,107	0	0	0	0	0
	5:15 PM	47	483	5	8	177	126	80	47	69	7	40	20	1,109	5	1	0	0	6
	5:30 PM	38	457	13	12	177	137	73	37	84	5	33	23	1,089	1	0	0	0	1
	5:45 PM	29	465	7	8	183	133	80	19	76	4	23	29	1,056	2	0	0	0	2
	VOLUMES	316	3,586	43	64	1,311	957	650	281	658	36	261	168	8,331	11	4	0	0	15
	APPROACH %	8%	91%	1%	3%	56%	41%	41%	18%	41%	8%	56%	36%						
	APP/DEPART	3,945	/	4,408	2,332	/	2,016	1,589	/	384	465	/	1,523	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	162	1,832	30	33	727	516	318	194	306	19	130	94	4,361					
	APPROACH %	8%	91%	1%	3%	57%	40%	39%	24%	37%	8%	53%	39%						
	PEAK HR FACTOR		0.946			0.979			0.808			0.907		0.983					
	APP/DEPART	2,024	/	2,245	1,276	/	1,060	818	/	256	243	/	800	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

1862

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Thu, May 16, 19

LOCATION: Santa Ana
NORTH & SOUTH: Grand
EAST & WEST: 1st

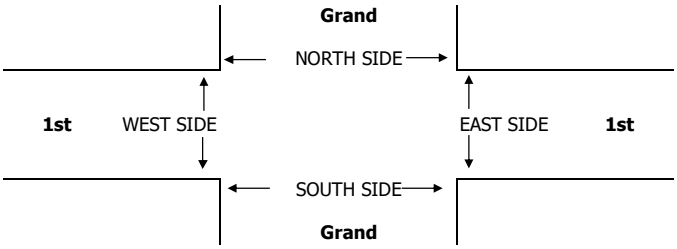
PROJECT #: SC2183
LOCATION #: 25
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N ◄ W E ► S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Grand			SOUTHBOUND Grand			EASTBOUND 1st			WESTBOUND 1st				U-TURNS				
	LANES:	NL 2	NT 3	NR 0	SL 2	ST 3	SR 1	EL 2	ET 3	ER 0	WL 2	WT 2	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	14	109	12	23	348	25	48	241	26	50	192	18	1,106	0	0	0	1	1
	7:15 AM	42	163	8	25	357	47	64	246	37	50	212	23	1,274	0	0	1	0	1
	7:30 AM	63	223	10	25	379	36	91	169	70	76	233	28	1,403	0	2	2	0	4
	7:45 AM	69	197	9	18	351	38	94	177	62	71	233	24	1,343	0	1	0	2	3
	8:00 AM	36	160	7	30	330	51	79	193	53	47	180	23	1,189	0	0	1	2	3
	8:15 AM	35	136	11	22	381	35	67	249	34	47	182	25	1,224	0	0	2	1	3
	8:30 AM	42	146	6	24	342	39	63	250	43	44	191	30	1,220	0	1	2	2	5
	8:45 AM	35	138	7	19	346	35	48	257	56	53	180	27	1,201	0	0	0	4	4
	VOLUMES	336	1,272	70	186	2,834	306	554	1,782	381	438	1,603	198	9,960	0	4	8	12	24
	APPROACH %	20%	76%	4%	6%	85%	9%	20%	66%	14%	20%	72%	9%						
	APP/DEPART	1,678	/	2,020	3,326	/	3,641	2,717	/	2,046	2,239	/	2,253	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	210	743	34	98	1,417	172	328	785	222	244	858	98	5,209					
	APPROACH %	21%	75%	3%	6%	84%	10%	25%	59%	17%	20%	72%	8%						
	PEAK HR FACTOR		0.834			0.959			0.962			0.890		0.928					
	APP/DEPART	987	/	1,168	1,687	/	1,879	1,335	/	918	1,200	/	1,244	0					
PM	4:00 PM	42	338	10	27	149	75	67	224	26	31	238	34	1,261	0	2	3	1	6
	4:15 PM	31	300	12	35	181	63	69	201	23	33	213	57	1,218	0	1	2	4	7
	4:30 PM	47	299	7	34	191	75	63	240	20	39	232	44	1,291	2	2	0	3	7
	4:45 PM	43	311	8	22	178	66	75	243	21	46	248	36	1,297	0	0	4	5	9
	5:00 PM	37	279	10	27	149	61	88	265	21	36	239	45	1,257	0	4	3	2	9
	5:15 PM	37	279	7	22	190	47	71	216	28	30	231	49	1,207	0	3	1	0	4
	5:30 PM	37	250	8	21	168	69	85	204	26	26	223	40	1,157	0	2	3	0	5
	5:45 PM	43	269	11	18	166	66	93	225	18	34	221	55	1,219	0	2	0	2	4
	VOLUMES	317	2,325	73	206	1,372	522	611	1,818	183	275	1,845	360	9,907	2	16	16	17	51
	APPROACH %	12%	86%	3%	10%	65%	25%	23%	70%	7%	11%	74%	15%						
	APP/DEPART	2,715	/	3,296	2,100	/	1,815	2,612	/	2,098	2,480	/	2,698	0					
	BEGIN PEAK HR		4:00 PM																
	VOLUMES	163	1,248	37	118	699	279	274	908	90	149	931	171	5,067					
	APPROACH %	11%	86%	3%	11%	64%	25%	22%	71%	7%	12%	74%	14%						
	PEAK HR FACTOR		0.928			0.913			0.938			0.948		0.977					
	APP/DEPART	1,448	/	1,689	1,096	/	927	1,272	/	1,071	1,251	/	1,380	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:15 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	4:00 PM					0	0	0	0	0					

1863

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

DATE: Thu, Feb 11, 16

LOCATION: Santa Ana
NORTH & SOUTH: Grand
EAST & WEST: Chestnut

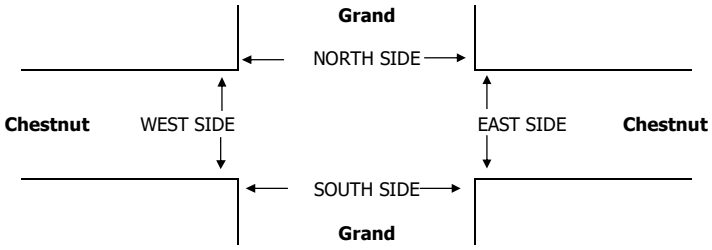
PROJECT #: SC0846
LOCATION #: 29
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Grand			SOUTHBOUND Grand			EASTBOUND Chestnut			WESTBOUND Chestnut				U-TURNS				
		NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL	NB	SB	EB	WB	TTL
	LANES:	1	3	0	1	3	0	1	2	0	1	2	0		0	0	0	0	
AM	7:00 AM	10	136	30	31	363	21	15	77	22	23	28	41	797	0	0	0	0	0
	7:15 AM	9	152	44	39	406	14	21	114	35	21	82	53	990	0	0	0	0	0
	7:30 AM	15	186	48	41	443	9	13	135	34	21	109	54	1,108	0	1	0	0	1
	7:45 AM	16	184	47	31	473	10	12	89	19	40	105	66	1,092	0	0	0	0	0
	8:00 AM	10	186	17	32	396	14	11	64	17	31	37	34	849	0	0	0	0	0
	8:15 AM	4	138	26	32	403	14	18	70	21	17	27	32	802	0	0	0	0	0
	8:30 AM	5	157	20	30	360	9	15	54	21	16	38	35	760	0	0	0	0	0
	8:45 AM	13	133	21	24	346	22	18	39	11	15	39	21	702	0	0	0	0	0
	VOLUMES	82	1,272	253	260	3,190	113	123	642	180	184	465	336	7,100	0	1	0	0	1
	APPROACH %	5%	79%	16%	7%	90%	3%	13%	68%	19%	19%	47%	34%						
	APP/DEPART	1,607	/	1,732	3,563	/	3,554	945	/	1,154	985	/	660	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	50	708	156	143	1,718	47	57	402	105	113	333	207	4,039					
	APPROACH %	5%	77%	17%	7%	90%	2%	10%	71%	19%	17%	51%	32%						
	PEAK HR FACTOR		0.918			0.928			0.775			0.774		0.911					
	APP/DEPART	914	/	973	1,908	/	1,936	564	/	700	653	/	430	0					
PM	4:00 PM	40	279	50	37	166	21	15	82	14	29	69	58	860	0	0	0	0	0
	4:15 PM	25	246	42	35	152	22	22	58	11	32	97	45	787	0	0	0	0	0
	4:30 PM	42	243	41	32	162	34	26	69	21	21	76	46	813	0	1	0	0	1
	4:45 PM	27	241	41	39	158	30	26	64	13	20	78	37	774	0	1	0	0	1
	5:00 PM	28	182	20	24	176	35	23	59	13	26	96	43	725	0	0	0	0	0
	5:15 PM	35	232	42	36	192	19	25	81	13	18	74	26	793	0	0	0	1	1
	5:30 PM	30	222	61	31	184	21	16	86	16	26	84	62	839	0	0	0	0	0
	5:45 PM	18	278	42	28	163	22	25	66	14	33	84	45	818	0	2	0	0	2
	VOLUMES	245	1,923	339	262	1,353	204	178	565	115	205	658	362	6,409	0	4	0	1	5
	APPROACH %	10%	77%	14%	14%	74%	11%	21%	66%	13%	17%	54%	30%						
	APP/DEPART	2,507	/	2,467	1,819	/	1,672	858	/	1,163	1,225	/	1,107	0					
	BEGIN PEAK HR		4:00 PM																
	VOLUMES	134	1,009	174	143	638	107	89	273	59	102	320	186	3,234					
	APPROACH %	10%	77%	13%	16%	72%	12%	21%	65%	14%	17%	53%	31%						
	PEAK HR FACTOR		0.892			0.974			0.907			0.874		0.940					
	APP/DEPART	1,317	/	1,286	888	/	799	421	/	588	608	/	561	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	18	9	10	2	39	17	8	7	2	34	1	1	3	0	5
	7:15 AM	59	25	29	13	126	57	20	29	12	118	2	5	0	1	8
	7:30 AM	178	35	63	42	318	175	33	62	39	309	3	2	1	3	9
	7:45 AM	122	11	20	21	174	119	10	19	18	166	3	1	1	3	8
	8:00 AM	4	3	2	2	11	3	1	1	2	7	1	2	1	0	4
	8:15 AM	2	6	7	0	15	2	5	5	0	12	0	1	2	0	3
	8:30 AM	5	2	3	2	12	4	1	2	2	9	1	1	1	0	3
	8:45 AM	0	1	2	1	4	0	0	2	1	3	0	1	0	0	1
	TOTAL	388	92	136	83	699	377	78	127	76	658	11	14	9	7	41
PM	4:00 PM	69	11	4	17	101	68	9	3	16	96	1	2	1	1	5
	4:15 PM	29	9	4	7	49	27	7	2	6	42	2	2	2	1	7
	4:30 PM	28	6	9	7	50	23	4	5	5	37	5	2	4	2	13
	4:45 PM	9	4	6	8	27	6	1	2	5	14	3	3	4	3	13
	5:00 PM	10	8	7	4	29	10	4	5	2	21	0	4	2	2	8
	5:15 PM	6	1	3	2	12	4	1	2	1	8	2	0	1	1	4
	5:30 PM	6	7	6	6	25	1	7	2	4	14	5	0	4	2	11
	5:45 PM	5	4	3	6	18	4	4	2	5	15	1	0	1	1	3
	TOTAL	162	50	42	57	311	143	37	23	44	247	19	13	19	13	64

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

DATE: Tue, Feb 9, 16

LOCATION: Santa Ana
NORTH & SOUTH: Grand
EAST & WEST: McFadden

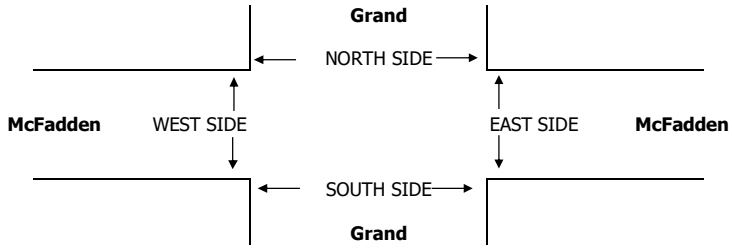
PROJECT #: SC0846
LOCATION #: 38
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Grand			SOUTHBOUND Grand			EASTBOUND McFadden			WESTBOUND McFadden				U-TURNS				
	LANES:	NL 1	NT 3	NR 0	SL 1	ST 3	SR 0	EL 1	ET 2	ER 0	WL 1	WT 2	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	22	137	16	40	320	28	37	136	31	22	90	20	899	0	0	0	0	0
	7:15 AM	28	144	22	54	321	35	46	156	48	25	93	14	986	0	0	0	0	0
	7:30 AM	37	180	29	44	439	41	48	127	52	20	81	26	1,124	0	0	0	0	0
	7:45 AM	50	192	24	68	362	43	43	135	43	28	133	29	1,150	0	0	0	0	0
	8:00 AM	49	177	28	61	405	29	41	159	31	39	96	32	1,147	0	0	0	0	0
	8:15 AM	19	130	28	45	317	29	28	143	20	29	78	29	895	0	0	0	0	0
	8:30 AM	16	125	18	35	329	21	31	106	21	25	57	9	793	0	0	0	0	0
	8:45 AM	13	114	18	47	270	27	25	87	17	18	70	30	736	0	0	0	0	0
	VOLUMES	234	1,199	183	394	2,763	253	299	1,049	263	206	698	189	7,730	0	0	0	0	0
	APPROACH %	14%	74%	11%	12%	81%	7%	19%	65%	16%	19%	64%	17%						
	APP/DEPART	1,616	/	1,687	3,410	/	3,232	1,611	/	1,626	1,093	/	1,185	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	164	693	103	227	1,527	148	178	577	174	112	403	101	4,407					
	APPROACH %	17%	72%	11%	12%	80%	8%	19%	62%	19%	18%	65%	16%						
	PEAK HR FACTOR		0.902			0.907			0.929			0.811		0.958					
	APP/DEPART	960	/	972	1,902	/	1,813	929	/	907	616	/	715	0					
PM	4:00 PM	39	278	39	28	135	53	33	96	19	24	138	35	917	0	0	0	0	0
	4:15 PM	42	244	50	41	143	56	28	93	19	21	169	26	932	0	0	0	0	0
	4:30 PM	52	280	47	40	161	33	30	99	25	17	163	34	981	0	0	0	0	0
	4:45 PM	52	285	47	30	118	56	25	96	16	16	143	36	920	0	0	0	0	0
	5:00 PM	42	264	41	34	172	36	38	127	24	27	178	39	1,022	0	0	0	0	0
	5:15 PM	48	276	55	28	150	36	30	135	14	22	175	36	1,005	0	0	0	0	0
	5:30 PM	39	291	61	35	123	49	20	96	20	22	176	26	958	0	0	0	0	0
	5:45 PM	27	275	50	22	130	45	21	121	14	15	160	37	917	0	0	0	0	0
	VOLUMES	341	2,193	390	258	1,132	364	225	863	151	164	1,302	269	7,652	0	0	0	0	0
	APPROACH %	12%	75%	13%	15%	65%	21%	18%	70%	12%	9%	75%	16%						
	APP/DEPART	2,924	/	2,687	1,754	/	1,447	1,239	/	1,511	1,735	/	2,007	0					
	BEGIN PEAK HR		4:30 PM																
	VOLUMES	194	1,105	190	132	601	161	123	457	79	82	659	145	3,928					
	APPROACH %	13%	74%	13%	15%	67%	18%	19%	69%	12%	9%	74%	16%						
	PEAK HR FACTOR		0.969			0.924			0.872			0.908		0.961					
	APP/DEPART	1,489	/	1,373	894	/	762	659	/	779	886	/	1,014	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	14	11	8	7	40	10	8	7	4	29	4	3	1	3	11
	7:15 AM	23	31	13	23	90	19	30	12	20	81	4	1	1	3	9
	7:30 AM	23	51	22	21	117	21	49	22	18	110	2	2	0	3	7
	7:45 AM	52	105	38	55	250	50	104	38	49	241	2	1	0	6	9
	8:00 AM	17	26	5	8	56	16	24	4	6	50	1	2	1	2	6
	8:15 AM	11	7	6	5	29	9	7	5	5	26	2	0	1	0	3
	8:30 AM	13	8	9	6	36	9	8	7	6	30	4	0	2	0	6
	8:45 AM	5	2	2	3	12	3	2	2	3	10	2	0	0	0	2
	TOTAL	158	241	103	128	630	137	232	97	111	577	21	9	6	17	53
PM	4:00 PM	21	18	14	7	60	16	12	12	7	47	5	6	2	0	13
	4:15 PM	16	11	8	8	43	13	10	5	7	35	3	1	3	1	8
	4:30 PM	12	19	14	21	66	6	15	11	16	48	6	4	3	5	18
	4:45 PM	10	9	7	3	29	7	4	5	2	18	3	5	2	1	11
	5:00 PM	16	11	12	7	46	13	10	12	6	41	3	1	0	1	5
	5:15 PM	18	15	6	4	43	14	11	6	4	35	4	4	0	0	8
	5:30 PM	7	13	2	5	27	4	9	2	2	17	3	4	0	3	10
	5:45 PM	15	13	3	3	34	11	8	1	1	21	4	5	2	2	13
	TOTAL	115	109	66	58	348	84	79	54	45	262	31	30	12	13	86

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Thu, May 16, 19

LOCATION: Santa Ana
NORTH & SOUTH: Grand
EAST & WEST: Edinger

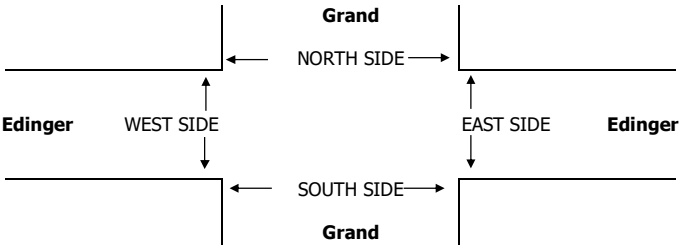
PROJECT #: SC2183
LOCATION #: 26
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Grand			SOUTHBOUND Grand			EASTBOUND Edinger			WESTBOUND Edinger				U-TURNS				
	LANES:	NL 1	NT 3	NR 0	SL 1	ST 3	SR 0	EL 1	ET 3	ER 0	WL 1	WT 3	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	11	97	25	61	253	50	63	309	35	18	141	30	1,093	0	0	0	0	0
	7:15 AM	4	119	20	58	263	55	82	333	19	30	125	27	1,135	0	0	0	1	1
	7:30 AM	15	143	26	54	288	49	88	328	33	46	144	34	1,248	0	0	1	3	4
	7:45 AM	9	120	28	51	274	68	90	320	34	45	160	35	1,234	0	0	1	2	3
	8:00 AM	18	85	17	58	309	49	55	337	40	28	144	28	1,168	0	0	0	1	1
	8:15 AM	25	94	31	58	249	43	48	306	28	22	191	33	1,128	0	0	3	2	5
	8:30 AM	11	94	25	52	237	39	50	239	34	29	125	33	968	0	0	3	4	7
	8:45 AM	15	105	25	46	236	37	44	245	39	24	109	32	957	0	0	2	5	7
	VOLUMES	108	857	197	438	2,109	390	520	2,417	262	242	1,139	252	8,931	0	0	10	18	28
	APPROACH %	9%	74%	17%	15%	72%	13%	16%	76%	8%	15%	70%	15%						
	APP/DEPART	1,162	/	1,619	2,937	/	2,595	3,199	/	3,070	1,633	/	1,647	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	46	467	91	221	1,134	221	315	1,318	126	149	573	124	4,785					
	APPROACH %	8%	77%	15%	14%	72%	14%	18%	75%	7%	18%	68%	15%						
	PEAK HR FACTOR		0.821			0.947			0.979			0.881		0.959					
	APP/DEPART	604	/	904	1,576	/	1,402	1,759	/	1,637	846	/	842	0					
PM	4:00 PM	70	317	45	41	88	52	57	193	14	23	299	43	1,242	0	0	2	1	3
	4:15 PM	59	317	35	36	92	62	32	190	13	21	332	54	1,243	0	0	1	3	4
	4:30 PM	68	323	23	44	111	73	52	186	16	26	321	39	1,282	0	0	4	1	5
	4:45 PM	54	334	35	38	111	54	46	190	24	20	331	45	1,282	0	0	2	0	2
	5:00 PM	65	356	28	48	122	75	53	211	17	17	328	38	1,358	0	0	5	2	7
	5:15 PM	65	348	28	29	87	49	44	229	15	19	335	49	1,297	0	0	2	3	5
	5:30 PM	55	345	28	48	83	48	56	229	13	17	325	39	1,286	0	0	0	0	0
	5:45 PM	41	300	22	36	66	64	55	181	11	14	336	45	1,171	0	0	1	0	1
	VOLUMES	477	2,640	244	320	760	477	395	1,609	123	157	2,607	352	10,161	0	0	17	10	27
	APPROACH %	14%	79%	7%	21%	49%	31%	19%	76%	6%	5%	84%	11%						
	APP/DEPART	3,361	/	3,370	1,557	/	1,030	2,127	/	2,183	3,116	/	3,578	0					
	BEGIN PEAK HR		4:45 PM																
	VOLUMES	239	1,383	119	163	403	226	199	859	69	73	1,319	171	5,223					
	APPROACH %	14%	79%	7%	21%	51%	29%	18%	76%	6%	5%	84%	11%						
	PEAK HR FACTOR		0.969			0.808			0.945			0.970		0.962					
	APP/DEPART	1,741	/	1,744	792	/	540	1,127	/	1,146	1,563	/	1,793	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:15 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	4:45 PM					0	0	0	0	0					

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Thu, May 16, 19

LOCATION: Santa Ana
NORTH & SOUTH: Grand
EAST & WEST: Warner

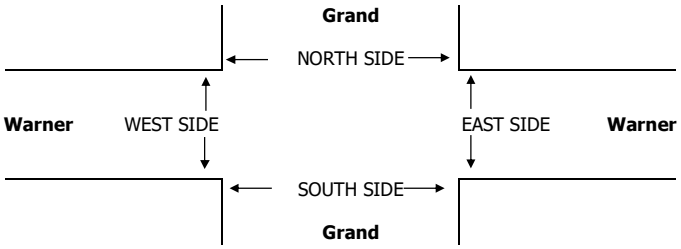
PROJECT #: SC2183
LOCATION #: 27
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N ◄ W E ► S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Grand			SOUTHBOUND Grand			EASTBOUND Warner			WESTBOUND Warner				U-TURNS				
	LANES:	NL 1	NT 3	NR 0	SL 1	ST 3	SR 0	EL 2	ET 3	ER 1	WL 2	WT 2	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	41	102	39	31	90	48	46	182	77	10	73	14	753	0	0	0	0	0
	7:15 AM	50	116	50	42	124	53	67	206	82	26	74	24	914	0	0	0	0	0
	7:30 AM	46	125	49	47	158	44	62	207	108	19	114	23	1,002	0	0	1	0	1
	7:45 AM	36	133	51	60	146	50	57	286	89	21	107	39	1,075	0	0	0	0	0
	8:00 AM	35	130	65	48	140	45	67	205	76	21	89	43	964	0	1	0	0	1
	8:15 AM	26	98	56	52	128	52	75	188	56	23	113	27	894	1	0	0	0	1
	8:30 AM	32	87	40	57	142	51	49	188	49	26	84	13	818	0	0	0	0	0
	8:45 AM	30	87	32	58	125	30	66	169	55	19	91	37	799	0	0	0	0	0
	VOLUMES	296	878	382	395	1,053	373	489	1,631	592	165	745	220	7,219	1	1	1	0	3
	APPROACH %	19%	56%	25%	22%	58%	20%	18%	60%	22%	15%	66%	19%						
	APP/DEPART	1,556	/	1,587	1,821	/	1,811	2,712	/	2,407	1,130	/	1,414	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	167	504	215	197	568	192	253	904	355	87	384	129	3,955					
	APPROACH %	19%	57%	24%	21%	59%	20%	17%	60%	23%	15%	64%	22%						
	PEAK HR FACTOR		0.963			0.935			0.875			0.898		0.920					
	APP/DEPART	886	/	886	957	/	1,010	1,512	/	1,315	600	/	744	0					
PM	4:00 PM	45	159	48	29	68	61	108	146	35	28	242	82	1,051	0	0	0	0	0
	4:15 PM	51	222	40	26	73	56	93	125	36	36	204	76	1,038	0	0	0	2	2
	4:30 PM	56	211	32	32	106	57	86	120	42	45	211	71	1,069	0	1	0	0	1
	4:45 PM	65	249	45	29	132	54	77	163	36	40	248	65	1,203	0	0	0	0	0
	5:00 PM	60	188	43	38	131	62	84	164	36	49	220	81	1,156	0	0	0	0	0
	5:15 PM	56	218	46	31	104	60	86	161	39	29	254	91	1,175	0	1	0	0	1
	5:30 PM	58	209	57	46	99	38	91	161	29	31	232	75	1,126	0	0	0	1	1
	5:45 PM	53	233	40	24	75	58	75	159	15	23	208	79	1,042	0	0	0	0	0
	VOLUMES	444	1,689	351	255	788	446	700	1,199	268	281	1,819	620	8,860	0	2	0	3	5
	APPROACH %	18%	68%	14%	17%	53%	30%	32%	55%	12%	10%	67%	23%						
	APP/DEPART	2,484	/	3,011	1,489	/	1,334	2,167	/	1,806	2,720	/	2,709	0					
	BEGIN PEAK HR		4:45 PM																
	VOLUMES	239	864	191	144	466	214	338	649	140	149	954	312	4,660					
	APPROACH %	18%	67%	15%	17%	57%	26%	30%	58%	12%	11%	67%	22%						
	PEAK HR FACTOR		0.901			0.892			0.985			0.946		0.968					
	APP/DEPART	1,294	/	1,515	824	/	754	1,127	/	984	1,415	/	1,407	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR			7:15 AM			0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR			4:45 PM			0	0	0	0	0					

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Thu, May 16, 19

LOCATION: Santa Ana
NORTH & SOUTH: SR-55 NB Ramps
EAST & WEST: Dyer

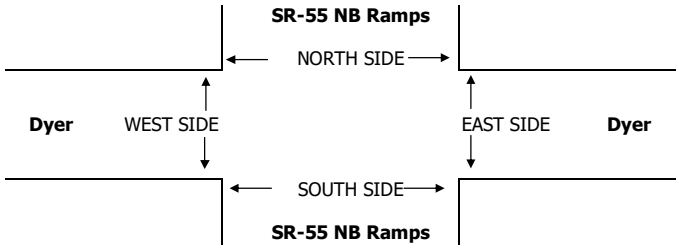
PROJECT #: SC2183
LOCATION #: 29
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☐ Add U-Turns to Left Turns

		NORTHBOUND SR-55 NB Ramps			SOUTHBOUND SR-55 NB Ramps			EASTBOUND Dyer			WESTBOUND Dyer				U-TURNS				
	LANES:	NL 1.5	NT X	NR 1.5	SL X	ST X	SR X	EL X	ET 3	ER 1	WL X	WT 3	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	171	0	88	0	0	0	0	333	152	0	120	84	948	0	0	0	0	0
	7:15 AM	167	0	116	0	0	0	0	357	172	0	152	69	1,033	0	0	0	0	0
	7:30 AM	142	0	114	0	0	0	0	390	139	0	157	95	1,037	0	0	0	0	0
	7:45 AM	169	0	121	0	0	0	0	385	158	0	179	82	1,094	0	0	0	0	0
	8:00 AM	128	0	99	0	0	0	0	366	142	0	168	110	1,013	0	0	1	0	1
	8:15 AM	112	0	117	0	0	0	0	376	167	0	207	141	1,120	0	0	0	0	0
	8:30 AM	110	0	107	0	0	0	0	300	146	0	195	97	955	0	0	0	0	0
	8:45 AM	140	0	99	0	0	0	0	276	139	0	200	89	943	0	0	0	0	0
	VOLUMES	1,139	0	861	0	0	0	0	2,783	1,215	0	1,378	767	8,144	0	0	1	0	1
	APPROACH %	57%	0%	43%	0%	0%	0%	0%	70%	30%	0%	64%	36%						
	APP/DEPART	2,000	/	767	0	/	1,215	3,999	/	3,644	2,145	/	2,518	0					
	BEGIN PEAK HR		7:30 AM																
	VOLUMES	551	0	451	0	0	0	0	1,517	606	0	711	428	4,265					
	APPROACH %	55%	0%	45%	0%	0%	0%	0%	71%	29%	0%	62%	38%						
	PEAK HR FACTOR		0.864			0.000			0.978			0.818		0.952					
	APP/DEPART	1,002	/	428	0	/	606	2,124	/	1,968	1,139	/	1,263	0					
PM	4:00 PM	56	0	16	0	0	0	0	241	175	0	342	249	1,079	0	0	0	0	0
	4:15 PM	37	0	16	0	0	0	0	279	169	0	370	258	1,129	0	0	0	0	0
	4:30 PM	42	0	8	0	0	0	0	278	170	0	366	248	1,112	0	0	0	0	0
	4:45 PM	48	0	14	0	0	0	0	312	155	0	360	241	1,130	0	0	0	0	0
	5:00 PM	40	0	11	0	0	0	0	320	177	0	399	241	1,188	0	0	0	0	0
	5:15 PM	42	0	14	0	0	0	0	351	176	0	395	242	1,220	0	0	0	0	0
	5:30 PM	56	0	11	0	0	0	0	342	166	0	375	231	1,181	0	0	0	0	0
	5:45 PM	38	0	24	0	0	0	0	278	138	0	318	197	993	0	0	0	0	0
	VOLUMES	359	0	114	0	0	0	0	2,401	1,326	0	2,925	1,907	9,032	0	0	0	0	0
	APPROACH %	76%	0%	24%	0%	0%	0%	0%	64%	36%	0%	61%	39%						
	APP/DEPART	473	/	1,907	0	/	1,326	3,727	/	2,515	4,832	/	3,284	0					
	BEGIN PEAK HR		4:45 PM																
	VOLUMES	186	0	50	0	0	0	0	1,325	674	0	1,529	955	4,719					
	APPROACH %	79%	0%	21%	0%	0%	0%	0%	66%	34%	0%	62%	38%						
	PEAK HR FACTOR		0.881			0.000			0.948			0.970		0.967					
	APP/DEPART	236	/	955	0	/	674	1,999	/	1,375	2,484	/	1,715	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	AM BEGIN PEAK HR	7:30 AM					0	0	0	0	0					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	PM BEGIN PEAK HR	4:45 PM					0	0	0	0	0					

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 pacific@aimtd.com

DATE: Tue, Mar 1, 16

LOCATION: Santa Ana
NORTH & SOUTH: Cambridge
EAST & WEST: La Veta

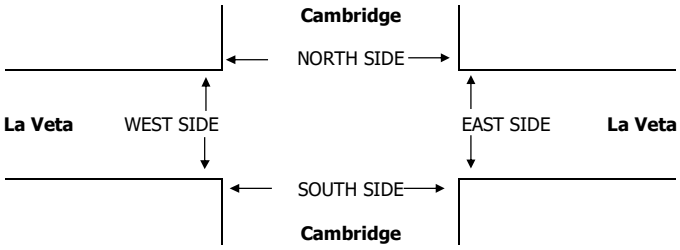
PROJECT #: SC0846
LOCATION #: 6
CONTROL: STOP ALL

NOTES:

AM PM MD OTHER OTHER — ▲ N, ◄ W, E ►, S ▼

☑ Add U-Turns to Left Turns

		NORTHBOUND Cambridge			SOUTHBOUND Cambridge			EASTBOUND La Veta			WESTBOUND La Veta				U-TURNS				
		NL	NT	NR	SL	ST	SR	EL	ET	ER	WL	WT	WR	TOTAL	NB	SB	EB	WB	TTL
	LANES:	1	1	X	X	1	1	1	X	1	X	X	X		0	0	0	0	
AM	7:00 AM	19	21	0	0	76	37	16	0	20	0	0	0	189	0	0	0	0	0
	7:15 AM	22	34	0	0	114	47	18	0	40	0	0	0	275	0	0	0	0	0
	7:30 AM	35	46	0	0	113	62	33	0	46	0	0	0	335	0	0	0	0	0
	7:45 AM	51	68	0	0	106	68	43	0	50	0	0	0	386	0	0	0	0	0
	8:00 AM	45	48	0	0	128	58	18	0	43	0	0	0	340	0	0	0	0	0
	8:15 AM	27	32	0	0	112	55	23	0	24	0	0	0	273	0	0	0	0	0
	8:30 AM	16	27	0	0	63	36	15	0	14	0	0	0	171	0	0	0	0	0
	8:45 AM	12	36	0	0	65	48	23	0	18	0	0	0	202	0	0	0	0	0
	VOLUMES	227	312	0	0	777	411	189	0	255	0	0	0	2,171	0	0	0	0	0
	APPROACH %	42%	58%	0%	0%	65%	35%	43%	0%	57%	0%	0%	0%						
	APP/DEPART	539	/	501	1,188	/	1,032	444	/	0	0	/	638	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	153	196	0	0	461	235	112	0	179	0	0	0	1,336					
	APPROACH %	44%	56%	0%	0%	66%	34%	38%	0%	62%	0%	0%	0%						
	PEAK HR FACTOR		0.733			0.935			0.782			0.000		0.865					
	APP/DEPART	349	/	308	696	/	640	291	/	0	0	/	388	0					
PM	4:00 PM	22	37	0	0	49	27	59	0	28	0	0	0	222	0	0	0	0	0
	4:15 PM	25	47	0	0	30	33	59	0	30	0	0	0	224	0	0	0	0	0
	4:30 PM	29	49	0	0	49	23	49	0	22	0	0	0	221	0	0	0	0	0
	4:45 PM	34	60	0	0	43	30	53	0	33	0	0	0	253	1	0	0	0	1
	5:00 PM	36	62	0	0	40	33	40	0	31	0	0	0	242	0	0	0	0	0
	5:15 PM	41	60	0	0	47	48	67	0	35	0	0	0	298	1	0	0	0	1
	5:30 PM	45	63	0	0	59	52	55	0	42	0	0	0	316	0	0	0	0	0
	5:45 PM	61	70	0	0	42	40	48	0	30	0	0	0	291	0	0	0	0	0
	VOLUMES	293	448	0	0	359	286	430	0	251	0	0	0	2,067	2	0	0	0	2
	APPROACH %	40%	60%	0%	0%	56%	44%	63%	0%	37%	0%	0%	0%						
	APP/DEPART	741	/	878	645	/	612	681	/	0	0	/	577	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	183	255	0	0	188	173	210	0	138	0	0	0	1,147					
	APPROACH %	42%	58%	0%	0%	52%	48%	60%	0%	40%	0%	0%	0%						
	PEAK HR FACTOR		0.836			0.813			0.853			0.000		0.907					
	APP/DEPART	438	/	465	361	/	327	348	/	0	0	/	355	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	1	1	3	5	0	0	0	2	2	0	1	1	1	3
	7:15 AM	1	2	1	1	5	0	2	0	0	2	1	0	1	1	3
	7:30 AM	4	1	0	5	10	4	1	0	1	6	0	0	0	4	4
	7:45 AM	0	4	0	3	7	0	4	0	1	5	0	0	0	2	2
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	1	1	0	2	0	1	0	0	1	0	0	1	0	1
	8:45 AM	0	0	1	4	5	0	0	0	2	2	0	0	1	2	3
	TOTAL	5	9	4	16	34	4	8	0	6	18	1	1	4	10	16
PM	4:00 PM	0	1	2	3	6	0	0	0	0	0	0	1	2	3	6
	4:15 PM	1	0	0	1	2	1	0	0	1	2	0	0	0	0	0
	4:30 PM	0	1	0	2	3	0	1	0	0	1	0	0	0	2	2
	4:45 PM	0	1	0	0	1	0	1	0	0	1	0	0	0	0	0
	5:00 PM	0	0	0	2	2	0	0	0	0	0	0	0	0	2	2
	5:15 PM	1	1	0	2	4	1	1	0	1	3	0	0	0	1	1
	5:30 PM	0	0	3	4	7	0	0	1	2	3	0	0	2	2	4
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	2	4	5	14	25	2	3	1	4	10	0	1	4	10	15

Counts Unlimited
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(951) 268-6268

City of Santa Ana
N/S: Cambridge Street
E/W: Fairhaven Avenue
Weather: Clear

File Name : 86_SNA_Cambridge_Fairhaven AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

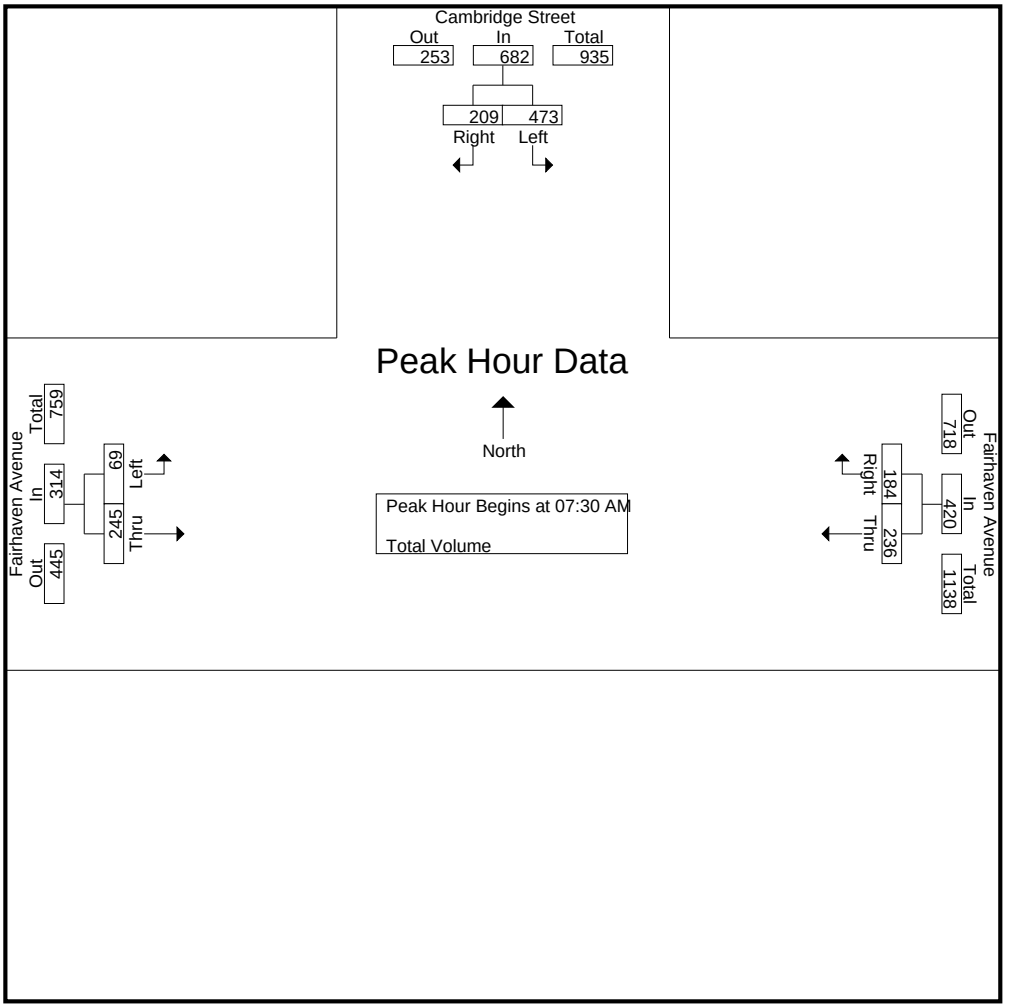
	Cambridge Street Southbound			Fairhaven Avenue Westbound			Fairhaven Avenue Eastbound			
Start Time	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	Int. Total
07:00 AM	74	26	100	16	15	31	5	36	41	172
07:15 AM	97	37	134	45	23	68	12	27	39	241
07:30 AM	117	58	175	59	42	101	16	50	66	342
07:45 AM	131	62	193	71	45	116	24	67	91	400
Total	419	183	602	191	125	316	57	180	237	1155
08:00 AM	122	56	178	62	57	119	13	69	82	379
08:15 AM	103	33	136	44	40	84	16	59	75	295
08:30 AM	73	34	107	37	26	63	9	41	50	220
08:45 AM	64	35	99	27	27	54	14	35	49	202
Total	362	158	520	170	150	320	52	204	256	1096
Grand Total	781	341	1122	361	275	636	109	384	493	2251
Apprch %	69.6	30.4		56.8	43.2		22.1	77.9		
Total %	34.7	15.1	49.8	16	12.2	28.3	4.8	17.1	21.9	

	Cambridge Street Southbound			Fairhaven Avenue Westbound			Fairhaven Avenue Eastbound			
Start Time	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 07:30 AM										
07:30 AM	117	58	175	59	42	101	16	50	66	342
07:45 AM	**131**	**62**	**193**	**71**	45	116	**24**	67	**91**	**400**
08:00 AM	122	56	178	62	**57**	**119**	13	**69**	82	379
08:15 AM	103	33	136	44	40	84	16	59	75	295
Total Volume	473	209	682	236	184	420	69	245	314	1416
% App. Total	69.4	30.6		56.2	43.8		22	78		
PHF	.903	.843	.883	.831	.807	.882	.719	.888	.863	.885

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City of Santa Ana
N/S: Cambridge Street
E/W: Fairhaven Avenue
Weather: Clear

File Name : 86_SNA_Cambridge_Fairhaven AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:30 AM			07:30 AM			07:30 AM		
+0 mins.	117	58	175	59	42	101	16	50	66
+15 mins.	**131**	**62**	**193**	**71**	45	116	**24**	67	**91**
+30 mins.	122	56	178	62	**57**	**119**	13	**69**	82
+45 mins.	103	33	136	44	40	84	16	59	75
Total Volume	473	209	682	236	184	420	69	245	314
% App. Total	69.4	30.6		56.2	43.8		22	78	
PHF	.903	.843	.883	.831	.807	.882	.719	.888	.863

Counts Unlimited
PO Box 1178
Corona, CA 92878
(951) 268-6268

City of Santa Ana
N/S: Cambridge Street
E/W: Fairhaven Avenue
Weather: Clear

File Name : 86_SNA_Cambridge_Fairhaven PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

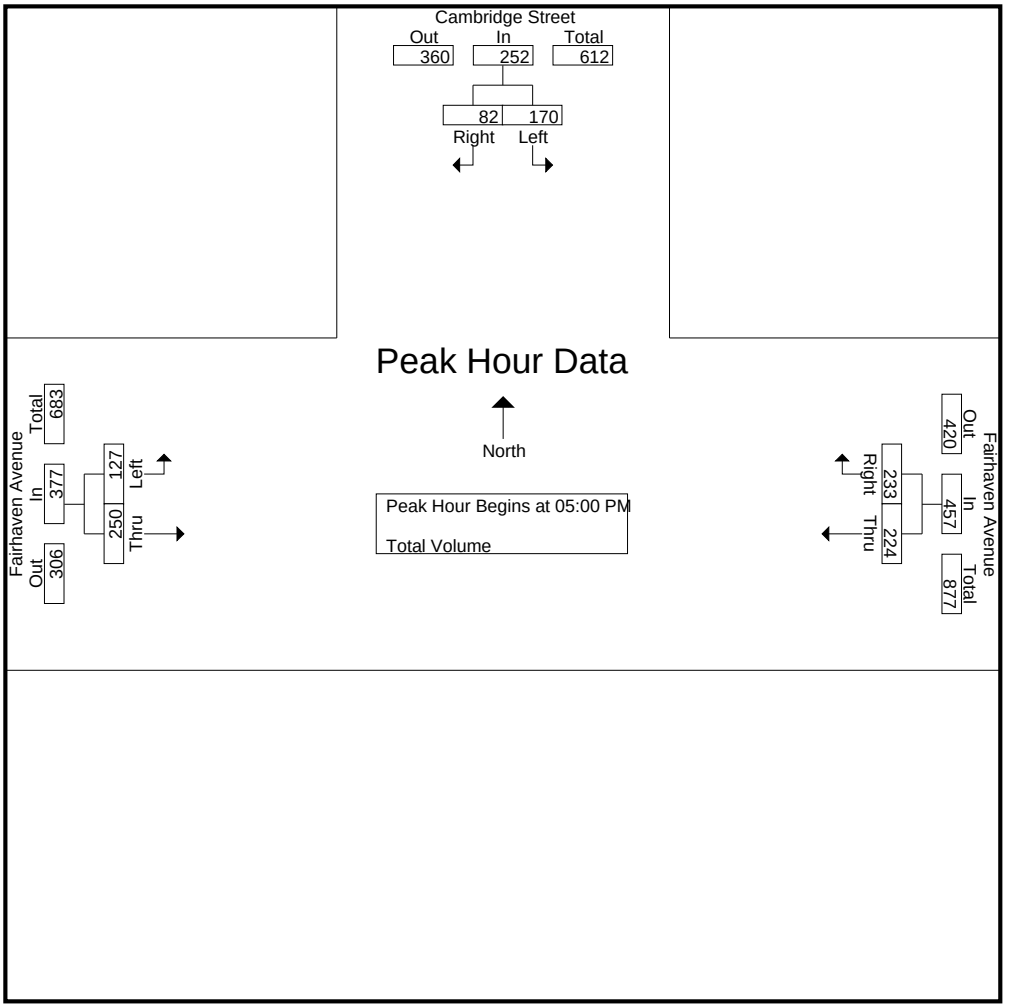
	Cambridge Street Southbound			Fairhaven Avenue Westbound			Fairhaven Avenue Eastbound			
Start Time	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	Int. Total
04:00 PM	41	19	60	39	46	85	15	40	55	200
04:15 PM	47	14	61	53	57	110	37	47	84	255
04:30 PM	40	20	60	45	52	97	31	63	94	251
04:45 PM	41	19	60	49	71	120	31	48	79	259
Total	169	72	241	186	226	412	114	198	312	965
05:00 PM	51	20	71	76	58	134	22	53	75	280
05:15 PM	38	20	58	47	59	106	42	79	121	285
05:30 PM	42	22	64	49	54	103	30	56	86	253
05:45 PM	39	20	59	52	62	114	33	62	95	268
Total	170	82	252	224	233	457	127	250	377	1086
Grand Total	339	154	493	410	459	869	241	448	689	2051
Apprch %	68.8	31.2		47.2	52.8		35	65		
Total %	16.5	7.5	24	20	22.4	42.4	11.8	21.8	33.6	

	Cambridge Street Southbound			Fairhaven Avenue Westbound			Fairhaven Avenue Eastbound			
Start Time	Left	Right	App. Total	Thru	Right	App. Total	Left	Thru	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 05:00 PM										
05:00 PM	**51**	20	**71**	**76**	58	**134**	22	53	75	280
05:15 PM	38	20	58	47	59	106	**42**	**79**	**121**	**285**
05:30 PM	42	**22**	64	49	54	103	30	56	86	253
05:45 PM	39	20	59	52	**62**	114	33	62	95	268
Total Volume	170	82	252	224	233	457	127	250	377	1086
% App. Total	67.5	32.5		49	51		33.7	66.3		
PHF	.833	.932	.887	.737	.940	.853	.756	.791	.779	.953

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City of Santa Ana
N/S: Cambridge Street
E/W: Fairhaven Avenue
Weather: Clear

File Name : 86_SNA_Cambridge_Fairhaven PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:45 PM			04:45 PM			05:00 PM		
+0 mins.	41	19	60	49	**71**	120	22	53	75
+15 mins.	**51**	20	**71**	**76**	58	**134**	**42**	**79**	**121**
+30 mins.	38	20	58	47	59	106	30	56	86
+45 mins.	42	**22**	64	49	54	103	33	62	95
Total Volume	172	81	253	221	242	463	127	250	377
% App. Total	68	32		47.7	52.3		33.7	66.3	
PHF	.843	.920	.891	.727	.852	.864	.756	.791	.779

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City of Santa Ana
N/S: Mabury St/Elk Lane
E/W: E First Street
Weather: Clear

File Name : 87_SNA_Mabury_1st AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

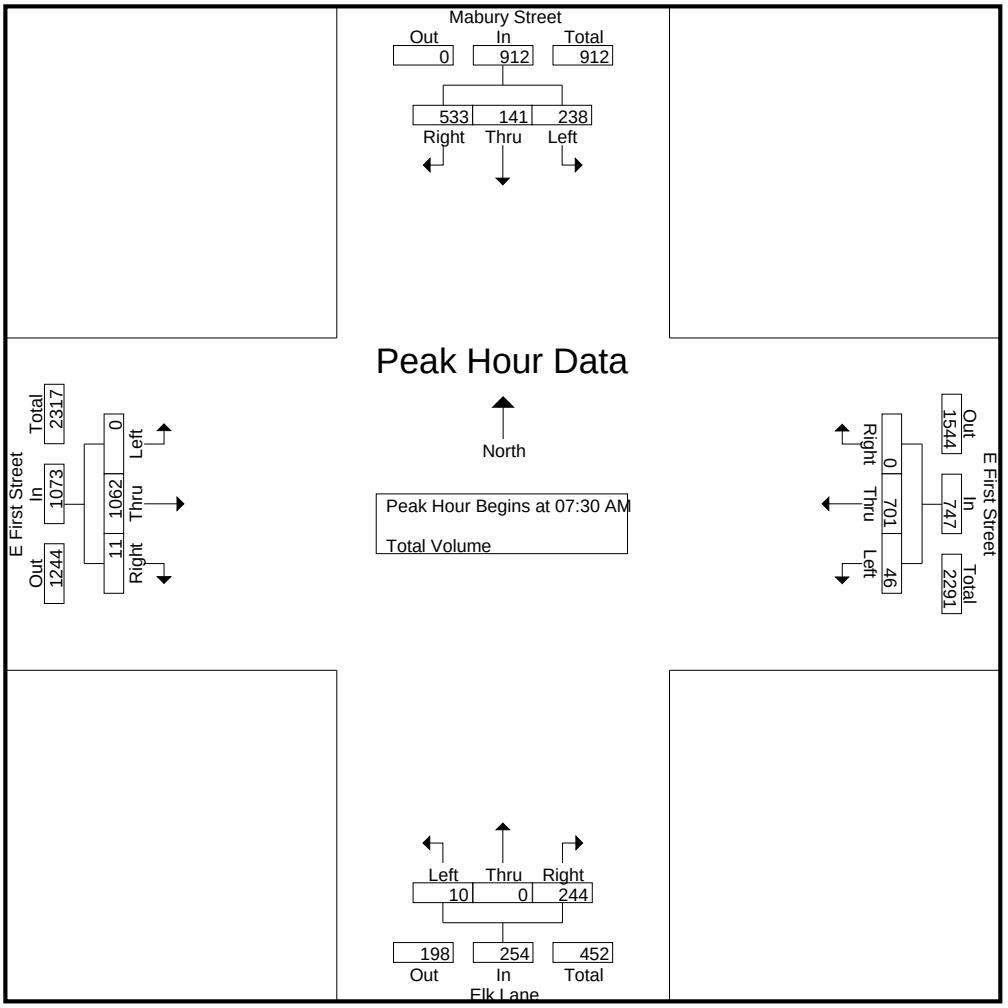
	Mabury Street Southbound				E First Street Westbound				Elk Lane Northbound				E First Street Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
07:00 AM	57	32	132	221	6	91	0	97	2	0	52	54	0	262	8	270	642
07:15 AM	50	30	157	237	6	144	0	150	3	0	58	61	0	264	2	266	714
07:30 AM	53	37	134	224	12	191	0	203	3	0	75	78	0	265	2	267	772
07:45 AM	67	35	139	241	13	190	0	203	3	0	67	70	0	256	3	259	773
Total	227	134	562	923	37	616	0	653	11	0	252	263	0	1047	15	1062	2901
08:00 AM	58	38	114	210	8	171	0	179	2	0	52	54	0	277	2	279	722
08:15 AM	60	31	146	237	13	149	0	162	2	0	50	52	0	264	4	268	719
08:30 AM	61	31	122	214	13	131	0	144	3	0	46	49	0	291	3	294	701
08:45 AM	56	35	130	221	3	127	0	130	1	0	48	49	0	282	4	286	686
Total	235	135	512	882	37	578	0	615	8	0	196	204	0	1114	13	1127	2828
Grand Total	462	269	1074	1805	74	1194	0	1268	19	0	448	467	0	2161	28	2189	5729
Apprch %	25.6	14.9	59.5		5.8	94.2	0		4.1	0	95.9		0	98.7	1.3		
Total %	8.1	4.7	18.7	31.5	1.3	20.8	0	22.1	0.3	0	7.8	8.2	0	37.7	0.5	38.2	

	Mabury Street Southbound				E First Street Westbound				Elk Lane Northbound				E First Street Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	53	37	134	224	12	**191**	0	**203**	**3**	0	**75**	**78**	0	265	2	267	772
07:45 AM	**67**	35	139	**241**	**13**	190	0	203	3	0	67	70	0	256	3	259	**773**
08:00 AM	58	**38**	114	210	8	171	0	179	2	0	52	54	0	**277**	2	**279**	722
08:15 AM	60	31	**146**	237	13	149	0	162	2	0	50	52	0	264	**4**	268	719
Total Volume	238	141	533	912	46	701	0	747	10	0	244	254	0	1062	11	1073	2986
% App. Total	26.1	15.5	58.4		6.2	93.8	0		3.9	0	96.1		0	99	1		
PHF	.888	.928	.913	.946	.885	.918	.000	.920	.833	.000	.813	.814	.000	.958	.688	.961	.966

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City of Santa Ana
N/S: Mabury St/Elk Lane
E/W: E First Street
Weather: Clear

File Name : 87_SNA_Mabury_1st AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:00 AM				07:30 AM				07:00 AM				08:00 AM			
+0 mins.	57	32	132	221	12	**191**	0	**203**	2	0	52	54	0	277	2	279
+15 mins.	50	30	**157**	237	**13**	190	0	203	**3**	0	58	61	0	264	**4**	268
+30 mins.	53	**37**	134	224	8	171	0	179	3	0	**75**	**78**	0	**291**	3	**294**
+45 mins.	**67**	35	139	**241**	13	149	0	162	3	0	67	70	0	282	4	286
Total Volume	227	134	562	923	46	701	0	747	11	0	252	263	0	1114	13	1127
% App. Total	24.6	14.5	60.9		6.2	93.8	0		4.2	0	95.8		0	98.8	1.2	
PHF	.847	.905	.895	.957	.885	.918	.000	.920	.917	.000	.840	.843	.000	.957	.813	.958

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City of Santa Ana
N/S: Mabury St/Elk Lane
E/W: E First Street
Weather: Clear

File Name : 87_SNA_Mabury_1st PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

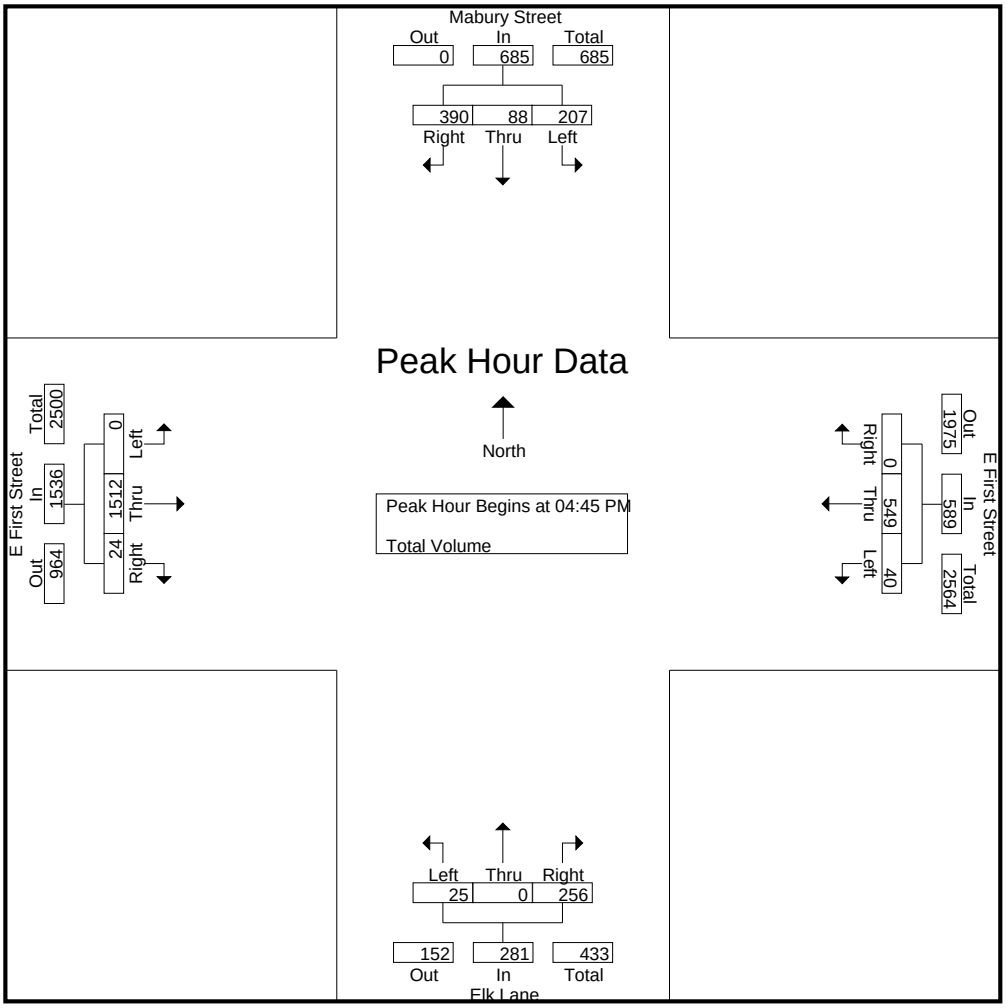
	Mabury Street Southbound				E First Street Westbound				Elk Lane Northbound				E First Street Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
04:00 PM	64	29	90	183	10	151	0	161	10	0	75	85	0	332	2	334	763
04:15 PM	39	15	97	151	15	153	0	168	4	0	55	59	0	314	4	318	696
04:30 PM	58	31	98	187	5	141	0	146	6	0	89	95	0	295	4	299	727
04:45 PM	37	23	100	160	12	126	0	138	5	0	70	75	0	379	4	383	756
Total	198	98	385	681	42	571	0	613	25	0	289	314	0	1320	14	1334	2942
05:00 PM	59	21	109	189	8	151	0	159	3	0	49	52	0	374	6	380	780
05:15 PM	64	25	96	185	13	166	0	179	9	0	75	84	0	351	8	359	807
05:30 PM	47	19	85	151	7	106	0	113	8	0	62	70	0	408	6	414	748
05:45 PM	54	28	106	188	15	99	0	114	5	0	72	77	0	327	7	334	713
Total	224	93	396	713	43	522	0	565	25	0	258	283	0	1460	27	1487	3048
Grand Total	422	191	781	1394	85	1093	0	1178	50	0	547	597	0	2780	41	2821	5990
Apprch %	30.3	13.7	56		7.2	92.8	0		8.4	0	91.6		0	98.5	1.5		
Total %	7	3.2	13	23.3	1.4	18.2	0	19.7	0.8	0	9.1	10	0	46.4	0.7	47.1	

	Mabury Street Southbound				E First Street Westbound				Elk Lane Northbound				E First Street Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	37	23	100	160	12	126	0	138	5	0	70	75	0	379	4	383	756
05:00 PM	59	21	**109**	**189**	8	151	0	159	3	0	49	52	0	374	6	380	780
05:15 PM	**64**	**25**	96	185	**13**	**166**	0	**179**	**9**	0	**75**	**84**	0	351	**8**	359	**807**
05:30 PM	47	19	85	151	7	106	0	113	8	0	62	70	0	**408**	6	**414**	748
Total Volume	207	88	390	685	40	549	0	589	25	0	256	281	0	1512	24	1536	3091
% App. Total	30.2	12.8	56.9		6.8	93.2	0		8.9	0	91.1		0	98.4	1.6		
PHF	.809	.880	.894	.906	.769	.827	.000	.823	.694	.000	.853	.836	.000	.926	.750	.928	.958

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City of Santa Ana
N/S: Mabury St/Elk Lane
E/W: E First Street
Weather: Clear

File Name : 87_SNA_Mabury_1st PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:30 PM				04:30 PM				04:00 PM				04:45 PM			
+0 mins.	58	**31**	98	187	5	141	0	146	**10**	0	75	85	0	379	4	383
+15 mins.	37	23	100	160	12	126	0	138	4	0	55	59	0	374	6	380
+30 mins.	59	21	**109**	**189**	8	151	0	159	6	0	**89**	**95**	0	351	**8**	359
+45 mins.	**64**	25	96	185	**13**	**166**	0	**179**	5	0	70	75	0	**408**	6	**414**
Total Volume	218	100	403	721	38	584	0	622	25	0	289	314	0	1512	24	1536
% App. Total	30.2	13.9	55.9		6.1	93.9	0		8	0	92		0	98.4	1.6	
PHF	.852	.806	.924	.954	.731	.880	.000	.869	.625	.000	.812	.826	.000	.926	.750	.928

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City of Orange
N/S: Tustin Street
E/W: Rock Creek Drive/La Veta Avenue
Weather: Clear

File Name : 88_ORN_Tustin_La Veta AM
Site Code : 20220171
Start Date : 1/8/2020
Page No : 1

Groups Printed- Total Volume

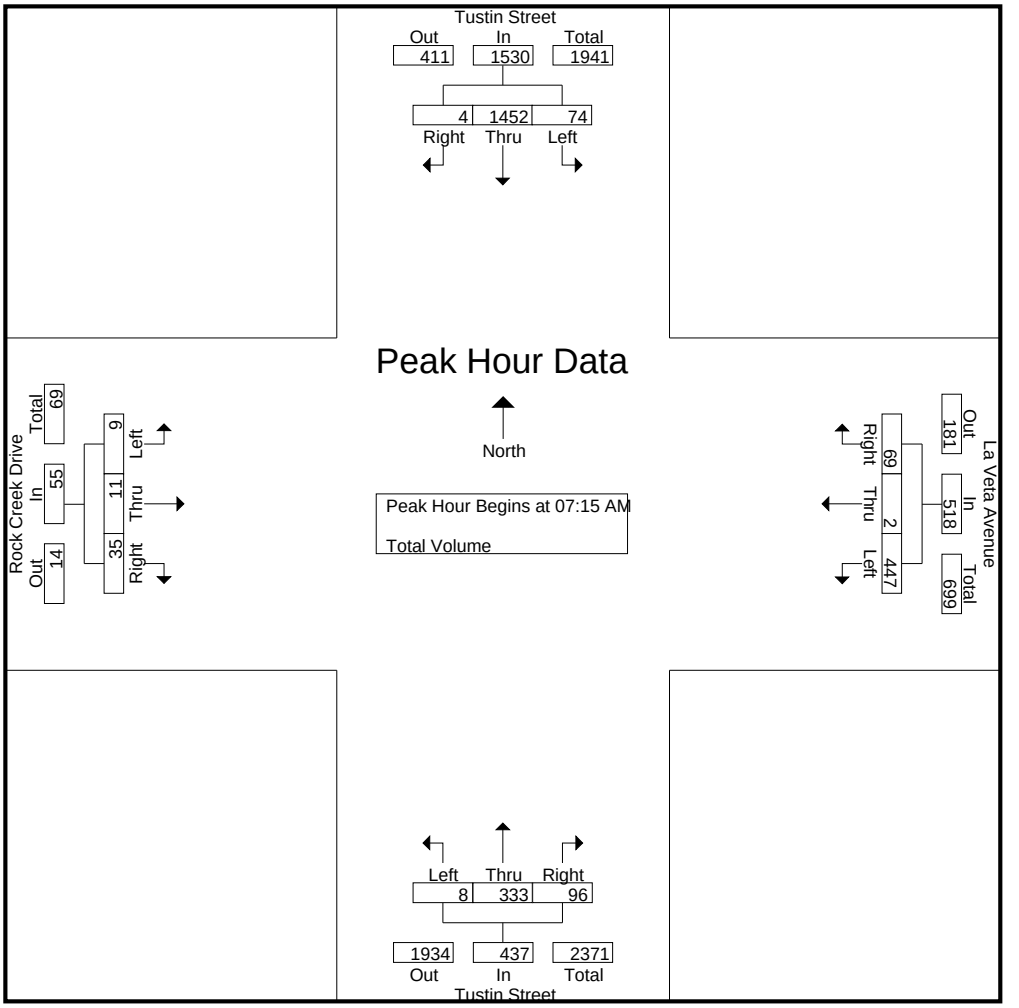
	Tustin Street Southbound				La Veta Avenue Westbound				Tustin Street Northbound				Rock Creek Drive Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
07:00 AM	6	346	1	353	84	2	10	96	5	72	22	99	1	1	7	9	557
07:15 AM	19	385	0	404	114	2	11	127	2	79	28	109	1	3	9	13	653
07:30 AM	21	367	0	388	144	0	14	158	0	70	17	87	3	1	9	13	646
07:45 AM	20	378	2	400	95	0	24	119	5	94	21	120	2	6	9	17	656
Total	66	1476	3	1545	437	4	59	500	12	315	88	415	7	11	34	52	2512
08:00 AM	14	322	2	338	94	0	20	114	1	90	30	121	3	1	8	12	585
08:15 AM	14	314	0	328	92	1	24	117	7	89	31	127	0	3	10	13	585
08:30 AM	16	323	0	339	69	0	19	88	4	108	26	138	5	1	9	15	580
08:45 AM	25	295	0	320	60	6	21	87	3	122	25	150	5	0	3	8	565
Total	69	1254	2	1325	315	7	84	406	15	409	112	536	13	5	30	48	2315
Grand Total	135	2730	5	2870	752	11	143	906	27	724	200	951	20	16	64	100	4827
Apprch %	4.7	95.1	0.2		83	1.2	15.8		2.8	76.1	21		20	16	64		
Total %	2.8	56.6	0.1	59.5	15.6	0.2	3	18.8	0.6	15	4.1	19.7	0.4	0.3	1.3	2.1	

	Tustin Street Southbound				La Veta Avenue Westbound				Tustin Street Northbound				Rock Creek Drive Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:15 AM																	
07:15 AM	19	**385**	0	**404**	114	**2**	11	127	2	79	28	109	1	3	**9**	13	653
07:30 AM	**21**	367	0	388	**144**	0	14	**158**	0	70	17	87	**3**	1	9	13	646
07:45 AM	20	378	**2**	400	95	0	**24**	119	**5**	**94**	21	120	2	**6**	9	**17**	**656**
08:00 AM	14	322	2	338	94	0	20	114	1	90	**30**	**121**	3	1	8	12	585
Total Volume	74	1452	4	1530	447	2	69	518	8	333	96	437	9	11	35	55	2540
% App. Total	4.8	94.9	0.3		86.3	0.4	13.3		1.8	76.2	22		16.4	20	63.6		
PHF	.881	.943	.500	.947	.776	.250	.719	.820	.400	.886	.800	.903	.750	.458	.972	.809	.968

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City of Orange
N/S: Tustin Street
E/W: Rock Creek Drive/La Veta Avenue
Weather: Clear

File Name : 88_ORN_Tustin_La Veta AM
Site Code : 20220171
Start Date : 1/8/2020
Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:00 AM				07:15 AM				08:00 AM				07:45 AM			
+0 mins.	6	346	1	353	114	**2**	11	127	1	90	30	121	2	**6**	9	**17**
+15 mins.	19	**385**	0	**404**	**144**	0	14	**158**	**7**	89	**31**	127	3	1	8	12
+30 mins.	**21**	367	0	388	95	0	**24**	119	4	108	26	138	0	3	**10**	13
+45 mins.	20	378	**2**	400	94	0	20	114	3	**122**	25	**150**	**5**	1	9	15
Total Volume	66	1476	3	1545	447	2	69	518	15	409	112	536	10	11	36	57
% App. Total	4.3	95.5	0.2		86.3	0.4	13.3		2.8	76.3	20.9		17.5	19.3	63.2	
PHF	.786	.958	.375	.956	.776	.250	.719	.820	.536	.838	.903	.893	.500	.458	.900	.838

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City of Orange
N/S: Tustin Street
E/W: Rock Creek Drive/La Veta Avenue
Weather: Clear

File Name : 88_ORN_Tustin_La Veta PM
Site Code : 20220171
Start Date : 1/8/2020
Page No : 1

Groups Printed- Total Volume

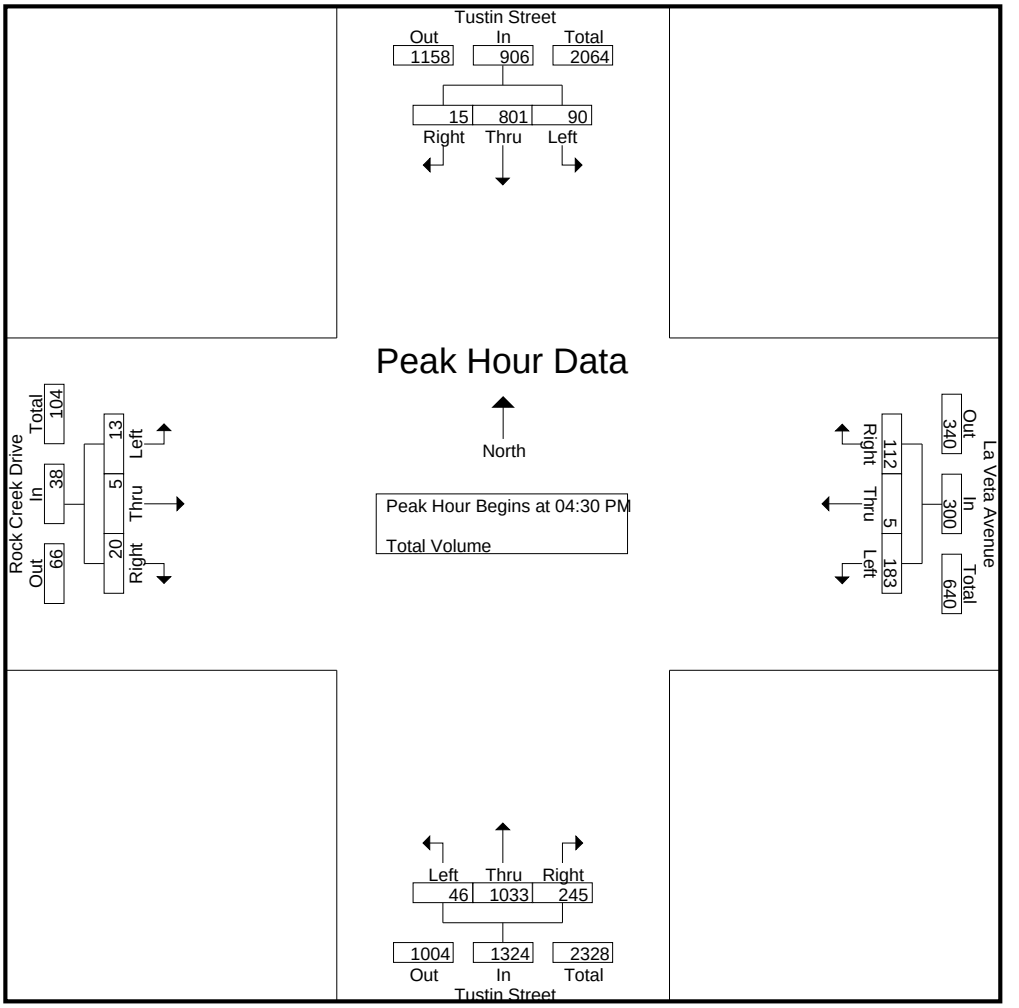
	Tustin Street Southbound				La Veta Avenue Westbound				Tustin Street Northbound				Rock Creek Drive Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
04:00 PM	26	208	1	235	59	3	32	94	5	285	56	346	4	1	2	7	682
04:15 PM	16	163	1	180	32	0	33	65	12	253	51	316	3	2	4	9	570
04:30 PM	18	191	5	214	55	0	24	79	9	272	52	333	2	1	5	8	634
04:45 PM	19	195	2	216	50	4	24	78	12	232	76	320	2	1	1	4	618
Total	79	757	9	845	196	7	113	316	38	1042	235	1315	11	5	12	28	2504
05:00 PM	28	224	6	258	33	1	35	69	15	278	63	356	6	1	10	17	700
05:15 PM	25	191	2	218	45	0	29	74	10	251	54	315	3	2	4	9	616
05:30 PM	34	168	4	206	51	3	23	77	19	231	49	299	5	4	2	11	593
05:45 PM	16	195	1	212	37	4	32	73	10	255	67	332	4	9	3	16	633
Total	103	778	13	894	166	8	119	293	54	1015	233	1302	18	16	19	53	2542
Grand Total	182	1535	22	1739	362	15	232	609	92	2057	468	2617	29	21	31	81	5046
Apprch %	10.5	88.3	1.3		59.4	2.5	38.1		3.5	78.6	17.9		35.8	25.9	38.3		
Total %	3.6	30.4	0.4	34.5	7.2	0.3	4.6	12.1	1.8	40.8	9.3	51.9	0.6	0.4	0.6	1.6	

	Tustin Street Southbound				La Veta Avenue Westbound				Tustin Street Northbound				Rock Creek Drive Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:30 PM																	
04:30 PM	18	191	5	214	**55**	0	24	**79**	9	272	52	333	2	1	5	8	634
04:45 PM	19	195	2	216	50	**4**	24	78	12	232	**76**	320	2	1	1	4	618
05:00 PM	**28**	**224**	**6**	**258**	33	1	**35**	69	**15**	**278**	63	**356**	**6**	1	**10**	**17**	**700**
05:15 PM	25	191	2	218	45	0	29	74	10	251	54	315	3	**2**	4	9	616
Total Volume	90	801	15	906	183	5	112	300	46	1033	245	1324	13	5	20	38	2568
% App. Total	9.9	88.4	1.7		61	1.7	37.3		3.5	78	18.5		34.2	13.2	52.6		
PHF	.804	.894	.625	.878	.832	.313	.800	.949	.767	.929	.806	.930	.542	.625	.500	.559	.917

Counts Unlimited
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City of Orange
N/S: Tustin Street
E/W: Rock Creek Drive/La Veta Avenue
Weather: Clear

File Name : 88_ORN_Tustin_La Veta PM
Site Code : 20220171
Start Date : 1/8/2020
Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:30 PM				04:00 PM				04:15 PM				05:00 PM			
+0 mins.	18	191	5	214	**59**	3	32	**94**	12	253	51	316	**6**	1	**10**	**17**
+15 mins.	19	195	2	216	32	0	**33**	65	9	272	52	333	3	2	4	9
+30 mins.	**28**	**224**	**6**	**258**	55	0	24	79	12	232	**76**	320	5	4	2	11
+45 mins.	25	191	2	218	50	**4**	24	78	**15**	**278**	63	**356**	4	**9**	3	16
Total Volume	90	801	15	906	196	7	113	316	48	1035	242	1325	18	16	19	53
% App. Total	9.9	88.4	1.7		62	2.2	35.8		3.6	78.1	18.3		34	30.2	35.8	
PHF	.804	.894	.625	.878	.831	.438	.856	.840	.800	.931	.796	.930	.750	.444	.475	.779

Counts Unlimited
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City of Orange
N/S: Tustin Street
E/W: SR-22 Westbound On Ramp
Weather: Clear

File Name : 89_ORN_Tustin_22W AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

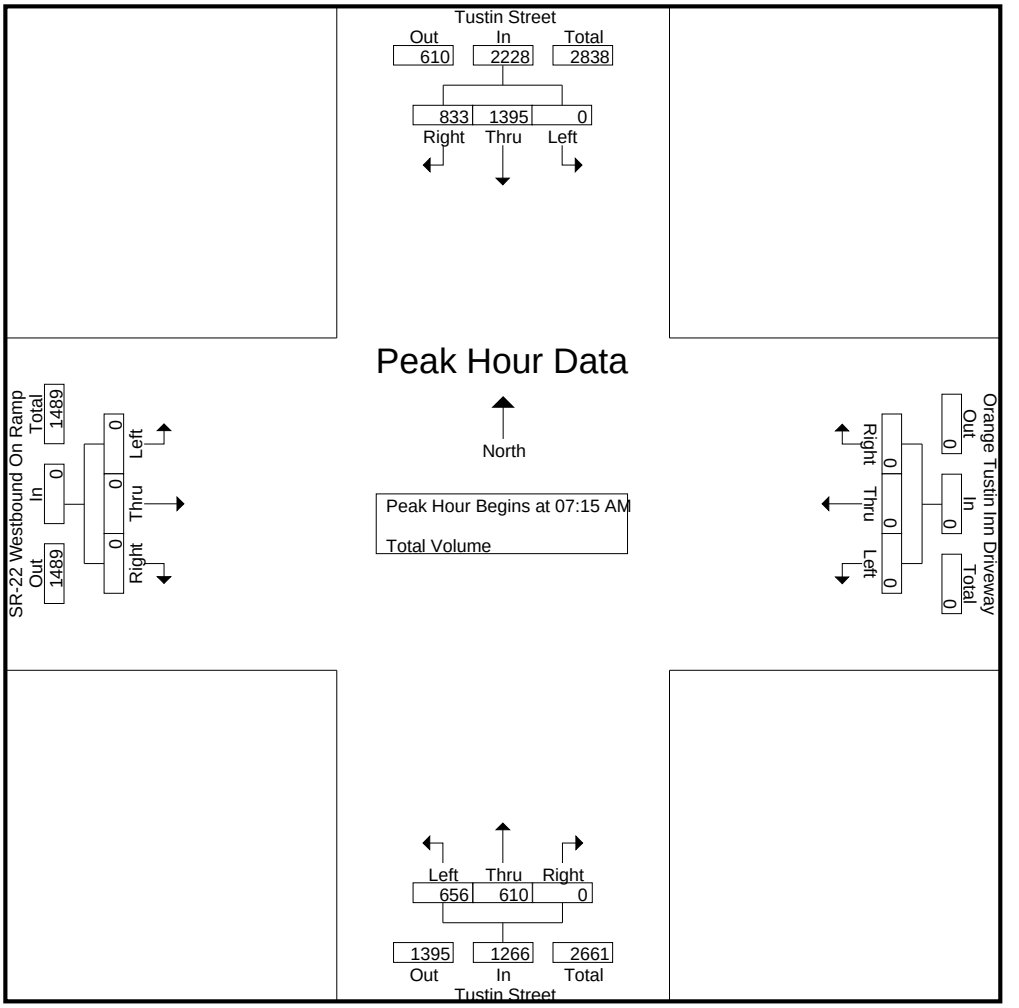
	Tustin Street Southbound				Orange Tustin Inn Driveway Westbound				Tustin Street Northbound				SR-22 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
07:00 AM	0	301	193	494	0	0	0	0	102	115	0	217	0	0	0	0	711
07:15 AM	0	389	195	584	0	0	0	0	127	115	0	242	0	0	0	0	826
07:30 AM	0	355	210	565	0	0	0	0	170	154	0	324	0	0	0	0	889
07:45 AM	0	329	233	562	0	0	0	0	216	161	0	377	0	0	0	0	939
Total	0	1374	831	2205	0	0	0	0	615	545	0	1160	0	0	0	0	3365
08:00 AM	0	322	195	517	0	0	0	0	143	180	0	323	0	0	0	0	840
08:15 AM	0	300	186	486	0	0	0	0	151	168	0	319	0	0	0	0	805
08:30 AM	0	269	151	420	0	0	0	0	121	156	0	277	0	0	0	0	697
08:45 AM	0	231	148	379	0	0	0	0	94	170	0	264	0	0	0	0	643
Total	0	1122	680	1802	0	0	0	0	509	674	0	1183	0	0	0	0	2985
Grand Total	0	2496	1511	4007	0	0	0	0	1124	1219	0	2343	0	0	0	0	6350
Apprch %	0	62.3	37.7		0	0	0		48	52	0		0	0	0		
Total %	0	39.3	23.8	63.1	0	0	0	0	17.7	19.2	0	36.9	0	0	0	0	

	Tustin Street Southbound				Orange Tustin Inn Driveway Westbound				Tustin Street Northbound				SR-22 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:15 AM																	
07:15 AM	0	**389**	195	**584**	0	0	0	0	127	115	0	242	0	0	0	0	826
07:30 AM	0	355	210	565	0	0	0	0	170	154	0	324	0	0	0	0	889
07:45 AM	0	329	**233**	562	0	0	0	0	**216**	161	0	**377**	0	0	0	0	**939**
08:00 AM	0	322	195	517	0	0	0	0	143	**180**	0	323	0	0	0	0	840
Total Volume	0	1395	833	2228	0	0	0	0	656	610	0	1266	0	0	0	0	3494
% App. Total	0	62.6	37.4		0	0	0		51.8	48.2	0		0	0	0		
PHF	.000	.897	.894	.954	.000	.000	.000	.000	.759	.847	.000	.840	.000	.000	.000	.000	.930

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City of Orange
N/S: Tustin Street
E/W: SR-22 Westbound On Ramp
Weather: Clear

File Name : 89_ORN_Tustin_22W AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:15 AM				07:00 AM				07:30 AM				07:00 AM			
+0 mins.	0	**389**	195	**584**	0	0	0	0	170	154	0	324	0	0	0	0
+15 mins.	0	355	210	565	0	0	0	0	**216**	161	0	**377**	0	0	0	0
+30 mins.	0	329	**233**	562	0	0	0	0	143	**180**	0	323	0	0	0	0
+45 mins.	0	322	195	517	0	0	0	0	151	168	0	319	0	0	0	0
Total Volume	0	1395	833	2228	0	0	0	0	680	663	0	1343	0	0	0	0
% App. Total	0	62.6	37.4		0	0	0		50.6	49.4	0		0	0	0	
PHF	.000	.897	.894	.954	.000	.000	.000	.000	.787	.921	.000	.891	.000	.000	.000	.000

Counts Unlimited
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City of Orange
N/S: Tustin Street
E/W: SR-22 Westbound On Ramp
Weather: Clear

File Name : 89_ORN_Tustin_22W PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

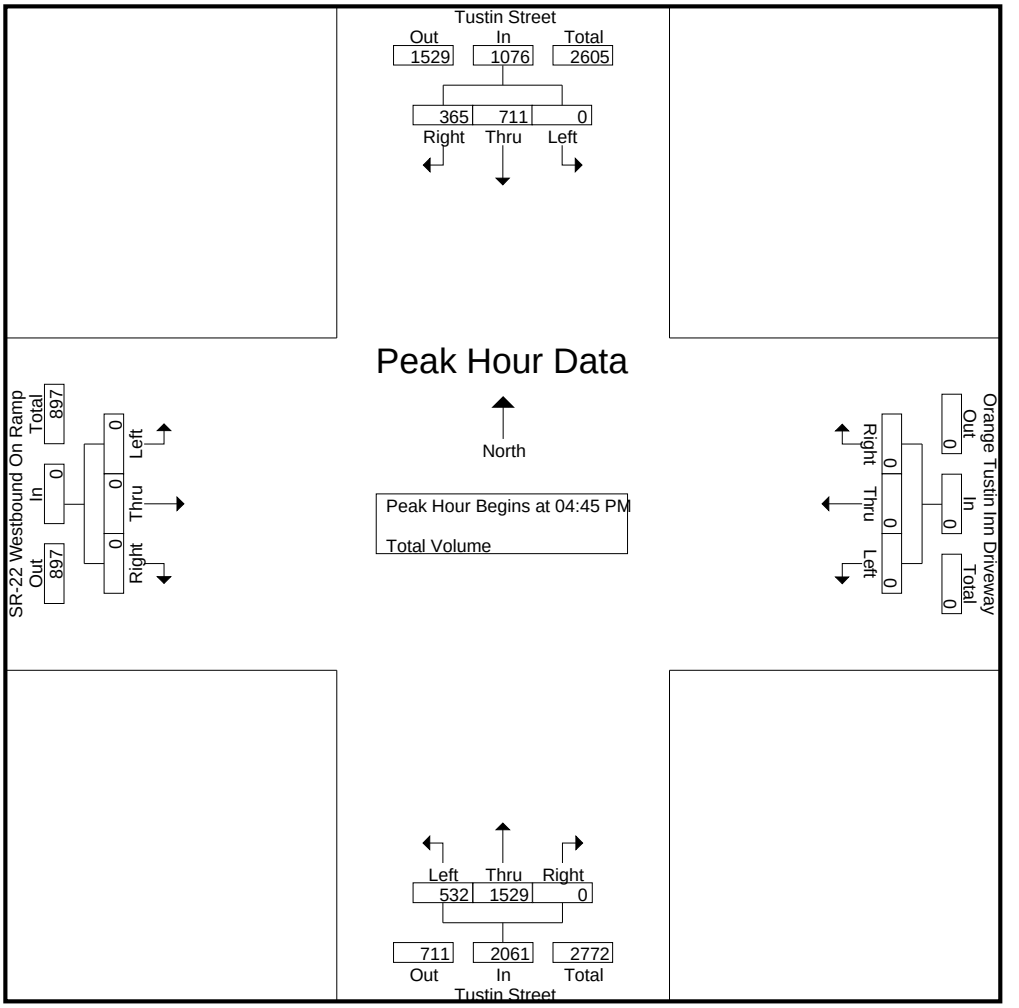
	Tustin Street Southbound				Orange Tustin Inn Driveway Westbound				Tustin Street Northbound				SR-22 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
04:00 PM	0	161	113	274	0	0	0	0	148	374	0	522	0	0	0	0	796
04:15 PM	0	146	88	234	0	0	0	0	142	377	0	519	0	0	0	0	753
04:30 PM	0	148	102	250	0	0	0	0	142	376	0	518	0	0	0	0	768
04:45 PM	0	166	100	266	0	0	0	0	111	373	0	484	0	0	0	0	750
Total	0	621	403	1024	0	0	0	0	543	1500	0	2043	0	0	0	0	3067
05:00 PM	0	173	99	272	0	0	0	0	149	379	0	528	0	0	0	0	800
05:15 PM	0	182	98	280	0	0	0	0	130	391	0	521	0	0	0	0	801
05:30 PM	0	190	68	258	0	0	0	0	142	386	0	528	0	0	0	0	786
05:45 PM	0	169	81	250	0	0	0	0	105	391	0	496	0	0	0	0	746
Total	0	714	346	1060	0	0	0	0	526	1547	0	2073	0	0	0	0	3133
Grand Total	0	1335	749	2084	0	0	0	0	1069	3047	0	4116	0	0	0	0	6200
Apprch %	0	64.1	35.9		0	0	0		26	74	0		0	0	0		
Total %	0	21.5	12.1	33.6	0	0	0	0	17.2	49.1	0	66.4	0	0	0	0	

	Tustin Street Southbound				Orange Tustin Inn Driveway Westbound				Tustin Street Northbound				SR-22 Westbound On Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	0	166	**100**	266	0	0	0	0	111	373	0	484	0	0	0	0	750
05:00 PM	0	173	99	272	0	0	0	0	**149**	379	0	**528**	0	0	0	0	800
05:15 PM	0	182	98	**280**	0	0	0	0	130	**391**	0	521	0	0	0	0	**801**
05:30 PM	0	**190**	68	258	0	0	0	0	142	386	0	528	0	0	0	0	786
Total Volume	0	711	365	1076	0	0	0	0	532	1529	0	2061	0	0	0	0	3137
% App. Total	0	66.1	33.9		0	0	0		25.8	74.2	0		0	0	0		
PHF	.000	.936	.913	.961	.000	.000	.000	.000	.893	.978	.000	.976	.000	.000	.000	.000	.979

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City of Orange
N/S: Tustin Street
E/W: SR-22 Westbound On Ramp
Weather: Clear

File Name : 89_ORN_Tustin_22W PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:45 PM				04:00 PM				05:00 PM				04:00 PM			
+0 mins.	0	166	**100**	266	0	0	0	0	**149**	379	0	**528**	0	0	0	0
+15 mins.	0	173	99	272	0	0	0	0	130	**391**	0	521	0	0	0	0
+30 mins.	0	182	98	**280**	0	0	0	0	142	386	0	528	0	0	0	0
+45 mins.	0	**190**	68	258	0	0	0	0	105	391	0	496	0	0	0	0
Total Volume	0	711	365	1076	0	0	0	0	526	1547	0	2073	0	0	0	0
% App. Total	0	66.1	33.9		0	0	0		25.4	74.6	0		0	0	0	
PHF	.000	.936	.913	.961	.000	.000	.000	.000	.883	.989	.000	.982	.000	.000	.000	.000

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City of Orange
N/S: Tustin Street
E/W: SR-22 EB Off Ramp/Seba Avenue
Weather: Clear

File Name : 90_ORN_Tustin_22E AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

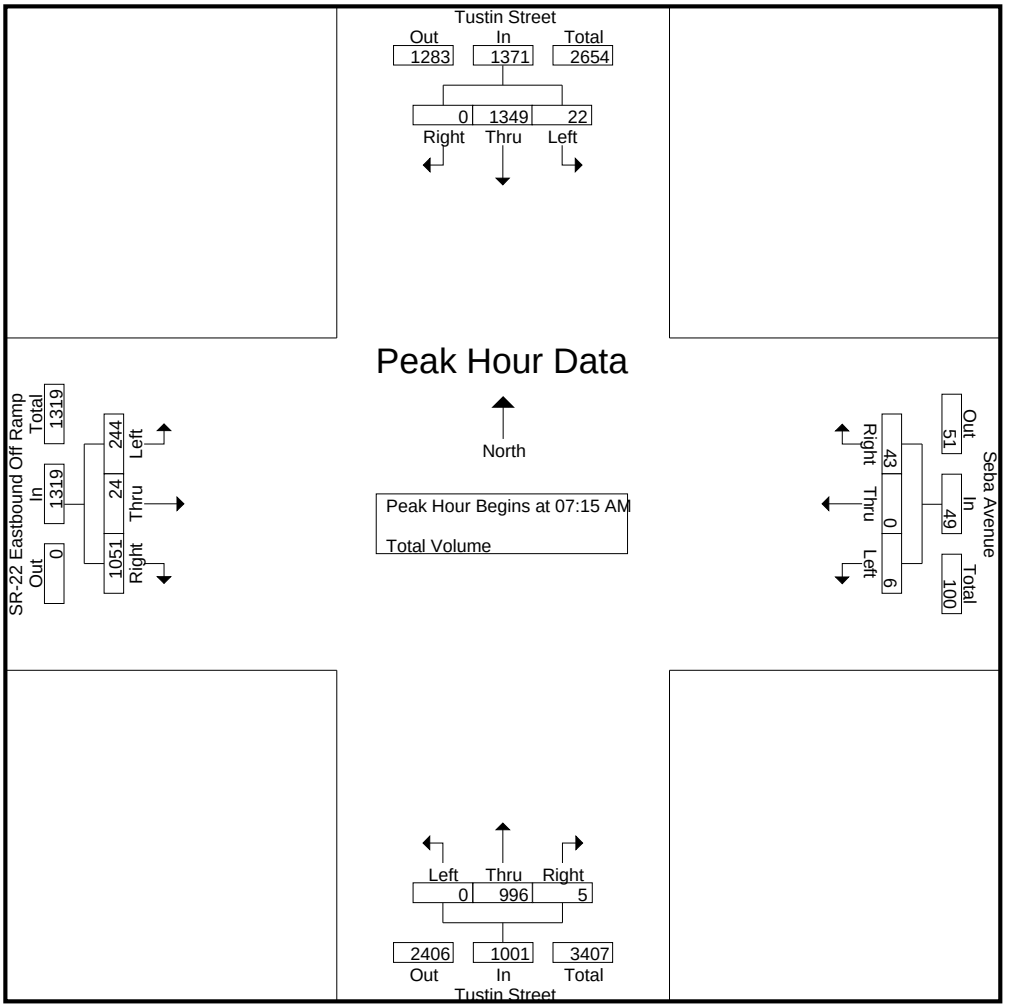
	Tustin Street Southbound				Seba Avenue Westbound				Tustin Street Northbound				SR-22 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
07:00 AM	2	299	0	301	3	0	4	7	0	170	0	170	49	2	208	259	737
07:15 AM	2	370	0	372	1	0	6	7	0	200	0	200	59	4	288	351	930
07:30 AM	2	316	0	318	0	0	10	10	0	263	1	264	62	9	269	340	932
07:45 AM	5	321	0	326	3	0	15	18	0	291	1	292	51	2	250	303	939
Total	11	1306	0	1317	7	0	35	42	0	924	2	926	221	17	1015	1253	3538
08:00 AM	13	342	0	355	2	0	12	14	0	242	3	245	72	9	244	325	939
08:15 AM	13	294	0	307	4	0	13	17	0	231	0	231	76	8	251	335	890
08:30 AM	6	261	0	267	6	0	9	15	0	217	2	219	63	8	184	255	756
08:45 AM	7	221	0	228	1	0	7	8	0	192	2	194	79	3	227	309	739
Total	39	1118	0	1157	13	0	41	54	0	882	7	889	290	28	906	1224	3324
Grand Total	50	2424	0	2474	20	0	76	96	0	1806	9	1815	511	45	1921	2477	6862
Apprch %	2	98	0		20.8	0	79.2		0	99.5	0.5		20.6	1.8	77.6		
Total %	0.7	35.3	0	36.1	0.3	0	1.1	1.4	0	26.3	0.1	26.5	7.4	0.7	28	36.1	

	Tustin Street Southbound				Seba Avenue Westbound				Tustin Street Northbound				SR-22 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:15 AM																	
07:15 AM	2	**370**	0	**372**	1	0	6	7	0	200	0	200	59	4	**288**	**351**	930
07:30 AM	2	316	0	318	0	0	10	10	0	263	1	264	62	**9**	269	340	932
07:45 AM	5	321	0	326	**3**	0	**15**	**18**	0	**291**	1	**292**	51	2	250	303	**939**
08:00 AM	**13**	342	0	355	2	0	12	14	0	242	**3**	245	**72**	9	244	325	939
Total Volume	22	1349	0	1371	6	0	43	49	0	996	5	1001	244	24	1051	1319	3740
% App. Total	1.6	98.4	0		12.2	0	87.8		0	99.5	0.5		18.5	1.8	79.7		
PHF	.423	.911	.000	.921	.500	.000	.717	.681	.000	.856	.417	.857	.847	.667	.912	.939	.996

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City of Orange
N/S: Tustin Street
E/W: SR-22 EB Off Ramp/Seba Avenue
Weather: Clear

File Name : 90_ORN_Tustin_22E AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:15 AM				07:45 AM				07:30 AM				07:15 AM			
+0 mins.	2	**370**	0	**372**	3	0	**15**	**18**	0	263	1	264	59	4	**288**	**351**
+15 mins.	2	316	0	318	2	0	12	14	0	**291**	1	**292**	62	**9**	269	340
+30 mins.	5	321	0	326	4	0	13	17	0	242	**3**	245	51	2	250	303
+45 mins.	**13**	342	0	355	**6**	0	9	15	0	231	0	231	**72**	9	244	325
Total Volume	22	1349	0	1371	15	0	49	64	0	1027	5	1032	244	24	1051	1319
% App. Total	1.6	98.4	0		23.4	0	76.6		0	99.5	0.5		18.5	1.8	79.7	
PHF	.423	.911	.000	.921	.625	.000	.817	.889	.000	.882	.417	.884	.847	.667	.912	.939

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City of Orange
N/S: Tustin Street
E/W: SR-22 EB Off Ramp/Seba Avenue
Weather: Clear

File Name : 90_ORN_Tustin_22E PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

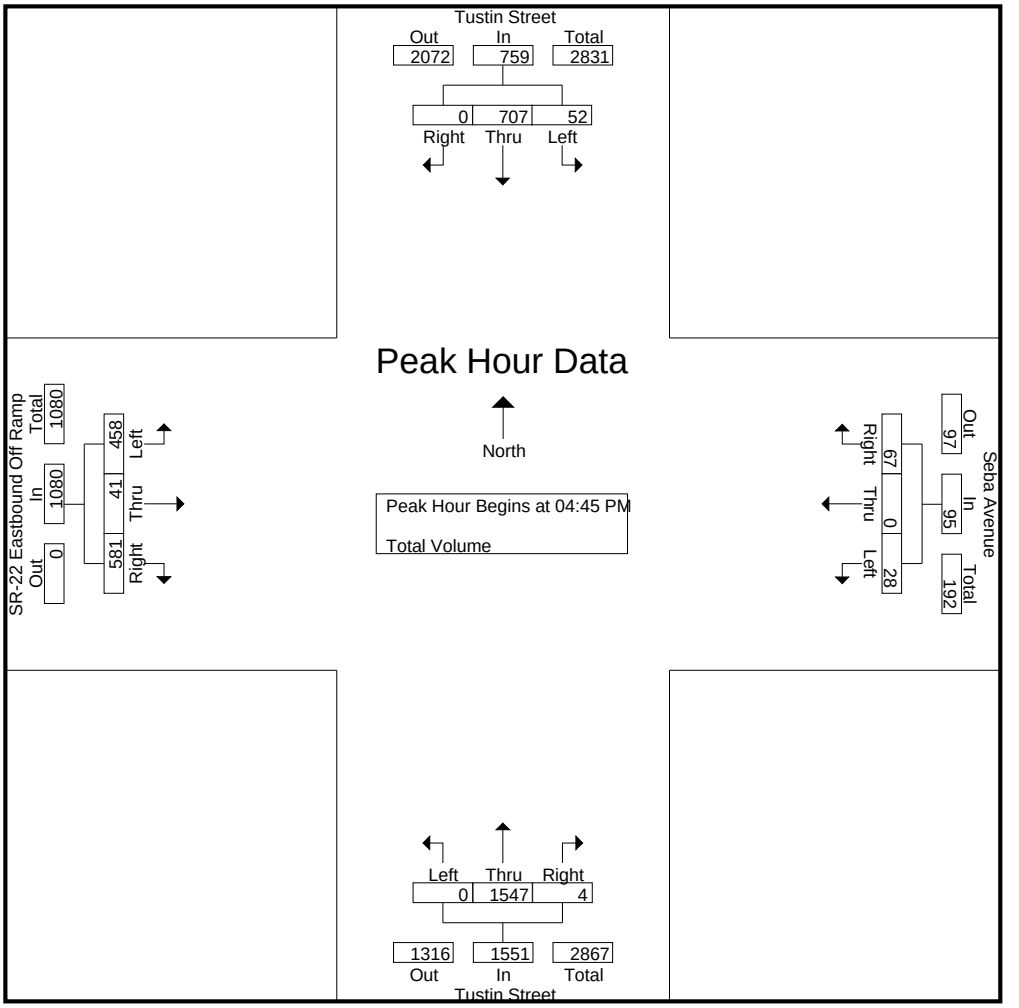
	Tustin Street Southbound				Seba Avenue Westbound				Tustin Street Northbound				SR-22 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
04:00 PM	7	144	0	151	8	0	10	18	0	359	1	360	164	11	142	317	846
04:15 PM	10	155	0	165	7	0	16	23	0	350	2	352	153	16	146	315	855
04:30 PM	17	142	0	159	6	0	18	24	0	359	2	361	144	7	142	293	837
04:45 PM	14	167	0	181	5	0	11	16	0	380	0	380	114	7	123	244	821
Total	48	608	0	656	26	0	55	81	0	1448	5	1453	575	41	553	1169	3359
05:00 PM	12	177	0	189	8	0	21	29	0	382	1	383	108	10	138	256	857
05:15 PM	10	155	0	165	9	0	18	27	0	372	3	375	120	11	162	293	860
05:30 PM	16	208	0	224	6	0	17	23	0	413	0	413	116	13	158	287	947
05:45 PM	14	144	0	158	5	0	24	29	0	344	2	346	123	7	142	272	805
Total	52	684	0	736	28	0	80	108	0	1511	6	1517	467	41	600	1108	3469
Grand Total	100	1292	0	1392	54	0	135	189	0	2959	11	2970	1042	82	1153	2277	6828
Apprch %	7.2	92.8	0		28.6	0	71.4		0	99.6	0.4		45.8	3.6	50.6		
Total %	1.5	18.9	0	20.4	0.8	0	2	2.8	0	43.3	0.2	43.5	15.3	1.2	16.9	33.3	

	Tustin Street Southbound				Seba Avenue Westbound				Tustin Street Northbound				SR-22 Eastbound Off Ramp Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	14	167	0	181	5	0	11	16	0	380	0	380	114	7	123	244	821
05:00 PM	12	177	0	189	8	0	**21**	**29**	0	382	1	383	108	10	138	256	857
05:15 PM	10	155	0	165	**9**	0	18	27	0	372	**3**	375	**120**	11	**162**	**293**	860
05:30 PM	**16**	**208**	0	**224**	6	0	17	23	0	**413**	0	**413**	116	**13**	158	287	**947**
Total Volume	52	707	0	759	28	0	67	95	0	1547	4	1551	458	41	581	1080	3485
% App. Total	6.9	93.1	0		29.5	0	70.5		0	99.7	0.3		42.4	3.8	53.8		
PHF	.813	.850	.000	.847	.778	.000	.798	.819	.000	.936	.333	.939	.954	.788	.897	.922	.920

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City of Orange
N/S: Tustin Street
E/W: SR-22 EB Off Ramp/Seba Avenue
Weather: Clear

File Name : 90_ORN_Tustin_22E PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:45 PM				05:00 PM				04:45 PM				04:00 PM			
+0 mins.	14	167	0	181	8	0	21	**29**	0	380	0	380	**164**	11	142	**317**
+15 mins.	12	177	0	189	**9**	0	18	27	0	382	1	383	153	**16**	**146**	315
+30 mins.	10	155	0	165	6	0	17	23	0	372	**3**	375	144	7	142	293
+45 mins.	**16**	**208**	0	**224**	5	0	**24**	29	0	**413**	0	**413**	114	7	123	244
Total Volume	52	707	0	759	28	0	80	108	0	1547	4	1551	575	41	553	1169
% App. Total	6.9	93.1	0		25.9	0	74.1		0	99.7	0.3		49.2	3.5	47.3	
PHF	.813	.850	.000	.847	.778	.000	.833	.931	.000	.936	.333	.939	.877	.641	.947	.922

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City of Santa Ana
N/S: Tustin Street/Tustin Avenue
E/W: Fairhaven Avenue
Weather: Clear

File Name : 91_SNA_Tustin_Fairhaven AM
Site Code : 20220171
Start Date : 3/12/2020

Groups Printed- Total Volume

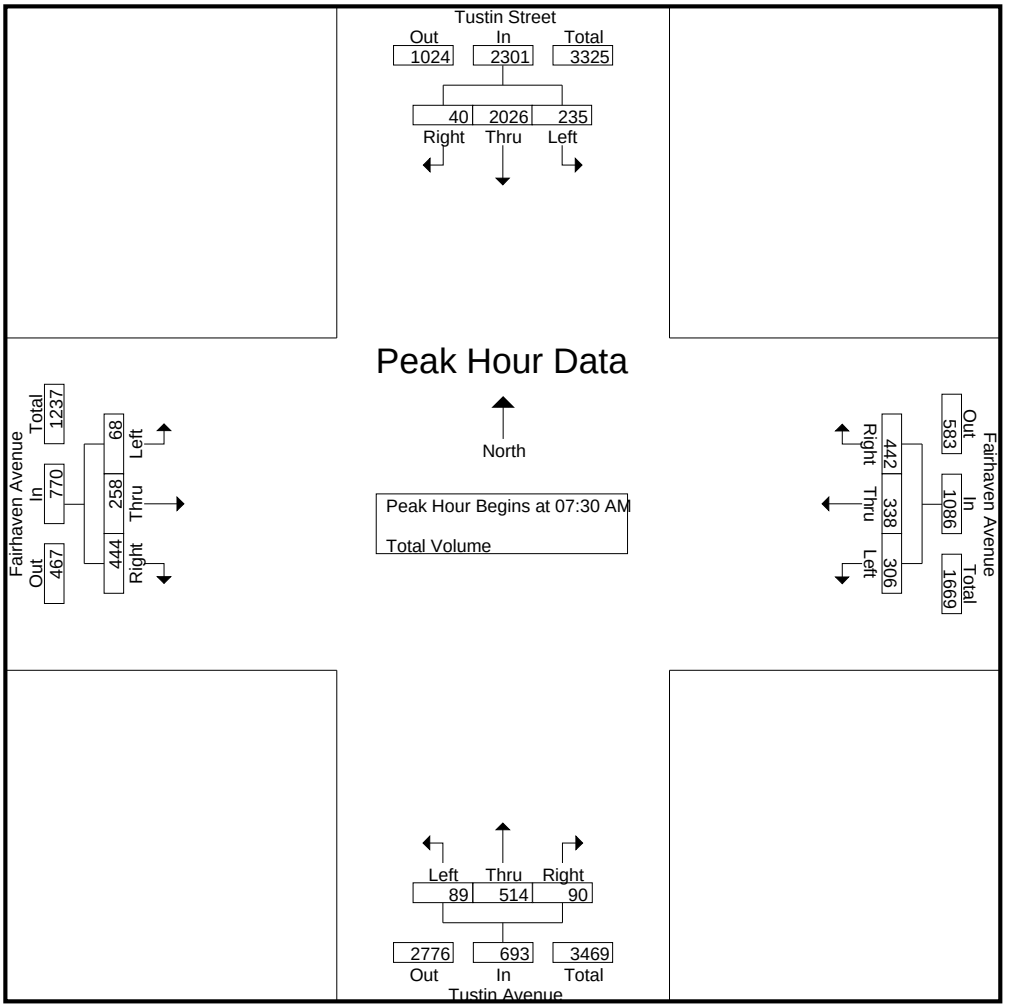
	Tustin Street Southbound				Fairhaven Avenue Westbound				Tustin Avenue Northbound				Fairhaven Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
07:00 AM	39	455	3	497	44	20	70	134	8	93	12	113	11	27	66	104	848
07:15 AM	59	554	8	621	51	45	85	181	14	108	19	141	14	45	85	144	1087
07:30 AM	61	526	4	591	77	71	112	260	21	123	30	174	17	72	119	208	1233
07:45 AM	67	501	13	581	80	92	139	311	28	138	17	183	16	73	101	190	1265
Total	226	2036	28	2290	252	228	406	886	71	462	78	611	58	217	371	646	4433
08:00 AM	53	520	11	584	81	106	102	289	24	125	23	172	19	57	126	202	1247
08:15 AM	54	479	12	545	68	69	89	226	16	128	20	164	16	56	98	170	1105
08:30 AM	53	399	6	458	54	39	82	175	16	136	19	171	21	50	71	142	946
08:45 AM	49	387	5	441	48	32	61	141	13	109	15	137	17	25	68	110	829
Total	209	1785	34	2028	251	246	334	831	69	498	77	644	73	188	363	624	4127
Grand Total	435	3821	62	4318	503	474	740	1717	140	960	155	1255	131	405	734	1270	8560
Apprch %	10.1	88.5	1.4		29.3	27.6	43.1		11.2	76.5	12.4		10.3	31.9	57.8		
Total %	5.1	44.6	0.7	50.4	5.9	5.5	8.6	20.1	1.6	11.2	1.8	14.7	1.5	4.7	8.6	14.8	

	Tustin Street Southbound				Fairhaven Avenue Westbound				Tustin Avenue Northbound				Fairhaven Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	61	**526**	4	**591**	77	71	112	260	21	123	**30**	174	17	72	119	**208**	1233
07:45 AM	**67**	501	**13**	581	80	92	**139**	**311**	**28**	**138**	17	**183**	16	**73**	101	190	**1265**
08:00 AM	53	520	11	584	**81**	**106**	102	289	24	125	23	172	**19**	57	**126**	202	1247
08:15 AM	54	479	12	545	68	69	89	226	16	128	20	164	16	56	98	170	1105
Total Volume	235	2026	40	2301	306	338	442	1086	89	514	90	693	68	258	444	770	4850
% App. Total	10.2	88	1.7		28.2	31.1	40.7		12.8	74.2	13		8.8	33.5	57.7		
PHF	.877	.963	.769	.973	.944	.797	.795	.873	.795	.931	.750	.947	.895	.884	.881	.925	.958

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City of Santa Ana
N/S: Tustin Street/Tustin Avenue
E/W: Fairhaven Avenue
Weather: Clear

File Name : 91_SNA_Tustin_Fairhaven AM
Site Code : 20220171
Start Date : 3/12/2020
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Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:15 AM				07:30 AM				07:30 AM				07:30 AM			
+0 mins.	59	**554**	8	**621**	77	71	112	260	21	123	**30**	174	17	72	119	**208**
+15 mins.	61	526	4	591	80	92	**139**	**311**	**28**	**138**	17	**183**	16	**73**	101	190
+30 mins.	**67**	501	**13**	581	**81**	**106**	102	289	24	125	23	172	**19**	57	**126**	202
+45 mins.	53	520	11	584	68	69	89	226	16	128	20	164	16	56	98	170
Total Volume	240	2101	36	2377	306	338	442	1086	89	514	90	693	68	258	444	770
% App. Total	10.1	88.4	1.5		28.2	31.1	40.7		12.8	74.2	13		8.8	33.5	57.7	
PHF	.896	.948	.692	.957	.944	.797	.795	.873	.795	.931	.750	.947	.895	.884	.881	.925

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City of Santa Ana
N/S: Tustin Street/Tustin Avenue
E/W: Fairhaven Avenue
Weather: Clear

File Name : 91_SNA_Tustin_Fairhaven PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

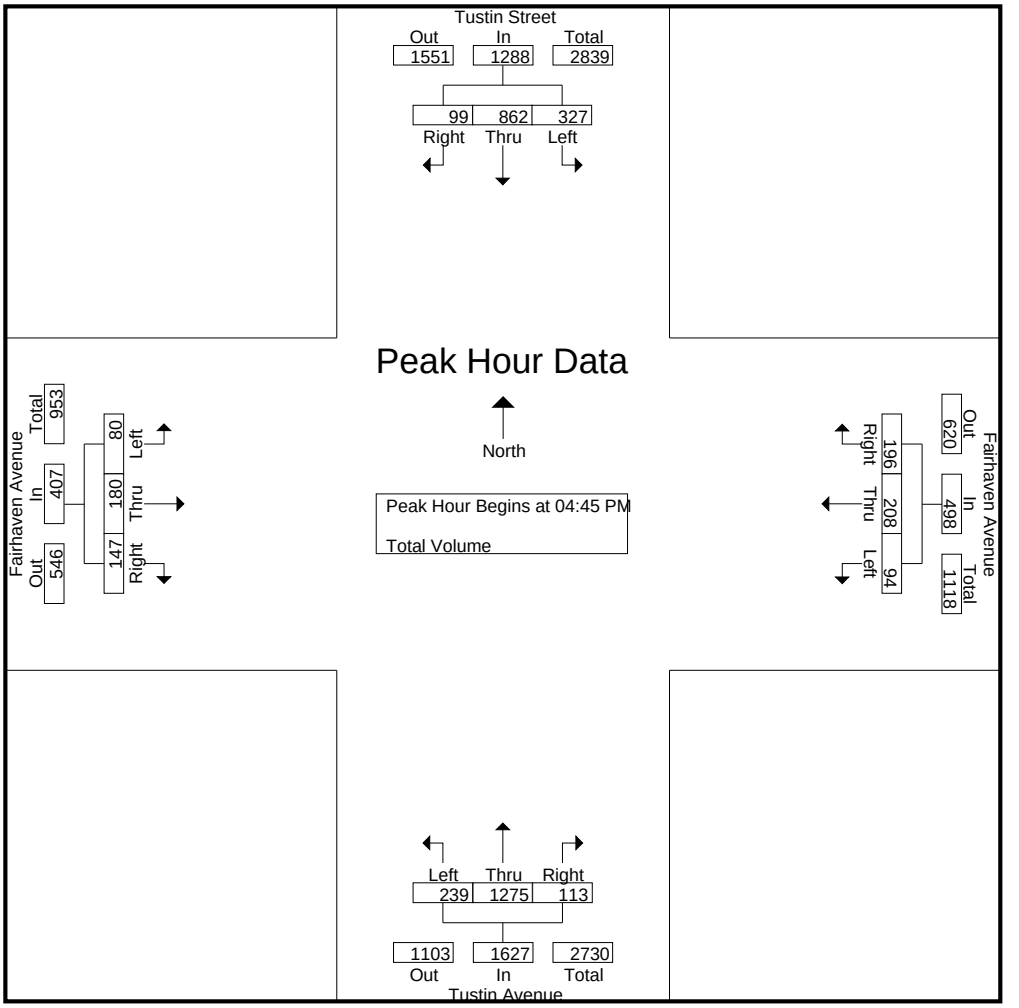
	Tustin Street Southbound				Fairhaven Avenue Westbound				Tustin Avenue Northbound				Fairhaven Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
04:00 PM	80	189	19	288	46	39	63	148	48	270	27	345	21	44	32	97	878
04:15 PM	84	210	14	308	25	47	50	122	50	277	35	362	18	40	35	93	885
04:30 PM	68	182	21	271	22	55	51	128	48	289	42	379	19	43	37	99	877
04:45 PM	71	205	18	294	20	47	42	109	66	311	33	410	20	32	38	90	903
Total	303	786	72	1161	113	188	206	507	212	1147	137	1496	78	159	142	379	3543
05:00 PM	72	205	32	309	19	56	51	126	61	324	32	417	23	56	37	116	968
05:15 PM	92	198	28	318	30	46	45	121	57	329	21	407	20	52	39	111	957
05:30 PM	92	254	21	367	25	59	58	142	55	311	27	393	17	40	33	90	992
05:45 PM	70	195	14	279	29	54	47	130	58	273	27	358	17	35	46	98	865
Total	326	852	95	1273	103	215	201	519	231	1237	107	1575	77	183	155	415	3782
Grand Total	629	1638	167	2434	216	403	407	1026	443	2384	244	3071	155	342	297	794	7325
Apprch %	25.8	67.3	6.9		21.1	39.3	39.7		14.4	77.6	7.9		19.5	43.1	37.4		
Total %	8.6	22.4	2.3	33.2	2.9	5.5	5.6	14	6	32.5	3.3	41.9	2.1	4.7	4.1	10.8	

	Tustin Street Southbound				Fairhaven Avenue Westbound				Tustin Avenue Northbound				Fairhaven Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	71	205	18	294	20	47	42	109	**66**	311	**33**	410	20	32	38	90	903
05:00 PM	72	205	**32**	309	19	56	51	126	61	324	32	**417**	**23**	**56**	37	**116**	968
05:15 PM	**92**	198	28	318	**30**	46	45	121	57	**329**	21	407	20	52	**39**	111	957
05:30 PM	92	**254**	21	**367**	25	**59**	**58**	**142**	55	311	27	393	17	40	33	90	**992**
Total Volume	327	862	99	1288	94	208	196	498	239	1275	113	1627	80	180	147	407	3820
% App. Total	25.4	66.9	7.7		18.9	41.8	39.4		14.7	78.4	6.9		19.7	44.2	36.1		
PHF	.889	.848	.773	.877	.783	.881	.845	.877	.905	.969	.856	.975	.870	.804	.942	.877	.963

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File Name : 91_SNA_Tustin_Fairhaven PM
Site Code : 20220171
Start Date : 3/12/2020
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Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:45 PM				05:00 PM				04:45 PM				04:30 PM			
+0 mins.	71	205	18	294	19	56	51	126	**66**	311	**33**	410	19	43	37	99
+15 mins.	72	205	**32**	309	**30**	46	45	121	61	324	32	**417**	20	32	38	90
+30 mins.	**92**	198	28	318	25	**59**	**58**	**142**	57	**329**	21	407	**23**	**56**	37	**116**
+45 mins.	92	**254**	21	**367**	29	54	47	130	55	311	27	393	20	52	**39**	111
Total Volume	327	862	99	1288	103	215	201	519	239	1275	113	1627	82	183	151	416
% App. Total	25.4	66.9	7.7		19.8	41.4	38.7		14.7	78.4	6.9		19.7	44	36.3	
PHF	.889	.848	.773	.877	.858	.911	.866	.914	.905	.969	.856	.975	.891	.817	.968	.897

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City of Santa Ana
N/S: Tustin Avenue
E/W: Santa Clara Avenue
Weather: Clear

File Name : 92_SNA_Tustin_Santa Clara AM
Site Code : 20220171
Start Date : 3/12/2020
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Groups Printed- Total Volume

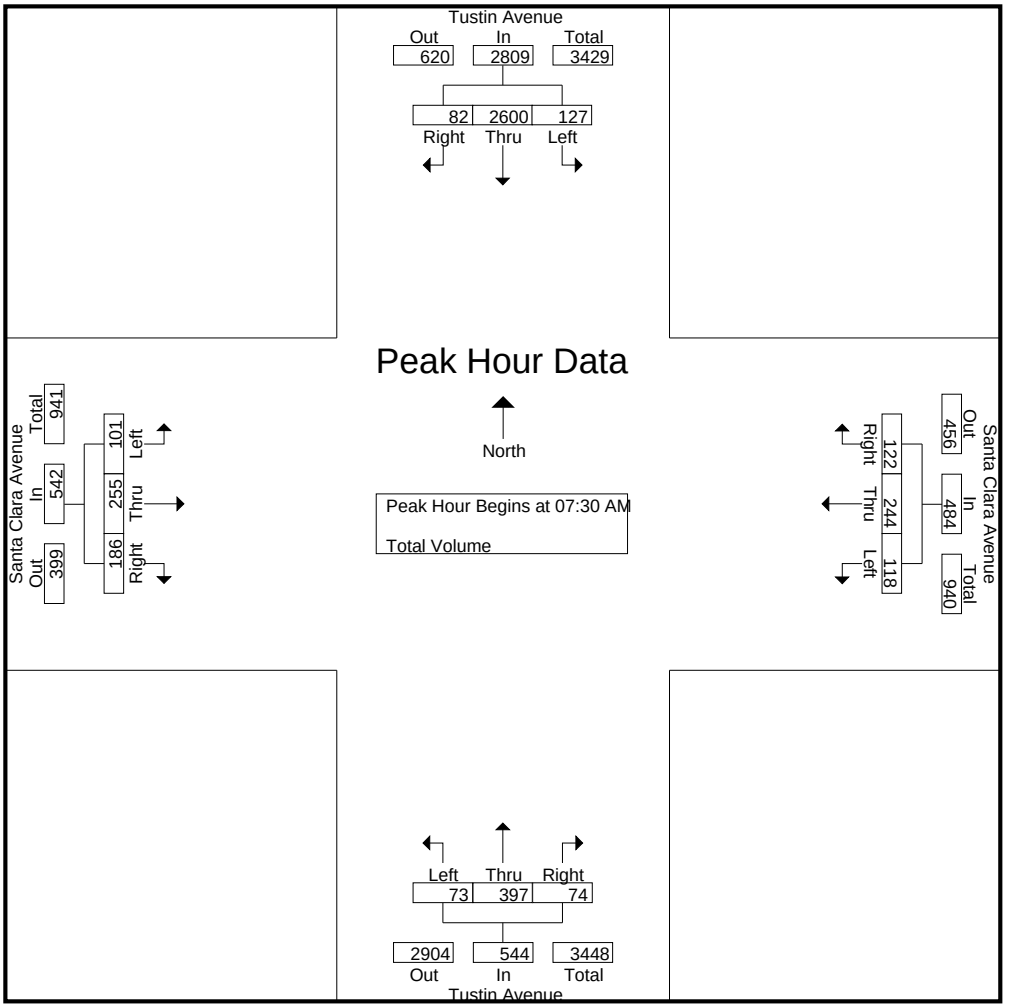
	Tustin Avenue Southbound				Santa Clara Avenue Westbound				Tustin Avenue Northbound				Santa Clara Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
07:00 AM	19	509	22	550	18	20	21	59	15	65	13	93	23	34	35	92	794
07:15 AM	31	655	24	710	20	29	32	81	9	78	24	111	16	86	40	142	1044
07:30 AM	24	633	22	679	31	67	32	130	17	92	20	129	33	82	54	169	1107
07:45 AM	26	654	15	695	23	59	29	111	22	101	16	139	19	73	32	124	1069
Total	100	2451	83	2634	92	175	114	381	63	336	73	472	91	275	161	527	4014
08:00 AM	33	661	28	722	30	63	37	130	17	101	20	138	32	61	56	149	1139
08:15 AM	44	652	17	713	34	55	24	113	17	103	18	138	17	39	44	100	1064
08:30 AM	23	473	17	513	19	33	23	75	20	94	13	127	25	31	37	93	808
08:45 AM	25	465	18	508	28	28	19	75	14	91	8	113	29	34	54	117	813
Total	125	2251	80	2456	111	179	103	393	68	389	59	516	103	165	191	459	3824
Grand Total	225	4702	163	5090	203	354	217	774	131	725	132	988	194	440	352	986	7838
Apprch %	4.4	92.4	3.2		26.2	45.7	28		13.3	73.4	13.4		19.7	44.6	35.7		
Total %	2.9	60	2.1	64.9	2.6	4.5	2.8	9.9	1.7	9.2	1.7	12.6	2.5	5.6	4.5	12.6	

	Tustin Avenue Southbound				Santa Clara Avenue Westbound				Tustin Avenue Northbound				Santa Clara Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	24	633	22	679	31	**67**	32	**130**	17	92	**20**	129	**33**	**82**	54	**169**	1107
07:45 AM	26	654	15	695	23	59	29	111	**22**	101	16	**139**	19	73	32	124	1069
08:00 AM	33	**661**	**28**	**722**	30	63	**37**	130	17	101	20	138	32	61	**56**	149	**1139**
08:15 AM	**44**	652	17	713	**34**	55	24	113	17	**103**	18	138	17	39	44	100	1064
Total Volume	127	2600	82	2809	118	244	122	484	73	397	74	544	101	255	186	542	4379
% App. Total	4.5	92.6	2.9		24.4	50.4	25.2		13.4	73	13.6		18.6	47	34.3		
PHF	.722	.983	.732	.973	.868	.910	.824	.931	.830	.964	.925	.978	.765	.777	.830	.802	.961

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City of Santa Ana
N/S: Tustin Avenue
E/W: Santa Clara Avenue
Weather: Clear

File Name : 92_SNA_Tustin_Santa Clara AM
Site Code : 20220171
Start Date : 3/12/2020
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Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:30 AM				07:30 AM				07:30 AM				07:15 AM			
+0 mins.	24	633	22	679	31	**67**	32	**130**	17	92	**20**	129	16	**86**	40	142
+15 mins.	26	654	15	695	23	59	29	111	**22**	101	16	**139**	**33**	82	54	**169**
+30 mins.	33	**661**	**28**	**722**	30	63	**37**	130	17	101	20	138	19	73	32	124
+45 mins.	**44**	652	17	713	**34**	55	24	113	17	**103**	18	138	32	61	**56**	149
Total Volume	127	2600	82	2809	118	244	122	484	73	397	74	544	100	302	182	584
% App. Total	4.5	92.6	2.9		24.4	50.4	25.2		13.4	73	13.6		17.1	51.7	31.2	
PHF	.722	.983	.732	.973	.868	.910	.824	.931	.830	.964	.925	.978	.758	.878	.813	.864

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City of Santa Ana
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Weather: Clear

File Name : 92_SNA_Tustin_Santa Clara PM
Site Code : 20220171
Start Date : 3/12/2020
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Groups Printed- Total Volume

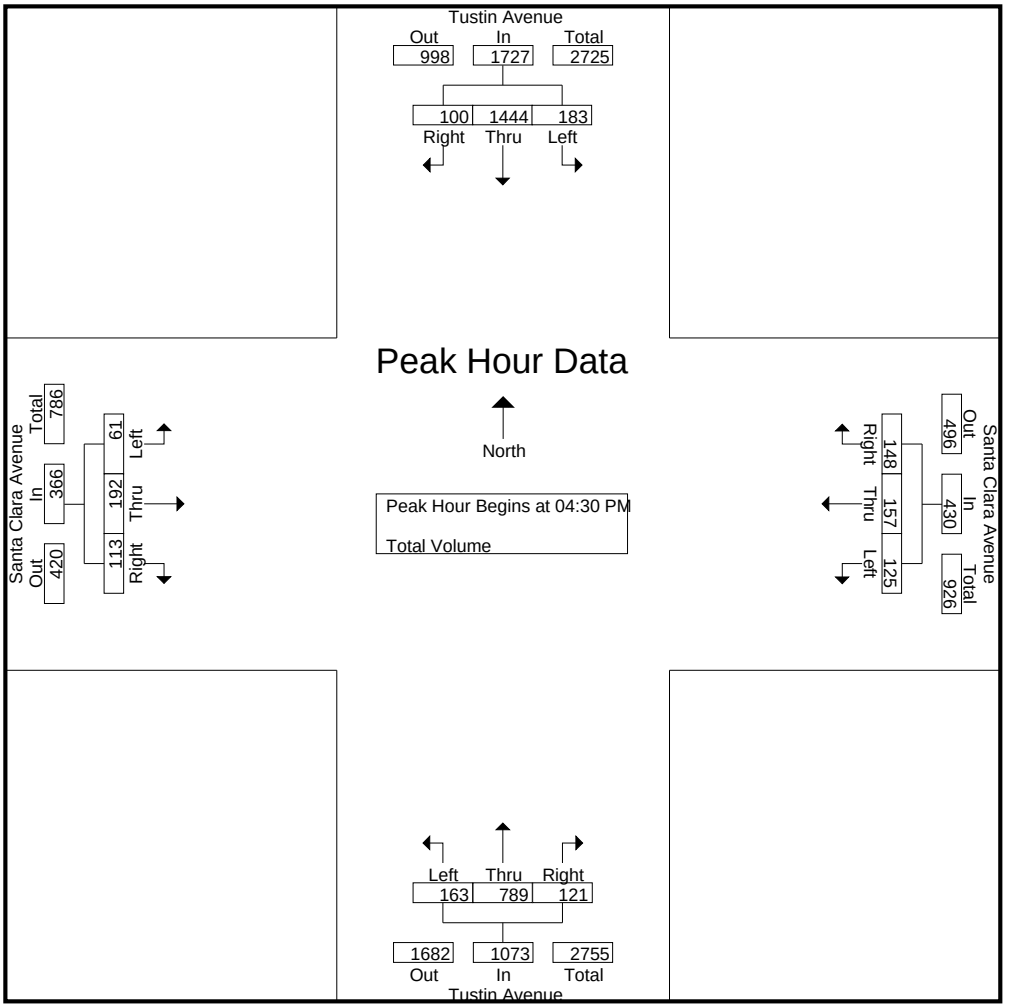
	Tustin Avenue Southbound				Santa Clara Avenue Westbound				Tustin Avenue Northbound				Santa Clara Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
04:00 PM	34	318	34	386	35	26	22	83	40	197	26	263	19	41	28	88	820
04:15 PM	53	301	29	383	29	31	28	88	32	182	30	244	14	45	30	89	804
04:30 PM	47	358	26	431	31	44	44	119	38	174	32	244	16	52	22	90	884
04:45 PM	44	349	18	411	23	40	32	95	32	197	31	260	15	47	24	86	852
Total	178	1326	107	1611	118	141	126	385	142	750	119	1011	64	185	104	353	3360
05:00 PM	37	372	30	439	28	40	30	98	37	199	29	265	17	45	40	102	904
05:15 PM	55	365	26	446	43	33	42	118	56	219	29	304	13	48	27	88	956
05:30 PM	41	344	17	402	34	50	32	116	36	203	24	263	16	52	33	101	882
05:45 PM	40	304	26	370	34	39	24	97	47	213	23	283	15	33	25	73	823
Total	173	1385	99	1657	139	162	128	429	176	834	105	1115	61	178	125	364	3565
Grand Total	351	2711	206	3268	257	303	254	814	318	1584	224	2126	125	363	229	717	6925
Apprch %	10.7	83	6.3		31.6	37.2	31.2		15	74.5	10.5		17.4	50.6	31.9		
Total %	5.1	39.1	3	47.2	3.7	4.4	3.7	11.8	4.6	22.9	3.2	30.7	1.8	5.2	3.3	10.4	

	Tustin Avenue Southbound				Santa Clara Avenue Westbound				Tustin Avenue Northbound				Santa Clara Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:30 PM																	
04:30 PM	47	358	26	431	31	**44**	**44**	**119**	38	174	**32**	244	16	**52**	22	90	884
04:45 PM	44	349	18	411	23	40	32	95	32	197	31	260	15	47	24	86	852
05:00 PM	37	**372**	**30**	439	28	40	30	98	37	199	29	265	**17**	45	**40**	**102**	904
05:15 PM	**55**	365	26	**446**	**43**	33	42	118	**56**	**219**	29	**304**	13	48	27	88	**956**
Total Volume	183	1444	100	1727	125	157	148	430	163	789	121	1073	61	192	113	366	3596
% App. Total	10.6	83.6	5.8		29.1	36.5	34.4		15.2	73.5	11.3		16.7	52.5	30.9		
PHF	.832	.970	.833	.968	.727	.892	.841	.903	.728	.901	.945	.882	.897	.923	.706	.897	.940

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N/S: Tustin Avenue
E/W: Santa Clara Avenue
Weather: Clear

File Name : 92_SNA_Tustin_Santa Clara PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:30 PM				04:30 PM				05:00 PM				04:45 PM			
+0 mins.	47	358	26	431	31	**44**	**44**	**119**	37	199	**29**	265	15	47	24	86
+15 mins.	44	349	18	411	23	40	32	95	**56**	**219**	29	**304**	**17**	45	**40**	**102**
+30 mins.	37	**372**	**30**	439	28	40	30	98	36	203	24	263	13	48	27	88
+45 mins.	**55**	365	26	**446**	**43**	33	42	118	47	213	23	283	16	**52**	33	101
Total Volume	183	1444	100	1727	125	157	148	430	176	834	105	1115	61	192	124	377
% App. Total	10.6	83.6	5.8		29.1	36.5	34.4		15.8	74.8	9.4		16.2	50.9	32.9	
PHF	.832	.970	.833	.968	.727	.892	.841	.903	.786	.952	.905	.917	.897	.923	.775	.924

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City of Santa Ana
N/S: Tustin Avenue
E/W: 17th Street
Weather: Clear

File Name : 93_SNA_Tustin_17th St AM
Site Code : 20220171
Start Date : 3/12/2020
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Groups Printed- Total Volume

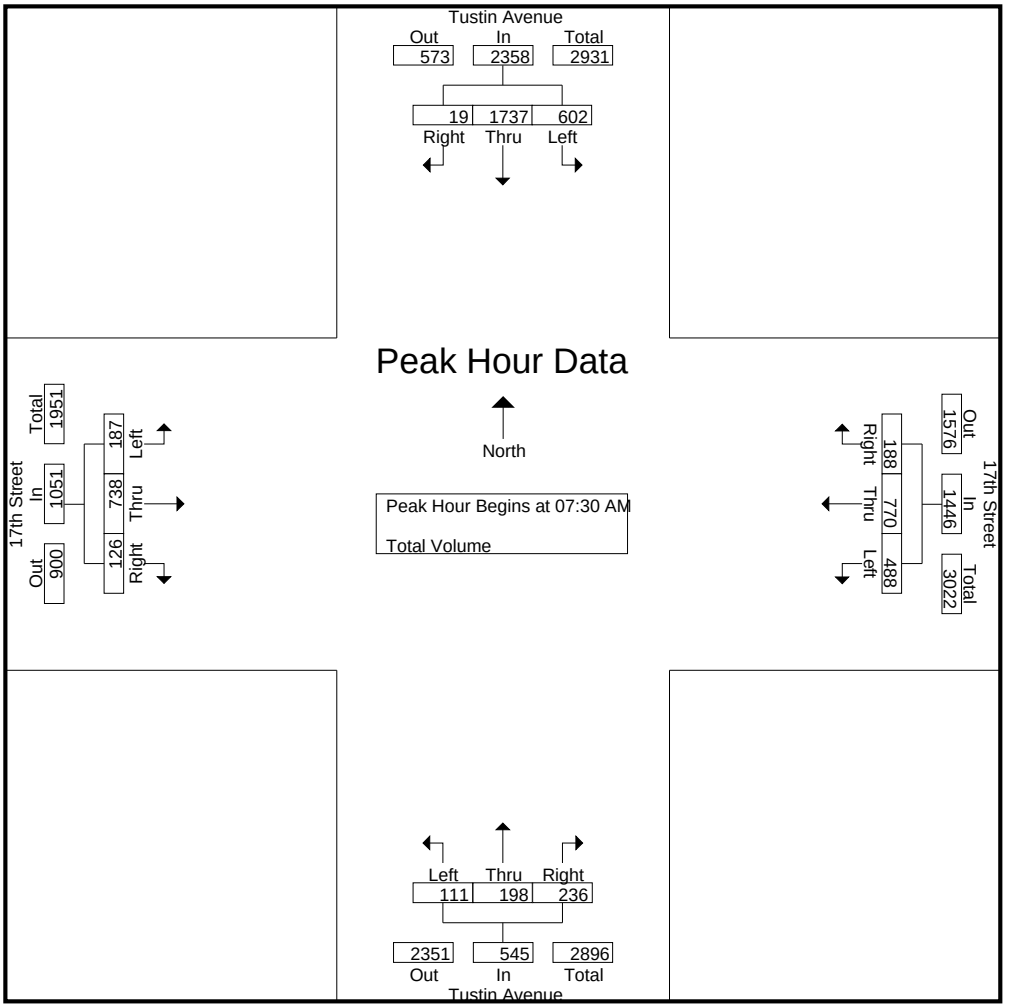
	Tustin Avenue Southbound				17th Street Westbound				Tustin Avenue Northbound				17th Street Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
07:00 AM	147	328	3	478	79	95	23	197	21	35	45	101	23	118	17	158	934
07:15 AM	133	414	1	548	77	131	33	241	20	41	55	116	32	228	22	282	1187
07:30 AM	174	475	6	655	79	147	28	254	26	41	61	128	27	216	22	265	1302
07:45 AM	123	408	1	532	139	227	51	417	25	54	58	137	46	188	26	260	1346
Total	577	1625	11	2213	374	600	135	1109	92	171	219	482	128	750	87	965	4769
08:00 AM	154	454	5	613	131	201	51	383	27	56	47	130	53	199	33	285	1411
08:15 AM	151	400	7	558	139	195	58	392	33	47	70	150	61	135	45	241	1341
08:30 AM	128	288	10	426	122	145	40	307	28	65	40	133	45	124	35	204	1070
08:45 AM	117	314	9	440	142	176	47	365	37	44	45	126	46	132	35	213	1144
Total	550	1456	31	2037	534	717	196	1447	125	212	202	539	205	590	148	943	4966
Grand Total	1127	3081	42	4250	908	1317	331	2556	217	383	421	1021	333	1340	235	1908	9735
Apprch %	26.5	72.5	1		35.5	51.5	12.9		21.3	37.5	41.2		17.5	70.2	12.3		
Total %	11.6	31.6	0.4	43.7	9.3	13.5	3.4	26.3	2.2	3.9	4.3	10.5	3.4	13.8	2.4	19.6	

	Tustin Avenue Southbound				17th Street Westbound				Tustin Avenue Northbound				17th Street Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	**174**	**475**	6	**655**	79	147	28	254	26	41	61	128	27	**216**	22	265	1302
07:45 AM	123	408	1	532	**139**	**227**	51	**417**	25	54	58	137	46	188	26	260	1346
08:00 AM	154	454	5	613	131	201	51	383	27	**56**	47	130	53	199	33	**285**	**1411**
08:15 AM	151	400	**7**	558	139	195	**58**	392	**33**	47	**70**	**150**	**61**	135	**45**	241	1341
Total Volume	602	1737	19	2358	488	770	188	1446	111	198	236	545	187	738	126	1051	5400
% App. Total	25.5	73.7	0.8		33.7	53.3	13		20.4	36.3	43.3		17.8	70.2	12		
PHF	.865	.914	.679	.900	.878	.848	.810	.867	.841	.884	.843	.908	.766	.854	.700	.922	.957

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City of Santa Ana
N/S: Tustin Avenue
E/W: 17th Street
Weather: Clear

File Name : 93_SNA_Tustin_17th St AM
Site Code : 20220171
Start Date : 3/12/2020
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Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:30 AM				07:45 AM				07:45 AM				07:15 AM			
+0 mins.	**174**	**475**	6	**655**	**139**	**227**	51	**417**	25	54	58	137	32	**228**	22	282
+15 mins.	123	408	1	532	131	201	51	383	27	56	47	130	27	216	22	265
+30 mins.	154	454	5	613	139	195	**58**	392	**33**	47	**70**	**150**	46	188	26	260
+45 mins.	151	400	**7**	558	122	145	40	307	28	**65**	40	133	**53**	199	**33**	**285**
Total Volume	602	1737	19	2358	531	768	200	1499	113	222	215	550	158	831	103	1092
% App. Total	25.5	73.7	0.8		35.4	51.2	13.3		20.5	40.4	39.1		14.5	76.1	9.4	
PHF	.865	.914	.679	.900	.955	.846	.862	.899	.856	.854	.768	.917	.745	.911	.780	.958

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City of Santa Ana
N/S: Tustin Avenue
E/W: 17th Street
Weather: Clear

File Name : 93_SNA_Tustin_17th St PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

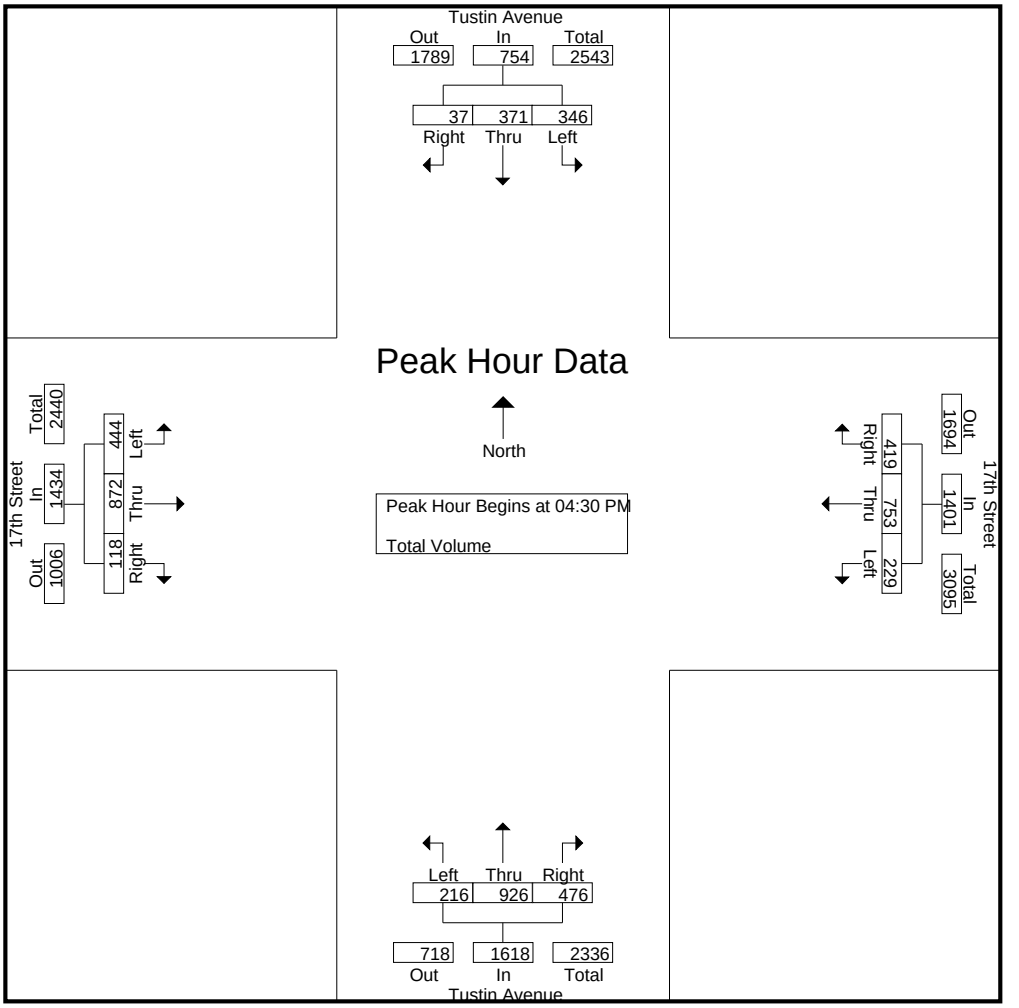
	Tustin Avenue Southbound				17th Street Westbound				Tustin Avenue Northbound				17th Street Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
04:00 PM	73	93	7	173	47	193	90	330	58	224	106	388	103	163	29	295	1186
04:15 PM	69	73	12	154	51	229	101	381	50	177	72	299	98	166	30	294	1128
04:30 PM	89	92	10	191	58	176	109	343	68	233	110	411	125	197	32	354	1299
04:45 PM	80	76	6	162	64	237	111	412	52	189	84	325	103	211	19	333	1232
Total	311	334	35	680	220	835	411	1466	228	823	372	1423	429	737	110	1276	4845
05:00 PM	90	92	9	191	46	160	108	314	57	238	169	464	115	244	29	388	1357
05:15 PM	87	111	12	210	61	180	91	332	39	266	113	418	101	220	38	359	1319
05:30 PM	89	88	12	189	48	205	88	341	47	176	102	325	129	219	37	385	1240
05:45 PM	89	101	18	208	49	186	88	323	44	207	88	339	94	180	24	298	1168
Total	355	392	51	798	204	731	375	1310	187	887	472	1546	439	863	128	1430	5084
Grand Total	666	726	86	1478	424	1566	786	2776	415	1710	844	2969	868	1600	238	2706	9929
Apprch %	45.1	49.1	5.8		15.3	56.4	28.3		14	57.6	28.4		32.1	59.1	8.8		
Total %	6.7	7.3	0.9	14.9	4.3	15.8	7.9	28	4.2	17.2	8.5	29.9	8.7	16.1	2.4	27.3	

	Tustin Avenue Southbound				17th Street Westbound				Tustin Avenue Northbound				17th Street Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:30 PM																	
04:30 PM	89	92	10	191	58	176	109	343	**68**	233	110	411	**125**	197	32	354	1299
04:45 PM	80	76	6	162	**64**	**237**	**111**	**412**	52	189	84	325	103	211	19	333	1232
05:00 PM	**90**	92	9	191	46	160	108	314	57	238	**169**	**464**	115	**244**	29	**388**	**1357**
05:15 PM	87	**111**	**12**	**210**	61	180	91	332	39	**266**	113	418	101	220	**38**	359	1319
Total Volume	346	371	37	754	229	753	419	1401	216	926	476	1618	444	872	118	1434	5207
% App. Total	45.9	49.2	4.9		16.3	53.7	29.9		13.3	57.2	29.4		31	60.8	8.2		
PHF	.961	.836	.771	.898	.895	.794	.944	.850	.794	.870	.704	.872	.888	.893	.776	.924	.959

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Site Code : 20220171
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Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	05:00 PM				04:00 PM				04:30 PM				04:45 PM			
+0 mins.	**90**	92	9	191	47	193	90	330	**68**	233	110	411	103	211	19	333
+15 mins.	87	**111**	12	**210**	51	229	101	381	52	189	84	325	115	**244**	29	**388**
+30 mins.	89	88	12	189	58	176	109	343	57	238	**169**	**464**	101	220	**38**	359
+45 mins.	89	101	**18**	208	**64**	**237**	**111**	**412**	39	**266**	113	418	**129**	219	37	385
Total Volume	355	392	51	798	220	835	411	1466	216	926	476	1618	448	894	123	1465
% App. Total	44.5	49.1	6.4		15	57	28		13.3	57.2	29.4		30.6	61	8.4	
PHF	.986	.883	.708	.950	.859	.881	.926	.890	.794	.870	.704	.872	.868	.916	.809	.944

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City of Santa Ana
N/S: Tustin Avenue
E/W: 4th Street
Weather: Clear

File Name : 94_SNA_Tustin_4th St AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

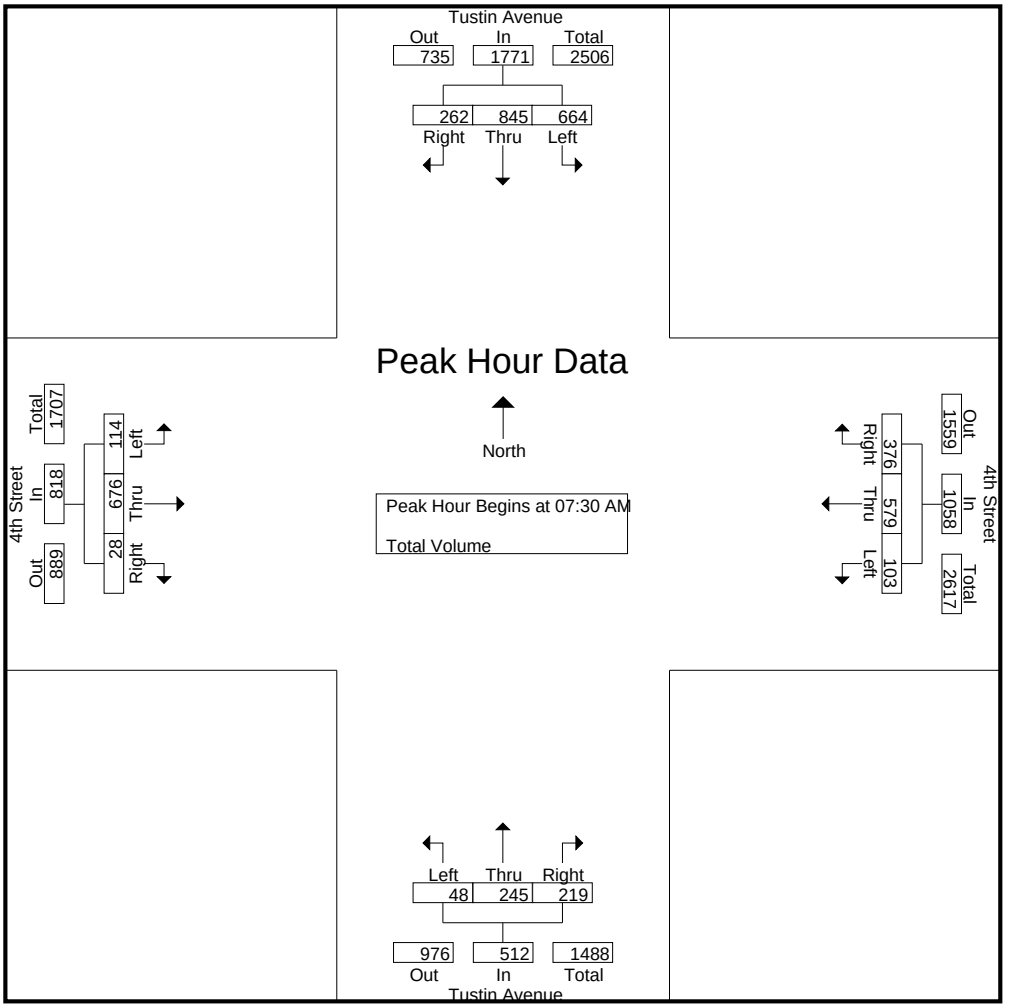
	Tustin Avenue Southbound				4th Street Westbound				Tustin Avenue Northbound				4th Street Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
07:00 AM	177	130	38	345	10	64	56	130	5	40	33	78	13	87	5	105	658
07:15 AM	185	164	39	388	23	92	63	178	9	43	32	84	31	130	12	173	823
07:30 AM	178	258	72	508	25	152	64	241	13	53	48	114	21	198	3	222	1085
07:45 AM	155	203	74	432	27	150	104	281	13	67	68	148	22	208	5	235	1096
Total	695	755	223	1673	85	458	287	830	40	203	181	424	87	623	25	735	3662
08:00 AM	156	191	63	410	20	131	96	247	10	67	57	134	38	161	12	211	1002
08:15 AM	175	193	53	421	31	146	112	289	12	58	46	116	33	109	8	150	976
08:30 AM	168	151	55	374	26	132	87	245	15	71	49	135	32	104	13	149	903
08:45 AM	131	135	64	330	17	146	85	248	10	54	25	89	38	108	13	159	826
Total	630	670	235	1535	94	555	380	1029	47	250	177	474	141	482	46	669	3707
Grand Total	1325	1425	458	3208	179	1013	667	1859	87	453	358	898	228	1105	71	1404	7369
Apprch %	41.3	44.4	14.3		9.6	54.5	35.9		9.7	50.4	39.9		16.2	78.7	5.1		
Total %	18	19.3	6.2	43.5	2.4	13.7	9.1	25.2	1.2	6.1	4.9	12.2	3.1	15	1	19.1	

	Tustin Avenue Southbound				4th Street Westbound				Tustin Avenue Northbound				4th Street Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	**178**	**258**	72	**508**	25	**152**	64	241	**13**	53	48	114	21	198	3	222	1085
07:45 AM	155	203	**74**	432	27	150	104	281	13	**67**	**68**	**148**	22	**208**	5	**235**	**1096**
08:00 AM	156	191	63	410	20	131	96	247	10	67	57	134	**38**	161	**12**	211	1002
08:15 AM	175	193	53	421	**31**	146	**112**	**289**	12	58	46	116	33	109	8	150	976
Total Volume	664	845	262	1771	103	579	376	1058	48	245	219	512	114	676	28	818	4159
% App. Total	37.5	47.7	14.8		9.7	54.7	35.5		9.4	47.9	42.8		13.9	82.6	3.4		
PHF	.933	.819	.885	.872	.831	.952	.839	.915	.923	.914	.805	.865	.750	.813	.583	.870	.949

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City of Santa Ana
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E/W: 4th Street
Weather: Clear

File Name : 94_SNA_Tustin_4th St AM
Site Code : 20220171
Start Date : 3/12/2020
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Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:30 AM				07:45 AM				07:45 AM				07:15 AM			
+0 mins.	**178**	**258**	72	**508**	27	**150**	104	281	13	67	**68**	**148**	31	130	**12**	173
+15 mins.	155	203	**74**	432	20	131	96	247	10	67	57	134	21	198	3	222
+30 mins.	156	191	63	410	**31**	146	**112**	**289**	12	58	46	116	22	**208**	5	**235**
+45 mins.	175	193	53	421	26	132	87	245	**15**	**71**	49	135	**38**	161	12	211
Total Volume	664	845	262	1771	104	559	399	1062	50	263	220	533	112	697	32	841
% App. Total	37.5	47.7	14.8		9.8	52.6	37.6		9.4	49.3	41.3		13.3	82.9	3.8	
PHF	.933	.819	.885	.872	.839	.932	.891	.919	.833	.926	.809	.900	.737	.838	.667	.895

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Weather: Clear

File Name : 94_SNA_Tustin_4th St PM
Site Code : 20220171
Start Date : 3/12/2020
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Groups Printed- Total Volume

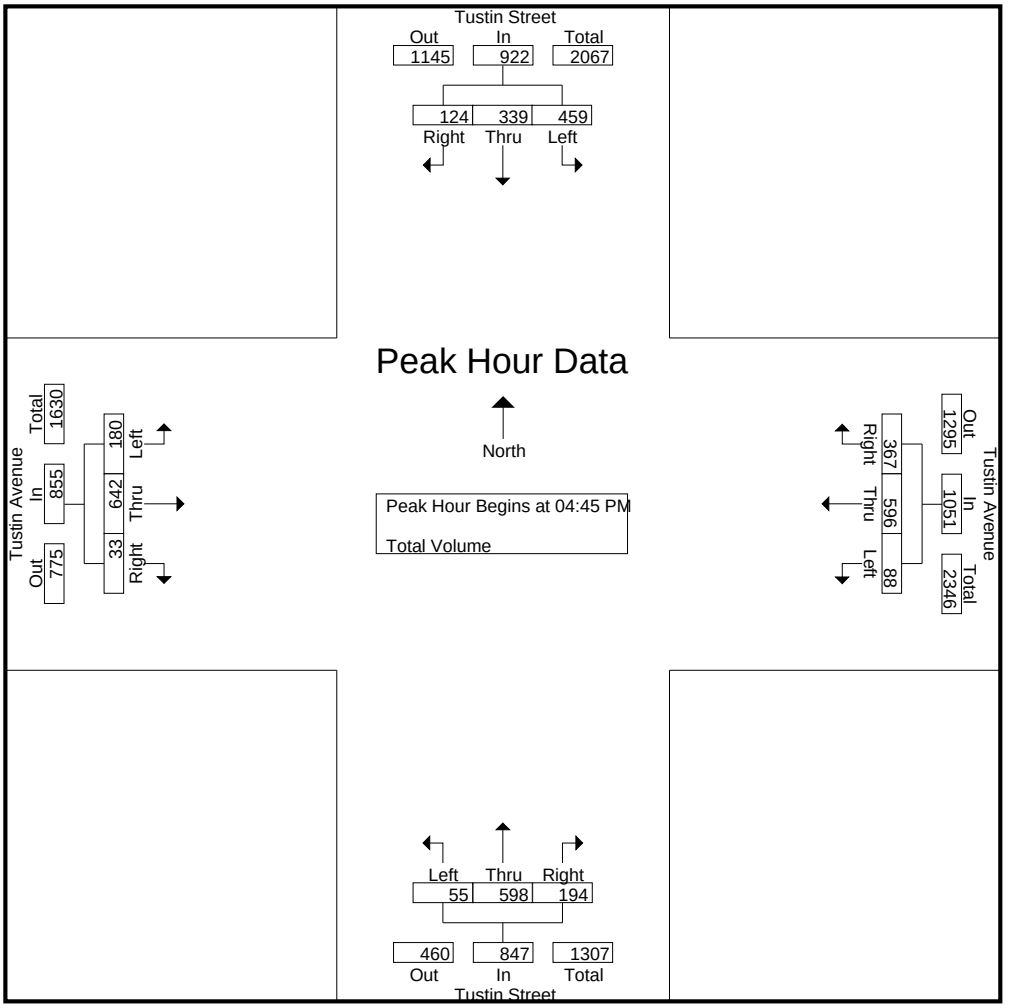
	Tustin Street Southbound				Tustin Avenue Westbound				Tustin Street Northbound				Tustin Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
04:00 PM	100	89	30	219	29	160	59	248	21	145	26	192	41	152	7	200	859
04:15 PM	98	96	23	217	23	222	101	346	19	156	40	215	71	155	21	247	1025
04:30 PM	87	74	34	195	14	69	115	198	14	140	30	184	43	118	10	171	748
04:45 PM	118	79	37	234	22	180	103	305	14	137	41	192	51	164	13	228	959
Total	403	338	124	865	88	631	378	1097	68	578	137	783	206	589	51	846	3591
05:00 PM	109	86	34	229	18	124	77	219	15	166	56	237	40	163	5	208	893
05:15 PM	105	89	35	229	23	139	93	255	12	137	46	195	42	149	6	197	876
05:30 PM	127	85	18	230	25	153	94	272	14	158	51	223	47	166	9	222	947
05:45 PM	92	70	27	189	26	154	83	263	7	145	58	210	42	115	6	163	825
Total	433	330	114	877	92	570	347	1009	48	606	211	865	171	593	26	790	3541
Grand Total	836	668	238	1742	180	1201	725	2106	116	1184	348	1648	377	1182	77	1636	7132
Apprch %	48	38.3	13.7		8.5	57	34.4		7	71.8	21.1		23	72.2	4.7		
Total %	11.7	9.4	3.3	24.4	2.5	16.8	10.2	29.5	1.6	16.6	4.9	23.1	5.3	16.6	1.1	22.9	

	Tustin Street Southbound				Tustin Avenue Westbound				Tustin Street Northbound				Tustin Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	118	79	**37**	**234**	22	**180**	**103**	**305**	14	137	41	192	**51**	164	**13**	**228**	**959**
05:00 PM	109	86	34	229	18	124	77	219	**15**	**166**	**56**	**237**	40	163	5	208	893
05:15 PM	105	**89**	35	229	23	139	93	255	12	137	46	195	42	149	6	197	876
05:30 PM	**127**	85	18	230	**25**	153	94	272	14	158	51	223	47	**166**	9	222	947
Total Volume	459	339	124	922	88	596	367	1051	55	598	194	847	180	642	33	855	3675
% App. Total	49.8	36.8	13.4		8.4	56.7	34.9		6.5	70.6	22.9		21.1	75.1	3.9		
PHF	.904	.952	.838	.985	.880	.828	.891	.861	.917	.901	.866	.893	.882	.967	.635	.938	.958

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E/W: 4th Street
Weather: Clear

File Name : 94_SNA_Tustin_4th St PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:45 PM				04:00 PM				05:00 PM				04:45 PM			
+0 mins.	118	79	**37**	**234**	**29**	160	59	248	**15**	**166**	56	**237**	**51**	164	**13**	**228**
+15 mins.	109	86	34	229	23	**222**	101	**346**	12	137	46	195	40	163	5	208
+30 mins.	105	**89**	35	229	14	69	**115**	198	14	158	51	223	42	149	6	197
+45 mins.	**127**	85	18	230	22	180	103	305	7	145	**58**	210	47	**166**	9	222
Total Volume	459	339	124	922	88	631	378	1097	48	606	211	865	180	642	33	855
% App. Total	49.8	36.8	13.4		8	57.5	34.5		5.5	70.1	24.4		21.1	75.1	3.9	
PHF	.904	.952	.838	.985	.759	.711	.822	.793	.800	.913	.909	.912	.882	.967	.635	.938

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City of Santa Ana
N/S: Auto Mall Dr/SR-55 Southbound Ramps
E/W: Edinger Avenue
Weather: Clear

File Name : 95_SNA_55S_Edinger AM
Site Code : 20220171
Start Date : 3/12/2020
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Groups Printed- Total Volume

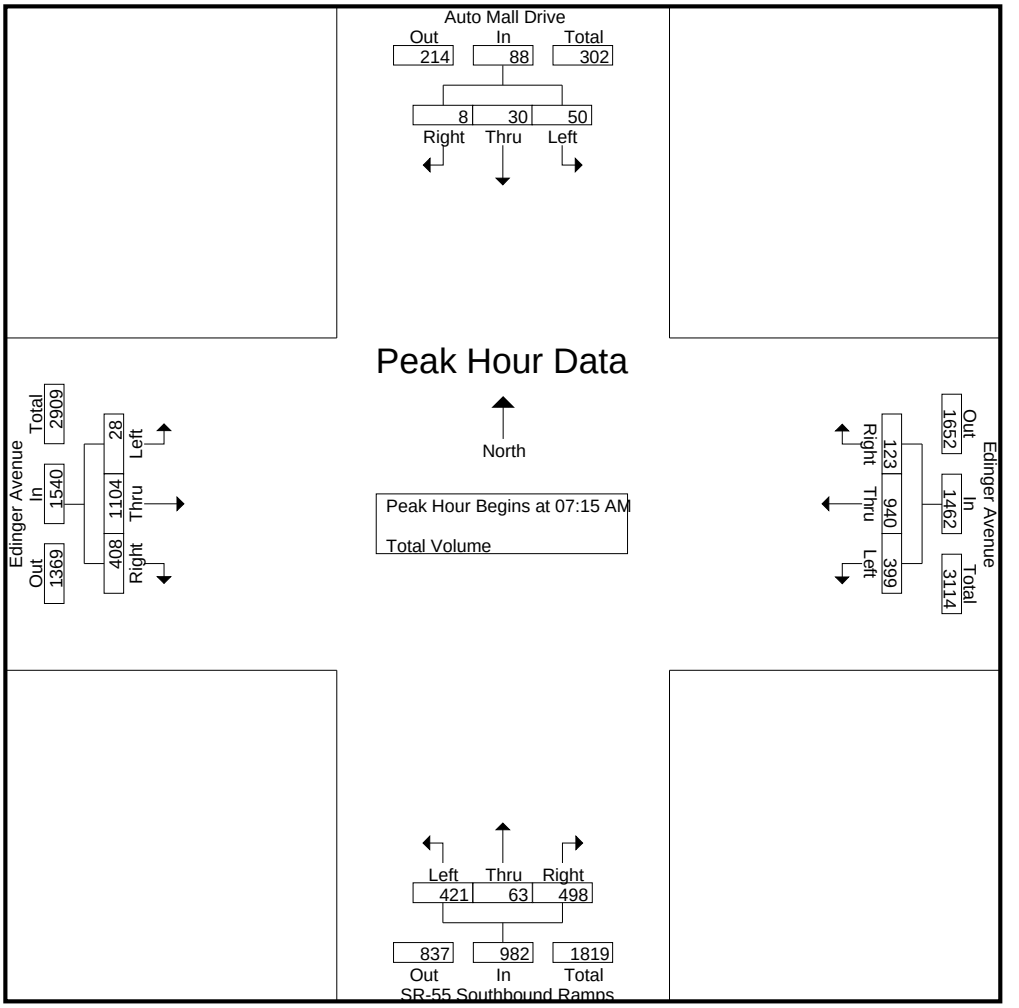
	Auto Mall Drive Southbound				Edinger Avenue Westbound				SR-55 Southbound Ramps Northbound				Edinger Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
07:00 AM	7	0	4	11	56	126	14	196	109	24	117	250	7	261	81	349	806
07:15 AM	11	8	1	20	70	182	29	281	121	15	137	273	6	260	194	460	1034
07:30 AM	16	10	3	29	91	230	25	346	109	18	135	262	3	294	106	403	1040
07:45 AM	8	5	2	15	129	269	34	432	91	12	130	233	3	302	58	363	1043
Total	42	23	10	75	346	807	102	1255	430	69	519	1018	19	1117	439	1575	3923
08:00 AM	15	7	2	24	109	259	35	403	100	18	96	214	16	248	50	314	955
08:15 AM	23	6	2	31	93	269	32	394	86	15	90	191	4	244	62	310	926
08:30 AM	10	5	3	18	88	220	25	333	76	10	88	174	8	256	93	357	882
08:45 AM	19	12	7	38	95	226	29	350	78	23	100	201	6	225	73	304	893
Total	67	30	14	111	385	974	121	1480	340	66	374	780	34	973	278	1285	3656
Grand Total	109	53	24	186	731	1781	223	2735	770	135	893	1798	53	2090	717	2860	7579
Apprch %	58.6	28.5	12.9		26.7	65.1	8.2		42.8	7.5	49.7		1.9	73.1	25.1		
Total %	1.4	0.7	0.3	2.5	9.6	23.5	2.9	36.1	10.2	1.8	11.8	23.7	0.7	27.6	9.5	37.7	

	Auto Mall Drive Southbound				Edinger Avenue Westbound				SR-55 Southbound Ramps Northbound				Edinger Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:15 AM																	
07:15 AM	11	8	1	20	70	182	29	281	**121**	15	**137**	**273**	6	260	**194**	**460**	1034
07:30 AM	**16**	**10**	**3**	**29**	91	230	25	346	109	**18**	135	262	3	294	106	403	1040
07:45 AM	8	5	2	15	**129**	**269**	34	**432**	91	12	130	233	3	**302**	58	363	**1043**
08:00 AM	15	7	2	24	109	259	**35**	403	100	18	96	214	**16**	248	50	314	955
Total Volume	50	30	8	88	399	940	123	1462	421	63	498	982	28	1104	408	1540	4072
% App. Total	56.8	34.1	9.1		27.3	64.3	8.4		42.9	6.4	50.7		1.8	71.7	26.5		
PHF	.781	.750	.667	.759	.773	.874	.879	.846	.870	.875	.909	.899	.438	.914	.526	.837	.976

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City of Santa Ana
N/S: Auto Mall Dr/SR-55 Southbound Ramps
E/W: Edinger Avenue
Weather: Clear

File Name : 95_SNA_55S_Edinger AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	08:00 AM				07:30 AM				07:00 AM				07:00 AM			
+0 mins.	15	7	2	24	91	230	25	346	109	**24**	117	250	**7**	261	81	349
+15 mins.	**23**	6	2	31	**129**	**269**	34	**432**	**121**	15	**137**	**273**	6	260	**194**	**460**
+30 mins.	10	5	3	18	109	259	**35**	403	109	18	135	262	3	294	106	403
+45 mins.	19	**12**	**7**	**38**	93	269	32	394	91	12	130	233	3	**302**	58	363
Total Volume	67	30	14	111	422	1027	126	1575	430	69	519	1018	19	1117	439	1575
% App. Total	60.4	27	12.6		26.8	65.2	8		42.2	6.8	51		1.2	70.9	27.9	
PHF	.728	.625	.500	.730	.818	.954	.900	.911	.888	.719	.947	.932	.679	.925	.566	.856

Counts Unlimited
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City of Santa Ana
N/S: Auto Mall Dr/SR-55 Southbound Ramps
E/W: Edinger Avenue
Weather: Clear

File Name : 95_SNA_55S_Edinger PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

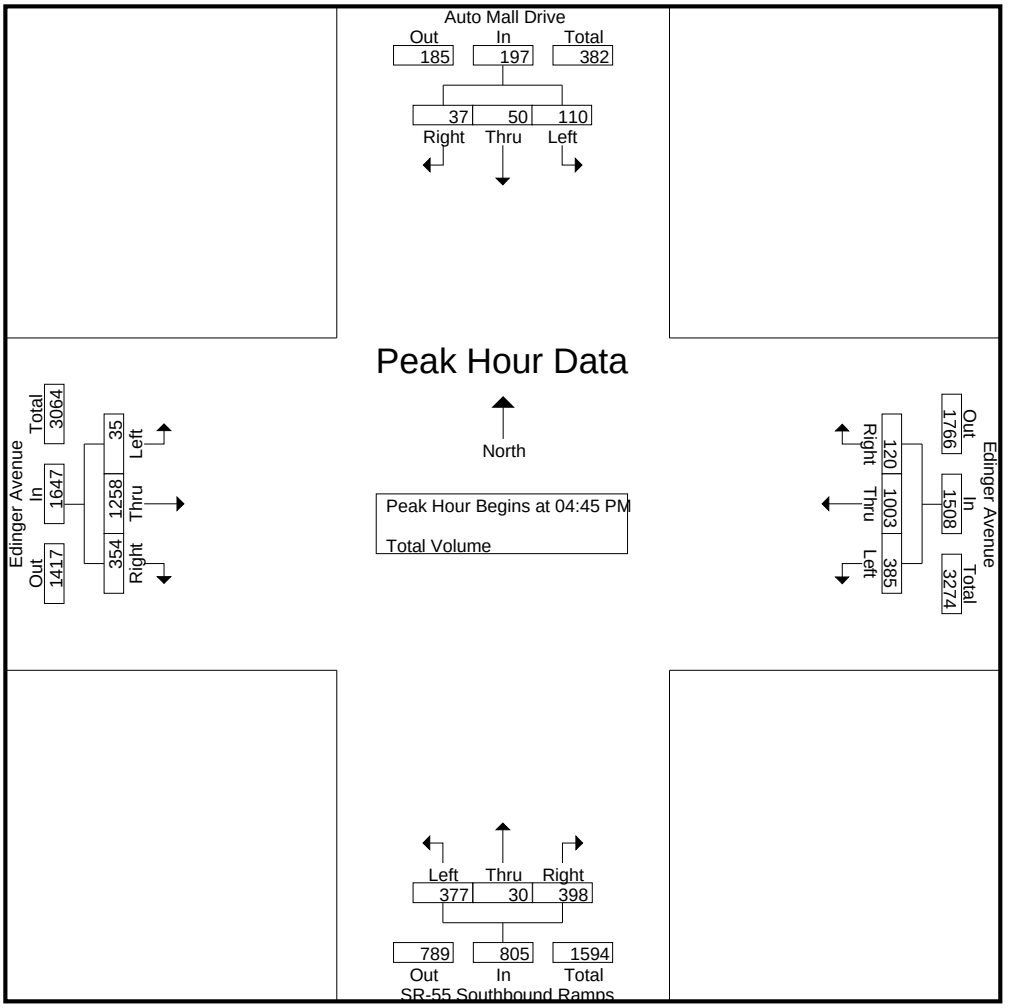
	Auto Mall Drive Southbound				Edinger Avenue Westbound				SR-55 Southbound Ramps Northbound				Edinger Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
04:00 PM	24	15	8	47	82	265	29	376	76	7	64	147	7	290	78	375	945
04:15 PM	25	7	7	39	61	248	23	332	79	8	59	146	12	226	62	300	817
04:30 PM	14	11	6	31	85	233	22	340	97	11	72	180	7	251	80	338	889
04:45 PM	26	12	8	46	97	210	27	334	86	5	82	173	6	239	73	318	871
Total	89	45	29	163	325	956	101	1382	338	31	277	646	32	1006	293	1331	3522
05:00 PM	39	19	8	66	96	218	27	341	89	5	85	179	13	322	103	438	1024
05:15 PM	21	9	11	41	78	252	25	355	95	12	97	204	12	296	73	381	981
05:30 PM	24	10	10	44	114	323	41	478	107	8	134	249	4	401	105	510	1281
05:45 PM	14	8	5	27	34	110	12	156	49	5	67	121	4	121	43	168	472
Total	98	46	34	178	322	903	105	1330	340	30	383	753	33	1140	324	1497	3758
Grand Total	187	91	63	341	647	1859	206	2712	678	61	660	1399	65	2146	617	2828	7280
Apprch %	54.8	26.7	18.5		23.9	68.5	7.6		48.5	4.4	47.2		2.3	75.9	21.8		
Total %	2.6	1.2	0.9	4.7	8.9	25.5	2.8	37.3	9.3	0.8	9.1	19.2	0.9	29.5	8.5	38.8	

	Auto Mall Drive Southbound				Edinger Avenue Westbound				SR-55 Southbound Ramps Northbound				Edinger Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	26	12	8	46	97	210	27	334	86	5	82	173	6	239	73	318	871
05:00 PM	**39**	**19**	8	**66**	96	218	27	341	89	5	85	179	**13**	322	103	438	1024
05:15 PM	21	9	**11**	41	78	252	25	355	95	**12**	97	204	12	296	73	381	981
05:30 PM	24	10	10	44	**114**	**323**	**41**	**478**	**107**	8	**134**	**249**	4	**401**	**105**	**510**	**1281**
Total Volume	110	50	37	197	385	1003	120	1508	377	30	398	805	35	1258	354	1647	4157
% App. Total	55.8	25.4	18.8		25.5	66.5	8		46.8	3.7	49.4		2.1	76.4	21.5		
PHF	.705	.658	.841	.746	.844	.776	.732	.789	.881	.625	.743	.808	.673	.784	.843	.807	.811

Counts Unlimited
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City of Santa Ana
N/S: Auto Mall Dr/SR-55 Southbound Ramps
E/W: Edinger Avenue
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File Name : 95_SNA_55S_Edinger PM
Site Code : 20220171
Start Date : 3/12/2020
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Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:45 PM				04:45 PM				04:45 PM				04:45 PM			
+0 mins.	26	12	8	46	97	210	27	334	86	5	82	173	6	239	73	318
+15 mins.	**39**	**19**	8	**66**	96	218	27	341	89	5	85	179	**13**	322	103	438
+30 mins.	21	9	**11**	41	78	252	25	355	95	**12**	97	204	12	296	73	381
+45 mins.	24	10	10	44	**114**	**323**	**41**	**478**	**107**	8	**134**	**249**	4	**401**	**105**	**510**
Total Volume	110	50	37	197	385	1003	120	1508	377	30	398	805	35	1258	354	1647
% App. Total	55.8	25.4	18.8		25.5	66.5	8		46.8	3.7	49.4		2.1	76.4	21.5	
PHF	.705	.658	.841	.746	.844	.776	.732	.789	.881	.625	.743	.808	.673	.784	.843	.807

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City of Tustin
N/S: Newport Avenue
E/W: SR-55 NB Ramps/Del Amo Avenue
Weather: Clear

File Name : 96_TUS_Newport_55N AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

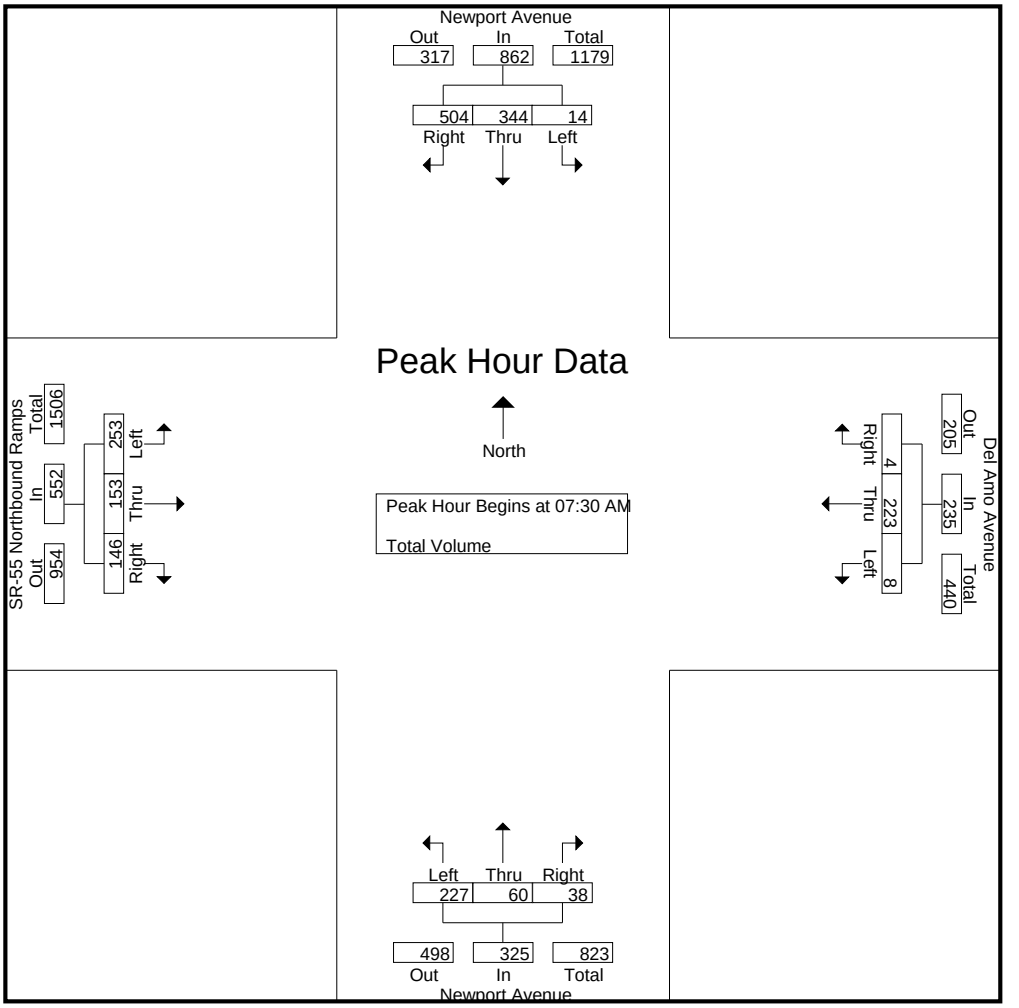
	Newport Avenue Southbound				Del Amo Avenue Westbound				Newport Avenue Northbound				SR-55 Northbound Ramps Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
07:00 AM	5	63	114	182	4	35	1	40	49	19	3	71	20	8	10	38	331
07:15 AM	3	87	136	226	4	43	1	48	54	11	6	71	53	22	20	95	440
07:30 AM	3	100	143	246	2	48	3	53	56	14	2	72	52	35	30	117	488
07:45 AM	2	104	116	222	4	68	1	73	63	14	2	79	66	46	48	160	534
Total	13	354	509	876	14	194	6	214	222	58	13	293	191	111	108	410	1793
08:00 AM	2	73	114	189	0	46	0	46	50	16	6	72	77	29	35	141	448
08:15 AM	7	67	131	205	2	61	0	63	58	16	28	102	58	43	33	134	504
08:30 AM	8	49	103	160	3	34	1	38	57	20	13	90	48	47	33	128	416
08:45 AM	6	44	120	170	5	42	2	49	38	15	15	68	69	55	34	158	445
Total	23	233	468	724	10	183	3	196	203	67	62	332	252	174	135	561	1813
Grand Total	36	587	977	1600	24	377	9	410	425	125	75	625	443	285	243	971	3606
Apprch %	2.2	36.7	61.1		5.9	92	2.2		68	20	12		45.6	29.4	25		
Total %	1	16.3	27.1	44.4	0.7	10.5	0.2	11.4	11.8	3.5	2.1	17.3	12.3	7.9	6.7	26.9	

	Newport Avenue Southbound				Del Amo Avenue Westbound				Newport Avenue Northbound				SR-55 Northbound Ramps Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	3	100	**143**	**246**	2	48	**3**	53	56	14	2	72	52	35	30	117	488
07:45 AM	2	**104**	116	222	**4**	**68**	1	**73**	**63**	14	2	79	66	**46**	**48**	**160**	**534**
08:00 AM	2	73	114	189	0	46	0	46	50	**16**	6	72	**77**	29	35	141	448
08:15 AM	**7**	67	131	205	2	61	0	63	58	16	**28**	**102**	58	43	33	134	504
Total Volume	14	344	504	862	8	223	4	235	227	60	38	325	253	153	146	552	1974
% App. Total	1.6	39.9	58.5		3.4	94.9	1.7		69.8	18.5	11.7		45.8	27.7	26.4		
PHF	.500	.827	.881	.876	.500	.820	.333	.805	.901	.938	.339	.797	.821	.832	.760	.863	.924

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File Name : 96_TUS_Newport_55N AM
Site Code : 20220171
Start Date : 3/12/2020
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Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:15 AM				07:30 AM				07:45 AM				07:45 AM			
+0 mins.	**3**	87	136	226	2	48	**3**	53	**63**	14	2	79	66	46	**48**	**160**
+15 mins.	3	100	**143**	**246**	**4**	**68**	1	**73**	50	16	6	72	**77**	29	35	141
+30 mins.	2	**104**	116	222	0	46	0	46	58	16	**28**	**102**	58	43	33	134
+45 mins.	2	73	114	189	2	61	0	63	57	**20**	13	90	48	**47**	33	128
Total Volume	10	364	509	883	8	223	4	235	228	66	49	343	249	165	149	563
% App. Total	1.1	41.2	57.6		3.4	94.9	1.7		66.5	19.2	14.3		44.2	29.3	26.5	
PHF	.833	.875	.890	.897	.500	.820	.333	.805	.905	.825	.438	.841	.808	.878	.776	.880

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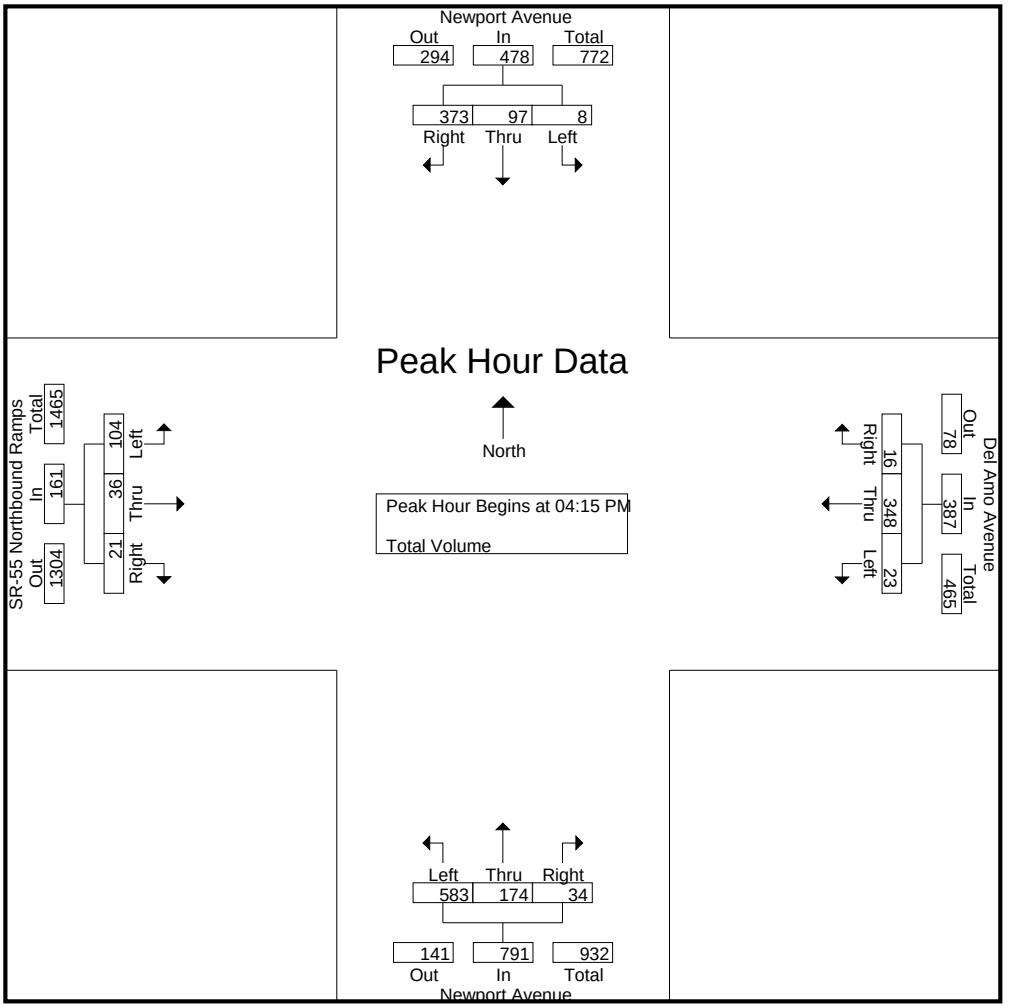
	Newport Avenue Southbound				Del Amo Avenue Westbound				Newport Avenue Northbound				SR-55 Northbound Ramps Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
04:00 PM	1	25	86	112	10	121	2	133	130	58	12	200	28	8	7	43	488
04:15 PM	4	26	94	124	6	88	2	96	164	48	11	223	19	10	6	35	478
04:30 PM	0	27	92	119	4	88	3	95	151	51	6	208	26	3	7	36	458
04:45 PM	1	23	93	117	3	74	5	82	117	30	6	153	26	10	3	39	391
Total	6	101	365	472	23	371	12	406	562	187	35	784	99	31	23	153	1815
05:00 PM	3	21	94	118	10	98	6	114	151	45	11	207	33	13	5	51	490
05:15 PM	3	18	82	103	4	87	4	95	149	46	11	206	24	15	3	42	446
05:30 PM	0	14	71	85	4	90	2	96	147	45	8	200	15	17	2	34	415
05:45 PM	3	23	74	100	2	59	3	64	129	32	2	163	26	7	6	39	366
Total	9	76	321	406	20	334	15	369	576	168	32	776	98	52	16	166	1717
Grand Total	15	177	686	878	43	705	27	775	1138	355	67	1560	197	83	39	319	3532
Apprch %	1.7	20.2	78.1		5.5	91	3.5		72.9	22.8	4.3		61.8	26	12.2		
Total %	0.4	5	19.4	24.9	1.2	20	0.8	21.9	32.2	10.1	1.9	44.2	5.6	2.3	1.1	9	

	Newport Avenue Southbound				Del Amo Avenue Westbound				Newport Avenue Northbound				SR-55 Northbound Ramps Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:15 PM																	
04:15 PM	**4**	26	**94**	**124**	6	88	2	96	**164**	48	**11**	**223**	19	10	6	35	478
04:30 PM	0	**27**	92	119	4	88	3	95	151	**51**	6	208	26	3	**7**	36	458
04:45 PM	1	23	93	117	3	74	5	82	117	30	6	153	26	10	3	39	391
05:00 PM	3	21	94	118	**10**	**98**	**6**	**114**	151	45	11	207	**33**	**13**	5	**51**	**490**
Total Volume	8	97	373	478	23	348	16	387	583	174	34	791	104	36	21	161	1817
% App. Total	1.7	20.3	78		5.9	89.9	4.1		73.7	22	4.3		64.6	22.4	13		
PHF	.500	.898	.992	.964	.575	.888	.667	.849	.889	.853	.773	.887	.788	.692	.750	.789	.927

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Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:15 PM				04:00 PM				04:15 PM				04:30 PM			
+0 mins.	**4**	26	**94**	**124**	**10**	**121**	2	**133**	**164**	48	**11**	**223**	26	3	**7**	36
+15 mins.	0	**27**	92	119	6	88	2	96	151	**51**	6	208	26	10	3	39
+30 mins.	1	23	93	117	4	88	3	95	117	30	6	153	**33**	13	5	**51**
+45 mins.	3	21	94	118	3	74	**5**	82	151	45	11	207	24	**15**	3	42
Total Volume	8	97	373	478	23	371	12	406	583	174	34	791	109	41	18	168
% App. Total	1.7	20.3	78		5.7	91.4	3		73.7	22	4.3		64.9	24.4	10.7	
PHF	.500	.898	.992	.964	.575	.767	.600	.763	.889	.853	.773	.887	.826	.683	.643	.824

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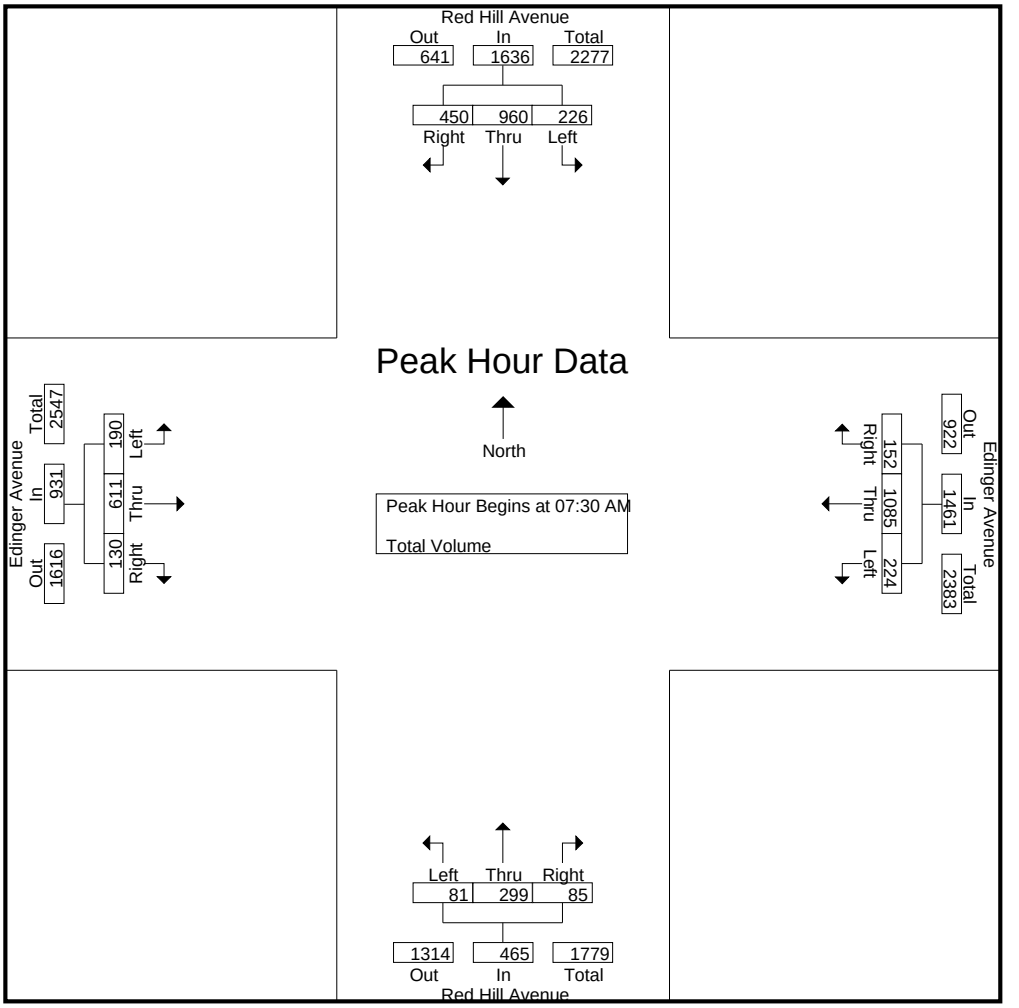
	Red Hill Avenue Southbound				Edinger Avenue Westbound				Red Hill Avenue Northbound				Edinger Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
07:00 AM	40	183	74	297	15	123	19	157	32	53	15	100	44	129	38	211	765
07:15 AM	53	201	87	341	37	182	28	247	40	87	18	145	52	149	28	229	962
07:30 AM	57	243	109	409	47	233	42	322	14	80	9	103	64	174	33	271	1105
07:45 AM	68	225	116	409	60	287	35	382	20	89	37	146	49	183	30	262	1199
Total	218	852	386	1456	159	825	124	1108	106	309	79	494	209	635	129	973	4031
08:00 AM	54	264	126	444	56	292	34	382	22	69	22	113	35	117	31	183	1122
08:15 AM	47	228	99	374	61	273	41	375	25	61	17	103	42	137	36	215	1067
08:30 AM	52	200	97	349	68	258	42	368	18	68	7	93	47	133	29	209	1019
08:45 AM	61	220	107	388	45	218	34	297	20	48	15	83	32	146	34	212	980
Total	214	912	429	1555	230	1041	151	1422	85	246	61	392	156	533	130	819	4188
Grand Total	432	1764	815	3011	389	1866	275	2530	191	555	140	886	365	1168	259	1792	8219
Apprch %	14.3	58.6	27.1		15.4	73.8	10.9		21.6	62.6	15.8		20.4	65.2	14.5		
Total %	5.3	21.5	9.9	36.6	4.7	22.7	3.3	30.8	2.3	6.8	1.7	10.8	4.4	14.2	3.2	21.8	

	Red Hill Avenue Southbound				Edinger Avenue Westbound				Red Hill Avenue Northbound				Edinger Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	57	243	109	409	47	233	**42**	322	14	80	9	103	**64**	174	33	**271**	1105
07:45 AM	**68**	225	116	409	60	287	35	**382**	20	**89**	**37**	**146**	49	**183**	30	262	**1199**
08:00 AM	54	**264**	**126**	**444**	56	**292**	34	382	22	69	22	113	35	117	31	183	1122
08:15 AM	47	228	99	374	**61**	273	41	375	**25**	61	17	103	42	137	**36**	215	1067
Total Volume	226	960	450	1636	224	1085	152	1461	81	299	85	465	190	611	130	931	4493
% App. Total	13.8	58.7	27.5		15.3	74.3	10.4		17.4	64.3	18.3		20.4	65.6	14		
PHF	.831	.909	.893	.921	.918	.929	.905	.956	.810	.840	.574	.796	.742	.835	.903	.859	.937

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Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:30 AM				07:45 AM				07:15 AM				07:00 AM			
+0 mins.	57	243	109	409	60	287	35	**382**	**40**	87	18	145	44	129	**38**	211
+15 mins.	**68**	225	116	409	56	**292**	34	382	14	80	9	103	52	149	28	229
+30 mins.	54	**264**	**126**	**444**	61	273	41	375	20	**89**	**37**	**146**	**64**	174	33	**271**
+45 mins.	47	228	99	374	**68**	258	**42**	368	22	69	22	113	49	**183**	30	262
Total Volume	226	960	450	1636	245	1110	152	1507	96	325	86	507	209	635	129	973
% App. Total	13.8	58.7	27.5		16.3	73.7	10.1		18.9	64.1	17		21.5	65.3	13.3	
PHF	.831	.909	.893	.921	.901	.950	.905	.986	.600	.913	.581	.868	.816	.867	.849	.898

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Groups Printed- Total Volume

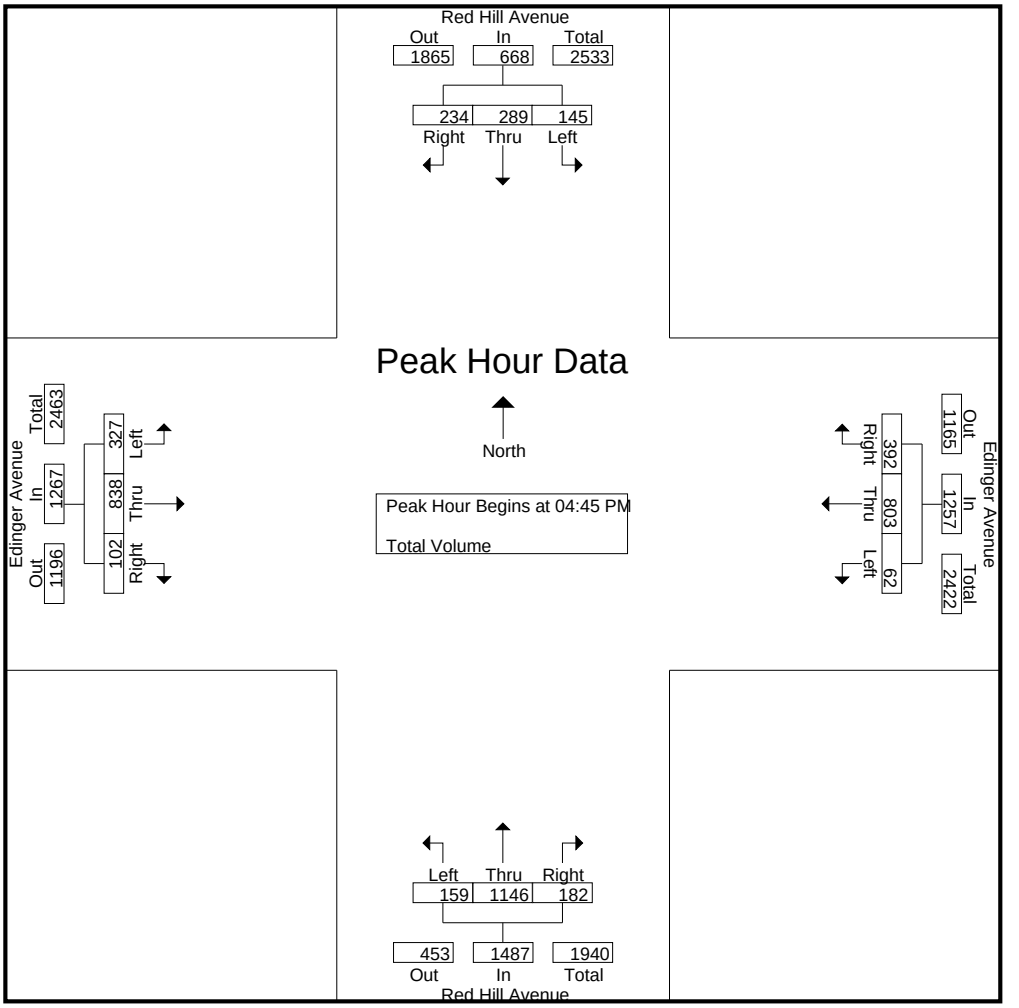
	Red Hill Avenue Southbound				Edinger Avenue Westbound				Red Hill Avenue Northbound				Edinger Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
04:00 PM	36	69	60	165	13	210	108	331	31	237	42	310	67	153	30	250	1056
04:15 PM	28	73	55	156	17	231	105	353	34	260	41	335	66	145	26	237	1081
04:30 PM	36	72	50	158	14	219	95	328	37	282	34	353	70	145	25	240	1079
04:45 PM	39	76	58	173	15	172	85	272	48	251	46	345	80	147	26	253	1043
Total	139	290	223	652	59	832	393	1284	150	1030	163	1343	283	590	107	980	4259
05:00 PM	30	72	58	160	15	225	93	333	34	281	47	362	80	210	27	317	1172
05:15 PM	40	74	59	173	15	195	119	329	29	276	49	354	95	218	22	335	1191
05:30 PM	36	67	59	162	17	211	95	323	48	338	40	426	72	263	27	362	1273
05:45 PM	40	74	42	156	17	161	93	271	33	249	40	322	58	153	27	238	987
Total	146	287	218	651	64	792	400	1256	144	1144	176	1464	305	844	103	1252	4623
Grand Total	285	577	441	1303	123	1624	793	2540	294	2174	339	2807	588	1434	210	2232	8882
Apprch %	21.9	44.3	33.8		4.8	63.9	31.2		10.5	77.4	12.1		26.3	64.2	9.4		
Total %	3.2	6.5	5	14.7	1.4	18.3	8.9	28.6	3.3	24.5	3.8	31.6	6.6	16.1	2.4	25.1	

	Red Hill Avenue Southbound				Edinger Avenue Westbound				Red Hill Avenue Northbound				Edinger Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	39	**76**	58	**173**	15	172	85	272	**48**	251	46	345	80	147	26	253	1043
05:00 PM	30	72	58	160	15	**225**	93	**333**	34	281	47	362	80	210	**27**	317	1172
05:15 PM	**40**	74	**59**	173	15	195	**119**	329	29	276	**49**	354	**95**	218	22	335	1191
05:30 PM	36	67	59	162	**17**	211	95	323	48	**338**	40	**426**	72	**263**	27	**362**	**1273**
Total Volume	145	289	234	668	62	803	392	1257	159	1146	182	1487	327	838	102	1267	4679
% App. Total	21.7	43.3	35		4.9	63.9	31.2		10.7	77.1	12.2		25.8	66.1	8.1		
PHF	.906	.951	.992	.965	.912	.892	.824	.944	.828	.848	.929	.873	.861	.797	.944	.875	.919

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City of Tustin
N/S: Red Hill Avenue
E/W: Edinger Avenue
Weather: Clear

File Name : 97_TUS_Red Hill_Edinger PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:45 PM				04:15 PM				04:45 PM				04:45 PM			
+0 mins.	39	**76**	58	**173**	**17**	**231**	**105**	**353**	**48**	251	46	345	80	147	26	253
+15 mins.	30	72	58	160	14	219	95	328	34	281	47	362	80	210	**27**	317
+30 mins.	**40**	74	**59**	173	15	172	85	272	29	276	**49**	354	**95**	218	22	335
+45 mins.	36	67	59	162	15	225	93	333	48	**338**	40	**426**	72	**263**	27	**362**
Total Volume	145	289	234	668	61	847	378	1286	159	1146	182	1487	327	838	102	1267
% App. Total	21.7	43.3	35		4.7	65.9	29.4		10.7	77.1	12.2		25.8	66.1	8.1	
PHF	.906	.951	.992	.965	.897	.917	.900	.911	.828	.848	.929	.873	.861	.797	.944	.875

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City of Santa Ana
N/S: Red Hill Avenue
E/W: Warner Avenue
Weather: Clear

File Name : 98_TUS_Red Hill_Warner_AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

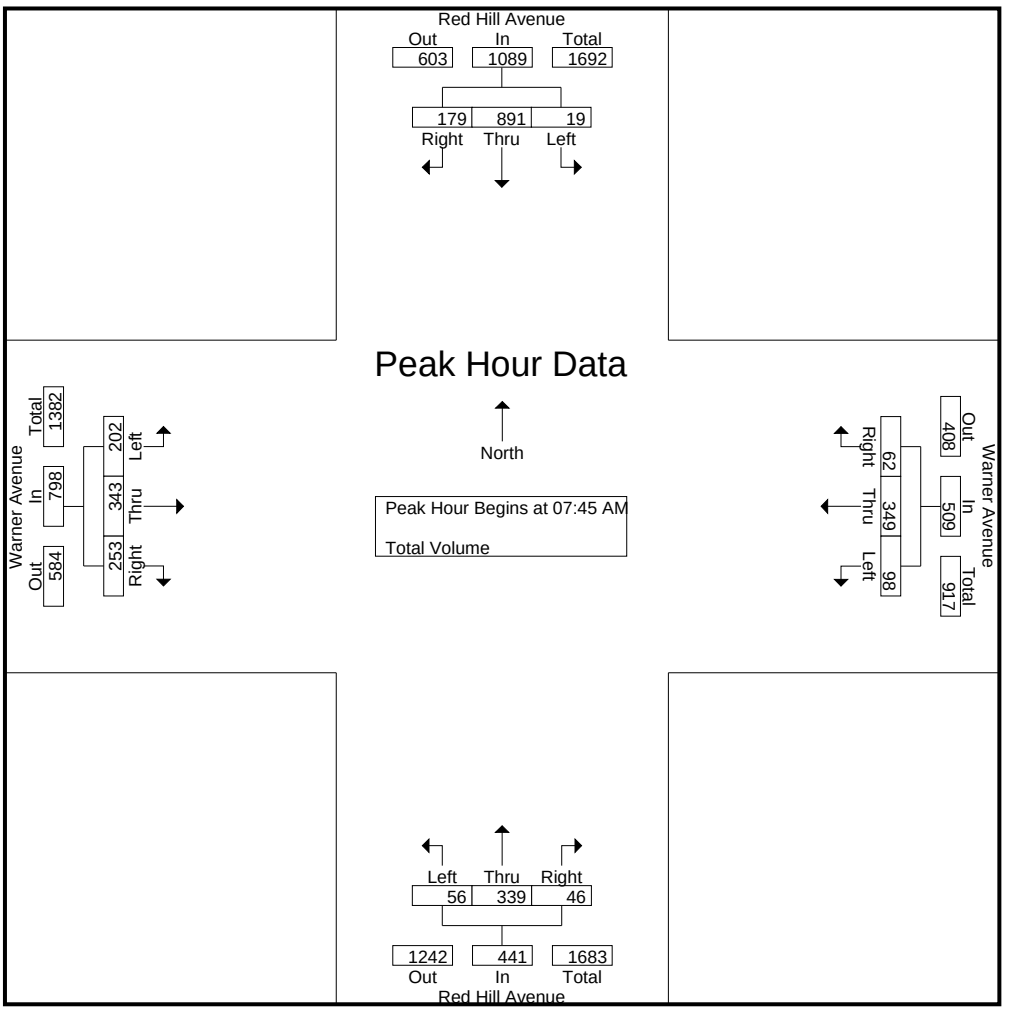
	Red Hill Avenue Southbound				Warner Avenue Westbound				Red Hill Avenue Northbound				Warner Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
07:00 AM	3	183	31	217	11	41	6	58	24	181	8	213	41	44	38	123	611
07:15 AM	1	163	50	214	17	59	14	90	19	154	11	184	41	60	62	163	651
07:30 AM	3	231	41	275	22	75	12	109	16	94	10	120	49	70	69	188	692
07:45 AM	5	214	42	261	17	75	14	106	15	92	8	115	55	99	76	230	712
Total	12	791	164	967	67	250	46	363	74	521	37	632	186	273	245	704	2666
08:00 AM	4	255	43	302	25	92	19	136	17	89	10	116	47	72	61	180	734
08:15 AM	3	228	45	276	23	90	16	129	9	74	11	94	56	86	54	196	695
08:30 AM	7	194	49	250	33	92	13	138	15	84	17	116	44	86	62	192	696
08:45 AM	13	191	54	258	22	82	16	120	9	96	16	121	32	71	53	156	655
Total	27	868	191	1086	103	356	64	523	50	343	54	447	179	315	230	724	2780
Grand Total	39	1659	355	2053	170	606	110	886	124	864	91	1079	365	588	475	1428	5446
Apprch %	1.9	80.8	17.3		19.2	68.4	12.4		11.5	80.1	8.4		25.6	41.2	33.3		
Total %	0.7	30.5	6.5	37.7	3.1	11.1	2	16.3	2.3	15.9	1.7	19.8	6.7	10.8	8.7	26.2	

	Red Hill Avenue Southbound				Warner Avenue Westbound				Red Hill Avenue Northbound				Warner Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:45 AM																	
07:45 AM	5	214	42	261	17	75	14	106	15	**92**	8	115	55	**99**	**76**	**230**	712
08:00 AM	4	**255**	43	**302**	25	**92**	**19**	136	**17**	89	10	**116**	47	72	61	180	**734**
08:15 AM	3	228	45	276	23	90	16	129	9	74	11	94	**56**	86	54	196	695
08:30 AM	**7**	194	**49**	250	**33**	92	13	**138**	15	84	**17**	116	44	86	62	192	696
Total Volume	19	891	179	1089	98	349	62	509	56	339	46	441	202	343	253	798	2837
% App. Total	1.7	81.8	16.4		19.3	68.6	12.2		12.7	76.9	10.4		25.3	43	31.7		
PHF	.679	.874	.913	.901	.742	.948	.816	.922	.824	.921	.676	.950	.902	.866	.832	.867	.966

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City of Santa Ana
N/S: Red Hill Avenue
E/W: Warner Avenue
Weather: Clear

File Name : 98_TUS_Red Hill_Warner_AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:30 AM				08:00 AM				07:00 AM				07:45 AM			
+0 mins.	3	231	41	275	25	**92**	**19**	136	**24**	**181**	8	**213**	55	**99**	**76**	**230**
+15 mins.	**5**	214	42	261	23	90	16	129	19	154	**11**	184	47	72	61	180
+30 mins.	4	**255**	43	**302**	**33**	92	13	**138**	16	94	10	120	**56**	86	54	196
+45 mins.	3	228	**45**	276	22	82	16	120	15	92	8	115	44	86	62	192
Total Volume	15	928	171	1114	103	356	64	523	74	521	37	632	202	343	253	798
% App. Total	1.3	83.3	15.4		19.7	68.1	12.2		11.7	82.4	5.9		25.3	43	31.7	
PHF	.750	.910	.950	.922	.780	.967	.842	.947	.771	.720	.841	.742	.902	.866	.832	.867

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City of Santa Ana
N/S: Red Hill Avenue
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Weather: Clear

File Name : 98_TUS_Red Hill_Warner_PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

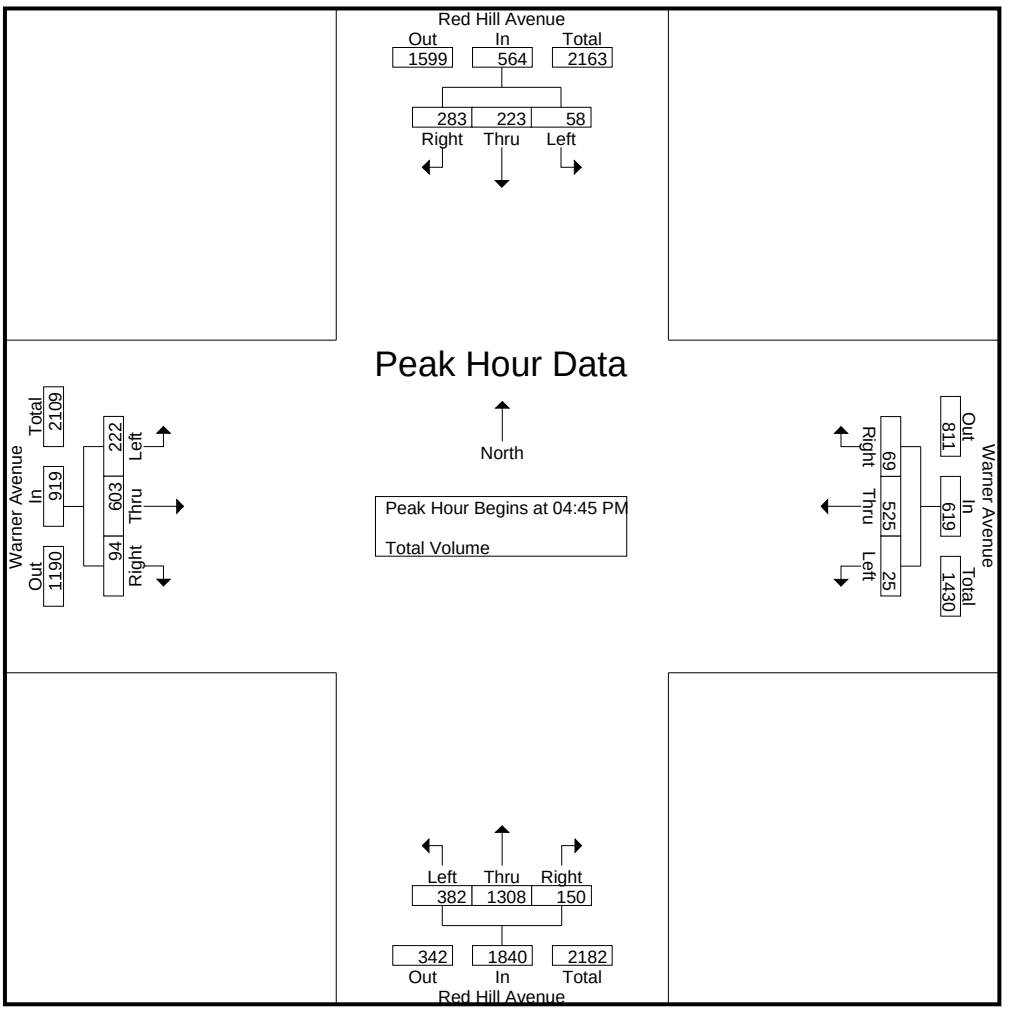
	Red Hill Avenue Southbound				Warner Avenue Westbound				Red Hill Avenue Northbound				Warner Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
04:00 PM	12	48	60	120	4	77	11	92	69	254	32	355	41	108	36	185	752
04:15 PM	14	48	80	142	8	123	13	144	76	273	31	380	52	94	35	181	847
04:30 PM	11	35	66	112	14	124	14	152	82	264	42	388	49	125	28	202	854
04:45 PM	15	39	74	128	10	133	13	156	93	325	24	442	56	128	18	202	928
Total	52	170	280	502	36	457	51	544	320	1116	129	1565	198	455	117	770	3381
05:00 PM	14	51	73	138	3	120	20	143	85	373	39	497	61	156	24	241	1019
05:15 PM	11	71	60	142	8	169	20	197	108	254	41	403	59	155	25	239	981
05:30 PM	18	62	76	156	4	103	16	123	96	356	46	498	46	164	27	237	1014
05:45 PM	11	55	73	139	6	121	20	147	87	257	53	397	55	151	21	227	910
Total	54	239	282	575	21	513	76	610	376	1240	179	1795	221	626	97	944	3924
Grand Total	106	409	562	1077	57	970	127	1154	696	2356	308	3360	419	1081	214	1714	7305
Apprch %	9.8	38	52.2		4.9	84.1	11		20.7	70.1	9.2		24.4	63.1	12.5		
Total %	1.5	5.6	7.7	14.7	0.8	13.3	1.7	15.8	9.5	32.3	4.2	46	5.7	14.8	2.9	23.5	

	Red Hill Avenue Southbound				Warner Avenue Westbound				Red Hill Avenue Northbound				Warner Avenue Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	15	39	74	128	**10**	133	13	156	93	325	24	442	56	128	18	202	928
05:00 PM	14	51	73	138	3	120	**20**	143	85	**373**	39	497	**61**	156	24	**241**	**1019**
05:15 PM	11	**71**	60	142	8	**169**	20	**197**	**108**	254	41	403	59	155	25	239	981
05:30 PM	**18**	62	**76**	**156**	4	103	16	123	96	356	**46**	**498**	46	**164**	**27**	237	1014
Total Volume	58	223	283	564	25	525	69	619	382	1308	150	1840	222	603	94	919	3942
% App. Total	10.3	39.5	50.2		4	84.8	11.1		20.8	71.1	8.2		24.2	65.6	10.2		
PHF	.806	.785	.931	.904	.625	.777	.863	.786	.884	.877	.815	.924	.910	.919	.870	.953	.967

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City of Santa Ana
N/S: Red Hill Avenue
E/W: Warner Avenue
Weather: Clear

File Name : 98_TUS_Red Hill_Warner_PM
Site Code : 20220171
Start Date : 3/12/2020



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	05:00 PM				04:30 PM				04:45 PM				05:00 PM			
+0 mins.	14	51	73	138	**14**	124	14	152	93	325	24	442	**61**	156	24	**241**
+15 mins.	11	**71**	60	142	10	133	13	156	85	**373**	39	497	59	155	25	239
+30 mins.	**18**	62	**76**	**156**	3	120	**20**	143	**108**	254	41	403	46	**164**	**27**	237
+45 mins.	11	55	73	139	8	**169**	20	**197**	96	356	**46**	**498**	55	151	21	227
Total Volume	54	239	282	575	35	546	67	648	382	1308	150	1840	221	626	97	944
% App. Total	9.4	41.6	49		5.4	84.3	10.3		20.8	71.1	8.2		23.4	66.3	10.3	
PHF	.750	.842	.928	.921	.625	.808	.838	.822	.884	.877	.815	.924	.906	.954	.898	.979

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City of Santa Ana
N/S: Red Hill Avenue
E/W: Dyer Road/Barranca Parkway
Weather: Clear

File Name : 99_SNA_Red Hill_Barranca AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

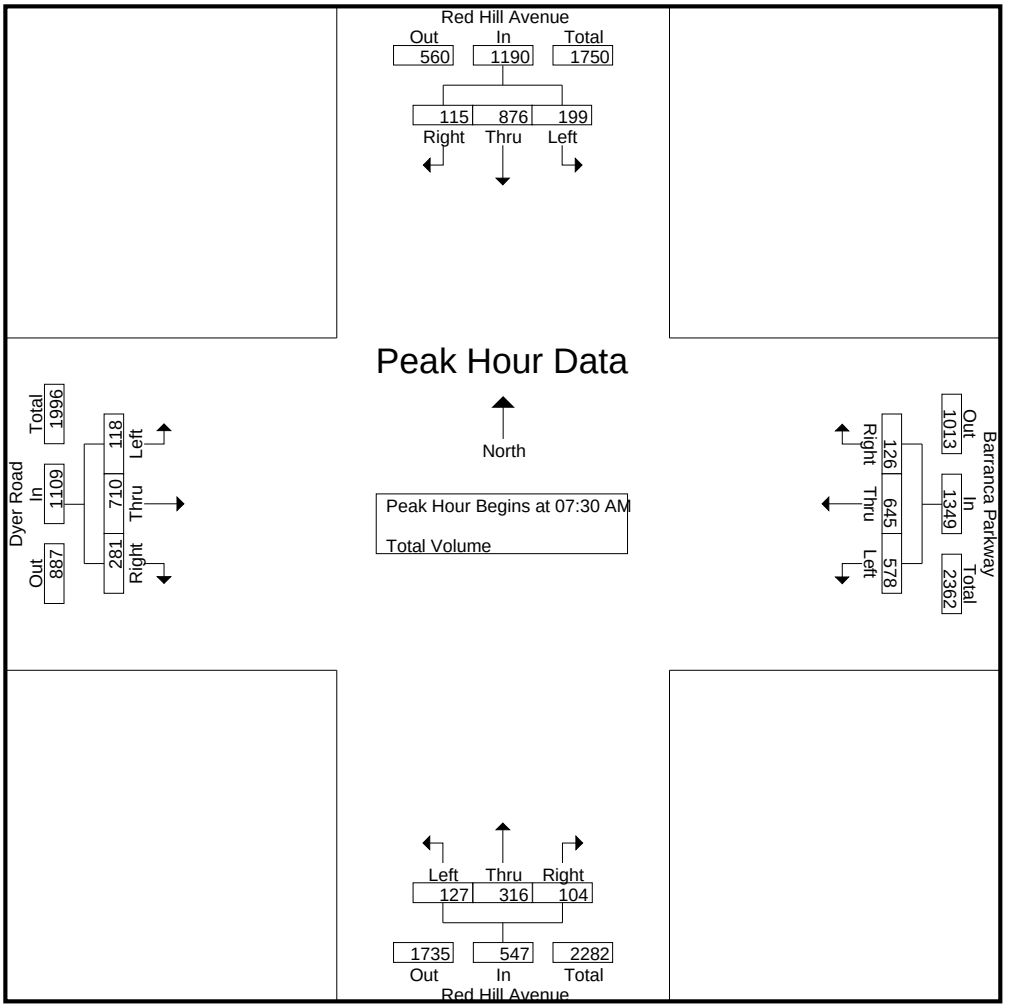
	Red Hill Avenue Southbound				Barranca Parkway Westbound				Red Hill Avenue Northbound				Dyer Road Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
07:00 AM	41	134	25	200	51	114	27	192	119	178	24	321	41	103	45	189	902
07:15 AM	50	127	22	199	90	136	21	247	100	153	24	277	37	154	90	281	1004
07:30 AM	37	206	30	273	105	141	25	271	48	85	27	160	23	182	52	257	961
07:45 AM	64	175	30	269	187	174	38	399	24	74	27	125	32	190	109	331	1124
Total	192	642	107	941	433	565	111	1109	291	490	102	883	133	629	296	1058	3991
08:00 AM	45	266	24	335	167	163	32	362	26	76	26	128	31	141	66	238	1063
08:15 AM	53	229	31	313	119	167	31	317	29	81	24	134	32	197	54	283	1047
08:30 AM	43	200	18	261	149	149	42	340	22	63	16	101	26	172	49	247	949
08:45 AM	46	187	27	260	148	146	37	331	25	77	16	118	25	149	90	264	973
Total	187	882	100	1169	583	625	142	1350	102	297	82	481	114	659	259	1032	4032
Grand Total	379	1524	207	2110	1016	1190	253	2459	393	787	184	1364	247	1288	555	2090	8023
Apprch %	18	72.2	9.8		41.3	48.4	10.3		28.8	57.7	13.5		11.8	61.6	26.6		
Total %	4.7	19	2.6	26.3	12.7	14.8	3.2	30.6	4.9	9.8	2.3	17	3.1	16.1	6.9	26.1	

	Red Hill Avenue Southbound				Barranca Parkway Westbound				Red Hill Avenue Northbound				Dyer Road Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	37	206	30	273	105	141	25	271	**48**	**85**	**27**	**160**	23	182	52	257	961
07:45 AM	**64**	175	30	269	**187**	**174**	**38**	**399**	24	74	27	125	**32**	190	**109**	**331**	**1124**
08:00 AM	45	**266**	24	**335**	167	163	32	362	26	76	26	128	31	141	66	238	1063
08:15 AM	53	229	**31**	313	119	167	31	317	29	81	24	134	32	**197**	54	283	1047
Total Volume	199	876	115	1190	578	645	126	1349	127	316	104	547	118	710	281	1109	4195
% App. Total	16.7	73.6	9.7		42.8	47.8	9.3		23.2	57.8	19		10.6	64	25.3		
PHF	.777	.823	.927	.888	.773	.927	.829	.845	.661	.929	.963	.855	.922	.901	.644	.838	.933

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City of Santa Ana
N/S: Red Hill Avenue
E/W: Dyer Road/Barranca Parkway
Weather: Clear

File Name : 99_SNA_Red Hill_Barranca AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:30 AM				07:45 AM				07:00 AM				07:30 AM			
+0 mins.	37	206	30	273	**187**	**174**	38	**399**	**119**	**178**	24	**321**	23	182	52	257
+15 mins.	**64**	175	30	269	167	163	32	362	100	153	24	277	**32**	190	**109**	**331**
+30 mins.	45	**266**	24	**335**	119	167	31	317	48	85	**27**	160	31	141	66	238
+45 mins.	53	229	**31**	313	149	149	**42**	340	24	74	27	125	32	**197**	54	283
Total Volume	199	876	115	1190	622	653	143	1418	291	490	102	883	118	710	281	1109
% App. Total	16.7	73.6	9.7		43.9	46.1	10.1		33	55.5	11.6		10.6	64	25.3	
PHF	.777	.823	.927	.888	.832	.938	.851	.888	.611	.688	.944	.688	.922	.901	.644	.838

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City of Santa Ana
N/S: Red Hill Avenue
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File Name : 99_SNA_Red Hill_Barranca PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

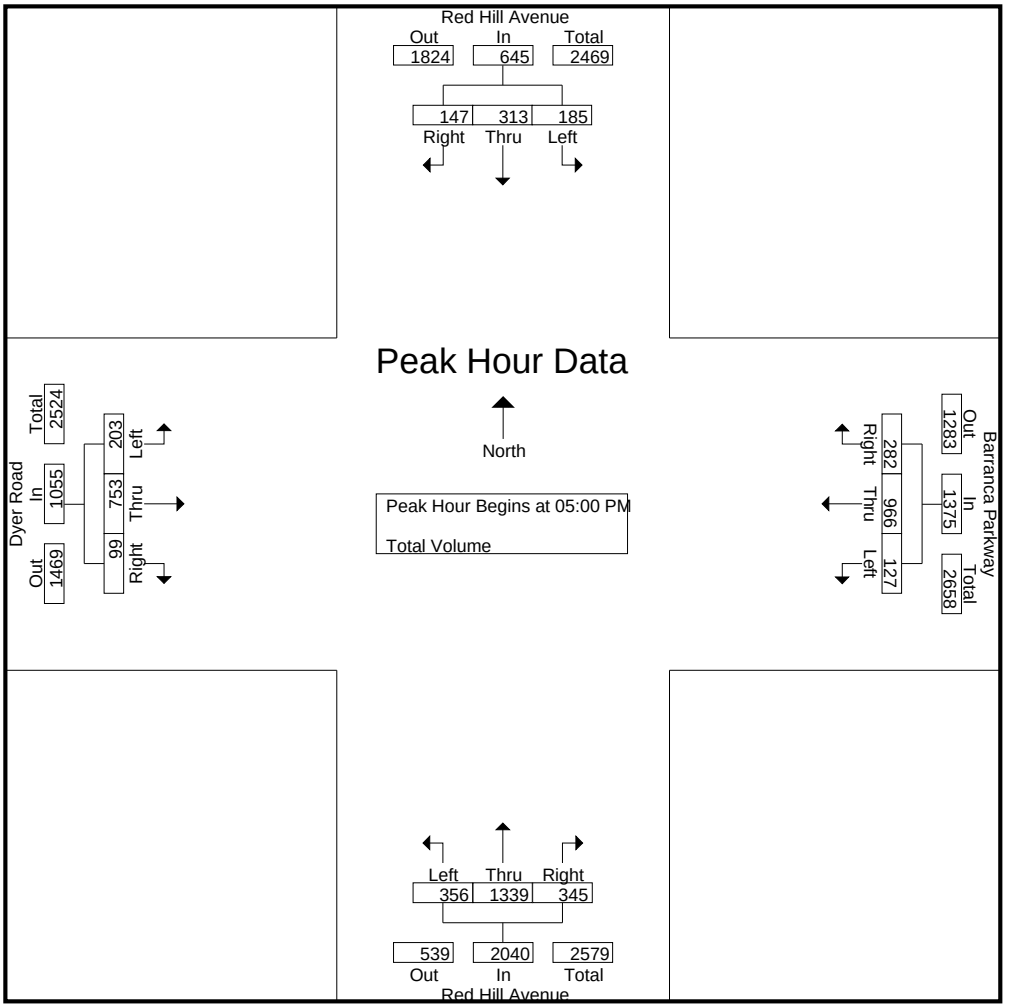
	Red Hill Avenue Southbound				Barranca Parkway Westbound				Red Hill Avenue Northbound				Dyer Road Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
04:00 PM	33	69	32	134	36	206	71	313	59	258	84	401	35	124	27	186	1034
04:15 PM	34	76	32	142	33	169	70	272	69	292	64	425	37	128	23	188	1027
04:30 PM	27	64	26	117	26	266	80	372	55	266	82	403	51	158	27	236	1128
04:45 PM	49	86	27	162	39	164	64	267	65	350	79	494	38	148	32	218	1141
Total	143	295	117	555	134	805	285	1224	248	1166	309	1723	161	558	109	828	4330
05:00 PM	42	93	45	180	19	273	87	379	102	393	81	576	38	182	20	240	1375
05:15 PM	53	68	38	159	35	212	65	312	91	323	99	513	49	191	18	258	1242
05:30 PM	39	83	32	154	39	216	68	323	78	354	91	523	62	187	32	281	1281
05:45 PM	51	69	32	152	34	265	62	361	85	269	74	428	54	193	29	276	1217
Total	185	313	147	645	127	966	282	1375	356	1339	345	2040	203	753	99	1055	5115
Grand Total	328	608	264	1200	261	1771	567	2599	604	2505	654	3763	364	1311	208	1883	9445
Apprch %	27.3	50.7	22		10	68.1	21.8		16.1	66.6	17.4		19.3	69.6	11		
Total %	3.5	6.4	2.8	12.7	2.8	18.8	6	27.5	6.4	26.5	6.9	39.8	3.9	13.9	2.2	19.9	

	Red Hill Avenue Southbound				Barranca Parkway Westbound				Red Hill Avenue Northbound				Dyer Road Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 05:00 PM																	
05:00 PM	42	**93**	**45**	**180**	19	**273**	**87**	**379**	**102**	**393**	81	**576**	38	182	20	240	**1375**
05:15 PM	**53**	68	38	159	35	212	65	312	91	323	**99**	513	49	191	18	258	1242
05:30 PM	39	83	32	154	**39**	216	68	323	78	354	91	523	**62**	187	**32**	**281**	1281
05:45 PM	51	69	32	152	34	265	62	361	85	269	74	428	54	**193**	29	276	1217
Total Volume	185	313	147	645	127	966	282	1375	356	1339	345	2040	203	753	99	1055	5115
% App. Total	28.7	48.5	22.8		9.2	70.3	20.5		17.5	65.6	16.9		19.2	71.4	9.4		
PHF	.873	.841	.817	.896	.814	.885	.810	.907	.873	.852	.871	.885	.819	.975	.773	.939	.930

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City of Santa Ana
N/S: Red Hill Avenue
E/W: Dyer Road/Barranca Parkway
Weather: Clear

File Name : 99_SNA_Red Hill_Barranca PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:45 PM				05:00 PM				04:45 PM				05:00 PM			
+0 mins.	49	86	27	162	19	**273**	**87**	**379**	65	350	79	494	38	182	20	240
+15 mins.	42	**93**	**45**	**180**	35	212	65	312	**102**	**393**	81	**576**	49	191	18	258
+30 mins.	**53**	68	38	159	**39**	216	68	323	91	323	**99**	513	**62**	187	**32**	**281**
+45 mins.	39	83	32	154	34	265	62	361	78	354	91	523	54	**193**	29	276
Total Volume	183	330	142	655	127	966	282	1375	336	1420	350	2106	203	753	99	1055
% App. Total	27.9	50.4	21.7		9.2	70.3	20.5		16	67.4	16.6		19.2	71.4	9.4	
PHF	.863	.887	.789	.910	.814	.885	.810	.907	.824	.903	.884	.914	.819	.975	.773	.939

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City of Irvine
N/S: Red Hill Avenue
E/W: Alton Parkway
Weather: Clear

File Name : 100_IRV_Red Hill_Alton AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

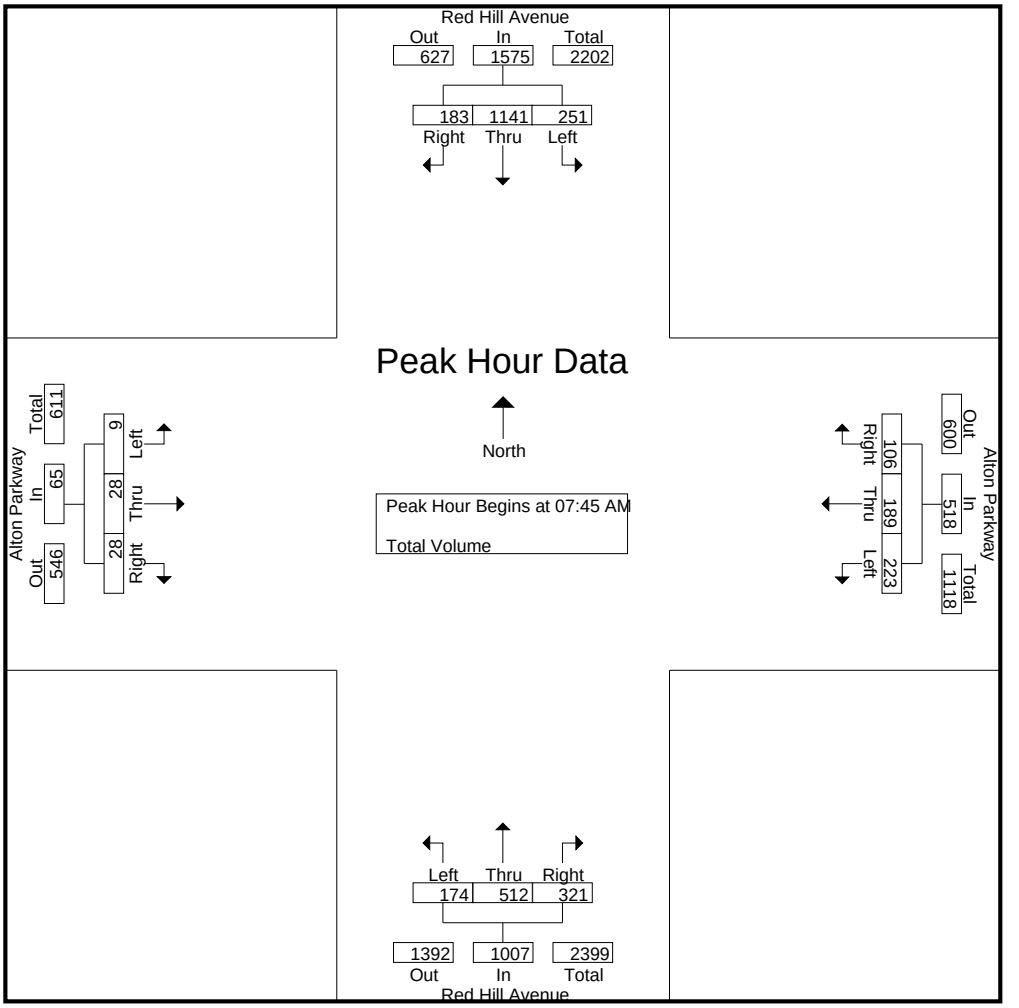
	Red Hill Avenue Southbound				Alton Parkway Westbound				Red Hill Avenue Northbound				Alton Parkway Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
07:00 AM	47	153	15	215	24	15	15	54	31	312	62	405	2	4	4	10	684
07:15 AM	39	189	19	247	31	25	15	71	39	288	76	403	2	8	2	12	733
07:30 AM	60	241	29	330	43	27	27	97	39	153	63	255	3	8	7	18	700
07:45 AM	58	314	41	413	48	46	24	118	46	149	93	288	2	5	8	15	834
Total	204	897	104	1205	146	113	81	340	155	902	294	1351	9	25	21	55	2951
08:00 AM	83	302	55	440	53	50	27	130	53	133	76	262	3	4	9	16	848
08:15 AM	52	242	44	338	61	46	33	140	34	115	78	227	2	7	5	14	719
08:30 AM	58	283	43	384	61	47	22	130	41	115	74	230	2	12	6	20	764
08:45 AM	43	281	48	372	71	47	20	138	26	109	80	215	3	11	5	19	744
Total	236	1108	190	1534	246	190	102	538	154	472	308	934	10	34	25	69	3075
Grand Total	440	2005	294	2739	392	303	183	878	309	1374	602	2285	19	59	46	124	6026
Apprch %	16.1	73.2	10.7		44.6	34.5	20.8		13.5	60.1	26.3		15.3	47.6	37.1		
Total %	7.3	33.3	4.9	45.5	6.5	5	3	14.6	5.1	22.8	10	37.9	0.3	1	0.8	2.1	

	Red Hill Avenue Southbound				Alton Parkway Westbound				Red Hill Avenue Northbound				Alton Parkway Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:45 AM																	
07:45 AM	58	**314**	41	413	48	46	24	118	46	**149**	**93**	**288**	2	5	8	15	834
08:00 AM	**83**	302	**55**	**440**	53	**50**	27	130	**53**	133	76	262	**3**	4	**9**	16	**848**
08:15 AM	52	242	44	338	**61**	46	**33**	**140**	34	115	78	227	2	7	5	14	719
08:30 AM	58	283	43	384	61	47	22	130	41	115	74	230	2	**12**	6	**20**	764
Total Volume	251	1141	183	1575	223	189	106	518	174	512	321	1007	9	28	28	65	3165
% App. Total	15.9	72.4	11.6		43.1	36.5	20.5		17.3	50.8	31.9		13.8	43.1	43.1		
PHF	.756	.908	.832	.895	.914	.945	.803	.925	.821	.859	.863	.874	.750	.583	.778	.813	.933

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City of Irvine
N/S: Red Hill Avenue
E/W: Alton Parkway
Weather: Clear

File Name : 100_IRV_Red Hill_Alton AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:45 AM				08:00 AM				07:00 AM				08:00 AM			
+0 mins.	58	**314**	41	413	53	**50**	27	130	31	**312**	62	**405**	**3**	4	**9**	16
+15 mins.	**83**	302	**55**	**440**	61	46	**33**	**140**	39	288	76	403	2	7	5	14
+30 mins.	52	242	44	338	61	47	22	130	39	153	63	255	2	**12**	6	**20**
+45 mins.	58	283	43	384	**71**	47	20	138	**46**	149	**93**	288	3	11	5	19
Total Volume	251	1141	183	1575	246	190	102	538	155	902	294	1351	10	34	25	69
% App. Total	15.9	72.4	11.6		45.7	35.3	19		11.5	66.8	21.8		14.5	49.3	36.2	
PHF	.756	.908	.832	.895	.866	.950	.773	.961	.842	.723	.790	.834	.833	.708	.694	.863

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City of Irvine
N/S: Red Hill Avenue
E/W: Alton Parkway
Weather: Clear

File Name : 100_IRV_Red Hill_Alton PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

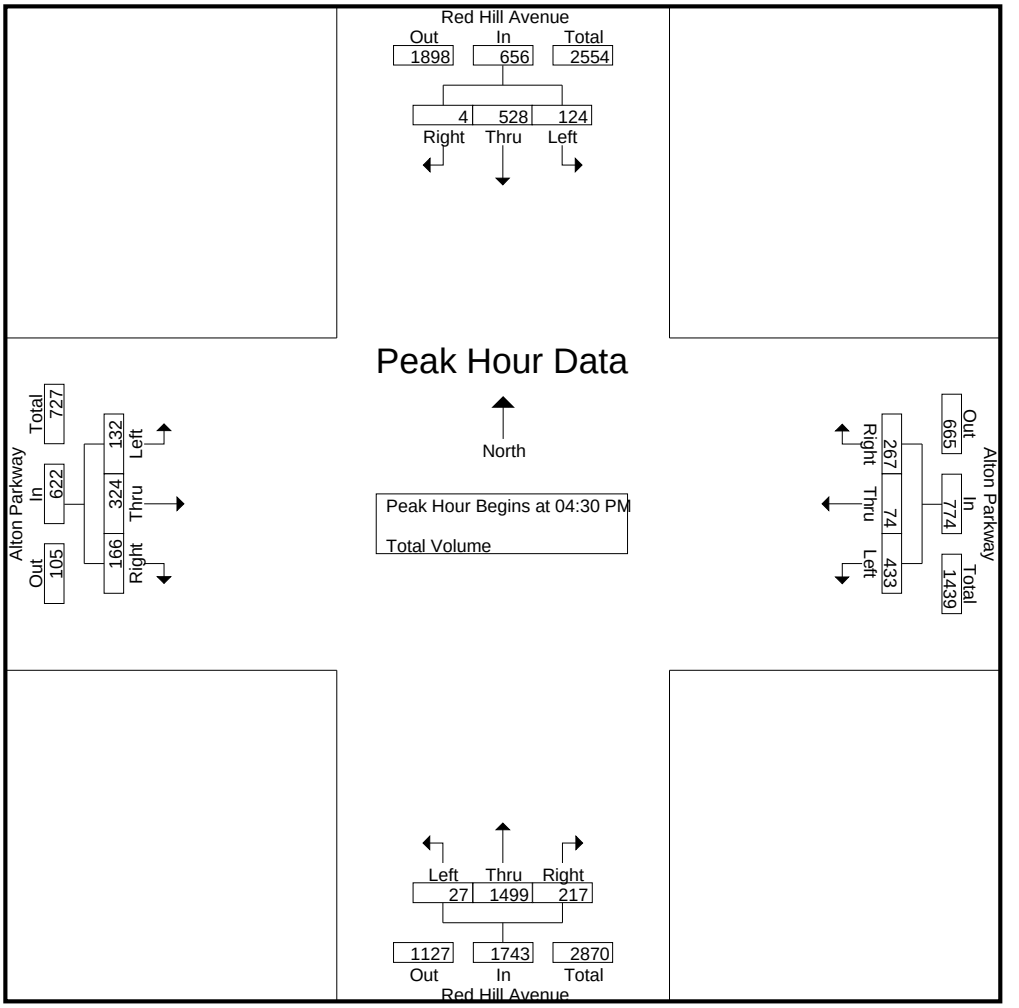
	Red Hill Avenue Southbound				Alton Parkway Westbound				Red Hill Avenue Northbound				Alton Parkway Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
04:00 PM	23	132	1	156	99	18	64	181	6	282	42	330	21	41	36	98	765
04:15 PM	34	117	5	156	72	18	40	130	7	331	44	382	19	42	41	102	770
04:30 PM	31	128	1	160	128	21	60	209	5	367	61	433	25	68	34	127	929
04:45 PM	30	138	0	168	81	22	58	161	6	358	40	404	28	73	36	137	870
Total	118	515	7	640	380	79	222	681	24	1338	187	1549	93	224	147	464	3334
05:00 PM	36	131	0	167	139	19	97	255	4	345	63	412	46	105	50	201	1035
05:15 PM	27	131	3	161	85	12	52	149	12	429	53	494	33	78	46	157	961
05:30 PM	29	123	0	152	110	20	66	196	4	406	47	457	19	60	33	112	917
05:45 PM	36	114	2	152	88	24	55	167	4	316	73	393	25	55	28	108	820
Total	128	499	5	632	422	75	270	767	24	1496	236	1756	123	298	157	578	3733
Grand Total	246	1014	12	1272	802	154	492	1448	48	2834	423	3305	216	522	304	1042	7067
Apprch %	19.3	79.7	0.9		55.4	10.6	34		1.5	85.7	12.8		20.7	50.1	29.2		
Total %	3.5	14.3	0.2	18	11.3	2.2	7	20.5	0.7	40.1	6	46.8	3.1	7.4	4.3	14.7	

	Red Hill Avenue Southbound				Alton Parkway Westbound				Red Hill Avenue Northbound				Alton Parkway Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:30 PM																	
04:30 PM	31	128	1	160	128	21	60	209	5	367	61	433	25	68	34	127	929
04:45 PM	30	**138**	0	**168**	81	**22**	58	161	6	358	40	404	28	73	36	137	870
05:00 PM	**36**	131	0	167	**139**	19	**97**	**255**	4	345	**63**	412	**46**	**105**	**50**	**201**	**1035**
05:15 PM	27	131	**3**	161	85	12	52	149	**12**	**429**	53	**494**	33	78	46	157	961
Total Volume	124	528	4	656	433	74	267	774	27	1499	217	1743	132	324	166	622	3795
% App. Total	18.9	80.5	0.6		55.9	9.6	34.5		1.5	86	12.4		21.2	52.1	26.7		
PHF	.861	.957	.333	.976	.779	.841	.688	.759	.563	.874	.861	.882	.717	.771	.830	.774	.917

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City of Irvine
N/S: Red Hill Avenue
E/W: Alton Parkway
Weather: Clear

File Name : 100_IRV_Red Hill_Alton PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:30 PM				04:30 PM				04:45 PM				04:30 PM			
+0 mins.	31	128	1	160	128	21	60	209	6	358	40	404	25	68	34	127
+15 mins.	30	**138**	0	**168**	81	**22**	58	161	4	345	**63**	412	28	73	36	137
+30 mins.	**36**	131	0	167	**139**	19	**97**	**255**	**12**	**429**	53	**494**	**46**	**105**	**50**	**201**
+45 mins.	27	131	**3**	161	85	12	52	149	4	406	47	457	33	78	46	157
Total Volume	124	528	4	656	433	74	267	774	26	1538	203	1767	132	324	166	622
% App. Total	18.9	80.5	0.6		55.9	9.6	34.5		1.5	87	11.5		21.2	52.1	26.7	
PHF	.861	.957	.333	.976	.779	.841	.688	.759	.542	.896	.806	.894	.717	.771	.830	.774

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City of Irvine
N/S: Red Hill Avenue
E/W: MacArthur Boulevard
Weather: Clear

File Name : 101_IRV_Red Hill_MacArthur AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

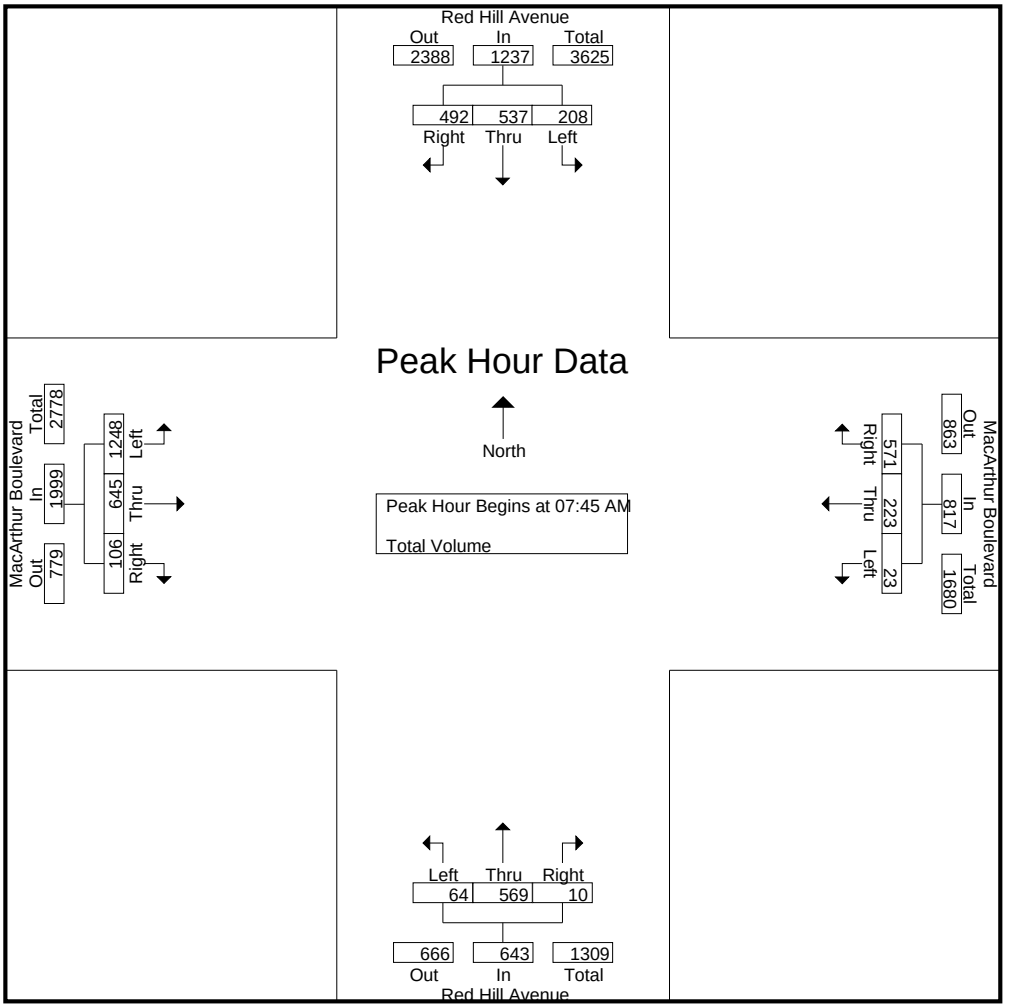
	Red Hill Avenue Southbound				MacArthur Boulevard Westbound				Red Hill Avenue Northbound				MacArthur Boulevard Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
07:00 AM	21	60	62	143	14	69	251	334	10	89	3	102	350	142	15	507	1086
07:15 AM	40	90	68	198	7	77	228	312	28	137	1	166	271	147	19	437	1113
07:30 AM	58	117	84	259	11	77	172	260	21	122	1	144	295	151	13	459	1122
07:45 AM	37	129	117	283	6	57	187	250	17	156	3	176	331	150	32	513	1222
Total	156	396	331	883	38	280	838	1156	76	504	8	588	1247	590	79	1916	4543
08:00 AM	67	158	119	344	4	52	140	196	14	174	2	190	299	159	22	480	1210
08:15 AM	51	118	127	296	7	59	109	175	16	145	2	163	300	173	22	495	1129
08:30 AM	53	132	129	314	6	55	135	196	17	94	3	114	318	163	30	511	1135
08:45 AM	67	132	133	332	3	88	102	193	16	101	3	120	282	150	24	456	1101
Total	238	540	508	1286	20	254	486	760	63	514	10	587	1199	645	98	1942	4575
Grand Total	394	936	839	2169	58	534	1324	1916	139	1018	18	1175	2446	1235	177	3858	9118
Apprch %	18.2	43.2	38.7		3	27.9	69.1		11.8	86.6	1.5		63.4	32	4.6		
Total %	4.3	10.3	9.2	23.8	0.6	5.9	14.5	21	1.5	11.2	0.2	12.9	26.8	13.5	1.9	42.3	

	Red Hill Avenue Southbound				MacArthur Boulevard Westbound				Red Hill Avenue Northbound				MacArthur Boulevard Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:45 AM																	
07:45 AM	37	129	117	283	6	57	**187**	**250**	**17**	156	**3**	176	**331**	150	**32**	**513**	**1222**
08:00 AM	**67**	**158**	119	**344**	4	52	140	196	14	**174**	2	**190**	299	159	22	480	1210
08:15 AM	51	118	127	296	**7**	**59**	109	175	16	145	2	163	300	**173**	22	495	1129
08:30 AM	53	132	**129**	314	6	55	135	196	17	94	3	114	318	163	30	511	1135
Total Volume	208	537	492	1237	23	223	571	817	64	569	10	643	1248	645	106	1999	4696
% App. Total	16.8	43.4	39.8		2.8	27.3	69.9		10	88.5	1.6		62.4	32.3	5.3		
PHF	.776	.850	.953	.899	.821	.945	.763	.817	.941	.818	.833	.846	.943	.932	.828	.974	.961

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City of Irvine
N/S: Red Hill Avenue
E/W: MacArthur Boulevard
Weather: Clear

File Name : 101_IRV_Red Hill_MacArthur AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	08:00 AM				07:00 AM				07:15 AM				07:45 AM			
+0 mins.	**67**	**158**	119	**344**	**14**	69	**251**	**334**	**28**	137	1	166	**331**	150	**32**	**513**
+15 mins.	51	118	127	296	7	**77**	228	312	21	122	1	144	299	159	22	480
+30 mins.	53	132	129	314	11	77	172	260	17	156	**3**	176	300	**173**	22	495
+45 mins.	67	132	**133**	332	6	57	187	250	14	**174**	2	**190**	318	163	30	511
Total Volume	238	540	508	1286	38	280	838	1156	80	589	7	676	1248	645	106	1999
% App. Total	18.5	42	39.5		3.3	24.2	72.5		11.8	87.1	1		62.4	32.3	5.3	
PHF	.888	.854	.955	.935	.679	.909	.835	.865	.714	.846	.583	.889	.943	.932	.828	.974

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City of Irvine
N/S: Red Hill Avenue
E/W: MacArthur Boulevard
Weather: Clear

File Name : 101_IRV_Red Hill_MacArthur PM
Site Code : 20220171
Start Date : 3/12/2020
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Groups Printed- Total Volume

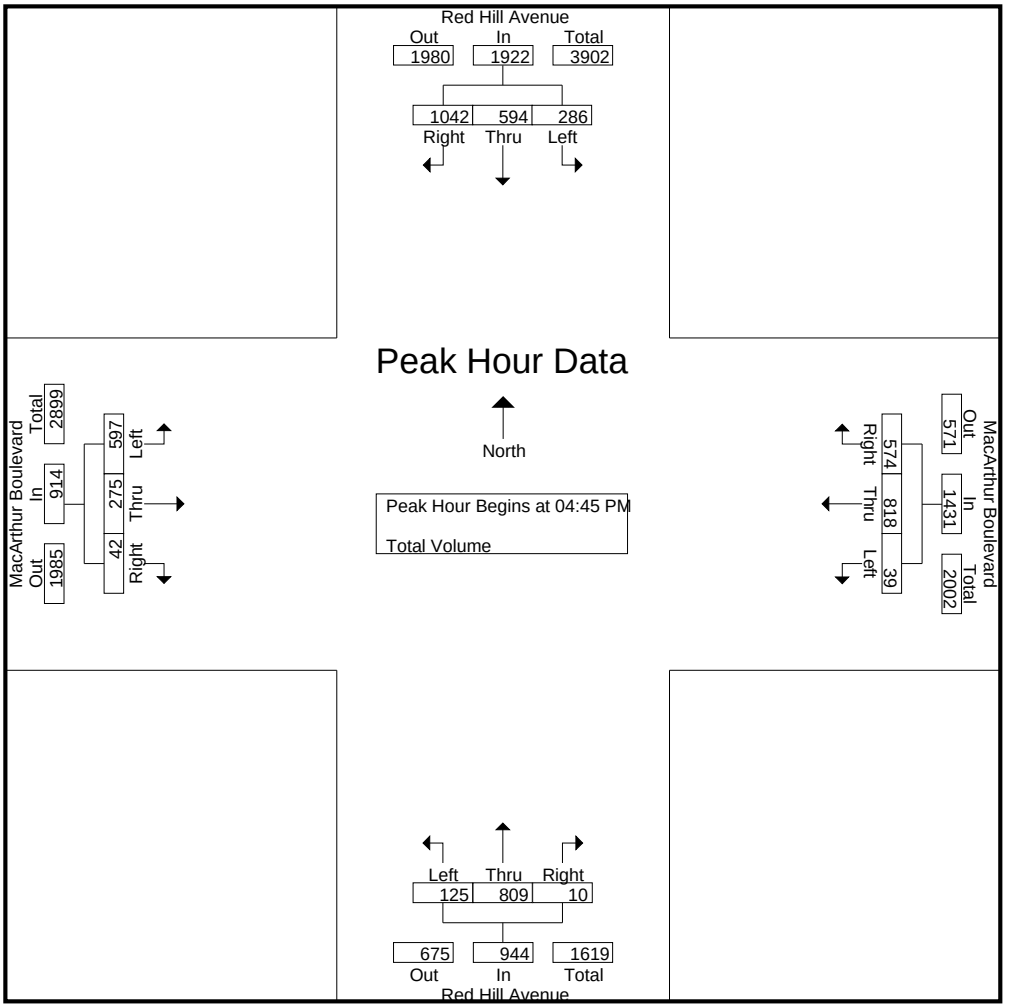
	Red Hill Avenue Southbound				MacArthur Boulevard Westbound				Red Hill Avenue Northbound				MacArthur Boulevard Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
04:00 PM	49	122	331	502	9	173	97	279	40	184	1	225	129	60	7	196	1202
04:15 PM	68	104	232	404	10	163	90	263	21	168	2	191	124	50	11	185	1043
04:30 PM	57	138	345	540	7	186	101	294	24	172	1	197	153	74	11	238	1269
04:45 PM	72	145	241	458	8	166	116	290	30	190	4	224	139	67	7	213	1185
Total	246	509	1149	1904	34	688	404	1126	115	714	8	837	545	251	36	832	4699
05:00 PM	69	166	308	543	14	231	153	398	34	208	1	243	135	76	13	224	1408
05:15 PM	78	146	254	478	11	248	163	422	27	221	2	250	117	65	12	194	1344
05:30 PM	67	137	239	443	6	173	142	321	34	190	3	227	206	67	10	283	1274
05:45 PM	39	106	193	338	4	216	118	338	22	160	4	186	181	74	10	265	1127
Total	253	555	994	1802	35	868	576	1479	117	779	10	906	639	282	45	966	5153
Grand Total	499	1064	2143	3706	69	1556	980	2605	232	1493	18	1743	1184	533	81	1798	9852
Apprch %	13.5	28.7	57.8		2.6	59.7	37.6		13.3	85.7	1		65.9	29.6	4.5		
Total %	5.1	10.8	21.8	37.6	0.7	15.8	9.9	26.4	2.4	15.2	0.2	17.7	12	5.4	0.8	18.3	

	Red Hill Avenue Southbound				MacArthur Boulevard Westbound				Red Hill Avenue Northbound				MacArthur Boulevard Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	72	145	241	458	8	166	116	290	30	190	**4**	224	139	67	7	213	1185
05:00 PM	69	**166**	**308**	**543**	**14**	231	153	398	**34**	208	1	243	135	**76**	**13**	224	**1408**
05:15 PM	**78**	146	254	478	11	**248**	**163**	**422**	27	**221**	2	**250**	117	65	12	194	1344
05:30 PM	67	137	239	443	6	173	142	321	34	190	3	227	**206**	67	10	**283**	1274
Total Volume	286	594	1042	1922	39	818	574	1431	125	809	10	944	597	275	42	914	5211
% App. Total	14.9	30.9	54.2		2.7	57.2	40.1		13.2	85.7	1.1		65.3	30.1	4.6		
PHF	.917	.895	.846	.885	.696	.825	.880	.848	.919	.915	.625	.944	.725	.905	.808	.807	.925

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City of Irvine
N/S: Red Hill Avenue
E/W: MacArthur Boulevard
Weather: Clear

File Name : 101_IRV_Red Hill_MacArthur PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:30 PM				05:00 PM				04:45 PM				05:00 PM			
+0 mins.	57	138	**345**	540	**14**	231	153	398	30	190	**4**	224	135	**76**	**13**	224
+15 mins.	72	145	241	458	11	**248**	**163**	**422**	**34**	208	1	243	117	65	12	194
+30 mins.	69	**166**	308	**543**	6	173	142	321	27	**221**	2	**250**	**206**	67	10	**283**
+45 mins.	**78**	146	254	478	4	216	118	338	34	190	3	227	181	74	10	265
Total Volume	276	595	1148	2019	35	868	576	1479	125	809	10	944	639	282	45	966
% App. Total	13.7	29.5	56.9		2.4	58.7	38.9		13.2	85.7	1.1		66.1	29.2	4.7	
PHF	.885	.896	.832	.930	.625	.875	.883	.876	.919	.915	.625	.944	.775	.928	.865	.853

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City of Irvine
N/S: Red Hill Avenue
E/W: Main Street
Weather: Clear

File Name : 102_IRV_Red Hill_Main_AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

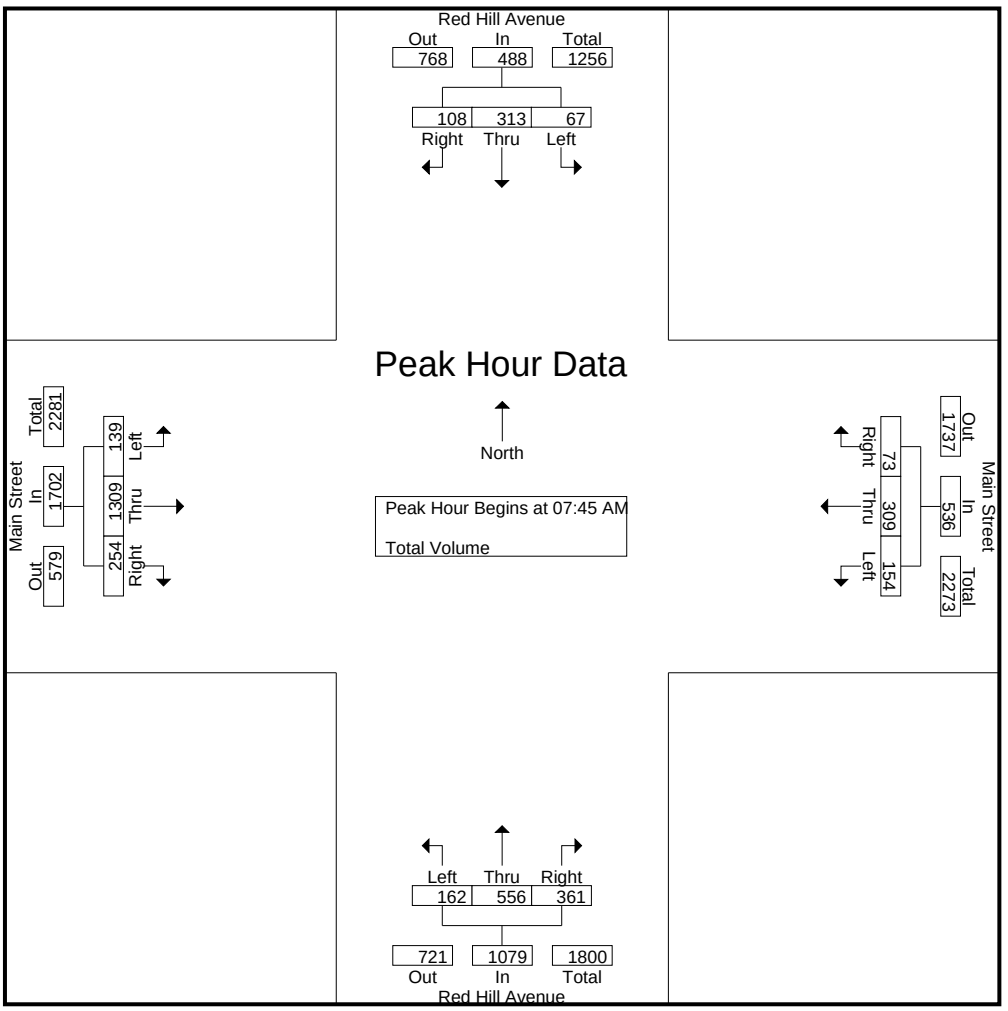
	Red Hill Avenue Southbound				Main Street Westbound				Red Hill Avenue Northbound				Main Street Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
07:00 AM	15	44	4	63	18	51	12	81	12	73	46	131	13	157	38	208	483
07:15 AM	11	40	12	63	26	54	19	99	25	113	55	193	25	234	51	310	665
07:30 AM	10	53	20	83	30	60	15	105	30	140	80	250	20	325	53	398	836
07:45 AM	19	76	26	121	41	67	20	128	35	151	82	268	30	374	76	480	997
Total	55	213	62	330	115	232	66	413	102	477	263	842	88	1090	218	1396	2981
08:00 AM	16	86	25	127	37	79	24	140	37	166	97	300	49	358	56	463	1030
08:15 AM	13	71	32	116	48	74	16	138	49	120	92	261	27	306	67	400	915
08:30 AM	19	80	25	124	28	89	13	130	41	119	90	250	33	271	55	359	863
08:45 AM	14	75	37	126	29	77	14	120	38	134	82	254	27	255	61	343	843
Total	62	312	119	493	142	319	67	528	165	539	361	1065	136	1190	239	1565	3651
Grand Total	117	525	181	823	257	551	133	941	267	1016	624	1907	224	2280	457	2961	6632
Apprch %	14.2	63.8	22		27.3	58.6	14.1		14	53.3	32.7		7.6	77	15.4		
Total %	1.8	7.9	2.7	12.4	3.9	8.3	2	14.2	4	15.3	9.4	28.8	3.4	34.4	6.9	44.6	

	Red Hill Avenue Southbound				Main Street Westbound				Red Hill Avenue Northbound				Main Street Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:45 AM																	
07:45 AM	**19**	76	26	121	41	67	20	128	35	151	82	268	30	**374**	**76**	**480**	997
08:00 AM	16	**86**	25	**127**	37	79	**24**	**140**	37	**166**	**97**	**300**	**49**	358	56	463	**1030**
08:15 AM	13	71	**32**	116	**48**	74	16	138	**49**	120	92	261	27	306	67	400	915
08:30 AM	19	80	25	124	28	**89**	13	130	41	119	90	250	33	271	55	359	863
Total Volume	67	313	108	488	154	309	73	536	162	556	361	1079	139	1309	254	1702	3805
% App. Total	13.7	64.1	22.1		28.7	57.6	13.6		15	51.5	33.5		8.2	76.9	14.9		
PHF	.882	.910	.844	.961	.802	.868	.760	.957	.827	.837	.930	.899	.709	.875	.836	.886	.924

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City of Irvine
N/S: Red Hill Avenue
E/W: Main Street
Weather: Clear

File Name : 102_IRV_Red Hill_Main_AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	08:00 AM				07:45 AM				07:30 AM				07:30 AM			
+0 mins.	16	**86**	25	**127**	41	67	20	128	30	140	80	250	20	325	53	398
+15 mins.	13	71	32	116	37	79	**24**	**140**	35	151	82	268	30	**374**	**76**	**480**
+30 mins.	**19**	80	25	124	**48**	74	16	138	37	**166**	**97**	**300**	**49**	358	56	463
+45 mins.	14	75	**37**	126	28	**89**	13	130	**49**	120	92	261	27	306	67	400
Total Volume	62	312	119	493	154	309	73	536	151	577	351	1079	126	1363	252	1741
% App. Total	12.6	63.3	24.1		28.7	57.6	13.6		14	53.5	32.5		7.2	78.3	14.5	
PHF	.816	.907	.804	.970	.802	.868	.760	.957	.770	.869	.905	.899	.643	.911	.829	.907

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City of Irvine
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E/W: Main Street
Weather: Clear

File Name : 102_IRV_Red Hill_Main_PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

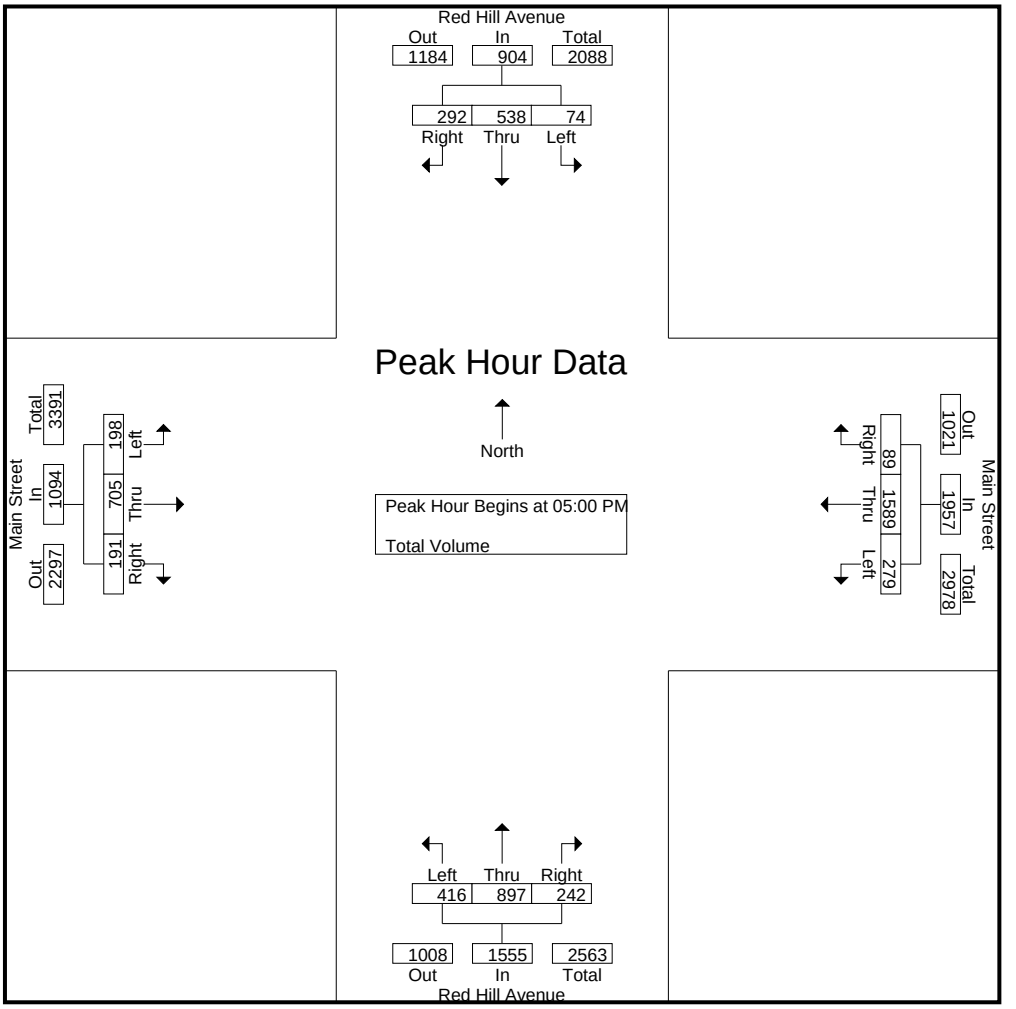
	Red Hill Avenue Southbound				Main Street Westbound				Red Hill Avenue Northbound				Main Street Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
04:00 PM	12	88	33	133	58	294	20	372	56	164	57	277	26	141	27	194	976
04:15 PM	23	90	47	160	53	364	17	434	81	220	45	346	39	126	26	191	1131
04:30 PM	26	124	53	203	60	267	16	343	91	236	60	387	44	138	46	228	1161
04:45 PM	26	133	61	220	63	362	27	452	85	232	62	379	45	136	37	218	1269
Total	87	435	194	716	234	1287	80	1601	313	852	224	1389	154	541	136	831	4537
05:00 PM	22	135	75	232	81	382	18	481	93	238	75	406	49	145	42	236	1355
05:15 PM	24	133	86	243	74	468	22	564	109	243	63	415	55	183	53	291	1513
05:30 PM	13	142	71	226	58	377	24	459	118	223	51	392	44	206	45	295	1372
05:45 PM	15	128	60	203	66	362	25	453	96	193	53	342	50	171	51	272	1270
Total	74	538	292	904	279	1589	89	1957	416	897	242	1555	198	705	191	1094	5510
Grand Total	161	973	486	1620	513	2876	169	3558	729	1749	466	2944	352	1246	327	1925	10047
Apprch %	9.9	60.1	30		14.4	80.8	4.7		24.8	59.4	15.8		18.3	64.7	17		
Total %	1.6	9.7	4.8	16.1	5.1	28.6	1.7	35.4	7.3	17.4	4.6	29.3	3.5	12.4	3.3	19.2	

	Red Hill Avenue Southbound				Main Street Westbound				Red Hill Avenue Northbound				Main Street Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 05:00 PM																	
05:00 PM	22	135	75	232	**81**	382	18	481	93	238	**75**	406	49	145	42	236	1355
05:15 PM	**24**	133	**86**	**243**	74	**468**	22	**564**	109	**243**	63	**415**	**55**	183	**53**	291	**1513**
05:30 PM	13	**142**	71	226	58	377	24	459	**118**	223	51	392	44	**206**	45	**295**	1372
05:45 PM	15	128	60	203	66	362	**25**	453	96	193	53	342	50	171	51	272	1270
Total Volume	74	538	292	904	279	1589	89	1957	416	897	242	1555	198	705	191	1094	5510
% App. Total	8.2	59.5	32.3		14.3	81.2	4.5		26.8	57.7	15.6		18.1	64.4	17.5		
PHF	.771	.947	.849	.930	.861	.849	.890	.867	.881	.923	.807	.937	.900	.856	.901	.927	.910

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City of Irvine
N/S: Red Hill Avenue
E/W: Main Street
Weather: Clear

File Name : 102_IRV_Red Hill_Main_PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:45 PM				05:00 PM				04:45 PM				05:00 PM			
+0 mins.	**26**	133	61	220	**81**	382	18	481	85	232	62	379	49	145	42	236
+15 mins.	22	135	75	232	74	**468**	22	**564**	93	238	**75**	406	**55**	183	**53**	291
+30 mins.	24	133	**86**	**243**	58	377	24	459	109	**243**	63	**415**	44	**206**	45	**295**
+45 mins.	13	**142**	71	226	66	362	**25**	453	**118**	223	51	392	50	171	51	272
Total Volume	85	543	293	921	279	1589	89	1957	405	936	251	1592	198	705	191	1094
% App. Total	9.2	59	31.8		14.3	81.2	4.5		25.4	58.8	15.8		18.1	64.4	17.5	
PHF	.817	.956	.852	.948	.861	.849	.890	.867	.858	.963	.837	.959	.900	.856	.901	.927

INTERSECTION TURNING MOVEMENT COUNTS

T816

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Thu, Feb 16, 17

LOCATION: Santa Ana
NORTH & SOUTH: I-5 SB Ramps
EAST & WEST: Santa Ana

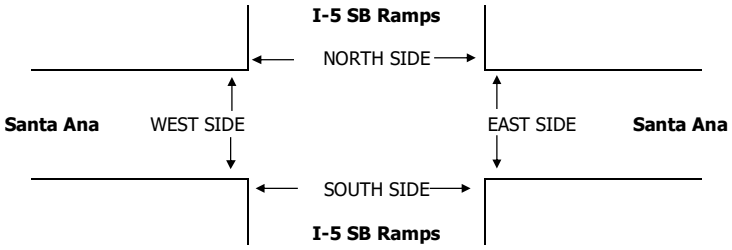
PROJECT #: SC1220
LOCATION #: 1
CONTROL: SIGNAL

NOTES:

AM PM MD OTHER OTHER — ▲ N ◄ W E ► S ▼

Add U-Turns to Left Turns

		NORTHBOUND I-5 SB Ramps			SOUTHBOUND I-5 SB Ramps			EASTBOUND Santa Ana			WESTBOUND Santa Ana				U-TURNS				
	LANES:	NL X	NT X	NR X	SL 2	ST X	SR 1	EL 2	ET 3	ER X	WL X	WT 3	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	0	0	0	76	0	7	50	100	0	0	190	81	504	0	0	0	0	0
	7:15 AM	0	0	0	87	0	8	65	145	0	0	226	79	610	0	0	0	0	0
	7:30 AM	0	0	0	79	0	10	85	138	0	0	327	60	699	0	0	1	0	1
	7:45 AM	0	0	0	101	0	7	84	123	0	0	329	88	732	0	0	0	0	0
	8:00 AM	0	0	0	89	0	9	79	131	0	0	214	93	615	0	0	0	0	0
	8:15 AM	0	0	0	67	0	12	39	114	0	0	188	79	499	0	0	0	0	0
	8:30 AM	0	0	0	66	0	14	43	75	0	0	174	54	426	0	0	0	0	0
	8:45 AM	0	0	0	105	0	10	27	81	0	0	203	46	472	0	0	0	0	0
	VOLUMES	0	0	0	670	0	77	472	907	0	0	1,851	580	4,557	0	0	1	0	1
	APPROACH %	0%	0%	0%	90%	0%	10%	34%	66%	0%	0%	76%	24%						
	APP/DEPART	0	/	1,051	747	/	0	1,379	/	1,577	2,431	/	1,929	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	0	0	0	356	0	34	313	537	0	0	1,096	320	2,656					
	APPROACH %	0%	0%	0%	91%	0%	9%	37%	63%	0%	0%	77%	23%						
	PEAK HR FACTOR		0.000			0.903			0.953			0.849		0.907					
	APP/DEPART	0	/	632	390	/	0	850	/	893	1,416	/	1,131	0					
PM	4:00 PM	0	0	0	54	0	27	123	138	0	0	128	33	503	0	0	0	0	0
	4:15 PM	0	0	0	67	0	24	78	100	0	0	159	28	456	0	0	0	0	0
	4:30 PM	0	0	0	76	0	14	140	152	0	0	167	39	588	0	0	0	0	0
	4:45 PM	0	0	0	72	0	15	121	147	0	0	156	49	560	0	0	0	0	0
	5:00 PM	0	0	0	66	0	8	146	190	0	0	150	54	614	0	0	0	0	0
	5:15 PM	0	0	0	55	0	22	137	126	0	0	175	35	550	0	0	1	0	1
	5:30 PM	0	0	0	64	0	16	122	131	0	0	141	41	515	0	0	0	0	0
	5:45 PM	0	0	0	63	0	20	102	112	0	0	135	33	465	0	0	0	0	0
	VOLUMES	0	0	0	517	0	146	969	1,096	0	0	1,211	312	4,251	0	0	1	0	1
	APPROACH %	0%	0%	0%	78%	0%	22%	47%	53%	0%	0%	80%	20%						
	APP/DEPART	0	/	1,280	663	/	0	2,065	/	1,613	1,523	/	1,358	0					
	BEGIN PEAK HR		4:30 PM																
	VOLUMES	0	0	0	269	0	59	544	615	0	0	648	177	2,312					
	APPROACH %	0%	0%	0%	82%	0%	18%	47%	53%	0%	0%	79%	21%						
	PEAK HR FACTOR		0.000			0.911			0.862			0.982		0.941					
	APP/DEPART	0	/	720	328	/	0	1,159	/	884	825	/	708	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

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City of Tustin
N/S: Tustin Ranch Road
E/W: Warner Avenue
Weather: Clear

File Name : 104_TUS_Tustin Ranch_Warner AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

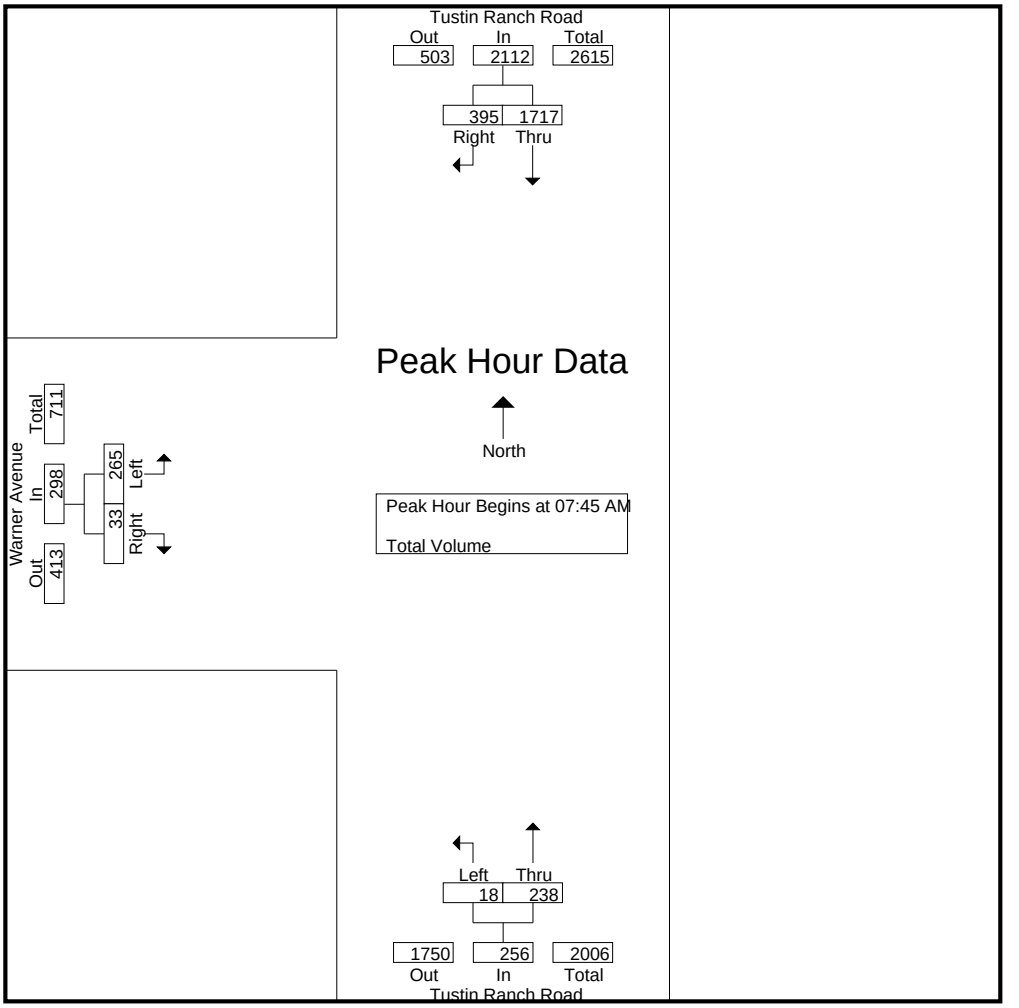
	Tustin Ranch Road Southbound			Tustin Ranch Road Northbound			Warner Avenue Eastbound			
Start Time	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	Int. Total
07:00 AM	140	44	184	2	46	48	57	6	63	295
07:15 AM	285	70	355	1	43	44	59	6	65	464
07:30 AM	308	80	388	4	56	60	80	5	85	533
07:45 AM	443	116	559	2	49	51	67	12	79	689
Total	1176	310	1486	9	194	203	263	29	292	1981
08:00 AM	396	82	478	2	58	60	83	14	97	635
08:15 AM	453	106	559	9	66	75	57	1	58	692
08:30 AM	425	91	516	5	65	70	58	6	64	650
08:45 AM	389	99	488	1	74	75	52	4	56	619
Total	1663	378	2041	17	263	280	250	25	275	2596
Grand Total	2839	688	3527	26	457	483	513	54	567	4577
Apprch %	80.5	19.5		5.4	94.6		90.5	9.5		
Total %	62	15	77.1	0.6	10	10.6	11.2	1.2	12.4	

	Tustin Ranch Road Southbound			Tustin Ranch Road Northbound			Warner Avenue Eastbound			
Start Time	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 07:45 AM										
07:45 AM	443	**116**	**559**	2	49	51	67	12	79	689
08:00 AM	396	82	478	2	58	60	**83**	**14**	**97**	635
08:15 AM	**453**	106	559	**9**	**66**	**75**	57	1	58	**692**
08:30 AM	425	91	516	5	65	70	58	6	64	650
Total Volume	1717	395	2112	18	238	256	265	33	298	2666
% App. Total	81.3	18.7		7	93		88.9	11.1		
PHF	.948	.851	.945	.500	.902	.853	.798	.589	.768	.963

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City of Tustin
N/S: Tustin Ranch Road
E/W: Warner Avenue
Weather: Clear

File Name : 104_TUS_Tustin Ranch_Warner AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	07:45 AM			08:00 AM			07:15 AM		
+0 mins.	443	**116**	**559**	2	58	60	59	6	65
+15 mins.	396	82	478	**9**	66	**75**	80	5	85
+30 mins.	**453**	106	559	5	65	70	67	12	79
+45 mins.	425	91	516	1	**74**	75	**83**	**14**	**97**
Total Volume	1717	395	2112	17	263	280	289	37	326
% App. Total	81.3	18.7		6.1	93.9		88.7	11.3	
PHF	.948	.851	.945	.472	.889	.933	.870	.661	.840

Counts Unlimited
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(951) 268-6268

City of Tustin
N/S: Tustin Ranch Road
E/W: Warner Avenue
Weather: Clear

File Name : 104_TUS_Tustin Ranch_Warner PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

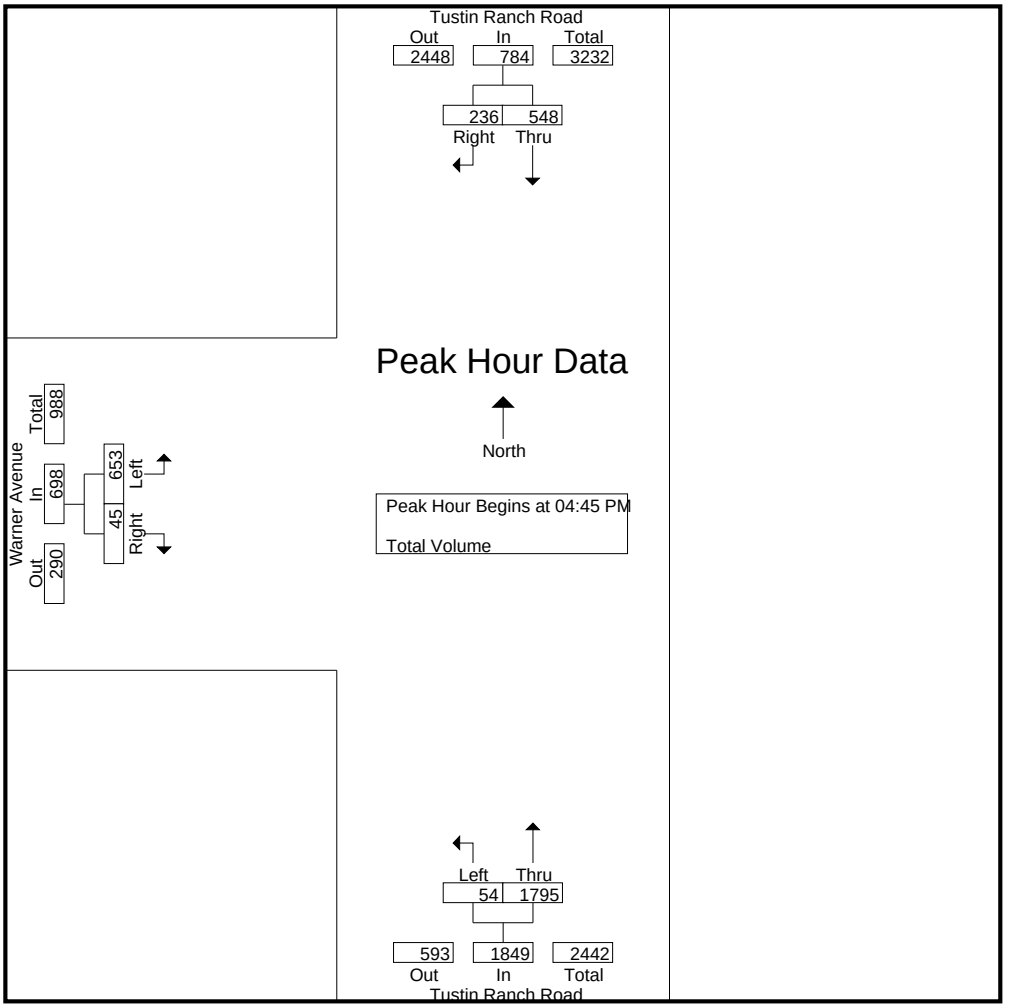
	Tustin Ranch Road Southbound			Tustin Ranch Road Northbound			Warner Avenue Eastbound			
Start Time	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	Int. Total
04:00 PM	99	49	148	11	351	362	100	7	107	617
04:15 PM	104	52	156	10	369	379	107	10	117	652
04:30 PM	117	55	172	18	376	394	106	9	115	681
04:45 PM	121	50	171	8	401	409	135	10	145	725
Total	441	206	647	47	1497	1544	448	36	484	2675
05:00 PM	141	63	204	19	466	485	166	12	178	867
05:15 PM	145	67	212	15	450	465	178	16	194	871
05:30 PM	141	56	197	12	478	490	174	7	181	868
05:45 PM	113	49	162	11	384	395	141	11	152	709
Total	540	235	775	57	1778	1835	659	46	705	3315
Grand Total	981	441	1422	104	3275	3379	1107	82	1189	5990
Apprch %	69	31		3.1	96.9		93.1	6.9		
Total %	16.4	7.4	23.7	1.7	54.7	56.4	18.5	1.4	19.8	

	Tustin Ranch Road Southbound			Tustin Ranch Road Northbound			Warner Avenue Eastbound			
Start Time	Thru	Right	App. Total	Left	Thru	App. Total	Left	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 04:45 PM										
04:45 PM	121	50	171	8	401	409	135	10	145	725
05:00 PM	141	63	204	**19**	466	485	166	12	178	867
05:15 PM	**145**	**67**	**212**	15	450	465	**178**	**16**	**194**	**871**
05:30 PM	141	56	197	12	**478**	**490**	174	7	181	868
Total Volume	548	236	784	54	1795	1849	653	45	698	3331
% App. Total	69.9	30.1		2.9	97.1		93.6	6.4		
PHF	.945	.881	.925	.711	.939	.943	.917	.703	.899	.956

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City of Tustin
N/S: Tustin Ranch Road
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Weather: Clear

File Name : 104_TUS_Tustin Ranch_Warner PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	04:45 PM			04:45 PM			05:00 PM		
+0 mins.	121	50	171	8	401	409	166	12	178
+15 mins.	141	63	204	**19**	466	485	**178**	**16**	**194**
+30 mins.	**145**	**67**	**212**	15	450	465	174	7	181
+45 mins.	141	56	197	12	**478**	**490**	141	11	152
Total Volume	548	236	784	54	1795	1849	659	46	705
% App. Total	69.9	30.1		2.9	97.1		93.5	6.5	
PHF	.945	.881	.925	.711	.939	.943	.926	.719	.909

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City of Irvine
N/S: Tustin Ranch Rd/Von Karman Ave
E/W: Barranca Parkway
Weather: Clear

File Name : 105_IRV_Von Karman_Barranca AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

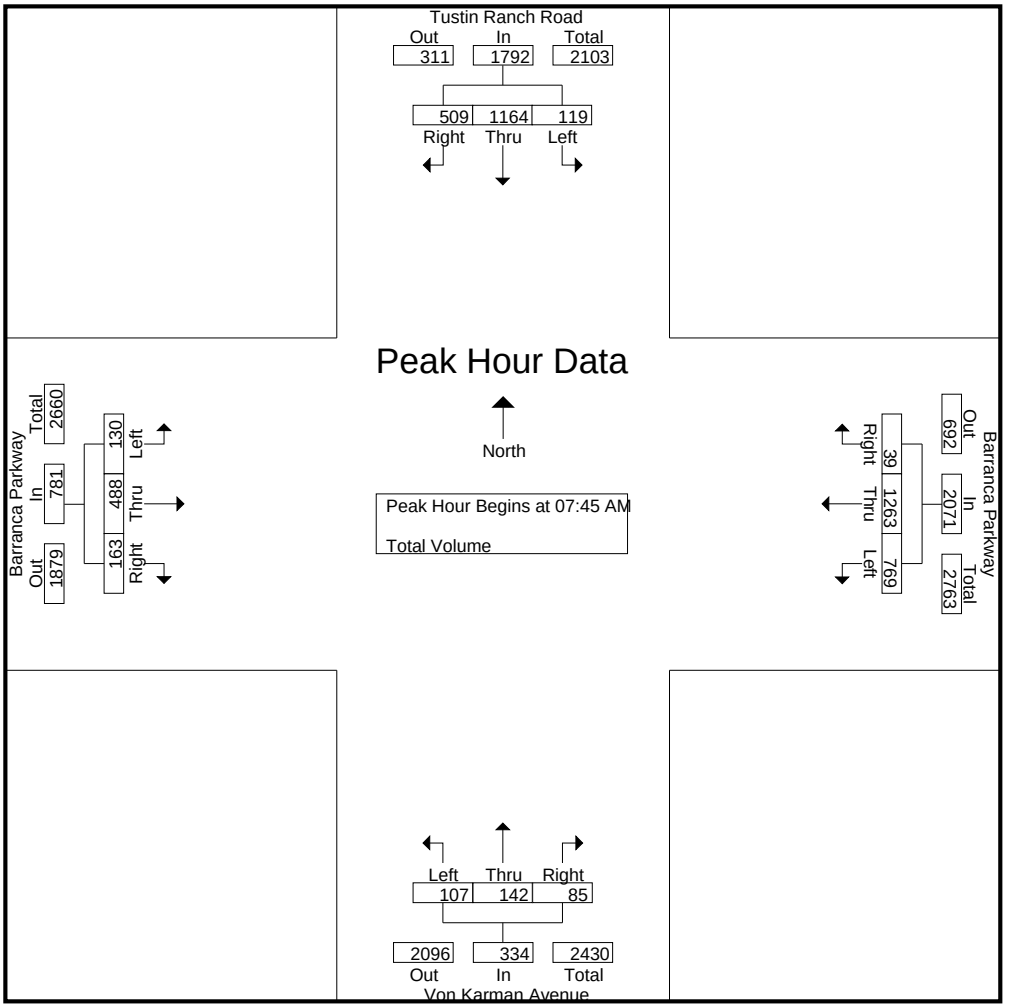
	Tustin Ranch Road Southbound				Barranca Parkway Westbound				Von Karman Avenue Northbound				Barranca Parkway Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
07:00 AM	5	117	38	160	81	170	11	262	17	26	17	60	17	103	35	155	637
07:15 AM	13	212	80	305	134	202	11	347	22	19	9	50	19	119	37	175	877
07:30 AM	38	258	99	395	140	264	13	417	26	29	18	73	25	101	45	171	1056
07:45 AM	30	280	118	428	186	361	8	555	21	29	16	66	30	120	51	201	1250
Total	86	867	335	1288	541	997	43	1581	86	103	60	249	91	443	168	702	3820
08:00 AM	23	301	119	443	175	290	12	477	24	36	28	88	38	115	31	184	1192
08:15 AM	44	294	141	479	192	312	10	514	34	36	22	92	32	127	35	194	1279
08:30 AM	22	289	131	442	216	300	9	525	28	41	19	88	30	126	46	202	1257
08:45 AM	30	269	134	433	156	291	17	464	28	44	23	95	49	90	44	183	1175
Total	119	1153	525	1797	739	1193	48	1980	114	157	92	363	149	458	156	763	4903
Grand Total	205	2020	860	3085	1280	2190	91	3561	200	260	152	612	240	901	324	1465	8723
Apprch %	6.6	65.5	27.9		35.9	61.5	2.6		32.7	42.5	24.8		16.4	61.5	22.1		
Total %	2.4	23.2	9.9	35.4	14.7	25.1	1	40.8	2.3	3	1.7	7	2.8	10.3	3.7	16.8	

	Tustin Ranch Road Southbound				Barranca Parkway Westbound				Von Karman Avenue Northbound				Barranca Parkway Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:45 AM																	
07:45 AM	30	280	118	428	186	**361**	8	**555**	21	29	16	66	30	120	**51**	201	1250
08:00 AM	23	**301**	119	443	175	290	**12**	477	24	36	**28**	88	**38**	115	31	184	1192
08:15 AM	**44**	294	**141**	**479**	192	312	10	514	**34**	36	22	**92**	32	**127**	35	194	**1279**
08:30 AM	22	289	131	442	**216**	300	9	525	28	**41**	19	88	30	126	46	**202**	1257
Total Volume	119	1164	509	1792	769	1263	39	2071	107	142	85	334	130	488	163	781	4978
% App. Total	6.6	65	28.4		37.1	61	1.9		32	42.5	25.4		16.6	62.5	20.9		
PHF	.676	.967	.902	.935	.890	.875	.813	.933	.787	.866	.759	.908	.855	.961	.799	.967	.973

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City of Irvine
N/S: Tustin Ranch Rd/Von Karman Ave
E/W: Barranca Parkway
Weather: Clear

File Name : 105_IRV_Von Karman_Barranca AM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	08:00 AM				07:45 AM				08:00 AM				07:45 AM			
+0 mins.	23	**301**	119	443	186	**361**	8	**555**	24	36	**28**	88	30	120	**51**	201
+15 mins.	**44**	294	**141**	**479**	175	290	**12**	477	**34**	36	22	92	**38**	115	31	184
+30 mins.	22	289	131	442	192	312	10	514	28	41	19	88	32	**127**	35	194
+45 mins.	30	269	134	433	**216**	300	9	525	28	**44**	23	**95**	30	126	46	**202**
Total Volume	119	1153	525	1797	769	1263	39	2071	114	157	92	363	130	488	163	781
% App. Total	6.6	64.2	29.2		37.1	61	1.9		31.4	43.3	25.3		16.6	62.5	20.9	
PHF	.676	.958	.931	.938	.890	.875	.813	.933	.838	.892	.821	.955	.855	.961	.799	.967

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City of Irvine
N/S: Tustin Ranch Rd/Von Karman Ave
E/W: Barranca Parkway
Weather: Clear

File Name : 105_IRV_Von Karman_Barranca PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 1

Groups Printed- Total Volume

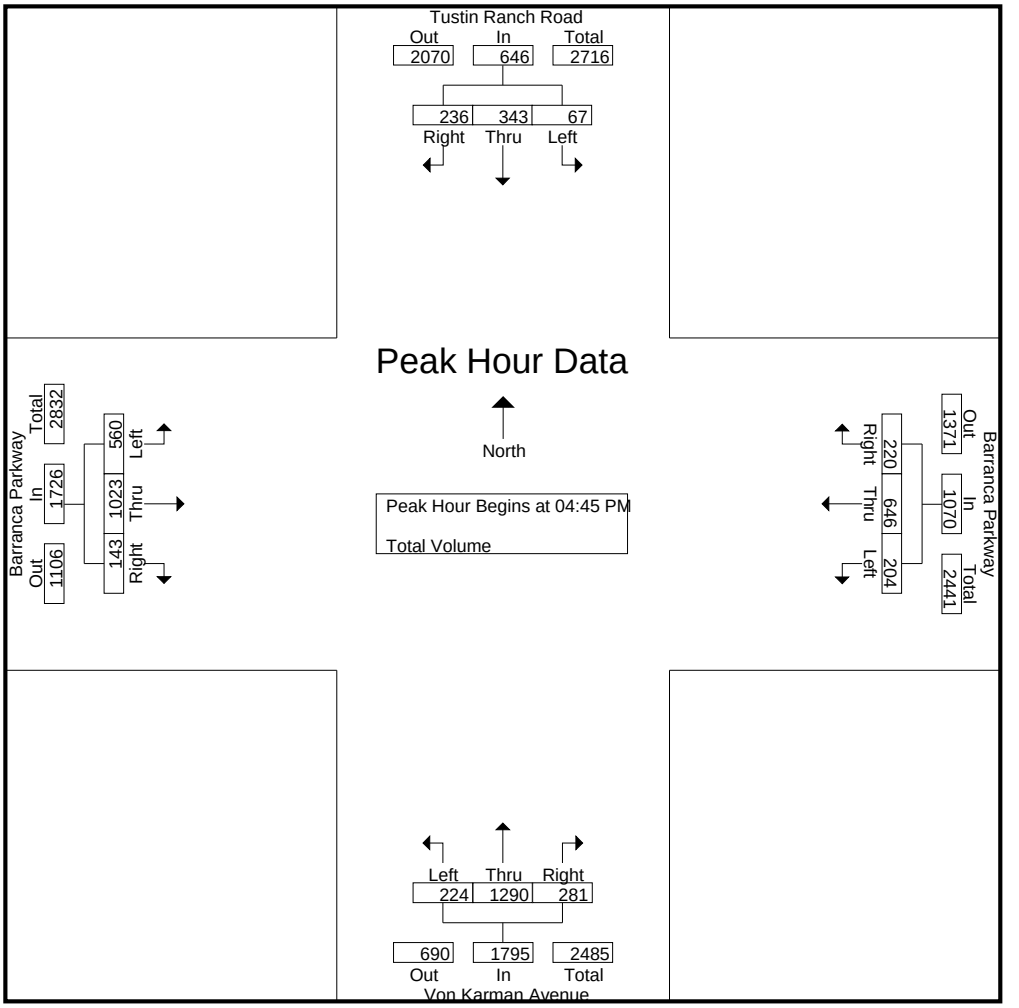
	Tustin Ranch Road Southbound				Barranca Parkway Westbound				Von Karman Avenue Northbound				Barranca Parkway Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
04:00 PM	13	71	53	137	67	171	56	294	71	229	53	353	85	197	19	301	1085
04:15 PM	9	77	62	148	36	125	41	202	38	277	63	378	137	214	27	378	1106
04:30 PM	14	59	48	121	53	157	59	269	83	257	68	408	99	207	23	329	1127
04:45 PM	14	88	59	161	37	149	47	233	48	328	78	454	119	221	30	370	1218
Total	50	295	222	567	193	602	203	998	240	1091	262	1593	440	839	99	1378	4536
05:00 PM	17	74	51	142	53	168	58	279	71	311	81	463	123	284	29	436	1320
05:15 PM	16	103	64	183	54	136	53	243	43	332	64	439	168	285	50	503	1368
05:30 PM	20	78	62	160	60	193	62	315	62	319	58	439	150	233	34	417	1331
05:45 PM	15	96	55	166	56	147	41	244	47	294	50	391	115	243	33	391	1192
Total	68	351	232	651	223	644	214	1081	223	1256	253	1732	556	1045	146	1747	5211
Grand Total	118	646	454	1218	416	1246	417	2079	463	2347	515	3325	996	1884	245	3125	9747
Apprch %	9.7	53	37.3		20	59.9	20.1		13.9	70.6	15.5		31.9	60.3	7.8		
Total %	1.2	6.6	4.7	12.5	4.3	12.8	4.3	21.3	4.8	24.1	5.3	34.1	10.2	19.3	2.5	32.1	

	Tustin Ranch Road Southbound				Barranca Parkway Westbound				Von Karman Avenue Northbound				Barranca Parkway Eastbound				
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 04:45 PM																	
04:45 PM	14	88	59	161	37	149	47	233	48	328	78	454	119	221	30	370	1218
05:00 PM	17	74	51	142	53	168	58	279	**71**	311	**81**	**463**	123	284	29	436	1320
05:15 PM	16	**103**	**64**	**183**	54	136	53	243	43	**332**	64	439	**168**	**285**	**50**	**503**	**1368**
05:30 PM	**20**	78	62	160	**60**	**193**	**62**	**315**	62	319	58	439	150	233	34	417	1331
Total Volume	67	343	236	646	204	646	220	1070	224	1290	281	1795	560	1023	143	1726	5237
% App. Total	10.4	53.1	36.5		19.1	60.4	20.6		12.5	71.9	15.7		32.4	59.3	8.3		
PHF	.838	.833	.922	.883	.850	.837	.887	.849	.789	.971	.867	.969	.833	.897	.715	.858	.957

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City of Irvine
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File Name : 105_IRV_Von Karman_Barranca PM
Site Code : 20220171
Start Date : 3/12/2020
Page No : 2



Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

	05:00 PM				05:00 PM				04:45 PM				05:00 PM			
+0 mins.	17	74	51	142	53	168	58	279	48	328	78	454	123	284	29	436
+15 mins.	16	**103**	**64**	**183**	54	136	53	243	**71**	311	**81**	**463**	**168**	**285**	**50**	**503**
+30 mins.	**20**	78	62	160	**60**	**193**	**62**	**315**	43	**332**	64	439	150	233	34	417
+45 mins.	15	96	55	166	56	147	41	244	62	319	58	439	115	243	33	391
Total Volume	68	351	232	651	223	644	214	1081	224	1290	281	1795	556	1045	146	1747
% App. Total	10.4	53.9	35.6		20.6	59.6	19.8		12.5	71.9	15.7		31.8	59.8	8.4	
PHF	.850	.852	.906	.889	.929	.834	.863	.858	.789	.971	.867	.969	.827	.917	.730	.868

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Tue, May 21, 19

LOCATION: Tustin
NORTH & SOUTH: Red Hill
EAST & WEST: El Camino Real

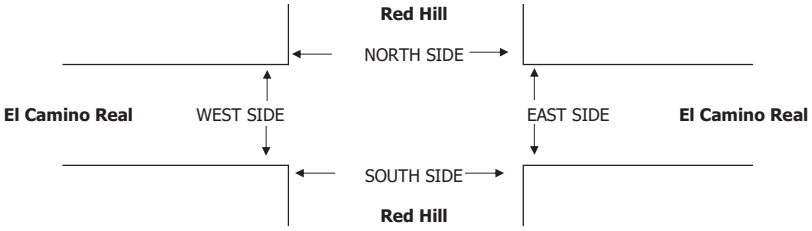
PROJECT #: SC2143
LOCATION #: 7
CONTROL: SIGNAL

NOTES:
Queue AM SB, PM NB/SB

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☒ Add U-Turns to Left Turns

		NORTHBOUND Red Hill			SOUTHBOUND Red Hill			EASTBOUND El Camino Real			WESTBOUND El Camino Real				U-TURNS				
	LANES:	NL 2	NT 3	NR 1	SL 1	ST 3	SR 0	EL 1	ET 1	ER 1	WL 1	WT 1	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	55	107	23	4	208	10	16	20	43	54	18	1	559	0	1	0	0	1
	7:15 AM	85	195	51	1	287	20	15	30	44	67	32	2	829	0	0	0	0	0
	7:30 AM	108	203	98	8	294	30	29	50	40	70	83	2	1,015	0	1	0	0	1
	7:45 AM	70	197	69	5	388	11	17	46	45	69	70	5	992	2	0	0	0	2
	8:00 AM	39	182	32	3	321	18	7	28	33	62	34	6	765	0	0	0	0	0
	8:15 AM	47	135	29	5	313	15	7	26	43	56	31	2	709	0	0	0	0	0
	8:30 AM	43	126	11	4	269	8	5	17	41	47	19	2	592	0	0	0	0	0
	8:45 AM	52	123	20	4	220	15	5	15	38	37	24	2	555	0	0	0	0	0
	VOLUMES	499	1,268	333	34	2,300	127	101	232	327	462	311	22	6,016	2	2	0	0	4
	APPROACH %	24%	60%	16%	1%	93%	5%	15%	35%	50%	58%	39%	3%						
	APP/DEPART	2,100	/	1,393	2,461	/	3,091	660	/	597	795	/	935	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	302	777	250	17	1,290	79	68	154	162	268	219	15	3,601					
	APPROACH %	23%	58%	19%	1%	93%	6%	18%	40%	42%	53%	44%	3%						
	PEAK HR FACTOR		0.812			0.858			0.807			0.810		0.887					
	APP/DEPART	1,329	/	861	1,386	/	1,722	384	/	420	502	/	598	0					
PM	4:00 PM	77	251	66	2	149	29	17	37	51	71	72	10	832	0	1	0	0	1
	4:15 PM	66	277	53	7	174	22	14	26	45	57	92	2	835	0	1	0	0	1
	4:30 PM	98	281	52	5	173	34	17	31	39	40	91	10	871	1	0	0	0	1
	4:45 PM	79	292	54	4	155	28	19	23	42	50	100	7	853	0	0	0	0	0
	5:00 PM	93	299	59	8	169	14	13	37	50	46	91	12	891	1	0	0	0	1
	5:15 PM	99	327	52	5	207	29	25	34	50	56	69	7	960	1	0	0	0	1
	5:30 PM	107	337	65	2	178	17	14	45	52	45	82	10	954	0	1	0	0	1
	5:45 PM	111	338	63	4	166	35	17	35	47	50	83	8	957	1	0	0	0	1
	VOLUMES	730	2,402	464	37	1,371	208	136	268	376	415	680	66	7,153	4	3	0	0	7
	APPROACH %	20%	67%	13%	2%	85%	13%	17%	34%	48%	36%	59%	6%						
	APP/DEPART	3,596	/	2,607	1,616	/	2,166	780	/	766	1,161	/	1,614	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	410	1,301	239	19	720	95	69	151	199	197	325	37	3,762					
	APPROACH %	21%	67%	12%	2%	86%	11%	16%	36%	47%	35%	58%	7%						
	PEAK HR FACTOR		0.952			0.865			0.944			0.938		0.980					
	APP/DEPART	1,950	/	1,408	834	/	1,119	419	/	408	559	/	827	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	3	0	7	6	16	3	0	7	4	14	0	0	0	2	2
	7:15 AM	29	0	35	13	77	29	0	34	13	76	0	0	1	0	1
	7:30 AM	67	0	30	25	122	64	0	30	24	118	3	0	0	1	4
	7:45 AM	16	0	7	6	29	16	0	7	6	29	0	0	0	0	0
	8:00 AM	6	0	2	3	11	5	0	2	2	9	1	0	0	1	2
	8:15 AM	1	0	2	2	5	1	0	1	1	3	0	0	1	1	2
	8:30 AM	7	0	2	3	12	7	0	2	3	12	0	0	0	0	0
	8:45 AM	3	0	2	3	8	2	0	1	2	5	1	0	1	1	3
	TOTAL	132	0	87	61	280	127	0	84	55	266	5	0	3	6	14
	AM BEGIN PEAK HR			7:15 AM			114	0	73	45	232					
PM	4:00 PM	21	0	16	4	41	18	0	16	4	38	3	0	0	0	3
	4:15 PM	14	0	14	12	40	14	0	13	12	39	0	0	1	0	1
	4:30 PM	12	0	10	9	31	10	0	10	8	28	2	0	0	1	3
	4:45 PM	6	0	4	9	19	4	0	4	8	16	2	0	0	1	3
	5:00 PM	6	0	4	7	17	4	0	4	5	13	2	0	0	2	4
	5:15 PM	12	0	8	1	21	10	0	8	1	19	2	0	0	0	2
	5:30 PM	13	0	6	7	26	13	0	6	7	26	0	0	0	0	0
	5:45 PM	4	0	8	5	17	1	0	5	3	9	3	0	3	2	8
	TOTAL	88	0	70	54	212	74	0	66	48	188	14	0	4	6	24
	PM BEGIN PEAK HR			5:00 PM			28	0	23	16	67					

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE:	LOCATION:	Tustin	PROJECT #:	SC2143
Tue, May 21, 19	NORTH & SOUTH:	Red Hill	LOCATION #:	6
	EAST & WEST:	I-5 NB Ramps	CONTROL:	SIGNAL

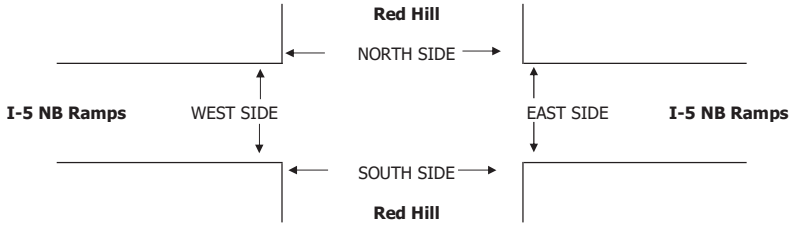
NOTES:

Queue AM NB, PM NB/SB

AM / PM / MD / OTHER / OTHER — N / W / E / S

Add U-Turns to Left Turns

		NORTHBOUND (Red Hill)			SOUTHBOUND (Red Hill)			EASTBOUND (I-5 NB Ramps)			WESTBOUND (I-5 NB Ramps)				U-TURNS				
	LANES:	NL 1	NT 3	NR X	SL X	ST 3	SR 1	EL X	ET X	ER X	WL 1.5	WT 0.5	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	47	118	0	0	193	112	0	0	0	49	0	59	578	0	0	0	0	0
	7:15 AM	71	267	0	0	266	124	0	0	0	59	0	74	861	0	0	0	0	0
	7:30 AM	73	351	0	0	315	94	0	0	0	57	0	58	948	0	0	0	0	0
	7:45 AM	91	273	0	0	359	144	0	0	0	66	0	64	997	0	0	0	0	0
	8:00 AM	88	182	0	0	310	108	0	0	0	43	0	62	793	0	0	0	0	0
	8:15 AM	63	127	0	0	290	117	0	0	0	50	0	79	726	0	0	0	0	0
	8:30 AM	65	117	0	0	260	94	0	0	0	77	1	76	690	0	0	0	0	0
	8:45 AM	49	104	0	0	221	86	0	0	0	83	1	94	638	0	0	0	0	0
	VOLUMES	547	1,539	0	0	2,214	879	0	0	0	484	2	566	6,231	0	0	0	0	0
	APPROACH %	26%	74%	0%	0%	72%	28%	0%	0%	0%	46%	0%	54%						
	APP/DEPART	2,086	/	2,105	3,093	/	2,698	0	/	0	1,052	/	1,428	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	323	1,073	0	0	1,250	470	0	0	0	225	0	258	3,599					
	APPROACH %	23%	77%	0%	0%	73%	27%	0%	0%	0%	47%	0%	53%						
	PEAK HR FACTOR		0.823			0.855			0.000			0.908		0.902					
	APP/DEPART	1,396	/	1,331	1,720	/	1,475	0	/	0	483	/	793	0					
PM	4:00 PM	75	321	0	0	209	67	0	0	0	59	11	53	795	0	0	0	0	0
	4:15 PM	101	329	0	0	214	64	0	0	0	42	11	58	819	0	0	0	0	0
	4:30 PM	68	342	0	0	192	58	0	0	0	58	19	107	844	0	0	0	0	0
	4:45 PM	85	354	0	0	198	50	0	0	0	35	25	85	832	0	0	0	0	0
	5:00 PM	69	368	0	0	219	48	0	0	0	57	21	85	867	0	0	0	0	0
	5:15 PM	73	378	0	0	253	48	0	0	0	65	30	98	945	0	0	0	0	0
	5:30 PM	60	409	0	0	244	46	0	0	0	65	25	113	962	0	0	0	0	0
	5:45 PM	67	386	0	0	213	46	0	0	0	66	19	114	911	0	0	0	0	0
	VOLUMES	598	2,887	0	0	1,742	427	0	0	0	447	161	713	6,975	0	0	0	0	0
	APPROACH %	17%	83%	0%	0%	80%	20%	0%	0%	0%	34%	12%	54%						
	APP/DEPART	3,485	/	3,600	2,169	/	2,189	0	/	0	1,321	/	1,186	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	269	1,541	0	0	929	188	0	0	0	253	95	410	3,685					
	APPROACH %	15%	85%	0%	0%	83%	17%	0%	0%	0%	33%	13%	54%						
	PEAK HR FACTOR		0.965			0.928			0.000			0.933		0.958					
	APP/DEPART	1,810	/	1,951	1,117	/	1,182	0	/	0	758	/	552	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	10	4	14	0	0	10	3	13	0	0	0	1	1
	7:15 AM	0	0	33	3	36	0	0	29	3	32	0	0	4	0	4
	7:30 AM	0	0	29	24	53	0	0	26	23	49	0	0	3	1	4
	7:45 AM	0	0	7	6	13	0	0	7	6	13	0	0	0	0	0
	8:00 AM	0	0	3	5	8	0	0	3	3	6	0	0	0	2	2
	8:15 AM	0	0	4	1	5	0	0	2	0	2	0	0	2	1	3
	8:30 AM	0	0	5	0	5	0	0	4	0	4	0	0	1	0	1
	8:45 AM	0	0	2	3	5	0	0	1	2	3	0	0	1	1	2
	TOTAL	0	0	93	46	139	0	0	82	40	122	0	0	11	6	17
	AM BEGIN PEAK HR			7:15 AM			0	0	65	35	100					
PM	4:00 PM	0	0	16	3	19	0	0	15	3	18	0	0	1	0	1
	4:15 PM	0	0	10	14	24	0	0	9	14	23	0	0	1	0	1
	4:30 PM	0	0	6	1	7	0	0	6	0	6	0	0	0	1	1
	4:45 PM	0	0	7	5	12	0	0	7	3	10	0	0	0	2	2
	5:00 PM	0	0	5	12	17	0	0	5	9	14	0	0	0	3	3
	5:15 PM	0	0	4	3	7	0	0	4	3	7	0	0	0	0	0
	5:30 PM	0	0	3	4	7	0	0	3	4	7	0	0	0	0	0
	5:45 PM	0	0	10	3	13	0	0	6	1	7	0	0	4	2	6
	TOTAL	0	0	61	45	106	0	0	55	37	92	0	0	6	8	14
	PM BEGIN PEAK HR			5:00 PM			0	0	18	17	35					

INTERSECTION TURNING MOVEMENT COUNTS

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE:					
Tue, May 21, 19	LOCATION:	Tustin	PROJECT #:	SC2143
	NORTH & SOUTH:	Red Hill	LOCATION #:	5
	EAST & WEST:	I-5 SB Ramps	CONTROL:	SIGNAL

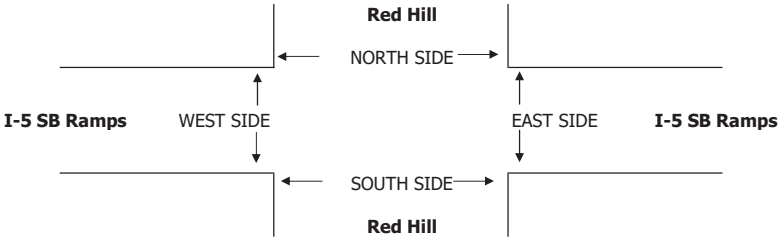
NOTES:

Queue AM WB/EB, PM EB.

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

Add U-Turns to Left Turns

		NORTHBOUND (Red Hill)			SOUTHBOUND (Red Hill)			EASTBOUND (I-5 SB Ramps)			WESTBOUND (I-5 SB Ramps)				U-TURNS				
	LANES:	NL X	NT 4	NR 1	SL 1	ST 3	SR X	EL 0.5	ET 0.5	ER 1	WL X	WT X	WR X	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	0	136	124	81	161	0	29	0	53	0	0	0	584	0	0	0	0	0
	7:15 AM	0	305	137	89	236	0	33	0	59	0	0	0	859	0	0	0	0	0
	7:30 AM	0	378	133	101	271	0	46	0	81	0	0	0	1,010	0	0	0	0	0
	7:45 AM	0	314	154	99	326	0	50	1	90	0	0	0	1,034	0	0	0	0	0
	8:00 AM	0	233	120	92	261	0	37	2	75	0	0	0	820	0	0	0	0	0
	8:15 AM	0	160	105	91	249	0	31	1	57	0	0	0	694	0	0	0	0	0
	8:30 AM	0	155	92	103	234	0	27	0	62	0	0	0	673	0	0	0	0	0
	8:45 AM	0	127	105	87	217	0	26	0	72	0	0	0	634	0	0	0	0	0
	VOLUMES	0	1,808	970	743	1,955	0	279	4	549	0	0	0	6,308	0	0	0	0	0
	APPROACH %	0%	65%	35%	28%	72%	0%	34%	0%	66%	0%	0%	0%						
	APP/DEPART	2,778	/	2,087	2,698	/	2,504	832	/	1,717	0	/	0	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	0	1,230	544	381	1,094	0	166	3	305	0	0	0	3,723					
	APPROACH %	0%	69%	31%	26%	74%	0%	35%	1%	64%	0%	0%	0%						
	PEAK HR FACTOR		0.868			0.868			0.840			0.000		0.900					
	APP/DEPART	1,774	/	1,396	1,475	/	1,399	474	/	928	0	/	0	0					
PM	4:00 PM	0	324	127	75	193	0	69	1	63	0	0	0	852	0	0	0	0	0
	4:15 PM	0	357	105	89	167	0	73	0	66	0	0	0	857	0	0	0	0	0
	4:30 PM	0	335	94	77	173	0	75	0	76	0	0	0	830	0	0	0	0	0
	4:45 PM	0	372	119	72	161	0	67	0	69	0	0	0	860	0	0	0	0	0
	5:00 PM	0	367	125	82	194	0	70	1	62	0	0	0	901	0	0	0	0	0
	5:15 PM	0	370	113	103	218	0	81	0	69	0	0	0	954	0	0	0	0	0
	5:30 PM	0	387	119	89	220	0	82	1	78	0	0	0	976	0	0	0	0	0
	5:45 PM	0	381	121	74	205	0	72	0	65	0	0	0	918	0	0	0	0	0
	VOLUMES	0	2,893	923	661	1,531	0	589	3	548	0	0	0	7,148	0	0	0	0	0
	APPROACH %	0%	76%	24%	30%	70%	0%	52%	0%	48%	0%	0%	0%						
	APP/DEPART	3,816	/	3,482	2,192	/	2,079	1,140	/	1,587	0	/	0	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	0	1,505	478	348	837	0	305	2	274	0	0	0	3,749					
	APPROACH %	0%	76%	24%	29%	71%	0%	52%	0%	47%	0%	0%	0%						
	PEAK HR FACTOR		0.980			0.923			0.902			0.000		0.960					
	APP/DEPART	1,983	/	1,810	1,185	/	1,111	581	/	828	0	/	0	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	0	17	4	21	0	0	17	3	20	0	0	0	1	1
	7:15 AM	0	0	33	3	36	0	0	30	3	33	0	0	3	0	3
	7:30 AM	0	0	26	25	51	0	0	25	24	49	0	0	1	1	2
	7:45 AM	0	0	7	4	11	0	0	7	4	11	0	0	0	0	0
	8:00 AM	0	0	1	4	5	0	0	1	3	4	0	0	0	1	1
	8:15 AM	0	0	5	2	7	0	0	3	0	3	0	0	2	2	4
	8:30 AM	0	0	5	0	5	0	0	3	0	3	0	0	2	0	2
	8:45 AM	0	0	2	3	5	0	0	0	2	2	0	0	2	1	3
	TOTAL	0	0	96	45	141	0	0	86	39	125	0	0	10	6	16
	AM BEGIN PEAK HR			7:15 AM			0	0	63	34	97					
PM	4:00 PM	0	0	14	3	17	0	0	12	3	15	0	0	2	0	2
	4:15 PM	0	0	6	9	15	0	0	5	9	14	0	0	1	0	1
	4:30 PM	0	0	7	2	9	0	0	7	1	8	0	0	0	1	1
	4:45 PM	0	0	8	6	14	0	0	8	4	12	0	0	0	2	2
	5:00 PM	0	0	5	7	12	0	0	3	5	8	0	0	2	2	4
	5:15 PM	0	0	6	6	12	0	0	5	6	11	0	0	1	0	1
	5:30 PM	0	0	3	4	7	0	0	3	4	7	0	0	0	0	0
	5:45 PM	0	0	11	3	14	0	0	7	0	7	0	0	4	3	7
	TOTAL	0	0	60	40	100	0	0	50	32	82	0	0	10	8	18
	PM BEGIN PEAK HR			5:00 PM			0	0	18	15	33					

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE:	LOCATION:	Tustin	PROJECT #:	SC2143
Tue, May 21, 19	NORTH & SOUTH:	Red Hill	LOCATION #:	4
	EAST & WEST:	Nisson	CONTROL:	SIGNAL

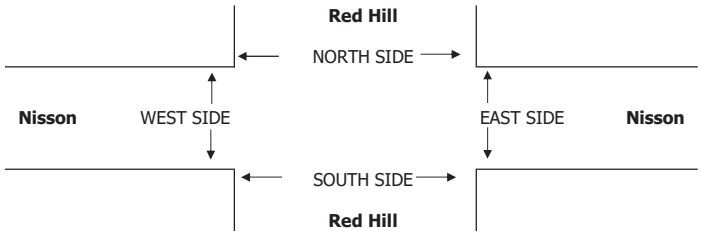
NOTES:

Queue AM/PM NB/SB

AM PM MD OTHER OTHER — ▲ N, ◄ W, E ►, S ▼

☐ Add U-Turns to Left Turns

		NORTHBOUND Red Hill			SOUTHBOUND Red Hill			EASTBOUND Nisson			WESTBOUND Nisson				U-TURNS				
	LANES:	NL 1	NT 3.5	NR 0.5	SL 1	ST 3	SR 0	EL 1	ET 1	ER 0	WL 1	WT 1	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	1	191	3	13	182	10	41	5	5	9	5	35	500	0	0	0	0	0
	7:15 AM	2	329	4	29	250	19	61	14	11	11	6	51	787	0	0	0	0	0
	7:30 AM	5	387	5	35	292	22	63	11	8	16	9	55	908	0	0	0	0	0
	7:45 AM	5	334	7	55	306	53	61	23	14	9	6	54	927	1	0	0	0	1
	8:00 AM	4	235	7	31	284	24	54	10	14	12	4	52	731	0	0	0	0	0
	8:15 AM	0	175	1	17	272	13	48	8	4	6	8	35	587	0	0	0	0	0
	8:30 AM	6	190	4	21	266	16	37	5	1	9	5	25	585	0	0	0	0	0
	8:45 AM	0	157	5	17	253	16	48	6	9	7	2	23	543	0	1	0	0	1
	VOLUMES	23	1,998	36	218	2,105	173	413	82	66	79	45	330	5,568	1	1	0	0	2
	APPROACH %	1%	97%	2%	9%	84%	7%	74%	15%	12%	17%	10%	73%						
	APP/DEPART	2,057	/	2,742	2,496	/	2,251	561	/	335	454	/	240	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	16	1,285	23	150	1,132	118	239	58	47	48	25	212	3,353					
	APPROACH %	1%	97%	2%	11%	81%	8%	69%	17%	14%	17%	9%	74%						
	PEAK HR FACTOR		0.834			0.845			0.878			0.891		0.904					
	APP/DEPART	1,324	/	1,736	1,400	/	1,228	344	/	231	285	/	158	0					
PM	4:00 PM	7	376	8	48	200	20	44	19	4	9	16	26	777	0	0	0	0	0
	4:15 PM	5	377	6	40	162	22	51	17	4	15	23	31	753	1	2	0	0	3
	4:30 PM	5	342	5	41	203	15	46	19	4	7	15	36	738	0	0	0	0	0
	4:45 PM	2	396	7	32	168	16	59	20	6	19	23	28	776	0	2	0	0	2
	5:00 PM	10	386	4	39	213	17	60	23	8	12	19	38	829	0	0	0	0	0
	5:15 PM	7	390	11	42	223	21	55	22	4	8	19	39	841	0	0	0	0	0
	5:30 PM	8	391	8	42	229	27	65	26	14	7	15	33	865	0	0	0	0	0
	5:45 PM	4	400	8	43	191	21	64	18	13	20	29	29	840	0	0	0	0	0
	VOLUMES	48	3,058	57	327	1,589	159	444	164	57	97	159	260	6,419	1	4	0	0	5
	APPROACH %	2%	97%	2%	16%	77%	8%	67%	25%	9%	19%	31%	50%						
	APP/DEPART	3,163	/	3,766	2,075	/	1,744	665	/	544	516	/	365	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	29	1,567	31	166	856	86	244	89	39	47	82	139	3,375					
	APPROACH %	2%	96%	2%	15%	77%	8%	66%	24%	10%	18%	31%	52%						
	PEAK HR FACTOR		0.987			0.930			0.886			0.859		0.975					
	APP/DEPART	1,627	/	1,950	1,108	/	942	372	/	286	268	/	197	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	0	1	6	3	10	0	1	6	2	9	0	0	0	1	1
	7:15 AM	0	5	16	1	22	0	5	13	1	19	0	0	3	0	3
	7:30 AM	0	3	18	1	22	0	3	17	1	21	0	0	1	0	1
	7:45 AM	0	7	4	5	16	0	6	2	5	13	0	1	2	0	3
	8:00 AM	0	0	0	3	3	0	0	0	2	2	0	0	0	1	1
	8:15 AM	0	2	1	2	5	0	1	1	0	2	0	1	0	2	3
	8:30 AM	1	5	3	1	10	0	4	2	1	7	1	1	1	0	3
	8:45 AM	0	2	4	2	8	0	2	4	1	7	0	0	0	1	1
	TOTAL	1	25	52	18	96	0	22	45	13	80	1	3	7	5	16
	AM BEGIN PEAK HR			7:15 AM			0	14	32	9	55					
PM	4:00 PM	0	11	10	2	23	0	11	9	2	22	0	0	1	0	1
	4:15 PM	0	7	6	9	22	0	7	5	9	21	0	0	1	0	1
	4:30 PM	0	2	4	1	7	0	2	4	0	6	0	0	0	1	1
	4:45 PM	0	13	9	4	26	0	12	9	2	23	0	1	0	2	3
	5:00 PM	0	3	5	8	16	0	3	5	5	13	0	0	0	3	3
	5:15 PM	0	6	5	6	17	0	4	5	6	15	0	2	0	0	2
	5:30 PM	0	12	2	3	17	0	12	2	3	17	0	0	0	0	0
	5:45 PM	0	22	10	2	34	0	22	6	0	28	0	0	4	2	6
	TOTAL	0	76	51	35	162	0	73	45	27	145	0	3	6	8	17
	PM BEGIN PEAK HR			5:00 PM			0	41	18	14	73					

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

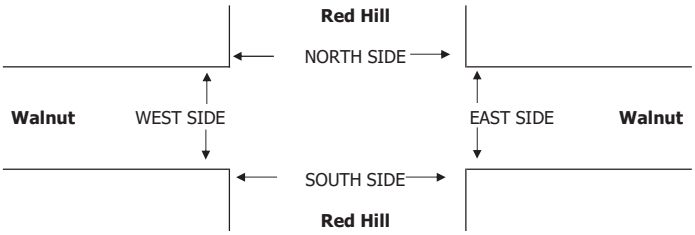
DATE:	LOCATION:	Tustin	PROJECT #:	SC2143
Tue, May 21, 19	NORTH & SOUTH:	Red Hill	LOCATION #:	2
	EAST & WEST:	Walnut	CONTROL:	SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ◄ W — N ▲ — S ▼ — E ►

☒ Add U-Turns to Left Turns

		NORTHBOUND Red Hill			SOUTHBOUND Red Hill			EASTBOUND Walnut			WESTBOUND Walnut				U-TURNS				
	LANES:	NL 1	NT 3	NR 0	SL 1	ST 3	SR 0	EL 1	ET 2	ER 0	WL 2	WT 2	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	8	89	27	18	159	13	23	75	50	31	38	15	546	4	0	0	0	4
	7:15 AM	17	112	14	24	211	22	42	92	92	38	66	30	760	2	0	0	0	2
	7:30 AM	27	139	35	23	303	24	44	99	113	57	59	36	959	2	0	0	0	2
	7:45 AM	52	156	26	30	261	38	44	127	101	48	69	33	985	4	0	0	0	4
	8:00 AM	37	112	23	25	260	31	28	76	82	47	57	20	798	6	0	0	0	6
	8:15 AM	12	69	26	25	239	26	24	89	74	43	55	24	706	2	0	0	0	2
	8:30 AM	17	110	25	30	236	17	21	83	59	40	46	24	708	4	0	0	0	4
	8:45 AM	24	89	15	19	243	13	14	76	56	34	43	24	650	2	1	0	0	3
	VOLUMES	194	876	191	194	1,912	184	240	717	627	338	433	206	6,112	26	1	0	0	27
	APPROACH %	15%	69%	15%	8%	83%	8%	15%	45%	40%	35%	44%	21%						
	APP/DEPART	1,261	/	1,323	2,290	/	2,903	1,584	/	1,101	977	/	785	0					
	BEGIN PEAK HR		7:15 AM																
	VOLUMES	133	519	98	102	1,035	115	158	394	388	190	251	119	3,502					
	APPROACH %	18%	69%	13%	8%	83%	9%	17%	42%	41%	34%	45%	21%						
	PEAK HR FACTOR		0.801			0.894			0.864			0.921		0.889					
	APP/DEPART	750	/	796	1,252	/	1,627	940	/	594	560	/	485	0					
PM	4:00 PM	71	342	26	30	133	24	38	50	30	53	210	33	1,040	3	1	0	0	4
	4:15 PM	59	305	27	25	104	27	19	76	21	54	176	38	931	2	1	0	0	3
	4:30 PM	82	326	19	30	94	34	32	68	20	42	152	43	942	5	0	0	0	5
	4:45 PM	69	338	14	31	89	29	27	74	24	51	162	47	955	5	0	0	0	5
	5:00 PM	55	375	20	22	97	41	24	70	30	53	164	39	990	3	0	0	0	3
	5:15 PM	67	311	15	39	125	37	26	68	25	53	186	59	1,011	4	0	0	0	4
	5:30 PM	87	361	30	37	96	50	28	62	28	55	164	44	1,042	0	0	0	0	0
	5:45 PM	75	334	34	36	99	46	32	66	26	41	161	34	984	3	0	0	0	3
	VOLUMES	565	2,692	185	250	837	288	226	534	204	402	1,375	337	7,895	25	2	0	0	27
	APPROACH %	16%	78%	5%	18%	61%	21%	23%	55%	21%	19%	65%	16%						
	APP/DEPART	3,442	/	3,257	1,375	/	1,468	964	/	967	2,114	/	2,203	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	284	1,381	99	134	417	174	110	266	109	202	675	176	4,027					
	APPROACH %	16%	78%	6%	18%	58%	24%	23%	55%	22%	19%	64%	17%						
	PEAK HR FACTOR		0.923			0.902			0.978			0.883		0.966					
	APP/DEPART	1,764	/	1,667	725	/	738	485	/	499	1,053	/	1,123	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	2	3	0	10	15	0	3	0	6	9	2	0	0	4	6
	7:15 AM	1	1	1	12	15	1	1	1	11	14	0	0	0	1	1
	7:30 AM	2	4	4	8	18	2	4	4	8	18	0	0	0	0	0
	7:45 AM	1	1	1	6	9	1	1	1	6	9	0	0	0	0	0
	8:00 AM	1	3	2	2	8	1	3	0	2	6	0	0	2	0	2
	8:15 AM	0	3	4	5	12	0	1	2	4	7	0	2	2	1	5
	8:30 AM	0	3	1	1	5	0	2	0	1	3	0	1	1	0	2
	8:45 AM	2	2	0	5	9	2	2	0	3	7	0	0	0	2	2
	TOTAL	9	20	13	49	91	7	17	8	41	73	2	3	5	8	18
	AM BEGIN PEAK HR			7:15 AM			5	9	6	27	47					
PM	4:00 PM	0	3	1	0	4	0	3	0	0	3	0	0	1	0	1
	4:15 PM	0	4	6	3	13	0	3	4	2	9	0	1	2	1	4
	4:30 PM	1	9	1	1	12	0	8	1	1	10	1	1	0	0	2
	4:45 PM	1	4	4	8	17	1	3	4	7	15	0	1	0	1	2
	5:00 PM	0	4	2	2	8	0	3	1	2	6	0	1	1	0	2
	5:15 PM	0	2	1	10	13	0	2	0	7	9	0	0	1	3	4
	5:30 PM	0	4	5	9	18	0	3	4	5	12	0	1	1	4	6
	5:45 PM	1	5	2	9	17	1	4	2	7	14	0	1	0	2	3
	TOTAL	3	35	22	42	102	2	29	16	31	78	1	6	6	11	24
	PM BEGIN PEAK HR			5:00 PM			1	12	7	21	41					

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Tue, Apr 16, 19

LOCATION: Tustin
NORTH & SOUTH: Red Hill
EAST & WEST: Valencia

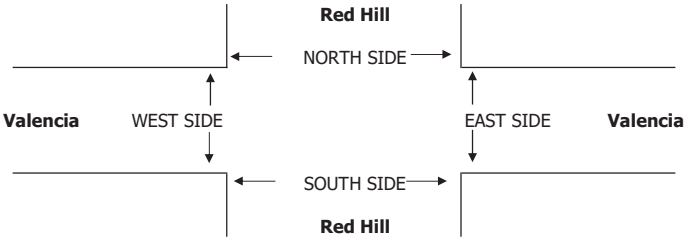
PROJECT #: SC2143
LOCATION #: 6
CONTROL: SIGNAL

NOTES:

AM PM MD OTHER OTHER — ▲ N ◄ W E ► S ▼

Add U-Turns to Left Turns

		NORTHBOUND Red Hill			SOUTHBOUND Red Hill			EASTBOUND Valencia			WESTBOUND Valencia				U-TURNS				
	LANES:	NL 1	NT 4	NR 0	SL 1	ST 3	SR 0	EL 1	ET 1	ER 1	WL 2	WT 1	WR 1	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	7	50	14	12	169	16	8	14	15	38	72	10	425	0	0	0	0	0
	7:15 AM	14	68	10	18	224	12	1	18	13	28	53	14	473	1	0	0	0	1
	7:30 AM	17	76	28	29	222	16	4	28	18	48	78	21	585	0	0	0	0	0
	7:45 AM	20	86	30	46	262	14	2	46	17	71	95	49	738	0	1	0	1	2
	8:00 AM	17	78	22	20	286	14	6	34	14	64	90	32	677	1	0	0	0	1
	8:15 AM	22	70	26	36	239	19	5	28	11	63	76	18	613	0	1	0	1	2
	8:30 AM	17	87	25	16	234	21	6	29	11	51	81	21	599	1	0	0	0	1
	8:45 AM	26	56	24	17	259	24	4	23	14	56	59	19	581	0	0	0	0	0
	VOLUMES	140	571	179	194	1,895	136	36	220	113	419	604	184	4,691	3	2	0	2	7
	APPROACH %	16%	64%	20%	9%	85%	6%	10%	60%	31%	35%	50%	15%						
	APP/DEPART	890	/	793	2,225	/	2,428	369	/	593	1,207	/	877	0					
	BEGIN PEAK HR		7:45 AM																
	VOLUMES	76	321	103	118	1,021	68	19	137	53	249	342	120	2,627					
	APPROACH %	15%	64%	21%	10%	85%	6%	9%	66%	25%	35%	48%	17%						
	PEAK HR FACTOR		0.919			0.937			0.804			0.827		0.890					
	APP/DEPART	500	/	462	1,207	/	1,323	209	/	358	711	/	484	0					
PM	4:00 PM	47	341	54	17	106	16	11	44	13	33	40	25	747	1	0	0	0	1
	4:15 PM	83	378	36	18	106	18	12	31	10	20	50	32	794	0	0	0	0	0
	4:30 PM	51	384	44	39	99	18	23	42	28	35	49	32	844	0	1	0	0	1
	4:45 PM	35	365	66	29	122	17	16	39	19	20	42	35	805	0	1	0	0	1
	5:00 PM	52	425	69	24	105	7	19	78	18	28	36	28	889	0	0	0	0	0
	5:15 PM	79	446	88	15	107	6	11	28	17	33	42	16	888	1	0	0	0	1
	5:30 PM	56	370	83	17	117	19	13	31	19	32	41	17	815	0	0	0	0	0
	5:45 PM	73	407	68	30	108	8	9	27	9	18	53	19	829	1	0	0	0	1
	VOLUMES	476	3,116	508	189	870	109	114	320	133	219	353	204	6,611	3	2	0	0	5
	APPROACH %	12%	76%	12%	16%	74%	9%	20%	56%	23%	28%	45%	26%						
	APP/DEPART	4,100	/	3,436	1,168	/	1,225	567	/	1,015	776	/	935	0					
	BEGIN PEAK HR		4:30 PM																
	VOLUMES	217	1,620	267	107	433	48	69	187	82	116	169	111	3,426					
	APPROACH %	10%	77%	13%	18%	74%	8%	20%	55%	24%	29%	43%	28%						
	PEAK HR FACTOR		0.858			0.875			0.735			0.853		0.963					
	APP/DEPART	2,104	/	1,802	588	/	632	338	/	559	396	/	433	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	1	0	1	0	2	1	0	0	0	1	0	0	1	0	1
	7:15 AM	0	0	0	1	1	0	0	0	1	1	0	0	0	0	0
	7:30 AM	0	1	0	1	2	0	1	0	0	1	0	0	0	1	1
	7:45 AM	0	1	0	1	2	0	0	0	1	1	0	1	0	0	1
	8:00 AM	0	0	1	0	1	0	0	1	0	1	0	0	0	0	0
	8:15 AM	1	0	0	2	3	0	0	0	2	2	1	0	0	0	1
	8:30 AM	0	0	1	0	1	0	0	1	0	1	0	0	0	0	0
	8:45 AM	0	2	0	2	4	0	1	0	2	3	0	1	0	0	1
	TOTAL	2	4	3	7	16	1	2	2	6	11	1	2	1	1	5
	AM BEGIN PEAK HR			7:45 AM			0	0	2	3	5					
PM	4:00 PM	2	0	3	0	5	1	0	0	0	1	1	0	3	0	4
	4:15 PM	0	0	0	2	2	0	0	0	2	2	0	0	0	0	0
	4:30 PM	1	0	2	1	4	0	0	0	0	0	1	0	2	1	4
	4:45 PM	0	1	0	0	1	0	0	0	0	0	0	1	0	0	1
	5:00 PM	1	1	0	1	3	0	1	0	0	1	1	0	0	1	2
	5:15 PM	0	0	1	1	2	0	0	1	0	1	0	0	0	1	1
	5:30 PM	0	0	2	1	3	0	0	2	1	3	0	0	0	0	0
	5:45 PM	0	2	0	2	4	0	0	0	1	1	0	2	0	1	3
	TOTAL	4	4	8	8	24	1	1	3	4	9	3	3	5	4	15
	PM BEGIN PEAK HR			4:30 PM			0	1	1	0	2					

INTERSECTION TURNING MOVEMENT COUNTS

T218

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: Thu, May 23, 19

LOCATION: Tustin
NORTH & SOUTH: Tustin Ranch
EAST & WEST: Warner North

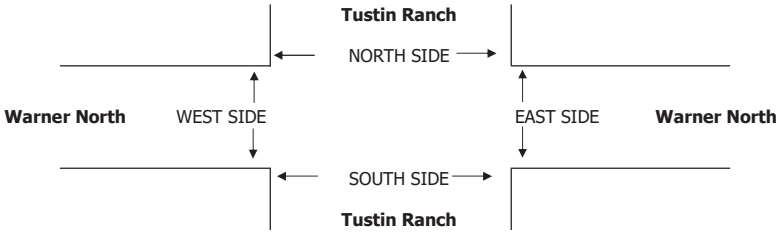
PROJECT #: SC2143
LOCATION #: 10
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N, ◄ W, E ►, S ▼

☐ Add U-Turns to Left Turns

		NORTHBOUND Tustin Ranch			SOUTHBOUND Tustin Ranch			EASTBOUND Warner North			WESTBOUND Warner North				U-TURNS				
	LANES:	NL X	NT 3	NR 1	SL 2	ST 3	SR X	EL X	ET X	ER X	WL 3	WT X	WR 2	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL
AM	7:00 AM	0	51	40	75	177	0	0	0	0	53	0	59	455	0	0	0	0	0
	7:15 AM	0	52	46	65	226	0	0	0	0	73	0	40	502	0	0	0	0	0
	7:30 AM	0	70	59	58	284	0	0	0	0	97	0	30	598	0	0	0	0	0
	7:45 AM	0	71	66	134	342	0	0	0	0	163	0	44	820	0	0	0	0	0
	8:00 AM	0	67	62	118	322	0	0	0	0	138	0	30	737	0	0	0	0	0
	8:15 AM	0	82	65	100	327	0	0	0	0	148	0	51	773	0	0	0	0	0
	8:30 AM	0	49	67	99	285	0	0	0	0	136	0	47	683	0	0	0	0	0
	8:45 AM	0	62	55	90	271	0	0	0	0	143	0	46	667	0	0	0	0	0
	VOLUMES	0	504	460	739	2,234	0	0	0	0	951	0	347	5,235	0	0	0	0	0
	APPROACH %	0%	52%	48%	25%	75%	0%	0%	0%	0%	73%	0%	27%						
	APP/DEPART	964	/	851	2,973	/	3,185	0	/	1,199	1,298	/	0	0					
	BEGIN PEAK HR		7:45 AM																
	VOLUMES	0	269	260	451	1,276	0	0	0	0	585	0	172	3,013					
	APPROACH %	0%	51%	49%	26%	74%	0%	0%	0%	0%	77%	0%	23%						
	PEAK HR FACTOR		0.900			0.907			0.000			0.914		0.919					
	APP/DEPART	529	/	441	1,727	/	1,861	0	/	711	757	/	0	0					
PM	4:00 PM	0	342	151	73	104	0	0	0	0	86	0	99	855	0	0	0	0	0
	4:15 PM	0	337	174	74	107	0	0	0	0	98	0	95	885	1	0	0	0	1
	4:30 PM	0	369	169	83	102	0	0	0	0	98	0	105	926	2	0	0	0	2
	4:45 PM	0	355	170	73	115	0	0	0	0	98	0	113	924	0	0	0	0	0
	5:00 PM	0	429	228	82	126	0	0	0	0	112	0	120	1,097	0	0	0	0	0
	5:15 PM	0	346	222	97	136	0	0	0	0	108	0	111	1,020	0	0	0	0	0
	5:30 PM	0	407	248	87	121	0	0	0	0	142	0	96	1,101	0	0	0	0	0
	5:45 PM	0	424	248	81	127	0	0	0	0	112	0	101	1,093	0	0	0	0	0
	VOLUMES	0	3,009	1,610	650	938	0	0	0	0	854	0	840	7,904	3	0	0	0	3
	APPROACH %	0%	65%	35%	41%	59%	0%	0%	0%	0%	50%	0%	50%						
	APP/DEPART	4,622	/	3,849	1,588	/	1,795	0	/	2,260	1,694	/	0	0					
	BEGIN PEAK HR		5:00 PM																
	VOLUMES	0	1,606	946	347	510	0	0	0	0	474	0	428	4,311					
	APPROACH %	0%	63%	37%	40%	60%	0%	0%	0%	0%	53%	0%	47%						
	PEAK HR FACTOR		0.949			0.920			0.000			0.947		0.979					
	APP/DEPART	2,552	/	2,034	857	/	984	0	/	1,293	902	/	0	0					



		PEDESTRIAN + BIKE CROSSINGS					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	7:00 AM	1	2	2	2	7	0	0	1	0	1	1	2	1	2	6
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	1	0	0	1	2	0	0	0	0	0	1	0	0	1	2
	8:00 AM	0	0	0	1	1	0	0	0	0	0	0	0	0	1	1
	8:15 AM	0	1	2	0	3	0	0	2	0	2	0	1	0	0	1
	8:30 AM	0	1	1	1	3	0	0	1	0	1	0	1	0	1	2
	8:45 AM	0	0	1	0	1	0	0	1	0	1	0	0	0	0	0
	TOTAL	2	4	6	5	17	0	0	5	0	5	2	4	1	5	12
	AM BEGIN PEAK HR			7:45 AM			0	0	3	0	3					
PM	4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:15 PM	1	1	0	0	2	0	1	0	0	1	1	0	0	0	1
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	1	0	1	0	0	1	0	1	0	0	0	0	0
	5:15 PM	0	1	1	0	2	0	0	0	0	0	0	1	1	0	2
	5:30 PM	0	0	1	0	1	0	0	0	0	0	0	0	1	0	1
	5:45 PM	1	1	1	2	5	0	0	0	0	0	1	1	1	2	5
	TOTAL	2	3	4	2	11	0	1	1	0	2	2	2	3	2	9
	PM BEGIN PEAK HR			5:00 PM			0	0	1	0	1					

National Data & Surveying Services

Intersection Turning Movement Count

Location: Walnut Ave & Tustin Ranch Rd
City: Tustin
Control: Signalized

Project ID: 19-01171-001
Date: 8/27/2019

Total

NS/EW Streets:	Walnut Ave				Walnut Ave				Tustin Ranch Rd				Tustin Ranch Rd				
AM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
	2	2	1	0	2	2	0	0	2	3	1	0	2	3	1	0	
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
7:00 AM	14	26	41	0	78	87	55	0	19	91	39	0	151	239	35	0	875
7:15 AM	16	30	35	0	133	123	46	0	8	93	27	0	116	266	40	0	933
7:30 AM	31	52	15	0	125	122	94	0	17	94	43	0	133	383	63	0	1172
7:45 AM	31	33	36	0	115	133	100	0	15	125	72	0	103	383	93	0	1239
8:00 AM	35	40	35	0	71	110	67	0	15	99	56	0	117	392	44	0	1081
8:15 AM	47	53	47	0	58	97	68	0	15	78	43	0	130	447	43	1	1127
8:30 AM	24	41	34	0	57	115	59	0	15	83	42	0	131	437	43	0	1081
8:45 AM	30	62	42	0	58	120	59	0	17	85	51	0	152	345	24	0	1045
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
TOTAL VOLUMES :	228	337	285	0	695	907	548	0	121	748	373	0	1033	2892	385	1	8553
APPROACH %'s :	26.82%	39.65%	33.53%	0.00%	32.33%	42.19%	25.49%	0.00%	9.74%	60.23%	30.03%	0.00%	23.96%	67.08%	8.93%	0.02%	
PEAK HR :	07:30 AM - 08:30 AM																TOTAL
PEAK HR VOL :	144	178	133	0	369	462	329	0	62	396	214	0	483	1605	243	1	4619
PEAK HR FACTOR :	0.766	0.840	0.707	0.000	0.738	0.868	0.823	0.000	0.912	0.792	0.743	0.000	0.908	0.898	0.653	0.250	0.932
	0.774				0.833				0.792				0.939				

NS/EW Streets:	Walnut Ave				Walnut Ave				Tustin Ranch Rd				Tustin Ranch Rd				
PM	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND				
	2	2	1	0	2	2	0	0	2	3	1	0	2	3	1	0	
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
4:00 PM	33	234	203	0	33	51	27	0	70	326	42	0	47	138	78	0	1282
4:15 PM	37	154	128	0	45	45	28	0	107	381	46	2	63	159	98	1	1294
4:30 PM	31	185	178	0	48	59	26	1	79	429	41	1	49	147	88	2	1364
4:45 PM	27	192	144	0	46	56	26	0	86	442	28	0	46	150	90	0	1333
5:00 PM	42	204	174	1	50	55	35	0	105	439	44	1	61	157	111	0	1479
5:15 PM	44	224	169	0	47	69	34	0	104	424	41	2	51	143	101	1	1454
5:30 PM	37	222	166	0	39	48	35	0	104	448	44	1	60	151	113	0	1468
5:45 PM	32	214	129	0	52	59	41	0	92	397	59	0	68	149	95	0	1387
	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
TOTAL VOLUMES :	283	1629	1291	1	360	442	252	1	747	3286	345	7	445	1194	774	4	11061
APPROACH %'s :	8.83%	50.84%	40.29%	0.03%	34.12%	41.90%	23.89%	0.09%	17.04%	74.94%	7.87%	0.16%	18.41%	49.40%	32.02%	0.17%	
PEAK HR :	05:00 PM - 06:00 PM																TOTAL
PEAK HR VOL :	155	864	638	1	188	231	145	0	405	1708	188	4	240	600	420	1	5788
PEAK HR FACTOR :	0.881	0.964	0.917	0.250	0.904	0.837	0.884	0.000	0.964	0.953	0.797	0.500	0.882	0.955	0.929	0.250	0.978
	0.949				0.928				0.965				0.958				

INTERSECTION TURNING MOVEMENT COUNTS AVERAGE SHEET

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: AVERAGE FOR THREE DAYS

LOCATION: Santa Ana
NORTH & SOUTH: SB SR-55 Ramps
EAST & WEST: Irvine

PROJECT #: CMP2019
LOCATION #: 194
CONTROL: SIGNAL

NOTES:

AM PM MD OTHER OTHER — ▲ N ◄ W E ► S ▼

Add U-Turns to Left Turns

		NORTHBOUND (SB SR-55 Ramps)			SOUTHBOUND (SB SR-55 Ramps)			EASTBOUND (Irvine)			WESTBOUND (Irvine)				U-TURNS					Right-turn Overlap			
	LANES:	NL X	NT X	NR X	SL 1.5	ST X	SR 1.5	EL X	ET 3	ER 0	WL 1	WT 3	WR X	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL	NRR X	SRR X	ERR X	WRR X
AM	6:00 AM	0	0	0	38	0	37	0	70	45	38	41	0	269	0	0	0	0	0	0	0	0	0
	6:15 AM	0	0	0	43	0	37	0	76	70	56	64	0	346	0	0	0	0	0	0	0	0	0
	6:30 AM	0	0	0	29	0	28	0	86	123	83	85	0	434	0	0	0	0	0	0	0	0	0
	6:45 AM	0	0	0	17	0	20	0	96	171	94	119	0	517	0	0	0	0	0	0	0	0	0
	7:00 AM	0	0	0	12	0	21	0	97	215	116	131	0	592	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	14	0	10	0	115	247	125	156	0	667	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	10	0	15	0	136	259	117	207	0	744	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	11	0	13	0	153	273	110	261	0	821	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	12	0	13	0	148	256	101	254	0	784	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	18	0	17	0	138	259	105	277	0	814	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	20	0	24	0	130	233	104	260	0	771	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	27	0	41	0	116	199	98	262	0	743	0	0	0	0	0	0	0	0	0
	VOLUMES	0	0	0	251	0	276	0	1,361	2,350	1,147	2,117	0	7,502	0	0	0	0	0	0	0	0	0
	APPROACH %	0%	0%	0%	48%	0%	52%	0%	37%	63%	35%	65%	0%										
	APP/DEPART	0	/	0	527	/	3,497	3,711	/	1,612	3,264	/	2,393										
	BEGIN PEAK HR		7:45 AM																				
	VOLUMES	0	0	0	61	0	67	0	569	1,021	420	1,052	0	3,190						0	0	0	0
	APPROACH %	0%	0%	0%	48%	0%	52%	0%	36%	64%	29%	71%	0%										
	PEAK HR FACTOR		0.000			0.727			0.933			0.963		0.971									
	APP/DEPART	0	/	0	128	/	1,441	1,590	/	630	1,472	/	1,119										
PM	03:00 PM	0	0	0	88	0	84	0	185	101	77	229	0	764	0	0	0	0	0	0	0	0	0
	3:15 PM	0	0	0	80	0	81	0	183	105	82	233	0	764	0	0	0	0	0	0	0	0	0
	3:30 PM	0	0	0	93	0	80	0	176	111	72	266	0	798	0	0	0	0	0	0	0	0	0
	3:45 PM	0	0	0	94	0	84	0	183	108	71	269	0	809	0	0	0	0	0	0	0	0	0
	4:00 PM	0	0	0	89	0	76	0	185	120	81	249	0	800	0	0	0	0	0	0	0	0	0
	4:15 PM	0	0	0	98	0	84	0	187	107	89	284	0	849	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	97	0	90	0	192	125	88	271	0	863	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	99	0	76	0	214	142	81	259	0	871	0	0	0	0	0	0	0	0	0
	5:00 PM	0	0	0	116	0	58	0	219	168	103	255	0	919	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	106	0	63	0	221	130	89	261	0	870	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	108	0	59	0	199	135	95	283	0	879	0	0	0	0	0	0	0	0	0
	5:45 PM	0	0	0	130	0	64	0	175	116	83	281	0	849	0	0	0	0	0	0	0	0	0
	6:00 PM	0	0	0	110	1	62	0	172	115	89	272	0	821	0	0	0	0	0	0	0	0	0
	6:15 PM	0	0	0	107	0	63	0	141	93	79	250	0	733	0	0	0	0	0	0	0	0	0
	6:30 PM	0	0	0	99	1	59	0	136	87	74	220	0	676	0	0	0	0	0	0	0	0	0
	6:45 PM	0	0	0	74	1	58	0	138	73	74	222	0	640	0	0	0	0	0	0	0	0	0
	VOLUMES	0	0	0	1,588	3	1,141	0	2,906	1,836	1,327	4,104	0	12,905	0	0	0	0	0	0	0	0	0
	APPROACH %	0%	0%	0%	58%	0%	42%	0%	61%	39%	24%	76%	0%										
	APP/DEPART	0	/	0	2,732	/	3,166	4,742	/	4,494	5,431	/	5,245										
	BEGIN PEAK HR		4:45 PM																				
	VOLUMES	0	0	0	429	0	256	0	853	575	368	1,058	0	3,539						0	0	0	0
	APPROACH %	0%	0%	0%	63%	0%	37%	0%	60%	40%	26%	74%	0%										
	PEAK HR FACTOR		0.000			0.979			0.922			0.943		0.963									
	APP/DEPART	0	/	0	685	/	943	1,428	/	1,282	1,426	/	1,314										

SB SR-55 Ramps

NORTH SIDE

Irvine WEST SIDE EAST SIDE **Irvine**

SOUTH SIDE

SB SR-55 Ramps

		ALL PED AND BIKE					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	6:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	6:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	6:30 AM	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0
	6:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:00 AM	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0
	7:15 AM	0	1	0	0	1	0	1	0	0	1	0	0	0	0	0
	7:30 AM	3	0	0	0	3	2	0	0	0	2	1	0	0	0	1
	7:45 AM	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0
	8:00 AM	2	1	0	0	3	2	0	0	0	2	0	1	0	0	1
	8:15 AM	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0
	8:30 AM	1	1	0	0	2	1	1	0	0	2	0	0	0	0	0
	8:45 AM	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0
	TOTAL	11	3	0	0	14	10	2	0	0	12	1	1	0	0	2
	AM BEGIN PEAK HR			7:45 AM			5	1	0	0	6	0	1	0	0	1
PM	3:00 PM	1	1	0	0	2	1	1	0	0	2	0	0	0	0	0
	3:15 PM	2	0	0	0	2	0	0	0	0	0	2	0	0	0	2
	3:30 PM	0	2	0	0	2	0	1	0	0	1	0	1	0	0	1
	3:45 PM	1	2	0	0	3	1	2	0	0	3	0	0	0	0	0
	4:00 PM	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	2	0	0	0	2	1	0	0	0	1	1	0	0	0	1
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	1	1	0	0	2	1	1	0	0	2	0	0	0	0	0
	5:15 PM	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0
	6:00 PM	0	1	0	0	1	0	1	0	0	1	0	0	0	0	0
	6:15 PM	0	1	0	0	1	0	0	0	0	0	0	1	0	0	1
	6:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	6:45 PM	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0
	TOTAL	11	8	0	0	19	8	6	0	0	14	3	2	0	0	5
	PM BEGIN PEAK HR			4:45 PM			2	1	0	0	3	0	0	0	0	0

INTERSECTION TURNING MOVEMENT COUNTS AVERAGE SHEET

T319

PREPARED BY: AimTD LLC. tel: 714 253 7888 cs@aimtd.com

DATE: AVERAGE FOR THREE DAYS

LOCATION: Tustin
NORTH & SOUTH: NB SR-55 Ramps
EAST & WEST: Irvine

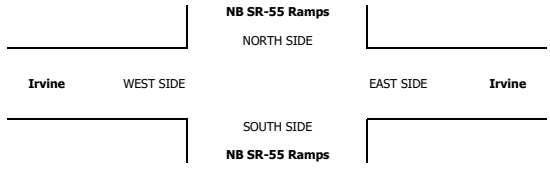
PROJECT #: CMP2019
LOCATION #: 232
CONTROL: SIGNAL

NOTES:

AM / PM / MD / OTHER / OTHER — ▲ N ◄ W E ► S ▼

Add U-Turns to Left Turns

		NORTHBOUND (NB SR-55 Ramps)			SOUTHBOUND (NB SR-55 Ramps)			EASTBOUND (Irvine)			WESTBOUND (Irvine)				U-TURNS					Right-turn Overlap			
	LANES:	NL 1.5	NT X	NR 1.5	SL X	ST X	SR X	EL 1	ET 3	ER X	WL X	WT 3	WR 0	TOTAL	NB 0	SB 0	EB 0	WB 0	TTL	NRR X	SRR X	ERR X	WRR X
AM	6:00 AM	23	0	25	0	0	0	32	75	0	0	55	44	254	0	0	0	0	0	0	0	0	0
	6:15 AM	39	0	18	0	0	0	34	82	0	0	80	54	307	0	0	0	0	0	0	0	0	0
	6:30 AM	45	0	30	0	0	0	38	76	0	0	123	64	376	0	0	0	0	0	0	0	0	0
	6:45 AM	65	0	59	0	0	0	37	75	0	0	149	92	477	0	0	0	0	0	0	0	0	0
	7:00 AM	56	0	48	0	0	0	38	70	0	0	191	113	516	0	0	0	0	0	0	0	0	0
	7:15 AM	59	0	72	0	0	0	48	80	0	0	223	126	608	0	0	0	0	0	0	0	0	0
	7:30 AM	65	0	73	0	0	0	38	108	0	0	259	177	720	0	0	0	0	0	0	0	0	0
	7:45 AM	93	0	110	0	0	0	51	113	0	0	278	175	820	0	0	0	0	0	0	0	0	0
	8:00 AM	89	0	87	0	0	0	48	110	0	0	267	195	796	0	0	1	0	1	0	0	0	0
	8:15 AM	105	0	93	0	0	0	48	106	0	0	277	185	814	0	0	1	0	1	0	0	0	0
	8:30 AM	118	0	100	0	0	0	38	110	0	0	246	187	799	0	0	1	0	1	0	0	0	0
	8:45 AM	122	0	110	0	0	0	41	100	0	0	238	145	756	0	0	1	0	1	0	0	0	0
	VOLUMES	879	0	825	0	0	0	491	1,105	0	0	2,386	1,557	7,247	0	0	4	0	4	0	0	0	0
	APPROACH %	52%	0%	48%	0%	0%	0%	31%	69%	0%	0%	61%	39%										
	APP/DEPART	1,704	/	2,048	0	/	0	1,600	/	1,930	3,943	/	3,269										
	BEGIN PEAK HR		7:45 AM																				
	VOLUMES	405	0	390	0	0	0	185	439	0	0	1,068	742	3,232						0	0	0	0
	APPROACH %	51%	0%	49%	0%	0%	0%	30%	70%	0%	0%	59%	41%										
	PEAK HR FACTOR		0.912			0.000			0.956			0.979		0.985									
	APP/DEPART	795	/	927	0	/	0	627	/	829	1,810	/	1,476										
PM	03:00 PM	65	2	80	0	0	0	84	186	0	0	238	176	831	0	0	4	0	4	0	0	0	0
	3:15 PM	66	1	61	0	0	0	65	194	0	0	245	193	825	0	0	4	0	4	0	0	0	0
	3:30 PM	67	1	56	0	0	0	74	192	0	0	268	224	882	0	0	3	0	3	0	0	0	0
	3:45 PM	58	1	63	0	0	0	76	198	0	0	282	191	869	0	0	2	0	2	0	0	0	0
	4:00 PM	65	2	61	0	0	0	78	191	0	0	261	188	846	0	0	3	0	3	0	0	0	0
	4:15 PM	77	1	49	0	0	0	73	209	0	0	294	198	901	0	0	2	0	2	0	0	0	0
	4:30 PM	60	2	55	0	0	0	71	215	0	0	297	179	879	0	0	3	0	3	0	0	0	0
	4:45 PM	63	3	66	0	0	0	70	240	0	0	274	173	889	0	0	3	0	3	0	0	0	0
	5:00 PM	60	5	69	0	0	0	67	263	0	0	293	189	946	0	0	4	0	4	0	0	0	0
	5:15 PM	77	2	66	0	0	0	60	266	0	0	273	176	920	0	0	1	0	1	0	0	0	0
	5:30 PM	102	4	82	0	0	0	50	255	0	0	275	181	949	0	0	2	0	2	0	0	0	0
	5:45 PM	85	4	78	0	0	0	49	253	0	0	279	162	910	0	0	2	0	2	0	0	0	0
	6:00 PM	83	3	72	0	0	0	57	223	0	0	277	169	884	0	0	2	0	2	0	0	0	0
	6:15 PM	72	4	59	0	0	0	53	194	0	0	256	171	809	0	0	2	0	2	0	0	0	0
	6:30 PM	72	1	65	0	0	0	58	175	0	0	221	172	764	0	0	1	0	1	0	0	0	0
	6:45 PM	77	1	74	0	0	0	59	151	0	0	218	168	748	0	0	2	0	2	0	0	0	0
	VOLUMES	1,149	37	1,056	0	0	0	1,044	3,405	0	0	4,251	2,910	13,892	0	0	40	0	40	0	0	0	0
	APPROACH %	51%	2%	47%	0%	0%	0%	23%	76%	0%	0%	59%	41%										
	APP/DEPART	2,242	/	3,991	0	/	0	4,489	/	4,461	7,161	/	5,440										
	BEGIN PEAK HR		5:00 PM																				
	VOLUMES	324	15	295	0	0	0	226	1,037	0	0	1,120	708	3,734						0	0	0	0
	APPROACH %	51%	2%	47%	0%	0%	0%	18%	82%	0%	0%	61%	39%										
	PEAK HR FACTOR		0.843			0.000			0.952			0.948		0.982									
	APP/DEPART	634	/	949	0	/	0	1,272	/	1,332	1,828	/	1,453										



		ALL PED AND BIKE					PEDESTRIAN CROSSINGS					BICYCLE CROSSINGS				
		N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	N SIDE	S SIDE	E SIDE	W SIDE	TOTAL	NS	SS	ES	WS	TOTAL
AM	6:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	6:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	6:30 AM	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0
	6:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:00 AM	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0
	7:15 AM	0	1	0	0	1	0	1	0	0	1	0	0	0	0	0
	7:30 AM	3	0	0	0	3	2	0	0	0	2	1	0	0	0	1
	7:45 AM	2	1	0	0	3	1	1	0	0	2	1	0	0	0	1
	8:00 AM	1	1	0	0	2	1	0	0	0	1	0	1	0	0	1
	8:15 AM	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0
	8:30 AM	1	1	0	0	2	1	1	0	0	2	0	0	0	0	0
	8:45 AM	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0
	TOTAL	11	4	0	0	15	9	3	0	0	12	2	1	0	0	3
	AM BEGIN PEAK HR			7:45 AM			4	2	0	0	6	1	1	0	0	2
PM	3:00 PM	2	1	0	0	3	2	1	0	0	3	0	0	0	0	0
	3:15 PM	2	0	0	0	2	1	0	0	0	1	1	0	0	0	1
	3:30 PM	1	2	0	0	3	0	1	0	0	1	1	1	0	0	2
	3:45 PM	1	2	0	0	3	1	2	0	0	3	0	0	0	0	0
	4:00 PM	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0
	4:15 PM	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0
	4:30 PM	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:00 PM	0	2	0	0	2	0	1	0	0	1	0	1	0	0	1
	5:15 PM	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:45 PM	1	0	0	0	1	1	0	0	0	1	0	0	0	0	0
	6:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	6:15 PM	0	1	0	0	1	0	0	0	0	0	0	1	0	0	1
	6:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	6:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	TOTAL	11	8	0	0	19	9	5	0	0	14	2	3	0	0	5
	PM BEGIN PEAK HR			5:00 PM			2	1	0	0	3	0	1	0	0	1

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : FIRST STREET — Site: SANTA ANA

Segment : WEST CITY LIMIT TO EUCLID ST — Date: 09/11/19

Client : CITY SANTA ANA — Day: Wednesday

Interval	WB				EB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	36	94	196	738	30	110	167	710	66	204	363	1,448
12:15	18		198		22		170		40		368	
12:30	20		178		28		187		48		365	
12:45	20		166		30		186		50		352	
01:00	18	62	170	710	12	41	163	702	30	103	333	1,412
01:15	17		172		10		172		27		344	
01:30	17		180		10		172		27		352	
01:45	10		188		9		195		19		383	
02:00	17	48	148	771	8	20	194	781	25	68	342	1,552
02:15	8		197		4		188		12		385	
02:30	10		212		3		176		13		388	
02:45	13		214		5		223		18		437	
03:00	8	38	224	860	8	53	210	848	16	91	434	1,708
03:15	8		208		12		220		20		428	
03:30	10		226		17		218		27		444	
03:45	12		202		16		200		28		402	
04:00	14	101	208	1,014	9	85	190	798	23	186	398	1,812
04:15	20		243		17		196		37		439	
04:30	36		280		25		196		61		476	
04:45	31		283		34		216		65		499	
05:00	26	184	276	1,178	47	194	260	950	73	378	536	2,128
05:15	37		303		31		252		68		555	
05:30	71		320		51		226		122		546	
05:45	50		279		65		212		115		491	
06:00	55	331	278	978	68	397	252	844	123	728	530	1,822
06:15	84		252		89		211		173		463	
06:30	78		246		98		197		176		443	
06:45	114		202		142		184		256		386	
07:00	118	685	214	743	182	865	170	576	300	1,550	384	1,319
07:15	136		151		204		138		340		289	
07:30	197		192		237		134		434		326	
07:45	234		186		242		134		476		320	
08:00	256	806	142	514	255	985	134	437	511	1,791	276	951
08:15	170		174		269		109		439		283	
08:30	182		102		233		92		415		194	
08:45	198		96		228		102		426		198	
09:00	175	737	110	414	152	641	101	351	327	1,378	211	765
09:15	188		108		188		100		376		208	
09:30	203		88		158		84		361		172	
09:45	171		108		143		66		314		174	
10:00	176	730	77	272	160	559	74	260	336	1,289	151	532
10:15	176		76		123		71		299		147	
10:30	201		53		138		56		339		109	
10:45	177		66		138		59		315		125	
11:00	176	709	51	203	178	696	58	160	354	1,405	109	363
11:15	146		50		168		33		314		83	
11:30	172		64		182		44		354		108	
11:45	215		38		168		25		383		63	
Totals	4,525		8,395		4,646		7,417		9,171		15,812	
Split%	49.3		53.1		50.7		46.9					
Day Totals		12,920				12,063				24,983		
Day Splits		51.7				48.3						
Peak Hour	07:30		04:45		07:30		04:45		07:30		04:45	
Volume	857		1,182		1,003		954		1,860		2,136	
Factor	0.84		0.92		0.93		0.92		0.91		0.96	

* Data File : D1909085

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : EUCLID STREET — Site: SANTA ANA

Segment : NCFADDEN AVE TO FIRST ST — Date: 10/09/19

Client : CITY SANTA ANA — Day: Wednesday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	58	202	246	964	44	131	224	983	102	333	470	1,947
12:15	52		242		34		258		86		500	
12:30	49		230		36		264		85		494	
12:45	43		246		17		237		60		483	
01:00	56	121	210	968	14	76	226	1,049	70	197	436	2,017
01:15	28		222		18		259		46		481	
01:30	18		266		26		280		44		546	
01:45	19		270		18		284		37		554	
02:00	20	72	316	1,292	12	68	266	1,194	32	140	582	2,486
02:15	15		288		23		284		38		572	
02:30	19		340		17		322		36		662	
02:45	18		348		16		322		34		670	
03:00	12	72	369	1,577	15	129	284	1,135	27	201	653	2,712
03:15	22		377		27		277		49		654	
03:30	20		424		40		322		60		746	
03:45	18		407		47		252		65		659	
04:00	26	126	396	1,693	46	347	302	1,177	72	473	698	2,870
04:15	30		418		60		278		90		696	
04:30	26		451		113		296		139		747	
04:45	44		428		128		301		172		729	
05:00	49	305	434	1,783	114	724	328	1,282	163	1,029	762	3,065
05:15	74		457		154		318		228		775	
05:30	82		446		198		312		280		758	
05:45	100		446		258		324		358		770	
06:00	88	576	411	1,520	197	1,327	337	1,167	285	1,903	748	2,687
06:15	123		393		288		294		411		687	
06:30	179		360		363		274		542		634	
06:45	186		356		479		262		665		618	
07:00	228	1,179	346	1,268	462	2,083	215	762	690	3,262	561	2,030
07:15	275		323		552		186		827		509	
07:30	299		326		593		201		892		527	
07:45	377		273		476		160		853		433	
08:00	314	1,212	206	759	474	1,930	171	613	788	3,142	377	1,372
08:15	352		211		521		164		873		375	
08:30	292		182		494		142		786		324	
08:45	254		160		441		136		695		296	
09:00	258	935	188	664	359	1,319	142	510	617	2,254	330	1,174
09:15	220		144		362		134		582		278	
09:30	240		148		310		127		550		275	
09:45	217		184		288		107		505		291	
10:00	213	829	172	547	234	965	100	356	447	1,794	272	903
10:15	208		158		252		100		460		258	
10:30	224		112		224		86		448		198	
10:45	184		105		255		70		439		175	
11:00	214	908	94	324	200	862	66	243	414	1,770	160	567
11:15	248		77		200		66		448		143	
11:30	226		92		232		60		458		152	
11:45	220		61		230		51		450		112	
Totals	6,537		13,359		9,961		10,471		16,498		23,830	
Split%	39.6		56.1		60.4		43.9					
Day Totals		19,896				20,432				40,328		
Day Splits		49.3				50.7						
Peak Hour	07:30		05:00		07:15		05:15		07:30		05:00	
Volume	1,342		1,783		2,095		1,291		3,406		3,065	
Factor	0.89		0.98		0.88		0.96		0.95		0.99	

* Data File : D1909059

Thursday, October 03, 2019 Location: Santa Ana PROJECT:

ADT227 SEVENTEENTH ST from HARBOR BL to CLINTON ST. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB	SB	EB		WB			PM Period	NB	SB	EB		WB		
0:30			23		18			12:00			189		248		
0:15			24		19			12:15			191		232		
0:30			21		20			12:30			192		214		
0:45			16	84	14	71	155	12:45			182	754	223	917	1671
1:00			15		15			13:00			213		204		
1:15			27		11			13:15			194		230		
1:30			19		14			13:30			184		265		
1:45			15	76	9	49	125	13:45			220	811	245	944	1755
2:00			12		9			14:00			255		241		
2:15			11		7			14:15			258		234		
2:30			8		10			14:30			292		253		
2:45			9	40	11	37	77	14:45			277	1082	271	999	2081
3:00			10		9			15:00			229		256		
3:15			11		13			15:15			242		270		
3:30			9		8			15:30			275		263		
3:45			8	38	15	45	83	15:45			232	978	291	1080	2058
4:00			12		17			16:00			256		304		
4:15			20		23			16:15			262		289		
4:30			16		26			16:30			294		293		
4:45			24	72	32	98	170	16:45			307	1119	303	1189	2308
5:00			32		49			17:00			302		313		
5:15			59		57			17:15			321		235		
5:30			68		65			17:30			276		241		
5:45			83	242	76	247	489	17:45			312	1211	285	1074	2285
6:00			103		85			18:00			300		274		
6:15			100		95			18:15			311		245		
6:30			167		112			18:30			268		231		
6:45			209	579	146	438	1017	18:45			238	1117	234	984	2101
7:00			285		159			19:00			215		203		
7:15			307		193			19:15			182		214		
7:30			366		256			19:30			152		201		
7:45			407	1365	283	891	2256	19:45			143	692	184	802	1494
8:00			393		274			20:00			117		174		
8:15			332		242			20:15			132		170		
8:30			319		249			20:30			114		182		
8:45			267	1311	232	997	2308	20:45			125	488	167	693	1181
9:00			196		195			21:00			103		142		
9:15			217		185			21:15			95		132		
9:30			204		174			21:30			115		95		
9:45			234	851	177	731	1582	21:45			94	407	112	481	888
10:00			222		184			22:00			88		76		
10:15			187		204			22:15			58		81		
10:30			199		213			22:30			53		53		
10:45			212	820	206	807	1627	22:45			60	259	58	268	527
11:00			189		201			23:00			46		43		
11:15			203		204			23:15			40		26		
11:30			199		216			23:30			32		31		
11:45			217	808	224	845	1653	23:45			28	146	20	120	266
Total Vol.				6286		5256	**11542**					9064		9551	**18615**

Daily Totals

NB	SB	EB	WB	Combined
		15350	14807	**30157**

			AM				PM		
Split %			54.5%	45.5%	**38.3%**		48.7%	51.3%	**61.7%**
Peak Hour	0:30	0:30	7:30	7:30	**7:30**		16:30	16:15	**16:30**
Volume			1498	1055	**2553**		1224	1198	**2368**
P.H.F.			0.92	0.93	**0.93**		0.95	0.96	**0.96**

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : HARBOR BOULEVARD — Site: SANTA ANA

Segment : HAZARD AVE TO WESTMINSTER AVE — Date: 10/03/19

Client : CITY SANTA ANA — Day: Thursday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	92	310	350	1,400	85	314	390	1,421	177	624	740	2,821
12:15	72		382		72		334		144		716	
12:30	84		316		84		361		168		677	
12:45	62		352		73		336		135		688	
01:00	68	206	422	1,653	65	198	354	1,420	133	404	776	3,073
01:15	48		418		40		366		88		784	
01:30	50		372		43		354		93		726	
01:45	40		441		50		346		90		787	
02:00	50	219	446	2,014	40	153	386	1,525	90	372	832	3,539
02:15	69		482		40		354		109		836	
02:30	54		552		39		379		93		931	
02:45	46		534		34		406		80		940	
03:00	50	223	428	1,995	38	251	380	1,504	88	474	808	3,499
03:15	52		526		48		380		100		906	
03:30	65		551		69		380		134		931	
03:45	56		490		96		364		152		854	
04:00	70	404	526	1,970	68	452	368	1,556	138	856	894	3,526
04:15	90		528		76		362		166		890	
04:30	124		457		139		396		263		853	
04:45	120		459		169		430		289		889	
05:00	133	700	461	1,833	144	816	440	1,675	277	1,516	901	3,508
05:15	178		460		184		426		362		886	
05:30	218		432		232		418		450		850	
05:45	171		480		256		391		427		871	
06:00	200	1,022	454	1,777	245	1,441	350	1,327	445	2,463	804	3,104
06:15	252		452		299		323		551		775	
06:30	262		432		416		346		678		778	
06:45	308		439		481		308		789		747	
07:00	295	1,392	394	1,388	538	2,071	322	1,324	833	3,463	716	2,712
07:15	356		362		531		375		887		737	
07:30	391		330		522		333		913		663	
07:45	350		302		480		294		830		596	
08:00	404	1,409	298	1,060	478	1,943	278	1,045	882	3,352	576	2,105
08:15	344		280		498		280		842		560	
08:30	324		228		493		258		817		486	
08:45	337		254		474		229		811		483	
09:00	302	1,082	263	851	396	1,736	239	934	698	2,818	502	1,785
09:15	260		248		462		254		722		502	
09:30	264		194		436		247		700		441	
09:45	256		146		442		194		698		340	
10:00	301	1,200	189	658	354	1,412	231	709	655	2,612	420	1,367
10:15	327		162		380		190		707		352	
10:30	308		146		368		150		676		296	
10:45	264		161		310		138		574		299	
11:00	294	1,268	146	473	328	1,337	154	530	622	2,605	300	1,003
11:15	293		108		337		126		630		234	
11:30	372		116		314		118		686		234	
11:45	309		103		358		132		667		235	
Totals	9,435		17,072		12,124		14,970		21,559		32,042	
Split%	43.8		53.3		56.2		46.7					
Day Totals		26,507				27,094				53,601		
Day Splits		49.5				50.5						
Peak Hour	07:15		03:30		06:45		04:45		07:15		02:30	
Volume	1,501		2,095		2,072		1,714		3,512		3,585	
Factor	0.93		0.95		0.96		0.97		0.96		0.95	

* Data File : D1909140

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : FIRST STREET — Site: SANTA ANA

Segment : HARBOR BLVD TO FAIRVIEW ST — Date: 09/11/19

Client : CITY SANTA ANA — Day: Wednesday

Interval	EB				WB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	42	129	196	749	38	124	238	843	80	253	434	1,592
12:15	30		185		34		222		64		407	
12:30	24		188		26		169		50		357	
12:45	33		180		26		214		59		394	
01:00	24	79	214	890	29	90	220	924	53	169	434	1,814
01:15	22		228		17		240		39		468	
01:30	15		216		32		222		47		438	
01:45	18		232		12		242		30		474	
02:00	17	74	248	982	14	70	213	962	31	144	461	1,944
02:15	21		228		19		258		40		486	
02:30	18		238		17		225		35		463	
02:45	18		268		20		266		38		534	
03:00	16	84	268	1,096	18	80	267	1,186	34	164	535	2,282
03:15	16		266		17		318		33		584	
03:30	20		280		20		280		40		560	
03:45	32		282		25		321		57		603	
04:00	32	178	274	1,115	26	168	326	1,453	58	346	600	2,568
04:15	36		279		24		374		60		653	
04:30	56		282		54		372		110		654	
04:45	54		280		64		381		118		661	
05:00	56	382	278	1,164	55	304	382	1,520	111	686	660	2,684
05:15	68		302		79		398		147		700	
05:30	114		298		84		354		198		652	
05:45	144		286		86		386		230		672	
06:00	124	780	249	983	102	517	360	1,278	226	1,297	609	2,261
06:15	160		256		115		342		275		598	
06:30	234		254		141		322		375		576	
06:45	262		224		159		254		421		478	
07:00	336	1,556	236	824	156	835	253	889	492	2,391	489	1,713
07:15	384		198		204		208		588		406	
07:30	368		202		227		224		595		426	
07:45	468		188		248		204		716		392	
08:00	362	1,334	164	585	274	862	158	603	636	2,196	322	1,188
08:15	334		160		180		160		514		320	
08:30	326		126		204		147		530		273	
08:45	312		135		204		138		516		273	
09:00	252	914	137	484	175	730	118	477	427	1,644	255	961
09:15	232		134		188		127		420		261	
09:30	228		109		202		120		430		229	
09:45	202		104		165		112		367		216	
10:00	166	710	94	348	164	734	96	316	330	1,444	190	664
10:15	168		111		196		82		364		193	
10:30	216		67		186		70		402		137	
10:45	160		76		188		68		348		144	
11:00	200	785	66	202	176	815	54	205	376	1,600	120	407
11:15	176		58		206		65		382		123	
11:30	204		36		229		46		433		82	
11:45	205		42		204		40		409		82	
Totals	7,005		9,422		5,329		10,656		12,334		20,078	
Split%	56.8		46.9		43.2		53.1					
Day Totals		16,427				15,985				32,412		
Day Splits		50.7				49.3						
Peak Hour	07:15		05:00		07:15		04:30		07:15		05:00	
Volume	1,582		1,164		953		1,533		2,535		2,684	
Factor	0.85		0.96		0.87		0.96		0.89		0.96	

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : EDINGER AVENUE — Site: SANTA ANA

Segment : SANTA ANA RIVER TO FAIRVIEW ST — Date: 09/17/19

Client : CITY SANTA ANA — Day: Tuesday

Interval	EB				WB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	21	88	144	628	28	83	176	659	49	171	320	1,287
12:15	35		165		17		162		52		327	
12:30	18		167		14		162		32		329	
12:45	14		152		24		159		38		311	
01:00	11	49	176	794	10	53	144	685	21	102	320	1,479
01:15	17		169		18		157		35		326	
01:30	11		231		11		186		22		417	
01:45	10		218		14		198		24		416	
02:00	8	32	185	881	5	40	200	870	13	72	385	1,751
02:15	4		202		8		205		12		407	
02:30	16		196		9		206		25		402	
02:45	4		298		18		259		22		557	
03:00	5	51	291	1,015	16	61	253	1,096	21	112	544	2,111
03:15	12		270		10		291		22		561	
03:30	19		216		14		268		33		484	
03:45	15		238		21		284		36		522	
04:00	21	151	266	1,009	11	137	304	1,244	32	288	570	2,253
04:15	31		232		20		326		51		558	
04:30	48		264		46		294		94		558	
04:45	51		247		60		320		111		567	
05:00	40	298	250	994	42	255	336	1,367	82	553	586	2,361
05:15	74		290		64		343		138		633	
05:30	78		224		70		332		148		556	
05:45	106		230		79		356		185		586	
06:00	104	578	212	799	63	489	310	1,167	167	1,067	522	1,966
06:15	130		214		111		319		241		533	
06:30	158		175		144		264		302		439	
06:45	186		198		171		274		357		472	
07:00	212	1,072	169	692	168	956	210	737	380	2,028	379	1,429
07:15	306		185		230		190		536		375	
07:30	298		185		268		191		566		376	
07:45	256		153		290		146		546		299	
08:00	274	1,018	184	673	249	821	162	564	523	1,839	346	1,237
08:15	262		168		224		156		486		324	
08:30	262		188		188		124		450		312	
08:45	220		133		160		122		380		255	
09:00	200	699	125	443	147	526	108	408	347	1,225	233	851
09:15	189		128		130		112		319		240	
09:30	172		95		111		110		283		205	
09:45	138		95		138		78		276		173	
10:00	147	668	82	240	162	638	89	256	309	1,306	171	496
10:15	137		70		156		70		293		140	
10:30	206		50		159		52		365		102	
10:45	178		38		161		45		339		83	
11:00	139	615	42	161	178	635	52	167	317	1,250	94	328
11:15	170		38		162		43		332		81	
11:30	142		48		151		39		293		87	
11:45	164		33		144		33		308		66	
Totals	5,319		8,329		4,694		9,220		10,013		17,549	
Split%	53.1		47.5		46.9		52.5					
Day Totals		13,648				13,914				27,562		
Day Splits		49.5				50.5						
Peak Hour	07:15		02:45		07:15		05:00		07:15		05:00	
Volume	1,134		1,075		1,037		1,367		2,171		2,361	
Factor	0.93		0.90		0.89		0.96		0.96		0.93	

* Data File : D1909049

Thursday, October 10, 2019 Location: Santa Ana PROJECT:

ADT259 WARNER AVE from SANTA ANA RIVER to FAIRVIEW ST. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB	SB	EB		WB			PM Period	NB	SB	EB		WB		
0:30			37		21			12:00			194		204		
0:15			26		23			12:15			180		201		
0:30			24		20			12:30			211		176		
0:45			32	119	17	81	200	12:45			201	786	152	733	1519
1:00			22		8			13:00			212		204		
1:15			8		9			13:15			200		185		
1:30			14		11			13:30			231		211		
1:45			20	64	13	41	105	13:45			214	857	270	870	1727
2:00			15		9			14:00			259		308		
2:15			13		11			14:15			265		324		
2:30			14		8			14:30			290		386		
2:45			14	56	14	42	98	14:45			257	1071	337	1355	2426
3:00			17		13			15:00			231		377		
3:15			20		20			15:15			233		357		
3:30			21		12			15:30			238		398		
3:45			31	89	15	60	149	15:45			234	936	387	1519	2455
4:00			34		23			16:00			271		348		
4:15			43		14			16:15			253		346		
4:30			65		45			16:30			246		397		
4:45			63	205	101	183	388	16:45			303	1073	365	1456	2529
5:00			99		69			17:00			237		391		
5:15			123		91			17:15			216		418		
5:30			148		114			17:30			242		447		
5:45			240	610	126	400	1010	17:45			224	919	426	1682	2601
6:00			144		137			18:00			251		379		
6:15			145		123			18:15			214		336		
6:30			170		164			18:30			198		311		
6:45			250	709	189	613	1322	18:45			201	864	220	1246	2110
7:00			304		176			19:00			181		212		
7:15			400		223			19:15			157		156		
7:30			387		242			19:30			153		161		
7:45			375	1466	254	895	2361	19:45			144	635	154	683	1318
8:00			398		220			20:00			121		117		
8:15			408		245			20:15			149		137		
8:30			346		213			20:30			127		124		
8:45			334	1486	225	903	2389	20:45			103	500	114	492	992
9:00			284		198			21:00			125		87		
9:15			203		156			21:15			92		76		
9:30			214		158			21:30			109		82		
9:45			225	926	196	708	1634	21:45			116	442	54	299	741
10:00			200		185			22:00			76		56		
10:15			189		168			22:15			64		46		
10:30			192		157			22:30			53		38		
10:45			168	749	156	666	1415	22:45			47	240	41	181	421
11:00			176		147			23:00			67		57		
11:15			159		157			23:15			46		38		
11:30			188		166			23:30			41		52		
11:45			180	703	174	644	1347	23:45			34	188	37	184	372
Total Vol.				7182		5236	**12418**					8511		10700	**19211**

Daily Totals

NB	SB	EB	WB	Combined
		15693	15936	**31629**

			AM			PM		
Split %			57.8%	42.2%	**39.3%**	44.3%	55.7%	**60.7%**
Peak Hour	0:30	0:30	7:30	7:30	**7:30**	16:00	17:00	**16:45**
Volume			1568	961	**2529**	1073	1682	**2619**
P.H.F.			0.96	0.95	**0.97**	0.89	0.94	**0.95**

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : HARBOR BOULEVARD — Site: SANTA ANA

Segment : MACARTHUR TO SEGERSTROM — Date: 10/03/19

Client : CITY SANTA ANA — Day: Thursday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	67	199	312	1,356	41	122	402	1,498	108	321	714	2,854
12:15	54		340		24		373		78		713	
12:30	34		350		34		373		68		723	
12:45	44		354		23		350		67		704	
01:00	34	116	314	1,472	20	82	361	1,358	54	198	675	2,830
01:15	28		361		19		331		47		692	
01:30	26		382		28		356		54		738	
01:45	28		415		15		310		43		725	
02:00	25	100	385	1,670	16	78	314	1,316	41	178	699	2,986
02:15	26		440		13		330		39		770	
02:30	27		463		26		350		53		813	
02:45	22		382		23		322		45		704	
03:00	17	92	475	1,874	24	137	302	1,218	41	229	777	3,092
03:15	20		445		26		338		46		783	
03:30	26		498		41		289		67		787	
03:45	29		456		46		289		75		745	
04:00	20	134	515	1,899	56	408	308	1,239	76	542	823	3,138
04:15	22		473		70		282		92		755	
04:30	28		472		116		339		144		811	
04:45	64		439		166		310		230		749	
05:00	63	336	430	1,763	115	782	298	1,203	178	1,118	728	2,966
05:15	75		447		152		298		227		745	
05:30	90		432		221		316		311		748	
05:45	108		454		294		291		402		745	
06:00	116	561	488	1,876	236	1,432	280	1,016	352	1,993	768	2,892
06:15	121		458		310		260		431		718	
06:30	150		454		400		242		550		696	
06:45	174		476		486		234		660		710	
07:00	174	870	444	1,525	536	2,323	216	816	710	3,193	660	2,341
07:15	216		383		606		204		822		587	
07:30	204		388		599		210		803		598	
07:45	276		310		582		186		858		496	
08:00	282	1,104	239	905	575	2,323	192	666	857	3,427	431	1,571
08:15	291		218		574		177		865		395	
08:30	275		210		568		167		843		377	
08:45	256		238		606		130		862		368	
09:00	232	892	193	815	487	1,885	140	510	719	2,777	333	1,325
09:15	226		227		516		142		742		369	
09:30	228		203		455		122		683		325	
09:45	206		192		427		106		633		298	
10:00	250	930	226	667	362	1,490	103	339	612	2,420	329	1,006
10:15	226		193		376		104		602		297	
10:30	222		124		338		70		560		194	
10:45	232		124		414		62		646		186	
11:00	264	1,142	100	463	376	1,415	56	241	640	2,557	156	704
11:15	304		103		320		69		624		172	
11:30	278		114		353		56		631		170	
11:45	296		146		366		60		662		206	
Totals	6,476		16,285		12,477		11,420		18,953		27,705	
Split%	34.2		58.8		65.8		41.2					
Day Totals		22,761				23,897				46,658		
Day Splits		48.8				51.2						
Peak Hour	11:00		03:30		07:15		12:00		08:00		03:15	
Volume	1,142		1,942		2,362		1,498		3,427		3,138	
Factor	0.94		0.94		0.97		0.93		0.99		0.95	

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : FAIRVIEW STREET — Site: SANTA ANA

Segment : MCFADDEN AVE TO FIRST ST — Date: 10/01/19

Client : CITY SANTA ANA — Day: Tuesday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	65	195	223	970	48	148	202	829	113	343	425	1,799
12:15	63		244		41		240		104		484	
12:30	37		235		29		202		66		437	
12:45	30		268		30		185		60		453	
01:00	26	105	264	1,148	23	100	192	853	49	205	456	2,001
01:15	28		287		31		205		59		492	
01:30	25		280		19		226		44		506	
01:45	26		317		27		230		53		547	
02:00	25	102	366	1,576	21	84	238	987	46	186	604	2,563
02:15	29		376		25		196		54		572	
02:30	18		405		16		272		34		677	
02:45	30		429		22		281		52		710	
03:00	27	121	394	1,672	25	159	277	1,011	52	280	671	2,683
03:15	26		425		30		256		56		681	
03:30	42		414		50		256		92		670	
03:45	26		439		54		222		80		661	
04:00	54	306	434	1,769	50	396	260	1,180	104	702	694	2,949
04:15	56		446		68		316		124		762	
04:30	94		431		110		308		204		739	
04:45	102		458		168		296		270		754	
05:00	98	568	439	1,784	152	865	360	1,463	250	1,433	799	3,247
05:15	150		475		189		350		339		825	
05:30	156		448		232		370		388		818	
05:45	164		422		292		383		456		805	
06:00	150	856	470	1,737	245	1,230	368	1,298	395	2,086	838	3,035
06:15	184		451		228		330		412		781	
06:30	246		434		349		335		595		769	
06:45	276		382		408		265		684		647	
07:00	314	1,491	372	1,346	444	1,797	286	982	758	3,288	658	2,328
07:15	391		352		458		260		849		612	
07:30	424		318		476		202		900		520	
07:45	362		304		419		234		781		538	
08:00	346	1,155	266	976	423	1,562	207	710	769	2,717	473	1,686
08:15	299		258		454		180		753		438	
08:30	272		242		360		163		632		405	
08:45	238		210		325		160		563		370	
09:00	210	902	194	753	298	1,168	154	579	508	2,070	348	1,332
09:15	238		194		271		158		509		352	
09:30	210		195		334		118		544		313	
09:45	244		170		265		149		509		319	
10:00	238	922	157	536	288	1,021	122	414	526	1,943	279	950
10:15	202		162		260		117		462		279	
10:30	235		116		244		82		479		198	
10:45	247		101		229		93		476		194	
11:00	222	930	104	346	246	815	82	266	468	1,745	186	612
11:15	232		84		206		78		438		162	
11:30	244		92		177		59		421		151	
11:45	232		66		186		47		418		113	
Totals	7,653		14,613		9,345		10,572		16,998		25,185	
Split%	45.0		58.0		55.0		42.0					
Day Totals		22,266				19,917				42,183		
Day Splits		52.8				47.2						
Peak Hour	07:15		04:45		07:00		05:15		07:15		05:15	
Volume	1,523		1,820		1,797		1,471		3,299		3,286	
Factor	0.90		0.96		0.94		0.96		0.92		0.98	

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : FIRST STREET — Site: SANTA ANA

Segment : FAIRVIEW ST TO RAITT ST — Date: 09/11/19

Client : CITY SANTA ANA — Day: Wednesday

Interval	WB				EB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	50	149	268	1,002	50	161	230	914	100	310	498	1,916
12:15	41		266		50		217		91		483	
12:30	27		222		29		233		56		455	
12:45	31		246		32		234		63		480	
01:00	26	96	266	1,099	31	90	250	1,006	57	186	516	2,105
01:15	28		270		20		266		48		536	
01:30	30		289		21		232		51		521	
01:45	12		274		18		258		30		532	
02:00	25	85	292	1,206	15	74	276	1,054	40	159	568	2,260
02:15	20		316		17		248		37		564	
02:30	23		292		24		258		47		550	
02:45	17		306		18		272		35		578	
03:00	18	96	326	1,396	15	88	300	1,173	33	184	626	2,569
03:15	15		364		9		307		24		671	
03:30	30		350		26		294		56		644	
03:45	33		356		38		272		71		628	
04:00	26	176	372	1,563	30	205	296	1,192	56	381	668	2,755
04:15	43		376		40		300		83		676	
04:30	52		404		67		304		119		708	
04:45	55		411		68		292		123		703	
05:00	68	348	385	1,506	82	433	288	1,274	150	781	673	2,780
05:15	88		371		86		340		174		711	
05:30	92		360		115		318		207		678	
05:45	100		390		150		328		250		718	
06:00	106	582	334	1,328	155	842	284	1,101	261	1,424	618	2,429
06:15	128		364		185		276		313		640	
06:30	160		326		232		263		392		589	
06:45	188		304		270		278		458		582	
07:00	211	1,049	256	933	340	1,568	254	885	551	2,617	510	1,818
07:15	266		234		430		228		696		462	
07:30	274		237		398		215		672		452	
07:45	298		206		400		188		698		394	
08:00	250	899	195	695	410	1,426	172	652	660	2,325	367	1,347
08:15	210		182		357		178		567		360	
08:30	227		160		335		155		562		315	
08:45	212		158		324		147		536		305	
09:00	218	873	168	599	285	1,040	128	482	503	1,913	296	1,081
09:15	215		160		262		132		477		292	
09:30	232		153		249		124		481		277	
09:45	208		118		244		98		452		216	
10:00	211	872	118	357	206	822	104	334	417	1,694	222	691
10:15	231		96		188		82		419		178	
10:30	232		65		244		80		476		145	
10:45	198		78		184		68		382		146	
11:00	214	968	72	217	214	877	70	230	428	1,845	142	447
11:15	256		55		205		66		461		121	
11:30	262		42		216		54		478		96	
11:45	236		48		242		40		478		88	
Totals	6,193		11,901		7,626		10,297		13,819		22,198	
Split%	44.8		53.6		55.2		46.4					
Day Totals		18,094				17,923				36,017		
Day Splits		50.2				49.8						
Peak Hour	07:15		04:15		07:15		05:00		07:15		04:30	
Volume	1,088		1,576		1,638		1,274		2,726		2,795	
Factor	0.91		0.96		0.95		0.94		0.98		0.98	

* Data File : D1909089

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : BRISTOL STREET — Site: SANTA ANA

Segment : SEVENTEENTH ST TO SANTA CLARA — Date: 09/17/19

Client : CITY SANTA ANA — Day: Tuesday

Interval	SB				NB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	64	227	298	1,112	56	165	289	1,182	120	392	587	2,294
12:15	70		248		52		274		122		522	
12:30	39		288		23		321		62		609	
12:45	54		278		34		298		88		576	
01:00	58	179	274	1,147	22	111	312	1,288	80	290	586	2,435
01:15	48		288		27		316		75		604	
01:30	34		285		34		318		68		603	
01:45	39		300		28		342		67		642	
02:00	34	126	258	1,153	31	104	316	1,023	65	230	574	2,176
02:15	22		301		26		237		48		538	
02:30	36		292		24		214		60		506	
02:45	34		302		23		256		57		558	
03:00	28	102	340	1,457	28	173	257	1,238	56	275	597	2,695
03:15	22		363		34		231		56		594	
03:30	26		368		51		366		77		734	
03:45	26		386		60		384		86		770	
04:00	34	264	433	1,692	74	513	394	1,532	108	777	827	3,224
04:15	58		398		113		372		171		770	
04:30	74		422		158		400		232		822	
04:45	98		439		168		366		266		805	
05:00	82	514	412	1,684	179	919	440	1,645	261	1,433	852	3,329
05:15	113		454		226		406		339		860	
05:30	153		416		280		411		433		827	
05:45	166		402		234		388		400		790	
06:00	218	1,213	361	1,392	226	1,104	346	1,477	444	2,317	707	2,869
06:15	237		352		246		385		483		737	
06:30	356		358		332		386		688		744	
06:45	402		321		300		360		702		681	
07:00	388	1,469	296	1,065	288	1,307	354	1,254	676	2,776	650	2,319
07:15	344		284		290		322		634		606	
07:30	366		258		358		276		724		534	
07:45	371		227		371		302		742		529	
08:00	482	1,851	218	875	354	1,301	258	1,026	836	3,152	476	1,901
08:15	471		232		332		260		803		492	
08:30	455		238		302		238		757		476	
08:45	443		187		313		270		756		457	
09:00	324	1,360	225	866	278	1,036	264	949	602	2,396	489	1,815
09:15	328		224		260		252		588		476	
09:30	354		217		248		215		602		432	
09:45	354		200		250		218		604		418	
10:00	318	1,082	174	605	239	982	212	561	557	2,064	386	1,166
10:15	274		174		248		157		522		331	
10:30	244		133		250		102		494		235	
10:45	246		124		245		90		491		214	
11:00	268	1,061	142	472	242	1,122	84	244	510	2,183	226	716
11:15	261		128		262		66		523		194	
11:30	258		96		306		58		564		154	
11:45	274		106		312		36		586		142	
Totals	9,448		13,520		8,837		13,419		18,285		26,939	
Split%	51.7		50.2		48.3		49.8					
Day Totals		22,968				22,256				45,224		
Day Splits		50.8				49.2						
Peak Hour	08:00		04:30		07:30		05:00		08:00		04:45	
Volume	1,851		1,727		1,415		1,645		3,152		3,344	
Factor	0.96		0.95		0.95		0.93		0.94		0.97	

Thursday, October 10, 2019 Location: Santa Ana PROJECT:

ADT230 SEVENTEENTH ST from COLLEGE AVE to BRISTOL ST. **Prepared by AimTD tel. 714 753 7888**

AM Period	NB	SB	EB	WB			PM Period	NB	SB	EB	WB	
0:30			10	34			12:00			278	256	
0:15			12	27			12:15			290	245	
0:30			9	20			12:30			286	251	
0:45			11 42	17 98	140		12:45			301 1155	242 994	2149
1:00			13	23			13:00			303	267	
1:15			10	15			13:15			298	242	
1:30			8	16			13:30			285	246	
1:45			7 38	13 67	105		13:45			274 1160	294 1049	2209
2:00			6	10			14:00			296	286	
2:15			8	9			14:15			314	275	
2:30			11	12			14:30			348	258	
2:45			7 32	8 39	71		14:45			332 1290	301 1120	2410
3:00			10	12			15:00			320	304	
3:15			12	14			15:15			325	321	
3:30			13	9			15:30			336	313	
3:45			17 52	11 46	98		15:45			342 1323	326 1264	2587
4:00			22	15			16:00			326	343	
4:15			32	13			16:15			358	356	
4:30			33	18			16:30			339	324	
4:45			29 116	26 72	188		16:45			346 1369	375 1398	2767
5:00			56	17			17:00			314	359	
5:15			102	24			17:15			380	356	
5:30			98	51			17:30			419	364	
5:45			105 361	54 146	507		17:45			439 1552	312 1391	2943
6:00			127	75			18:00			377	321	
6:15			170	93			18:15			384	303	
6:30			229	84			18:30			352	297	
6:45			330 856	114 366	1222		18:45			313 1426	316 1237	2663
7:00			392	152			19:00			294	284	
7:15			460	195			19:15			252	252	
7:30			428	266			19:30			213	234	
7:45			361 1641	278 891	2532		19:45			229 988	250 1020	2008
8:00			393	234			20:00			186	210	
8:15			368	201			20:15			173	185	
8:30			374	173			20:30			191	178	
8:45			353 1488	194 802	2290		20:45			179 729	187 760	1489
9:00			344	203			21:00			190	201	
9:15			360	183			21:15			203	194	
9:30			332	198			21:30			173	165	
9:45			318 1354	203 787	2141		21:45			145 711	134 694	1405
10:00			286	216			22:00			113	114	
10:15			273	205			22:15			94	95	
10:30			263	200			22:30			65	75	
10:45			279 1101	231 852	1953		22:45			43 315	82 366	681
11:00			309	217			23:00			24	68	
11:15			314	246			23:15			18	45	
11:30			301	237			23:30			23	47	
11:45			293 1217	228 928	2145		23:45			15 80	32 192	272
Total Vol.			8298	5094	**13392**					12098	11485	**23583**

			Daily Totals		
	NB	SB	EB	WB	**Combined**
			20396	16579	**36975**
	AM		**PM**		
Split %			51.3%	48.7%	**63.8%**
Peak Hour	0:30	0:30	17:30	16:45	**17:15**
Volume			1619	1454	**2968**
P.H.F.			0.92	0.97	**0.95**

pacific@aimtd.com Tell. 714 753 7888

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : BRISTOL STREET — Site: SANTA ANA

Segment : CIVIC CENTER DR TO SEVENTEENTH — Date: 09/17/19

Client : CITY SANTA ANA — Day: Tuesday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	71	177	288	1,161	56	212	288	1,028	127	389	576	2,189
12:15	48		274		64		254		112		528	
12:30	26		297		42		242		68		539	
12:45	32		302		50		244		82		546	
01:00	38	118	270	1,194	44	146	256	1,124	82	264	526	2,318
01:15	26		308		42		280		68		588	
01:30	27		300		26		308		53		608	
01:45	27		316		34		280		61		596	
02:00	36	113	318	1,221	30	115	262	1,084	66	228	580	2,305
02:15	34		315		24		230		58		545	
02:30	18		322		30		256		48		578	
02:45	25		266		31		336		56		602	
03:00	30	179	314	1,333	28	103	286	1,138	58	282	600	2,471
03:15	40		308		23		278		63		586	
03:30	45		353		28		274		73		627	
03:45	64		358		24		300		88		658	
04:00	64	432	356	1,479	32	233	340	1,221	96	665	696	2,700
04:15	88		373		52		277		140		650	
04:30	126		374		71		266		197		640	
04:45	154		376		78		338		232		714	
05:00	168	772	359	1,535	75	451	294	1,211	243	1,223	653	2,746
05:15	190		418		88		332		278		750	
05:30	214		366		150		290		364		656	
05:45	200		392		138		295		338		687	
06:00	190	1,082	364	1,510	177	946	298	1,021	367	2,028	662	2,531
06:15	270		408		191		204		461		612	
06:30	292		372		266		264		558		636	
06:45	330		366		312		255		642		621	
07:00	302	1,451	346	1,228	342	1,366	226	926	644	2,817	572	2,154
07:15	376		302		356		252		732		554	
07:30	398		282		342		228		740		510	
07:45	375		298		326		220		701		518	
08:00	356	1,262	225	904	336	1,343	200	863	692	2,605	425	1,767
08:15	334		233		314		219		648		452	
08:30	310		220		356		210		666		430	
08:45	262		226		337		234		599		460	
09:00	264	1,064	204	762	287	1,164	240	933	551	2,228	444	1,695
09:15	272		194		288		222		560		416	
09:30	276		178		292		258		568		436	
09:45	252		186		297		213		549		399	
10:00	283	1,016	136	445	309	1,056	206	629	592	2,072	342	1,074
10:15	245		118		275		179		520		297	
10:30	236		99		234		128		470		227	
10:45	252		92		238		116		490		208	
11:00	210	1,121	84	275	252	1,033	133	409	462	2,154	217	684
11:15	278		87		260		112		538		199	
11:30	341		48		233		80		574		128	
11:45	292		56		288		84		580		140	
Totals	8,787		13,047		8,168		11,587		16,955		24,634	
Split%	51.8		53.0		48.2		47.0					
Day Totals		21,834				19,755				41,589		
Day Splits		52.5				47.5						
Peak Hour	07:15		05:15		07:00		04:45		07:15		04:45	
Volume	1,505		1,540		1,366		1,254		2,865		2,773	
Factor	0.95		0.92		0.96		0.93		0.97		0.92	

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : FAIRVIEW STREET — Site: SANTA ANA

Segment : SEVENTEENTH ST TO TRASK AVE — Date: 10/01/19

Client : CITY SANTA ANA — Day: Tuesday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	46	143	238	970	59	202	228	919	105	345	466	1,889
12:15	46		246		55		232		101		478	
12:30	23		238		48		204		71		442	
12:45	28		248		40		255		68		503	
01:00	24	109	225	1,056	38	131	214	968	62	240	439	2,024
01:15	29		258		36		234		65		492	
01:30	26		276		22		226		48		502	
01:45	30		297		35		294		65		591	
02:00	18	81	324	1,416	30	88	268	1,189	48	169	592	2,605
02:15	19		322		26		311		45		633	
02:30	21		380		13		276		34		656	
02:45	23		390		19		334		42		724	
03:00	30	178	379	1,461	20	107	342	1,267	50	285	721	2,728
03:15	38		334		19		298		57		632	
03:30	68		374		42		310		110		684	
03:45	42		374		26		317		68		691	
04:00	78	443	390	1,560	39	261	315	1,248	117	704	705	2,808
04:15	74		380		46		297		120		677	
04:30	144		394		78		304		222		698	
04:45	147		396		98		332		245		728	
05:00	167	819	420	1,635	102	634	338	1,280	269	1,453	758	2,915
05:15	202		404		132		336		334		740	
05:30	246		419		186		318		432		737	
05:45	204		392		214		288		418		680	
06:00	234	1,037	385	1,489	215	1,070	302	1,144	449	2,107	687	2,633
06:15	223		390		220		290		443		680	
06:30	286		388		287		288		573		676	
06:45	294		326		348		264		642		590	
07:00	296	1,230	362	1,197	359	1,401	225	908	655	2,631	587	2,105
07:15	302		304		322		264		624		568	
07:30	322		279		358		202		680		481	
07:45	310		252		362		217		672		469	
08:00	357	1,108	238	805	390	1,327	208	728	747	2,435	446	1,533
08:15	285		189		363		201		648		390	
08:30	256		186		306		146		562		332	
08:45	210		192		268		173		478		365	
09:00	184	851	188	728	274	1,156	188	636	458	2,007	376	1,364
09:15	245		200		280		145		525		345	
09:30	197		189		288		150		485		339	
09:45	225		151		314		153		539		304	
10:00	236	917	128	413	252	907	147	489	488	1,824	275	902
10:15	218		104		249		140		467		244	
10:30	226		95		204		94		430		189	
10:45	237		86		202		108		439		194	
11:00	197	908	84	261	213	863	94	294	410	1,771	178	555
11:15	213		60		189		65		402		125	
11:30	228		69		216		72		444		141	
11:45	270		48		245		63		515		111	
Totals	7,824		12,991		8,147		11,070		15,971		24,061	
Split%	49.0		54.0		51.0		46.0					
Day Totals		20,815				19,217				40,032		
Day Splits		52.0				48.0						
Peak Hour	07:15		04:45		07:30		04:45		07:30		04:45	
Volume	1,291		1,639		1,473		1,324		2,747		2,963	
Factor	0.90		0.98		0.94		0.98		0.92		0.98	

* Data File : D1909076

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : BRISTOL STREET
Segment : MCFADDEN AVE TO FIRST ST
Client : CITY SANTA ANA

Site: SANTA ANA
Date: 09/17/19
Day: Tuesday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	62	173	240	1,016	59	185	264	962	121	358	504	1,978
12:15	47		238		55		224		102		462	
12:30	30		253		37		230		67		483	
12:45	34		285		34		244		68		529	
01:00	32	114	234	1,114	44	135	202	943	76	249	436	2,057
01:15	24		302		38		218		62		520	
01:30	22		290		26		246		48		536	
01:45	36		288		27		277		63		565	
02:00	32	120	294	1,312	21	111	275	1,107	53	231	569	2,419
02:15	32		318		23		242		55		560	
02:30	29		324		27		306		56		630	
02:45	27		376		40		284		67		660	
03:00	27	136	412	1,701	26	107	316	1,282	53	243	728	2,983
03:15	33		468		17		342		50		810	
03:30	28		425		30		305		58		730	
03:45	48		396		34		319		82		715	
04:00	52	264	424	1,730	39	279	318	1,381	91	543	742	3,111
04:15	52		444		62		350		114		794	
04:30	84		435		100		338		184		773	
04:45	76		427		78		375		154		802	
05:00	117	493	432	1,778	94	499	360	1,504	211	992	792	3,282
05:15	112		448		82		394		194		842	
05:30	130		441		156		366		286		807	
05:45	134		457		167		384		301		841	
06:00	130	756	441	1,644	176	1,016	338	1,262	306	1,772	779	2,906
06:15	172		439		188		326		360		765	
06:30	216		393		264		296		480		689	
06:45	238		371		388		302		626		673	
07:00	239	1,371	346	1,289	412	1,829	262	1,004	651	3,200	608	2,293
07:15	352		344		484		296		836		640	
07:30	420		310		465		226		885		536	
07:45	360		289		468		220		828		509	
08:00	343	1,240	263	953	430	1,719	208	819	773	2,959	471	1,772
08:15	300		263		435		195		735		458	
08:30	292		189		446		227		738		416	
08:45	305		238		408		189		713		427	
09:00	208	872	168	677	265	1,107	223	820	473	1,979	391	1,497
09:15	220		190		260		191		480		381	
09:30	240		171		301		220		541		391	
09:45	204		148		281		186		485		334	
10:00	230	862	161	519	277	998	158	537	507	1,860	319	1,056
10:15	210		130		238		149		448		279	
10:30	216		110		240		122		456		232	
10:45	206		118		243		108		449		226	
11:00	205	967	78	286	218	942	82	306	423	1,909	160	592
11:15	254		76		227		96		481		172	
11:30	256		66		222		64		478		130	
11:45	252		66		275		64		527		130	
Totals	7,368		14,019		8,927		11,927		16,295		25,946	
Split%	45.2		54.0		54.8		46.0					
Day Totals		21,387				20,854				42,241		
Day Splits		50.6				49.4						
Peak Hour	07:15		05:15		07:15		05:00		07:15		05:00	
Volume	1,475		1,787		1,847		1,504		3,322		3,282	
Factor	0.88		0.98		0.95		0.95		0.94		0.97	

Data File : D1909017

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : CIVIC CENTER DRIVE

Segment : BRISTOL ST TO FLOWER ST

Client : CITY SANTA ANA

Site: SANTA ANA

Date: 09/18/19

Day: Wednesday

Interval	EB				WB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	13	36	98	441	8	42	139	489	21	78	237	930
12:15	8		97		13		122		21		219	
12:30	9		118		8		138		17		256	
12:45	6		128		13		90		19		218	
01:00	7	23	132	549	4	33	96	593	11	56	228	1,142
01:15	10		152		12		156		22		308	
01:30	3		145		15		186		18		331	
01:45	3		120		2		155		5		275	
02:00	6	21	122	507	5	19	129	572	11	40	251	1,079
02:15	4		130		6		129		10		259	
02:30	5		119		4		132		9		251	
02:45	6		136		4		182		10		318	
03:00	6	27	190	635	6	24	146	645	12	51	336	1,280
03:15	3		151		3		163		6		314	
03:30	9		160		12		170		21		330	
03:45	9		134		3		166		12		300	
04:00	7	65	154	588	7	60	206	859	14	125	360	1,447
04:15	11		138		7		181		18		319	
04:30	19		140		26		234		45		374	
04:45	28		156		20		238		48		394	
05:00	30	171	166	659	24	129	273	969	54	300	439	1,628
05:15	45		157		22		246		67		403	
05:30	41		168		28		250		69		418	
05:45	55		168		55		200		110		368	
06:00	67	425	156	501	58	310	198	573	125	735	354	1,074
06:15	83		130		58		140		141		270	
06:30	119		134		84		121		203		255	
06:45	156		81		110		114		266		195	
07:00	175	1,064	94	316	120	867	88	353	295	1,931	182	669
07:15	282		80		213		86		495		166	
07:30	293		82		258		92		551		174	
07:45	314		60		276		87		590		147	
08:00	240	836	62	240	196	571	78	260	436	1,407	140	500
08:15	243		61		177		74		420		135	
08:30	189		57		110		50		299		107	
08:45	164		60		88		58		252		118	
09:00	104	396	61	196	76	299	66	200	180	695	127	396
09:15	96		64		64		40		160		104	
09:30	92		40		76		46		168		86	
09:45	104		31		83		48		187		79	
10:00	70	322	40	122	90	324	45	127	160	646	85	249
10:15	82		33		68		34		150		67	
10:30	102		27		76		28		178		55	
10:45	68		22		90		20		158		42	
11:00	74	354	16	53	99	470	23	80	173	824	39	133
11:15	72		9		111		22		183		31	
11:30	88		12		128		20		216		32	
11:45	120		16		132		15		252		31	
Totals	3,740		4,807		3,148		5,720		6,888		10,527	
Split%	54.3		45.7		45.7		54.3					
Day Totals		8,547				8,868				17,415		
Day Splits		49.1				50.9						
Peak Hour	07:15		05:00		07:15		04:45		07:15		04:45	
Volume	1,129		659		943		1,007		2,072		1,654	
Factor	0.90		0.98		0.85		0.92		0.88		0.94	

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : FLOWER STREET — Site: SANTA ANA

Segment : MCFADDEN AVE TO FIRST ST — Date: 09/10/19

Client : CITY SANTA ANA — Day: Tuesday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	22	58	65	330	24	82	69	305	46	140	134	635
12:15	13		88		15		76		28		164	
12:30	12		92		18		82		30		174	
12:45	11		85		25		78		36		163	
01:00	6	29	76	370	12	44	78	332	18	73	154	702
01:15	16		83		8		70		24		153	
01:30	5		101		10		93		15		194	
01:45	2		110		14		91		16		201	
02:00	5	17	100	575	11	37	100	438	16	54	200	1,013
02:15	5		150		9		110		14		260	
02:30	1		169		9		98		10		267	
02:45	6		156		8		130		14		286	
03:00	8	34	174	682	6	38	158	531	14	72	332	1,213
03:15	5		178		7		120		12		298	
03:30	8		164		14		121		22		285	
03:45	13		166		11		132		24		298	
04:00	15	86	172	729	9	76	136	536	24	162	308	1,265
04:15	20		187		11		124		31		311	
04:30	29		190		24		140		53		330	
04:45	22		180		32		136		54		316	
05:00	44	230	175	788	28	205	142	543	72	435	317	1,331
05:15	54		220		46		132		100		352	
05:30	62		186		70		130		132		316	
05:45	70		207		61		139		131		346	
06:00	70	365	209	754	54	332	134	490	124	697	343	1,244
06:15	81		197		92		106		173		303	
06:30	96		190		96		132		192		322	
06:45	118		158		90		118		208		276	
07:00	122	627	122	404	112	498	96	383	234	1,125	218	787
07:15	196		105		128		96		324		201	
07:30	182		96		126		96		308		192	
07:45	127		81		132		95		259		176	
08:00	156	519	90	309	137	548	80	297	293	1,067	170	606
08:15	124		83		133		82		257		165	
08:30	133		64		142		74		275		138	
08:45	106		72		136		61		242		133	
09:00	76	293	60	217	82	306	82	276	158	599	142	493
09:15	75		61		64		62		139		123	
09:30	68		49		74		68		142		117	
09:45	74		47		86		64		160		111	
10:00	68	283	35	144	62	293	60	195	130	576	95	339
10:15	62		42		78		49		140		91	
10:30	77		39		72		51		149		90	
10:45	76		28		81		35		157		63	
11:00	53	283	25	84	76	322	52	150	129	605	77	234
11:15	73		27		89		43		162		70	
11:30	77		19		77		25		154		44	
11:45	80		13		80		30		160		43	
Totals	2,824		5,386		2,781		4,476		5,605		9,862	
Split%	50.4		54.6		49.6		45.4					
Day Totals		8,210				7,257				15,467		
Day Splits		53.1				46.9						
Peak Hour	07:15		05:15		08:00		04:30		07:15		05:15	
Volume	661		822		548		550		1,184		1,357	
Factor	0.84		0.93		0.96		0.97		0.91		0.96	

Thursday, October 03, 2019 Location: Santa Ana PROJECT:

ADT167 MAIN ST from SEVENTEENTH ST to SANTA CLARA AVE. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB		SB		EB	WB		PM Period	NB		SB		EB	WB	
0:00	58		42					12:00	227		212				
0:15	53		30					12:15	236		221				
0:30	54		41					12:30	224		230				
0:45	36	201	35	148			349	12:45	239	926	219	882			1808
1:00	46		29					13:00	268		218				
1:15	35		28					13:15	266		224				
1:30	27		26					13:30	241		205				
1:45	21	129	22	105			234	13:45	267	1042	197	844			1886
2:00	34		20					14:00	274		198				
2:15	41		34					14:15	257		201				
2:30	30		25					14:30	286		213				
2:45	26	131	21	100			231	14:45	272	1089	219	831			1920
3:00	21		16					15:00	269		216				
3:15	25		12					15:15	327		234				
3:30	30		8					15:30	312		265				
3:45	32	108	11	47			155	15:45	313	1221	255	970			2191
4:00	35		12					16:00	306		233				
4:15	56		21					16:15	341		238				
4:30	76		32					16:30	335		295				
4:45	87	254	38	103			357	16:45	291	1273	248	1014			2287
5:00	101		40					17:00	286		254				
5:15	99		62					17:15	295		266				
5:30	137		64					17:30	321		267				
5:45	144	481	98	264			745	17:45	309	1211	270	1057			2268
6:00	137		114					18:00	326		253				
6:15	142		157					18:15	335		246				
6:30	161		234					18:30	320		216				
6:45	159	599	291	796			1395	18:45	284	1265	204	919			2184
7:00	187		318					19:00	260		180				
7:15	214		278					19:15	256		160				
7:30	204		258					19:30	240		139				
7:45	267	872	269	1123			1995	19:45	222	978	109	588			1566
8:00	234		298					20:00	197		125				
8:15	213		276					20:15	195		118				
8:30	204		293					20:30	178		113				
8:45	202	853	258	1125			1978	20:45	186	756	96	452			1208
9:00	215		242					21:00	195		99				
9:15	186		223					21:15	175		90				
9:30	170		210					21:30	155		124				
9:45	182	753	201	876			1629	21:45	138	663	106	419			1082
10:00	195		197					22:00	116		87				
10:15	177		185					22:15	124		76				
10:30	171		175					22:30	99		64				
10:45	185	728	204	761			1489	22:45	54	393	45	272			665
11:00	198		189					23:00	58		55				
11:15	213		195					23:15	62		47				
11:30	217		201					23:30	56		42				
11:45	235	863	214	799			1662	23:45	70	246	53	197			443
Total Vol.		5972		6247			**12219**			11063		8445			**19508**

Daily Totals

NB	SB	EB	WB	**Combined**
17035	14692			**31727**

	AM			**PM**		
Split %	48.9%	51.1%	**38.5%**	56.7%	43.3%	**61.5%**
Peak Hour	11:45	6:45	**7:45**	15:45	16:30	**17:30**
Volume	922	1145	**2054**	1295	1063	**2327**
P.H.F.	0.98	0.90	**0.96**	0.95	0.90	**0.99**

cs@aimtd.com Tell. 714 253 7888

Wednesday, October 02, 2019 Location: Santa Ana PROJECT:

ADT165 MAIN ST from CIVIC CENTER DR to WASHINGTON AVE. **Prepared by AimTD tel. 714 753 7888**

AM Period	NB		SB		EB	WB		PM Period	NB		SB		EB	WB	
0:00	57		48					12:00	229		189				
0:15	51		43					12:15	242		182				
0:30	50		47					12:30	257		213				
0:45	30	188	38	176			364	12:45	279	1007	209	793			1800
1:00	17		34					13:00	311		199				
1:15	14		28					13:15	256		213				
1:30	21		23					13:30	276		199				
1:45	35	87	19	104			191	13:45	277	1120	208	819			1939
2:00	33		22					14:00	310		231				
2:15	26		17					14:15	289		186				
2:30	30		19					14:30	297		194				
2:45	18	107	18	76			183	14:45	284	1180	213	824			2004
3:00	17		16					15:00	291		232				
3:15	12		20					15:15	275		214				
3:30	24		30					15:30	304		257				
3:45	37	90	32	98			188	15:45	329	1199	217	920			2119
4:00	49		27					16:00	335		230				
4:15	45		35					16:15	344		206				
4:30	71		43					16:30	365		241				
4:45	98	263	38	143			406	16:45	356	1400	274	951			2351
5:00	91		50					17:00	334		246				
5:15	112		70					17:15	302		232				
5:30	150		108					17:30	316		245				
5:45	145	498	133	361			859	17:45	324	1276	234	957			2233
6:00	153		152					18:00	315		215				
6:15	146		194					18:15	349		157				
6:30	208		202					18:30	331		139				
6:45	223	730	238	786			1516	18:45	297	1292	147	658			1950
7:00	242		250					19:00	279		162				
7:15	257		303					19:15	259		158				
7:30	268		300					19:30	219		148				
7:45	276	1043	337	1190			2233	19:45	218	975	165	633			1608
8:00	271		314					20:00	233		145				
8:15	238		286					20:15	197		150				
8:30	203		320					20:30	216		153				
8:45	194	906	297	1217			2123	20:45	198	844	134	582			1426
9:00	186		257					21:00	189		140				
9:15	203		231					21:15	176		157				
9:30	199		228					21:30	170		135				
9:45	186	774	230	946			1720	21:45	154	689	125	557			1246
10:00	214		198					22:00	180		90				
10:15	220		210					22:15	145		79				
10:30	212		178					22:30	118		78				
10:45	204	850	206	792			1642	22:45	106	549	85	332			881
11:00	201		193					23:00	85		92				
11:15	226		177					23:15	75		86				
11:30	217		189					23:30	56		44				
11:45	215	859	210	769			1628	23:45	52	268	57	279			547
Total Vol.	6395		6658				**13053**		11799		8305				**20104**

Daily Totals

NB	SB	EB	WB	Combined
18194	14963			**33157**

	AM			PM		
Split %	49.0%	51.0%	**39.4%**	58.7%	41.3%	**60.6%**
Peak Hour	7:15	7:45	**7:15**	16:00	16:45	**16:15**
Volume	1072	1257	**2326**	1400	997	**2366**
P.H.F.	0.97	0.93	**0.95**	0.97	0.91	**0.94**

pacific@aimtd.com Tell. 714 753 7888

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : CIVIC CENTER DRIVE
Segment : FLOWER ST TO ROSS ST
Client : CITY SANTA ANA

Site: SANTA ANA
Date: 09/18/19
Day: Wednesday

Interval	WB				EB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	10	48	122	473	17	53	148	517	27	101	270	990
12:15	17		114		10		131		27		245	
12:30	15		122		12		116		27		238	
12:45	6		115		14		122		20		237	
01:00	9	27	128	640	12	37	137	580	21	64	265	1,220
01:15	5		166		7		146		12		312	
01:30	10		170		12		178		22		348	
01:45	3		176		6		119		9		295	
02:00	4	10	142	510	1	22	149	622	5	32	291	1,132
02:15	4		116		3		138		7		254	
02:30	0		122		15		144		15		266	
02:45	2		130		3		191		5		321	
03:00	5	18	120	524	7	19	215	712	12	37	335	1,236
03:15	3		132		0		169		3		301	
03:30	4		140		4		174		8		314	
03:45	6		132		8		154		14		286	
04:00	6	48	172	710	6	46	172	827	12	94	344	1,537
04:15	4		144		11		182		15		326	
04:30	13		202		12		231		25		433	
04:45	25		192		17		242		42		434	
05:00	24	113	276	929	21	131	248	760	45	244	524	1,689
05:15	16		241		28		172		44		413	
05:30	23		240		40		162		63		402	
05:45	50		172		42		178		92		350	
06:00	52	255	174	490	53	366	163	568	105	621	337	1,058
06:15	54		124		68		154		122		278	
06:30	56		98		114		138		170		236	
06:45	93		94		131		113		224		207	
07:00	112	755	69	285	140	807	92	302	252	1,562	161	587
07:15	172		73		196		78		368		151	
07:30	224		78		214		80		438		158	
07:45	247		65		257		52		504		117	
08:00	238	750	66	220	218	682	66	233	456	1,432	132	453
08:15	218		64		176		50		394		114	
08:30	153		42		140		50		293		92	
08:45	141		48		148		67		289		115	
09:00	100	376	39	178	102	440	47	158	202	816	86	336
09:15	94		46		121		52		215		98	
09:30	100		47		116		35		216		82	
09:45	82		46		101		24		183		70	
10:00	87	335	32	101	94	402	26	101	181	737	58	202
10:15	84		23		104		36		188		59	
10:30	74		28		103		21		177		49	
10:45	90		18		101		18		191		36	
11:00	102	397	17	64	106	555	14	58	208	952	31	122
11:15	91		17		143		18		234		35	
11:30	88		15		150		14		238		29	
11:45	116		15		156		12		272		27	
Totals	3,132		5,124		3,560		5,438		6,692		10,562	
Split%	46.8		48.5		53.2		51.5					
Day Totals		8,256				8,998				17,254		
Day Splits		47.8				52.2						
Peak Hour	07:30		04:45		07:15		04:15		07:30		04:30	
Volume	927		949		885		903		1,792		1,804	
Factor	0.94		0.86		0.86		0.91		0.89		0.86	

Data File : D1909039

Wednesday, October 02, 2019 Location: Santa Ana PROJECT:

ADT208 SANTA ANA BLVD from FLOWER ST to ROSS ST. **Prepared by AimTD tel. 714 753 7888**

AM Period	NB	SB	EB		WB			PM Period	NB	SB	EB		WB		
0:30			7		9			12:00			98		97		
0:15			8		11			12:15			86		89		
0:30			7		9			12:30			94		104		
0:45			2	24	13	42	66	12:45			106	384	108	398	782
1:00			1		10			13:00			89		123		
1:15			1		7			13:15			86		132		
1:30			0		6			13:30			95		105		
1:45			3	5	8	31	36	13:45			79	349	101	461	810
2:00			2		11			14:00			108		96		
2:15			0		7			14:15			89		95		
2:30			2		4			14:30			87		103		
2:45			3	7	2	24	31	14:45			97	381	123	417	798
3:00			6		5			15:00			102		127		
3:15			4		1			15:15			113		145		
3:30			2		0			15:30			103		153		
3:45			3	15	2	8	23	15:45			107	425	167	592	1017
4:00			6		4			16:00			123		159		
4:15			10		5			16:15			156		153		
4:30			13		3			16:30			178		163		
4:45			11	40	8	20	60	16:45			168	625	175	650	1275
5:00			16		12			17:00			156		213		
5:15			24		24			17:15			134		234		
5:30			29		36			17:30			121		204		
5:45			39	108	65	137	245	17:45			102	513	195	846	1359
6:00			54		64			18:00			97		172		
6:15			75		75			18:15			87		142		
6:30			86		78			18:30			103		104		
6:45			103	318	95	312	630	18:45			76	363	123	541	904
7:00			123		121			19:00			86		96		
7:15			189		158			19:15			78		86		
7:30			254		175			19:30			64		74		
7:45			304	870	145	599	1469	19:45			61	289	76	332	621
8:00			234		157			20:00			58		78		
8:15			153		134			20:15			53		61		
8:30			145		109			20:30			43		56		
8:45			126	658	89	489	1147	20:45			37	191	64	259	450
9:00			103		76			21:00			45		64		
9:15			100		89			21:15			35		43		
9:30			87		75			21:30			32		63		
9:45			95	385	78	318	703	21:45			40	152	53	223	375
10:00			76		100			22:00			28		47		
10:15			68		89			22:15			17		41		
10:30			79		96			22:30			23		36		
10:45			81	304	78	363	667	22:45			24	92	27	151	243
11:00			74		86			23:00			14		32		
11:15			89		75			23:15			8		18		
11:30			94		95			23:30			12		20		
11:45			90	347	99	355	702	23:45			11	45	16	86	131
Total Vol.			3081		2698		**5779**				3809		4956		**8765**

Daily Totals

NB	SB	EB	WB	Combined
		6890	7654	**14544**

			AM					PM		
Split %			53.3%	46.7%	**39.7%**			43.5%	56.5%	**60.3%**
Peak Hour	0:30	0:30	7:15	7:15	**7:15**			16:15	17:00	**16:30**
Volume			981	635	**1616**			658	846	**1421**
P.H.F.			0.81	0.91	**0.90**			0.92	0.90	**0.96**

pacific@aimtd.com Tell. 714 753 7888

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : FIRST STREET — Site: SANTA ANA

Segment : MAIN ST TO STANDARD AVE — Date: 09/11/19

Client : CITY SANTA ANA — Day: Wednesday

Interval	EB				WB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	54	193	299	1,249	58	185	274	1,115	112	378	573	2,364
12:15	49		306		52		282		101		588	
12:30	42		320		46		269		88		589	
12:45	48		324		29		290		77		614	
01:00	38	144	325	1,375	28	112	316	1,211	66	256	641	2,586
01:15	32		362		34		248		66		610	
01:30	28		356		23		334		51		690	
01:45	46		332		27		313		73		645	
02:00	31	116	330	1,314	36	136	333	1,369	67	252	663	2,683
02:15	31		324		34		326		65		650	
02:30	25		323		36		350		61		673	
02:45	29		337		30		360		59		697	
03:00	22	163	352	1,395	21	117	378	1,446	43	280	730	2,841
03:15	37		356		24		360		61		716	
03:30	42		315		32		362		74		677	
03:45	62		372		40		346		102		718	
04:00	56	376	324	1,440	33	135	356	1,451	89	511	680	2,891
04:15	86		362		26		368		112		730	
04:30	101		392		38		352		139		744	
04:45	133		362		38		375		171		737	
05:00	135	791	381	1,560	60	304	374	1,454	195	1,095	755	3,014
05:15	162		380		61		358		223		738	
05:30	236		406		72		354		308		760	
05:45	258		393		111		368		369		761	
06:00	258	1,243	330	1,294	118	566	352	1,279	376	1,809	682	2,573
06:15	286		338		134		313		420		651	
06:30	365		324		133		306		498		630	
06:45	334		302		181		308		515		610	
07:00	378	1,557	310	1,171	230	1,266	298	1,109	608	2,823	608	2,280
07:15	394		339		311		273		705		612	
07:30	384		264		355		278		739		542	
07:45	401		258		370		260		771		518	
08:00	375	1,422	221	866	291	1,086	192	772	666	2,508	413	1,638
08:15	384		213		276		208		660		421	
08:30	331		210		269		184		600		394	
08:45	332		222		250		188		582		410	
09:00	344	1,238	166	639	259	1,014	171	659	603	2,252	337	1,298
09:15	322		183		213		162		535		345	
09:30	278		154		276		162		554		316	
09:45	294		136		266		164		560		300	
10:00	270	1,088	125	446	248	998	158	566	518	2,086	283	1,012
10:15	270		118		242		140		512		258	
10:30	256		119		242		136		498		255	
10:45	292		84		266		132		558		216	
11:00	276	1,157	90	285	246	1,068	102	336	522	2,225	192	621
11:15	270		69		268		80		538		149	
11:30	290		68		276		66		566		134	
11:45	321		58		278		88		599		146	
Totals	9,488		13,034		6,987		12,767		16,475		25,801	
Split%	57.6		50.5		42.4		49.5					
Day Totals		22,522				19,754				42,276		
Day Splits		53.3				46.7						
Peak Hour	07:00		05:00		07:15		04:15		07:15		05:00	
Volume	1,557		1,560		1,327		1,469		2,881		3,014	
Factor	0.97		0.96		0.90		0.98		0.93		0.99	

Thursday, October 10, 2019 Location: Santa Ana PROJECT:

ADT161 MAIN ST from McFADDEN AVE to CHESTNUT AVE. **Prepared by AimTD tel. 714 753 7888**

AM Period	NB		SB		EB	WB		PM Period	NB		SB		EB	WB	
0:00	32		29					12:00	221		230				
0:15	21		24					12:15	195		205				
0:30	24		14					12:30	162		265				
0:45	18	95	17	84			179	12:45	182	760	257	957			1717
1:00	23		23					13:00	156		244				
1:15	14		16					13:15	186		214				
1:30	18		18					13:30	207		225				
1:45	16	71	21	78			149	13:45	174	723	251	934			1657
2:00	18		24					14:00	207		263				
2:15	22		25					14:15	173		236				
2:30	19		21					14:30	187		244				
2:45	23	82	24	94			176	14:45	201	768	303	1046			1814
3:00	21		17					15:00	197		285				
3:15	29		19					15:15	199		295				
3:30	20		25					15:30	178		300				
3:45	28	98	24	85			183	15:45	175	749	336	1216			1965
4:00	24		27					16:00	196		354				
4:15	34		48					16:15	204		313				
4:30	42		54					16:30	193		334				
4:45	39	139	66	195			334	16:45	213	806	326	1327			2133
5:00	52		94					17:00	264		295				
5:15	76		107					17:15	268		303				
5:30	92		100					17:30	248		265				
5:45	106	326	112	413			739	17:45	256	1036	314	1177			2213
6:00	148		122					18:00	224		317				
6:15	203		120					18:15	220		336				
6:30	261		126					18:30	189		309				
6:45	250	862	142	510			1372	18:45	176	809	299	1261			2070
7:00	268		210					19:00	187		275				
7:15	271		189					19:15	183		231				
7:30	294		264					19:30	164		207				
7:45	289	1122	252	915			2037	19:45	149	683	175	888			1571
8:00	285		231					20:00	139		154				
8:15	270		207					20:15	123		167				
8:30	316		197					20:30	110		125				
8:45	234	1105	179	814			1919	20:45	114	486	103	549			1035
9:00	205		155					21:00	99		132				
9:15	203		200					21:15	112		109				
9:30	187		185					21:30	118		113				
9:45	181	776	192	732			1508	21:45	96	425	94	448			873
10:00	175		200					22:00	75		104				
10:15	164		209					22:15	88		92				
10:30	160		184					22:30	78		108				
10:45	170	669	213	806			1475	22:45	63	304	85	389			693
11:00	158		197					23:00	52		81				
11:15	196		215					23:15	50		79				
11:30	209		194					23:30	46		56				
11:45	195	758	215	821			1579	23:45	34	182	38	254			436
Total Vol.		6103		5547			**11650**			7731		10446			**18177**

Daily Totals

NB	SB	EB	WB	Combined
13834	15993			**29827**

	AM			PM		
Split %	52.4%	47.6%	**39.1%**	42.5%	57.5%	**60.9%**
Peak Hour	7:45	7:30	**7:30**	17:00	15:45	**17:00**
Volume	1160	954	**2092**	1036	1337	**2213**
P.H.F.	0.92	0.90	**0.94**	0.96	0.94	**0.97**

pacific@aimtd.com Tell. 714 753 7888

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : GRAND AVENUE — Site: SANTA ANA

Segment : SANTA CLARA AVE TO FAIRHAVEN — Date: 09/12/19

Client : CITY SANTA ANA — Day: Thursday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	21	72	192	780	21	81	199	746	42	153	391	1,526
12:15	15		200		27		177		42		377	
12:30	20		212		17		170		37		382	
12:45	16		176		16		200		32		376	
01:00	12	43	190	804	10	41	214	722	22	84	404	1,526
01:15	11		213		12		156		23		369	
01:30	11		198		12		164		23		362	
01:45	9		203		7		188		16		391	
02:00	14	57	209	968	7	36	184	814	21	93	393	1,782
02:15	16		234		12		202		28		436	
02:30	15		251		6		222		21		473	
02:45	12		274		11		206		23		480	
03:00	10	59	248	1,055	8	51	208	900	18	110	456	1,955
03:15	12		244		14		254		26		498	
03:30	21		285		6		210		27		495	
03:45	16		278		23		228		39		506	
04:00	24	143	304	1,253	16	116	213	917	40	259	517	2,170
04:15	32		307		26		226		58		533	
04:30	41		316		38		250		79		566	
04:45	46		326		36		228		82		554	
05:00	62	302	353	1,463	26	260	287	1,099	88	562	640	2,562
05:15	62		332		56		294		118		626	
05:30	87		406		74		270		161		676	
05:45	91		372		104		248		195		620	
06:00	78	432	354	1,319	132	1,016	224	769	210	1,448	578	2,088
06:15	96		360		199		172		295		532	
06:30	132		320		306		180		438		500	
06:45	126		285		379		193		505		478	
07:00	140	777	228	799	383	1,728	176	659	523	2,505	404	1,458
07:15	192		214		414		178		606		392	
07:30	227		189		481		153		708		342	
07:45	218		168		450		152		668		320	
08:00	189	675	134	524	477	1,806	144	509	666	2,481	278	1,033
08:15	170		128		469		134		639		262	
08:30	168		142		415		117		583		259	
08:45	148		120		445		114		593		234	
09:00	148	575	126	404	422	1,174	124	427	570	1,749	250	831
09:15	125		102		274		111		399		213	
09:30	136		90		230		94		366		184	
09:45	166		86		248		98		414		184	
10:00	175	627	96	292	184	686	88	241	359	1,313	184	533
10:15	156		84		184		54		340		138	
10:30	150		64		164		49		314		113	
10:45	146		48		154		50		300		98	
11:00	164	693	43	149	166	674	48	170	330	1,367	91	319
11:15	160		46		152		42		312		88	
11:30	176		36		178		34		354		70	
11:45	193		24		178		46		371		70	
Totals	4,455		9,810		7,669		7,973		12,124		17,783	
Split%	36.7		55.2		63.3		44.8					
Day Totals		14,265				15,642				29,907		
Day Splits		47.7				52.3						
Peak Hour	07:15		05:30		07:30		05:00		07:30		05:00	
Volume	826		1,492		1,877		1,099		2,681		2,562	
Factor	0.91		0.92		0.98		0.93		0.95		0.95	

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : GRAND AVENUE — Site: SANTA ANA

Segment : FOURTH ST TO SANTA ANA BLVD — Date: 09/12/19

Client : CITY SANTA ANA — Day: Thursday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	44	105	252	1,022	25	101	257	918	69	206	509	1,940
12:15	26		266		33		227		59		493	
12:30	15		250		24		237		39		487	
12:45	20		254		19		197		39		451	
01:00	27	103	292	1,179	23	84	242	842	50	187	534	2,021
01:15	20		270		21		187		41		457	
01:30	31		293		27		192		58		485	
01:45	25		324		13		221		38		545	
02:00	19	80	360	1,415	18	61	183	866	37	141	543	2,281
02:15	26		331		13		229		39		560	
02:30	22		336		10		204		32		540	
02:45	13		388		20		250		33		638	
03:00	14	103	386	1,579	13	85	224	872	27	188	610	2,451
03:15	27		379		14		218		41		597	
03:30	30		423		26		206		56		629	
03:45	32		391		32		224		64		615	
04:00	48	245	370	1,627	33	279	242	886	81	524	612	2,513
04:15	55		409		58		218		113		627	
04:30	62		398		80		212		142		610	
04:45	80		450		108		214		188		664	
05:00	95	492	442	1,780	85	614	224	947	180	1,106	666	2,727
05:15	110		462		134		271		244		733	
05:30	164		444		175		229		339		673	
05:45	123		432		220		223		343		655	
06:00	138	692	480	1,766	184	1,045	221	763	322	1,737	701	2,529
06:15	158		460		205		181		363		641	
06:30	206		410		260		175		466		585	
06:45	190		416		396		186		586		602	
07:00	225	1,055	350	1,080	384	1,448	176	605	609	2,503	526	1,685
07:15	260		270		382		148		642		418	
07:30	288		270		348		143		636		413	
07:45	282		190		334		138		616		328	
08:00	230	886	206	722	460	1,892	134	507	690	2,778	340	1,229
08:15	228		178		487		111		715		289	
08:30	224		180		499		142		723		322	
08:45	204		158		446		120		650		278	
09:00	210	912	141	564	257	1,028	123	448	467	1,940	264	1,012
09:15	226		144		316		115		542		259	
09:30	234		149		227		118		461		267	
09:45	242		130		228		92		470		222	
10:00	242	896	118	395	212	788	104	271	454	1,684	222	666
10:15	190		112		186		65		376		177	
10:30	225		85		194		46		419		131	
10:45	239		80		196		56		435		136	
11:00	264	1,003	63	219	182	877	44	168	446	1,880	107	387
11:15	245		62		237		48		482		110	
11:30	256		60		198		35		454		95	
11:45	238		34		260		41		498		75	
Totals	6,572		13,348		8,302		8,093		14,874		21,441	
Split%	44.2		62.3		55.8		37.7					
Day Totals		19,920				16,395				36,315		
Day Splits		54.9				45.1						
Peak Hour	07:15		05:15		08:00		05:00		08:00		05:15	
Volume	1,060		1,818		1,892		947		2,778		2,762	
Factor	0.92		0.95		0.95		0.87		0.96		0.94	

* Data File : D1909124

Tuesday, September 24, 2019 CITY: Santa Ana PROJECT: SC

ADT218 SANTA CLARA AVE from GR to AVE to TUSTIN AVE. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB	SB	EB		WB			PM Period	NB	SB	EB		WB		
0:00			4		4			12:00			72		69		
0:15			8		3			12:15			77		73		
0:30			5		5			12:30			82		67		
0:45			3	20	3	15	35	12:45			69	300	87	296	596
1:00			6		5			13:00			82		66		
1:15			4		2			13:15			64		97		
1:30			2		3			13:30			88		85		
1:45			3	15	3	13	28	13:45			71	305	95	343	648
2:00			3		8			14:00			83		76		
2:15			0		0			14:15			95		81		
2:30			0		6			14:30			127		84		
2:45			2	5	0	14	19	14:45			108	413	88	329	742
3:00			4		2			15:00			93		99		
3:15			6		4			15:15			103		94		
3:30			3		6			15:30			110		97		
3:45			4	17	4	16	33	15:45			106	412	107	397	809
4:00			8		4			16:00			118		98		
4:15			6		4			16:15			110		115		
4:30			10		4			16:30			83		117		
4:45			16	40	11	23	63	16:45			115	426	113	443	869
5:00			11		16			17:00			117		136		
5:15			14		12			17:15			110		124		
5:30			28		26			17:30			144		136		
5:45			24	77	17	71	148	17:45			98	469	128	524	993
6:00			22		20			18:00			116		148		
6:15			42		34			18:15			92		98		
6:30			61		52			18:30			82		116		
6:45			49	174	58	164	338	18:45			77	367	87	449	816
7:00			70		61			19:00			52		79		
7:15			121		78			19:15			72		83		
7:30			119		108			19:30			68		53		
7:45			120	430	112	359	789	19:45			42	234	49	264	498
8:00			107		85			20:00			40		47		
8:15			92		67			20:15			55		25		
8:30			67		70			20:30			47		45		
8:45			77	343	68	290	633	20:45			35	177	39	156	333
9:00			81		57			21:00			48		34		
9:15			69		76			21:15			38		41		
9:30			72		54			21:30			28		27		
9:45			70	292	61	248	540	21:45			41	155	23	125	280
10:00			58		51			22:00			25		19		
10:15			80		61			22:15			25		19		
10:30			72		64			22:30			15		19		
10:45			65	275	62	238	513	22:45			19	84	19	76	160
11:00			61		51			23:00			15		18		
11:15			56		60			23:15			13		6		
11:30			82		68			23:30			8		6		
11:45			72	271	69	248	519	23:45			7	43	5	35	78
Total Vol.			1959		1699		**3658**				3385		3437		**6822**

Daily Totals

NB	SB	EB	WB	Combined
		5344	5136	**10480**

	AM			PM		
Split %	53.6%	46.4%	**34.9%**	49.6%	50.4%	**65.1%**
Peak Hour	7:15	7:15	**7:15**	16:45	17:15	**17:15**
Volume	467	383	**850**	486	536	**1004**
P.H.F.	0.96	0.85	**0.92**	0.84	0.91	**0.90**

Wednesday, September 25, 2019 CITY: Santa Ana PROJECT: SC

ADT258 TUSTIN AVE from SANTA CLARA AVE to FAIRHAVEN AVE. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB		SB		EB	WB		PM Period	NB		SB		EB	WB	
0:00	19		25					12:00	196		232				
0:15	17		18					12:15	257		239				
0:30	21		24					12:30	219		251				
0:45	21	78	16	83			161	12:45	262	934	279	1001			1935
1:00	21		24					13:00	231		256				
1:15	9		14					13:15	246		221				
1:30	9		11					13:30	259		248				
1:45	9	48	4	53			101	13:45	272	1008	245	970			1978
2:00	11		12					14:00	263		229				
2:15	11		11					14:15	280		248				
2:30	8		12					14:30	270		220				
2:45	8	38	10	45			83	14:45	286	1099	253	950			2049
3:00	8		8					15:00	277		267				
3:15	2		8					15:15	279		300				
3:30	6		16					15:30	294		259				
3:45	20	36	16	48			84	15:45	318	1168	257	1083			2251
4:00	9		14					16:00	366		275				
4:15	24		26					16:15	348		267				
4:30	25		36					16:30	388		272				
4:45	24	82	41	117			199	16:45	432	1534	271	1085			2619
5:00	36		49					17:00	516		335				
5:15	50		58					17:15	437		307				
5:30	57		60					17:30	445		325				
5:45	69	212	110	277			489	17:45	429	1827	269	1236			3063
6:00	68		157					18:00	477		283				
6:15	74		218					18:15	347		224				
6:30	97		410					18:30	300		211				
6:45	92	331	481	1266			1597	18:45	310	1434	206	924			2358
7:00	119		548					19:00	260		208				
7:15	125		663					19:15	243		185				
7:30	148		672					19:30	203		156				
7:45	175	567	682	2565			3132	19:45	148	854	162	711			1565
8:00	166		671					20:00	205		162				
8:15	161		665					20:15	145		142				
8:30	156		647					20:30	117		125				
8:45	164	647	572	2555			3202	20:45	134	601	130	559			1160
9:00	172		479					21:00	108		125				
9:15	152		349					21:15	130		94				
9:30	168		270					21:30	94		94				
9:45	169	661	278	1376			2037	21:45	77	409	100	413			822
10:00	177		241					22:00	89		83				
10:15	181		238					22:15	61		62				
10:30	165		231					22:30	66		41				
10:45	187	710	221	931			1641	22:45	50	266	65	251			517
11:00	211		203					23:00	69		49				
11:15	188		240					23:15	40		32				
11:30	189		201					23:30	34		33				
11:45	233	821	234	878			1699	23:45	31	174	29	143			317
Total Vol.	4231		10194				**14425**		11308		9326				**20634**

Daily Totals

NB	SB	EB	WB	**Combined**
15539	19520			**35059**

	AM			**PM**		
Split %	29.3%	70.7%	**41.1%**	54.8%	45.2%	**58.9%**
Peak Hour	11:45	7:30	**7:30**	16:45	16:45	**16:45**
Volume	905	2690	**3340**	1830	1238	**3068**
P.H.F.	0.88	0.99	**0.97**	0.88	0.92	**0.90**

cs@aimtd.com Tell. 714 253 7888

Thursday, October 10, 2019 Location: Santa Ana PROJECT:

ADT239 SEVENTEENTH ST from CABRILLO PARK DR to TUSTIN AVE. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB	SB	EB		WB			PM Period	NB	SB	EB		WB		
0:30			12		34			12:00			256		258		
0:15			15		23			12:15			262		290		
0:30			9		17			12:30			304		306		
0:45			8	44	19	93	137	12:45			256	1078	317	1171	2249
1:00			7		12			13:00			245		291		
1:15			10		14			13:15			215		295		
1:30			7		11			13:30			277		326		
1:45			9	33	8	45	78	13:45			244	981	283	1195	2176
2:00			7		10			14:00			283		278		
2:15			5		14			14:15			261		229		
2:30			10		16			14:30			258		263		
2:45			5	27	9	49	76	14:45			235	1037	293	1063	2100
3:00			7		8			15:00			252		276		
3:15			5		13			15:15			245		287		
3:30			9		15			15:30			250		263		
3:45			9	30	8	44	74	15:45			248	995	319	1145	2140
4:00			12		7			16:00			273		316		
4:15			15		13			16:15			319		276		
4:30			25		21			16:30			302		311		
4:45			41	93	27	68	161	16:45			313	1207	290	1193	2400
5:00			52		28			17:00			323		314		
5:15			67		41			17:15			356		313		
5:30			62		57			17:30			374		349		
5:45			68	249	54	180	429	17:45			309	1362	353	1329	2691
6:00			86		58			18:00			303		316		
6:15			104		70			18:15			256		332		
6:30			176		97			18:30			289		283		
6:45			197	563	134	359	922	18:45			259	1107	282	1213	2320
7:00			191		186			19:00			277		283		
7:15			189		201			19:15			226		265		
7:30			207		268			19:30			180		190		
7:45			224	811	218	873	1684	19:45			175	858	214	952	1810
8:00			251		234			20:00			165		155		
8:15			272		271			20:15			150		171		
8:30			261		262			20:30			116		156		
8:45			248	1032	256	1023	2055	20:45			118	549	127	609	1158
9:00			217		233			21:00			123		154		
9:15			208		198			21:15			104		127		
9:30			200		209			21:30			97		106		
9:45			195	820	194	834	1654	21:45			105	429	72	459	888
10:00			188		226			22:00			71		80		
10:15			213		228			22:15			79		86		
10:30			232		244			22:30			78		54		
10:45			217	850	194	892	1742	22:45			70	298	47	267	565
11:00			218		252			23:00			67		42		
11:15			238		235			23:15			45		53		
11:30			257		232			23:30			32		41		
11:45			242	955	238	957	1912	23:45			26	170	35	171	341
Total Vol.				5507		5417	**10924**					10071		10767	**20838**

Daily Totals

NB	SB	EB	WB	Combined
		15578	16184	**31762**

			AM				PM		
Split %			50.4%	49.6%	**34.4%**		48.3%	51.7%	**65.6%**
Peak Hour	0:30	0:30	11:45	11:45	**11:45**		16:45	17:30	**17:00**
Volume			1064	1092	**2156**		1366	1350	**2691**
P.H.F.			0.88	0.89	**0.88**		0.91	0.96	**0.93**

Tuesday, September 24, 2019 CITY: Santa Ana PROJECT: SC

ADT255 TUSTIN AVE from FOURTH ST to FRUIT ST. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB		SB		EB	WB		PM Period	NB		SB		EB	WB	
0:00	12		11					12:00	197		198				
0:15	12		15					12:15	182		177				
0:30	12		12					12:30	192		195				
0:45	8	44	9	47			91	12:45	153	724	197	767			1491
1:00	6		4					13:00	178		196				
1:15	9		11					13:15	189		202				
1:30	7		8					13:30	199		181				
1:45	7	29	4	27			56	13:45	204	770	154	733			1503
2:00	5		7					14:00	185		162				
2:15	7		5					14:15	183		167				
2:30	3		7					14:30	192		153				
2:45	6	21	4	23			44	14:45	259	819	145	627			1446
3:00	3		6					15:00	259		176				
3:15	8		13					15:15	264		196				
3:30	8		6					15:30	286		191				
3:45	9	28	5	30			58	15:45	269	1078	187	750			1828
4:00	18		13					16:00	263		181				
4:15	24		10					16:15	316		152				
4:30	13		24					16:30	319		202				
4:45	24	79	25	72			151	16:45	311	1209	206	741			1950
5:00	27		27					17:00	327		247				
5:15	41		26					17:15	317		205				
5:30	30		57					17:30	340		201				
5:45	41	139	49	159			298	17:45	371	1355	166	819			2174
6:00	51		91					18:00	361		133				
6:15	65		130					18:15	332		110				
6:30	100		205					18:30	309		147				
6:45	102	318	216	642			960	18:45	296	1298	113	503			1801
7:00	115		310					19:00	189		105				
7:15	119		309					19:15	166		115				
7:30	138		320					19:30	143		106				
7:45	164	536	329	1268			1804	19:45	95	593	85	411			1004
8:00	245		354					20:00	96		74				
8:15	235		333					20:15	97		73				
8:30	199		337					20:30	91		84				
8:45	239	918	334	1358			2276	20:45	73	357	62	293			650
9:00	183		266					21:00	60		74				
9:15	187		240					21:15	77		51				
9:30	153		197					21:30	82		61				
9:45	197	720	195	898			1618	21:45	51	270	59	245			515
10:00	160		179					22:00	43		56				
10:15	178		159					22:15	44		40				
10:30	173		149					22:30	31		34				
10:45	154	665	129	616			1281	22:45	32	150	27	157			307
11:00	161		170					23:00	30		27				
11:15	177		158					23:15	20		35				
11:30	173		198					23:30	24		29				
11:45	172	683	210	736			1419	23:45	15	89	20	111			200
Total Vol.	4180		5876				**10056**		8712		6157				**14869**

Daily Totals

NB	SB	EB	WB	**Combined**
12892	12033			**24925**

	AM			**PM**		
Split %	41.6%	58.4%	**40.3%**	58.6%	41.4%	**59.7%**
Peak Hour	8:00	8:00	**8:00**	17:30	16:30	**17:00**
Volume	918	1358	**2276**	1404	860	**2174**
P.H.F.	0.94	0.96	**0.95**	0.97	0.87	**0.95**

cs@aimtd.com Tell. 714 253 7888

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : FIRST STREET — Site: SANTA ANA

Segment : GRAND AVE TO LYON ST — Date: 09/10/19

Client : CITY SANTA ANA — Day: Tuesday

Interval	EB				WB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	24	74	158	612	58	186	220	963	82	260	378	1,575
12:15	15		153		49		260		64		413	
12:30	14		158		43		225		57		383	
12:45	21		143		36		258		57		401	
01:00	15	48	162	668	30	139	243	976	45	187	405	1,644
01:15	12		172		34		236		46		408	
01:30	5		174		42		241		47		415	
01:45	16		160		33		256		49		416	
02:00	8	43	180	870	24	101	246	1,084	32	144	426	1,954
02:15	10		192		24		248		34		440	
02:30	14		260		25		300		39		560	
02:45	11		238		28		290		39		528	
03:00	6	65	202	800	15	101	289	1,137	21	166	491	1,937
03:15	14		209		28		271		42		480	
03:30	16		177		18		321		34		498	
03:45	29		212		40		256		69		468	
04:00	25	180	206	859	31	157	307	1,301	56	337	513	2,160
04:15	37		218		38		300		75		518	
04:30	50		229		39		326		89		555	
04:45	68		206		49		368		117		574	
05:00	70	375	186	505	70	335	344	1,360	140	710	530	1,865
05:15	78		62		70		342		148		404	
05:30	91		91		83		354		174		445	
05:45	136		166		112		320		248		486	
06:00	126	581	196	749	130	637	288	1,026	256	1,218	484	1,775
06:15	144		188		129		268		273		456	
06:30	148		187		164		240		312		427	
06:45	163		178		214		230		377		408	
07:00	170	608	147	497	240	1,222	222	852	410	1,830	369	1,349
07:15	172		140		305		210		477		350	
07:30	132		106		324		222		456		328	
07:45	134		104		353		198		487		302	
08:00	140	602	98	379	294	1,041	180	635	434	1,643	278	1,014
08:15	146		110		272		163		418		273	
08:30	150		101		234		150		384		251	
08:45	166		70		241		142		407		212	
09:00	162	620	78	283	236	938	130	581	398	1,558	208	864
09:15	150		86		266		162		416		248	
09:30	160		53		208		140		368		193	
09:45	148		66		228		149		376		215	
10:00	164	613	68	202	211	880	124	450	375	1,493	192	652
10:15	158		48		216		132		374		180	
10:30	138		42		220		104		358		146	
10:45	153		44		233		90		386		134	
11:00	144	630	40	138	240	948	94	303	384	1,578	134	441
11:15	148		36		234		64		382		100	
11:30	170		32		232		69		402		101	
11:45	168		30		242		76		410		106	
Totals	4,439		6,562		6,685		10,668		11,124		17,230	
Split%	39.9		38.1		60.1		61.9					
Day Totals		11,001				17,353				28,354		
Day Splits		38.8				61.2						
Peak Hour	06:30		02:30		07:15		04:45		07:15		04:15	
Volume	653		909		1,276		1,408		1,854		2,177	
Factor	0.95		0.87		0.90		0.96		0.95		0.95	

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : FIRST STREET — Site: SANTA ANA

Segment : SANTA ANA FWY TO GOLDEN CIRCLE — Date: 09/11/19

Client : CITY SANTA ANA — Day: Wednesday

Interval	WB				EB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	22	85	132	605	21	78	212	691	43	163	344	1,296
12:15	31		148		24		162		55		310	
12:30	17		168		17		165		34		333	
12:45	15		157		16		152		31		309	
01:00	24	72	180	669	16	66	161	705	40	138	341	1,374
01:15	14		168		14		168		28		336	
01:30	17		158		19		186		36		344	
01:45	17		163		17		190		34		353	
02:00	18	54	172	664	20	56	164	732	38	110	336	1,396
02:15	17		164		12		166		29		330	
02:30	14		184		11		203		25		387	
02:45	5		144		13		199		18		343	
03:00	12	57	192	725	12	76	196	826	24	133	388	1,551
03:15	15		170		21		199		36		369	
03:30	14		169		19		203		33		372	
03:45	16		194		24		228		40		422	
04:00	14	80	222	795	21	135	188	871	35	215	410	1,666
04:15	14		177		29		232		43		409	
04:30	32		188		45		229		77		417	
04:45	20		208		40		222		60		430	
05:00	41	199	222	817	58	247	250	978	99	446	472	1,795
05:15	46		222		58		240		104		462	
05:30	54		186		44		228		98		414	
05:45	58		187		87		260		145		447	
06:00	62	354	194	654	90	448	251	860	152	802	445	1,514
06:15	69		170		90		224		159		394	
06:30	91		160		116		210		207		370	
06:45	132		130		152		175		284		305	
07:00	155	811	144	503	120	484	138	482	275	1,295	282	985
07:15	176		138		115		130		291		268	
07:30	240		119		136		122		376		241	
07:45	240		102		113		92		353		194	
08:00	192	787	94	342	172	742	102	400	364	1,529	196	742
08:15	216		94		186		92		402		186	
08:30	191		82		166		119		357		201	
08:45	188		72		218		87		406		159	
09:00	164	612	82	277	192	659	77	281	356	1,271	159	558
09:15	150		67		146		60		296		127	
09:30	160		62		157		78		317		140	
09:45	138		66		164		66		302		132	
10:00	140	497	66	218	130	533	56	205	270	1,030	122	423
10:15	127		57		143		61		270		118	
10:30	126		61		134		46		260		107	
10:45	104		34		126		42		230		76	
11:00	160	563	38	138	162	602	37	129	322	1,165	75	267
11:15	134		35		130		36		264		71	
11:30	123		27		162		26		285		53	
11:45	146		38		148		30		294		68	
Totals	4,171		6,407		4,126		7,160		8,297		13,567	
Split%	50.3		47.2		49.7		52.8					
Day Totals		10,578				11,286				21,864		
Day Splits		48.4				51.6						
Peak Hour	07:30		04:30		08:15		05:15		08:00		05:00	
Volume	888		840		762		979		1,529		1,795	
Factor	0.93		0.95		0.87		0.94		0.94		0.95	

* Data File : D1909141

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : FAIRVIEW STREET — Site: SANTA ANA

Segment : HARVARD AVE TO EDINGER AVE — Date: 09/25/19

Client : CITY SANTA ANA — Day: Wednesday

Interval	SB				NB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	18	72	204	942	56	175	268	969	74	247	472	1,911
12:15	21		254		44		211		65		465	
12:30	16		236		41		246		57		482	
12:45	17		248		34		244		51		492	
01:00	10	52	247	991	29	106	282	1,237	39	158	529	2,228
01:15	10		227		35		280		45		507	
01:30	16		245		21		328		37		573	
01:45	16		272		21		347		37		619	
02:00	14	66	283	1,116	39	110	440	1,615	53	176	723	2,731
02:15	16		265		23		352		39		617	
02:30	18		290		12		426		30		716	
02:45	18		278		36		397		54		675	
03:00	12	135	246	886	25	99	406	1,781	37	234	652	2,667
03:15	23		232		21		402		44		634	
03:30	42		206		21		475		63		681	
03:45	58		202		32		498		90		700	
04:00	64	471	216	841	43	183	462	1,914	107	654	678	2,755
04:15	68		226		38		486		106		712	
04:30	139		195		44		467		183		662	
04:45	200		204		58		499		258		703	
05:00	127	816	210	867	60	329	502	1,913	187	1,145	712	2,780
05:15	167		224		76		470		243		694	
05:30	228		204		96		467		324		671	
05:45	294		229		97		474		391		703	
06:00	218	1,231	195	685	124	600	454	1,687	342	1,831	649	2,372
06:15	278		170		140		399		418		569	
06:30	332		178		160		460		492		638	
06:45	403		142		176		374		579		516	
07:00	400	1,487	130	509	275	1,278	324	1,115	675	2,765	454	1,624
07:15	428		154		276		283		704		437	
07:30	350		113		375		256		725		369	
07:45	309		112		352		252		661		364	
08:00	370	1,480	107	403	250	886	238	829	620	2,366	345	1,232
08:15	374		118		220		207		594		325	
08:30	390		82		208		190		598		272	
08:45	346		96		208		194		554		290	
09:00	253	988	94	324	210	824	217	765	463	1,812	311	1,089
09:15	271		88		190		206		461		294	
09:30	239		68		234		165		473		233	
09:45	225		74		190		177		415		251	
10:00	182	812	52	190	181	731	136	531	363	1,543	188	721
10:15	205		45		181		159		386		204	
10:30	215		49		195		140		410		189	
10:45	210		44		174		96		384		140	
11:00	195	787	54	146	196	841	86	337	391	1,628	140	483
11:15	197		32		196		97		393		129	
11:30	191		33		226		88		417		121	
11:45	204		27		223		66		427		93	
Totals	8,397		7,900		6,162		14,693		14,559		22,593	
Split%	57.7		35.0		42.3		65.0					
Day Totals		16,297				20,855				37,152		
Day Splits		43.9				56.1						
Peak Hour	06:45		02:00		07:00		04:15		07:00		04:15	
Volume	1,581		1,116		1,278		1,954		2,765		2,789	
Factor	0.92		0.96		0.85		0.97		0.95		0.98	

* Data File : D1909070

1990

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : FAIRVIEW STREET — Site: SANTA ANA

Segment : SEGERSTROM AVE TO WARNER AVE — Date: 10/09/19

Client : CITY SANTA ANA — Day: Wednesday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	51	162	218	888	22	96	239	996	73	258	457	1,884
12:15	40		222		30		222		70		444	
12:30	38		222		25		265		63		487	
12:45	33		226		19		270		52		496	
01:00	25	90	292	1,227	11	47	262	1,092	36	137	554	2,319
01:15	28		280		11		246		39		526	
01:30	19		310		14		284		33		594	
01:45	18		345		11		300		29		645	
02:00	17	87	394	1,627	22	81	292	1,070	39	168	686	2,697
02:15	15		380		14		251		29		631	
02:30	19		394		16		279		35		673	
02:45	36		459		29		248		65		707	
03:00	21	77	418	1,738	24	176	277	1,111	45	253	695	2,849
03:15	17		424		22		289		39		713	
03:30	17		428		56		277		73		705	
03:45	22		468		74		268		96		736	
04:00	26	152	518	1,919	58	435	267	1,071	84	587	785	2,990
04:15	25		483		98		234		123		717	
04:30	46		474		128		276		174		750	
04:45	55		444		151		294		206		738	
05:00	60	292	470	1,933	116	856	270	1,071	176	1,148	740	3,004
05:15	48		501		188		274		236		775	
05:30	96		486		269		267		365		753	
05:45	88		476		283		260		371		736	
06:00	86	482	434	1,618	264	1,511	243	879	350	1,993	677	2,497
06:15	132		406		312		208		444		614	
06:30	132		434		420		228		552		662	
06:45	132		344		515		200		647		544	
07:00	192	983	302	1,032	490	2,095	232	743	682	3,078	534	1,775
07:15	213		290		556		182		769		472	
07:30	293		228		555		179		848		407	
07:45	285		212		494		150		779		362	
08:00	219	829	190	726	549	2,197	138	558	768	3,026	328	1,284
08:15	218		196		558		142		776		338	
08:30	196		160		557		152		753		312	
08:45	196		180		533		126		729		306	
09:00	214	782	156	616	374	1,270	94	405	588	2,052	250	1,021
09:15	184		162		334		128		518		290	
09:30	198		154		284		109		482		263	
09:45	186		144		278		74		464		218	
10:00	192	720	156	522	192	900	74	243	384	1,620	230	765
10:15	174		144		234		52		408		196	
10:30	176		128		234		50		410		178	
10:45	178		94		240		67		418		161	
11:00	167	778	76	276	213	875	38	149	380	1,653	114	425
11:15	205		72		198		33		403		105	
11:30	209		66		220		41		429		107	
11:45	197		62		244		37		441		99	
Totals	5,434		14,122		10,539		9,388		15,973		23,510	
Split%	34.0		60.1		66.0		39.9					
Day Totals		19,556				19,927				39,483		
Day Splits		49.5				50.5						
Peak Hour	07:30		03:45		08:00		01:30		07:30		04:45	
Volume	1,015		1,943		2,197		1,127		3,171		3,006	
Factor	0.87		0.94		0.98		0.94		0.93		0.97	

Wednesday, September 11, 2019 Location: Santa Ana PROJECT:

ADT147 MacARTHUR BL from SUSAN ST to FAIRVIEW ST. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB	SB	EB		WB			PM Period	NB	SB	EB		WB		
0:30			16		15			12:00			217		153		
0:15			6		15			12:15			221		175		
0:30			9		14			12:30			213		145		
0:45			6	37	4	48	85	12:45			192	843	200	673	1516
1:00			7		6			13:00			191		187		
1:15			6		8			13:15			186		198		
1:30			13		5			13:30			217		199		
1:45			9	35	7	26	61	13:45			178	772	218	802	1574
2:00			6		4			14:00			210		215		
2:15			6		4			14:15			163		218		
2:30			11		0			14:30			259		210		
2:45			9	32	0	8	40	14:45			232	864	232	875	1739
3:00			18		6			15:00			220		315		
3:15			9		5			15:15			202		278		
3:30			21		6			15:30			232		290		
3:45			31	79	8	25	104	15:45			177	831	374	1257	2088
4:00			35		21			16:00			236		362		
4:15			25		10			16:15			235		385		
4:30			23		41			16:30			258		383		
4:45			42	125	64	136	261	16:45			234	963	386	1516	2479
5:00			35		32			17:00			313		414		
5:15			32		36			17:15			249		358		
5:30			62		62			17:30			262		341		
5:45			60	189	83	213	402	17:45			205	1029	407	1520	2549
6:00			62		55			18:00			219		379		
6:15			71		68			18:15			205		318		
6:30			129		73			18:30			180		240		
6:45			154	416	96	292	708	18:45			174	778	265	1202	1980
7:00			210		101			19:00			165		212		
7:15			246		127			19:15			131		162		
7:30			337		141			19:30			164		154		
7:45			390	1183	191	560	1743	19:45			104	564	135	663	1227
8:00			257		174			20:00			105		123		
8:15			336		132			20:15			127		81		
8:30			298		148			20:30			98		56		
8:45			291	1182	159	613	1795	20:45			96	426	86	346	772
9:00			219		144			21:00			106		85		
9:15			186		141			21:15			80		52		
9:30			169		142			21:30			74		51		
9:45			145	719	144	571	1290	21:45			57	317	34	222	539
10:00			143		142			22:00			64		55		
10:15			144		118			22:15			40		49		
10:30			158		139			22:30			41		34		
10:45			163	608	164	563	1171	22:45			33	178	29	167	345
11:00			180		117			23:00			40		25		
11:15			171		136			23:15			25		34		
11:30			165		152			23:30			19		26		
11:45			209	725	171	576	1301	23:45			11	95	26	111	206
Total Vol.				5330		3631	**8961**					7660		9354	**17014**

Daily Totals

NB	SB	EB	WB	**Combined**
		12990	12985	**25975**

			AM				**PM**		
Split %			59.5%	40.5%	**34.5%**		45.0%	55.0%	**65.5%**
Peak Hour	0:30	0:30	7:30	11:30	**7:30**		16:45	16:15	**16:15**
Volume			1320	651	**1958**		1058	1568	**2608**
P.H.F.			0.85	0.93	**0.84**		0.85	0.95	**0.90**

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : EDINGER AVENUE
Segment : FAIRVIEW ST TO RAITT ST
Client : CITY SANTA ANA

Site: SANTA ANA
Date: 09/17/19
Day: Tuesday

Interval	EB				WB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	35	112	166	666	34	104	172	665	69	216	338	1,331
12:15	32		158		22		160		54		318	
12:30	30		186		21		175		51		361	
12:45	15		156		27		158		42		314	
01:00	17	79	193	791	14	57	162	750	31	136	355	1,541
01:15	18		182		18		168		36		350	
01:30	30		202		12		194		42		396	
01:45	14		214		13		226		27		440	
02:00	8	45	194	961	8	55	202	865	16	100	396	1,826
02:15	8		229		14		204		22		433	
02:30	15		276		15		207		30		483	
02:45	14		262		18		252		32		514	
03:00	5	48	300	1,073	14	78	264	1,093	19	126	564	2,166
03:15	8		272		12		280		20		552	
03:30	16		231		20		273		36		504	
03:45	19		270		32		276		51		546	
04:00	24	163	298	1,053	22	199	284	1,197	46	362	582	2,250
04:15	31		234		33		308		64		542	
04:30	56		269		66		294		122		563	
04:45	52		252		78		311		130		563	
05:00	49	337	280	1,103	62	379	317	1,213	111	716	597	2,316
05:15	64		282		84		288		148		570	
05:30	104		276		116		287		220		563	
05:45	120		265		117		321		237		586	
06:00	105	624	251	951	100	558	296	1,112	205	1,182	547	2,063
06:15	146		224		134		292		280		516	
06:30	176		252		152		242		328		494	
06:45	197		224		172		282		369		506	
07:00	244	1,080	204	856	198	905	240	797	442	1,985	444	1,653
07:15	308		218		216		202		524		420	
07:30	273		238		239		196		512		434	
07:45	255		196		252		159		507		355	
08:00	222	900	236	796	218	727	166	645	440	1,627	402	1,441
08:15	238		197		169		188		407		385	
08:30	220		195		176		149		396		344	
08:45	220		168		164		142		384		310	
09:00	164	669	155	561	150	580	126	464	314	1,249	281	1,025
09:15	188		144		156		147		344		291	
09:30	172		120		138		109		310		229	
09:45	145		142		136		82		281		224	
10:00	150	620	105	328	153	644	99	311	303	1,264	204	639
10:15	146		94		174		81		320		175	
10:30	167		70		145		71		312		141	
10:45	157		59		172		60		329		119	
11:00	148	635	58	200	170	614	50	164	318	1,249	108	364
11:15	158		59		162		44		320		103	
11:30	151		42		148		39		299		81	
11:45	178		41		134		31		312		72	
Totals	5,312		9,339		4,900		9,276		10,212		18,615	
Split%	52.0		50.2		48.0		49.8					
Day Totals		14,651				14,176				28,827		
Day Splits		50.8				49.2						
Peak Hour	07:00		02:30		07:15		04:15		07:00		05:00	
Volume	1,080		1,110		925		1,230		1,985		2,316	
Factor	0.88		0.93		0.92		0.97		0.95		0.97	

Data File : D1909050

1993

Thursday, October 10, 2019 Location: Santa Ana PROJECT:

ADT174 McFADDEN AVE from FAIRVIEW ST to RAITT ST. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB	SB	EB		WB			PM Period	NB	SB	EB		WB		
0:30			21		35			12:00			103		104		
0:15			29		24			12:15			113		123		
0:30			23		20			12:30			98		113		
0:45			19	92	13	92	184	12:45			110	424	130	470	894
1:00			16		9			13:00			140		109		
1:15			10		11			13:15			158		121		
1:30			11		13			13:30			148		132		
1:45			14	51	12	45	96	13:45			156	602	126	488	1090
2:00			13		8			14:00			136		129		
2:15			8		5			14:15			169		132		
2:30			7		10			14:30			174		157		
2:45			12	40	16	39	79	14:45			184	663	184	602	1265
3:00			13		11			15:00			221		183		
3:15			19		13			15:15			193		203		
3:30			12		14			15:30			176		191		
3:45			15	59	17	55	114	15:45			187	777	205	782	1559
4:00			10		23			16:00			208		195		
4:15			19		25			16:15			194		175		
4:30			29		23			16:30			187		179		
4:45			35	93	41	112	205	16:45			181	770	190	739	1509
5:00			39		53			17:00			195		204		
5:15			58		62			17:15			204		195		
5:30			73		78			17:30			213		222		
5:45			87	257	86	279	536	17:45			217	829	216	837	1666
6:00			85		84			18:00			223		230		
6:15			91		78			18:15			197		200		
6:30			106		82			18:30			186		207		
6:45			127	409	101	345	754	18:45			181	787	189	826	1613
7:00			140		123			19:00			198		186		
7:15			178		164			19:15			179		175		
7:30			219		189			19:30			187		156		
7:45			229	766	196	672	1438	19:45			161	725	140	657	1382
8:00			218		187			20:00			124		117		
8:15			186		159			20:15			115		103		
8:30			174		122			20:30			134		111		
8:45			157	735	103	571	1306	20:45			110	483	100	431	914
9:00			125		98			21:00			102		98		
9:15			130		104			21:15			98		104		
9:30			108		91			21:30			94		68		
9:45			114	477	100	393	870	21:45			85	379	95	365	744
10:00			100		93			22:00			77		61		
10:15			109		98			22:15			79		71		
10:30			115		101			22:30			67		52		
10:45			116	440	109	401	841	22:45			68	291	38	222	513
11:00			123		117			23:00			56		27		
11:15			131		108			23:15			52		35		
11:30			119		107			23:30			47		24		
11:45			99	472	115	447	919	23:45			32	187	25	111	298
Total Vol.			3891		3451		**7342**				6917		6530		**13447**

Daily Totals

NB	SB	EB	WB	Combined
		10808	9981	**20789**

			AM			PM		
Split %			53.0%	47.0%	**35.3%**	51.4%	48.6%	**64.7%**
Peak Hour	0:30	0:30	7:30	7:15	**7:30**	17:15	17:30	**17:15**
Volume			852	736	**1583**	857	868	**1720**
P.H.F.			0.93	0.94	**0.93**	0.96	0.94	**0.95**

Thursday, October 03, 2019 Location: Santa Ana PROJECT:

ADT148 MacARTHUR BL from FAIRVIEW ST to RAITT ST. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB	SB	EB		WB			PM Period	NB	SB	EB		WB		
0:30			13		17			12:00			199		185		
0:15			15		15			12:15			186		197		
0:30			12		13			12:30			170		183		
0:45			11	51	16	61	112	12:45			173	728	194	759	1487
1:00			7		12			13:00			168		199		
1:15			6		8			13:15			201		214		
1:30			8		11			13:30			234		221		
1:45			4	25	7	38	63	13:45			208	811	237	871	1682
2:00			4		5			14:00			206		264		
2:15			2		10			14:15			229		242		
2:30			8		11			14:30			312		278		
2:45			14	28	9	35	63	14:45			304	1051	341	1125	2176
3:00			13		6			15:00			281		359		
3:15			11		8			15:15			242		342		
3:30			15		12			15:30			235		336		
3:45			21	60	18	44	104	15:45			214	972	407	1444	2416
4:00			31		15			16:00			206		400		
4:15			34		23			16:15			211		395		
4:30			39		27			16:30			214		414		
4:45			46	150	37	102	252	16:45			231	862	463	1672	2534
5:00			42		43			17:00			240		486		
5:15			57		45			17:15			238		452		
5:30			76		41			17:30			242		463		
5:45			68	243	48	177	420	17:45			214	934	461	1862	2796
6:00			84		52			18:00			218		451		
6:15			96		50			18:15			196		355		
6:30			124		78			18:30			167		374		
6:45			206	510	107	287	797	18:45			139	720	311	1491	2211
7:00			241		112			19:00			156		273		
7:15			353		199			19:15			124		223		
7:30			429		266			19:30			127		184		
7:45			435	1458	343	920	2378	19:45			104	511	136	816	1327
8:00			407		257			20:00			118		104		
8:15			320		168			20:15			105		112		
8:30			356		152			20:30			94		85		
8:45			322	1405	131	708	2113	20:45			103	420	96	397	817
9:00			252		127			21:00			95		86		
9:15			153		132			21:15			100		75		
9:30			142		121			21:30			96		72		
9:45			145	692	136	516	1208	21:45			68	359	67	300	659
10:00			136		108			22:00			54		62		
10:15			127		114			22:15			46		54		
10:30			134		119			22:30			41		46		
10:45			125	522	127	468	990	22:45			34	175	41	203	378
11:00			135		157			23:00			31		36		
11:15			153		154			23:15			23		45		
11:30			188		152			23:30			17		34		
11:45			213	689	163	626	1315	23:45			12	83	28	143	226
Total Vol.				5833		3982	**9815**					7626		11083	**18709**

Daily Totals

NB	SB	EB	WB	**Combined**
		13459	15065	**28524**

			AM				**PM**		
Split %			59.4%	40.6%	**34.4%**		40.8%	59.2%	**65.6%**
Peak Hour	0:30	0:30	7:15	7:15	**7:15**		14:30	16:45	**16:45**
Volume			1624	1065	**2689**		1139	1864	**2815**
P.H.F.			0.93	0.78	**0.86**		0.91	0.96	**0.97**

Wednesday, October 02, 2019 Location: Santa Ana PROJECT:

ADT222 SEGERSTROM AVE from FAIRVIEW ST to RAITT ST. **Prepared by AimTD tel. 714 753 7888**

AM Period	NB	SB	EB		WB			PM Period	NB	SB	EB		WB		
0:30			34		35			12:00			94		89		
0:15			37		32			12:15			86		101		
0:30			26		29			12:30			68		85		
0:45			31	128	31	127	255	12:45			71	319	88	363	682
1:00			34		25			13:00			75		108		
1:15			26		21			13:15			78		93		
1:30			18		22			13:30			85		95		
1:45			20	98	19	87	185	13:45			104	342	113	409	751
2:00			19		17			14:00			87		105		
2:15			10		10			14:15			78		104		
2:30			11		12			14:30			90		101		
2:45			5	45	6	45	90	14:45			86	341	105	415	756
3:00			3		8			15:00			96		112		
3:15			5		7			15:15			105		103		
3:30			4		6			15:30			108		123		
3:45			6	18	11	32	50	15:45			110	419	147	485	904
4:00			7		12			16:00			106		139		
4:15			6		10			16:15			114		167		
4:30			4		9			16:30			195		184		
4:45			8	25	8	39	64	16:45			175	590	271	761	1351
5:00			6		6			17:00			189		242		
5:15			11		7			17:15			173		196		
5:30			16		8			17:30			191		207		
5:45			15	48	12	33	81	17:45			178	731	242	887	1618
6:00			21		8			18:00			172		257		
6:15			26		17			18:15			176		280		
6:30			27		23			18:30			163		268		
6:45			31	105	29	77	182	18:45			157	668	282	1087	1755
7:00			34		31			19:00			186		302		
7:15			42		51			19:15			188		330		
7:30			59		72			19:30			207		294		
7:45			84	219	68	222	441	19:45			193	774	280	1206	1980
8:00			81		63			20:00			198		271		
8:15			95		70			20:15			174		245		
8:30			134		80			20:30			137		179		
8:45			153	463	112	325	788	20:45			106	615	180	875	1490
9:00			156		135			21:00			96		156		
9:15			231		185			21:15			78		132		
9:30			276		249			21:30			86		83		
9:45			257	920	297	866	1786	21:45			89	349	73	444	793
10:00			252		251			22:00			101		80		
10:15			204		142			22:15			72		72		
10:30			178		123			22:30			61		70		
10:45			132	766	134	650	1416	22:45			52	286	64	286	572
11:00			104		108			23:00			56		54		
11:15			112		100			23:15			45		43		
11:30			101		96			23:30			37		42		
11:45			91	408	84	388	796	23:45			35	173	37	176	349
Total Vol.			3243		2891	**6134**					5607		7394		**13001**

Daily Totals

NB	SB	EB	WB	Combined
		8850	10285	**19135**

			AM			PM		
Split %			52.9%	47.1%	**32.1%**	43.1%	56.9%	**67.9%**
Peak Hour	0:30	0:30	9:15	9:15	**9:15**	19:15	18:45	**19:00**
Volume			1016	982	**1998**	786	1208	**1980**
P.H.F.			0.92	0.83	**0.90**	0.95	0.92	**0.96**

pacific@aimtd.com Tell. 714 753 7888

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : BRISTOL STREET — Site: SANTA ANA

Segment : WARNER AVE TO EDINGER AVE — Date: 09/17/19

Client : CITY SANTA ANA — Day: Tuesday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	72	181	226	944	34	130	234	885	106	311	460	1,829
12:15	42		228		40		222		82		450	
12:30	29		225		34		222		63		447	
12:45	38		265		22		207		60		472	
01:00	34	123	251	1,071	28	86	212	892	62	209	463	1,963
01:15	23		272		18		188		41		460	
01:30	38		282		24		246		62		528	
01:45	28		266		16		246		44		512	
02:00	29	120	279	1,293	18	83	233	1,003	47	203	512	2,296
02:15	30		288		23		236		53		524	
02:30	24		344		16		266		40		610	
02:45	37		382		26		268		63		650	
03:00	29	92	402	1.547	22	124	332	1.088	51	216	734	2.635
03:15	18		384		20		236		38		620	
03:30	21		384		32		278		53		662	
03:45	24		377		50		242		74		619	
04:00	22	121	394	1,549	51	395	228	924	73	516	622	2,473
04:15	30		378		84		226		114		604	
04:30	33		376		146		216		179		592	
04:45	36		401		114		254		150		655	
05:00	44	220	426	1,569	99	606	266	1,057	143	826	692	2,626
05:15	48		394		115		241		163		635	
05:30	64		377		186		272		250		649	
05:45	64		372		206		278		270		650	
06:00	94	425	362	1,401	211	1,195	234	933	305	1,620	596	2,334
06:15	90		379		234		228		324		607	
06:30	108		336		328		239		436		575	
06:45	133		324		422		232		555		556	
07:00	190	1,030	254	1,057	421	1,708	226	858	611	2,738	480	1,915
07:15	236		283		432		252		668		535	
07:30	302		264		425		206		727		470	
07:45	302		256		430		174		732		430	
08:00	309	1.125	230	793	384	1.640	198	712	693	2.765	428	1.505
08:15	346		192		375		199		721		391	
08:30	300		187		445		177		745		364	
08:45	170		184		436		138		606		322	
09:00	165	704	166	649	310	1,110	146	578	475	1,814	312	1,227
09:15	169		171		256		143		425		314	
09:30	194		164		276		145		470		309	
09:45	176		148		268		144		444		292	
10:00	170	718	151	531	220	866	88	367	390	1,584	239	898
10:15	188		126		212		103		400		229	
10:30	182		110		206		90		388		200	
10:45	178		144		228		86		406		230	
11:00	198	870	91	347	229	924	68	225	427	1,794	159	572
11:15	241		100		219		57		460		157	
11:30	221		82		234		50		455		132	
11:45	210		74		242		50		452		124	
Totals	5,729		12,751		8,867		9,522		14,596		22,273	
Split%	39.3		57.2		60.7		42.8					
Day Totals		18,480				18,389				36,869		
Day Splits		50.1				49.9						
Peak Hour	07:30		04:45		07:00		02:45		07:45		02:45	
Volume	1,259		1,598		1,708		1,114		2,891		2,666	
Factor	0.91		0.94		0.99		0.84		0.97		0.91	

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : BRISTOL STREET — Site: SANTA ANA

Segment : SEGERSTROM AVE TO WARNER AVE — Date: 09/17/19

Client : CITY SANTA ANA — Day: Tuesday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	58	170	250	1,064	40	130	261	1,063	98	300	511	2,127
12:15	48		256		36		264		84		520	
12:30	25		278		31		288		56		566	
12:45	39		280		23		250		62		530	
01:00	34	113	256	1,083	20	83	256	1,022	54	196	512	2,105
01:15	30		254		22		214		52		468	
01:30	30		304		28		280		58		584	
01:45	19		269		13		272		32		541	
02:00	25	90	304	1,360	20	81	228	1,087	45	171	532	2,447
02:15	22		298		20		267		42		565	
02:30	20		342		23		282		43		624	
02:45	23		416		18		310		41		726	
03:00	18	58	409	1,493	18	113	342	1,115	36	171	751	2,608
03:15	12		340		19		257		31		597	
03:30	13		320		32		258		45		578	
03:45	15		424		44		258		59		682	
04:00	15	75	390	1,460	53	396	212	922	68	471	602	2,382
04:15	24		365		96		248		120		613	
04:30	14		373		120		234		134		607	
04:45	22		332		127		228		149		560	
05:00	26	150	400	1,480	106	604	271	1,092	132	754	671	2,572
05:15	32		370		121		277		153		647	
05:30	40		364		196		272		236		636	
05:45	52		346		181		272		233		618	
06:00	64	340	372	1,324	204	1,218	247	987	268	1,558	619	2,311
06:15	70		332		224		252		294		584	
06:30	98		308		348		234		446		542	
06:45	108		312		442		254		550		566	
07:00	158	913	277	1,092	408	1,907	235	922	566	2,820	512	2,014
07:15	212		290		491		252		703		542	
07:30	243		269		525		216		768		485	
07:45	300		256		483		219		783		475	
08:00	292	999	220	804	444	1,814	220	730	736	2,813	440	1,534
08:15	326		205		441		192		767		397	
08:30	223		201		464		166		687		367	
08:45	158		178		465		152		623		330	
09:00	154	685	192	648	394	1,306	124	511	548	1,991	316	1,159
09:15	184		162		306		131		490		293	
09:30	171		137		292		130		463		267	
09:45	176		157		314		126		490		283	
10:00	177	741	141	493	254	942	86	379	431	1,683	227	872
10:15	170		132		229		98		399		230	
10:30	200		106		215		107		415		213	
10:45	194		114		244		88		438		202	
11:00	239	989	104	352	244	1,034	58	197	483	2,023	162	549
11:15	276		100		232		43		508		143	
11:30	226		84		280		54		506		138	
11:45	248		64		278		42		526		106	
Totals	5,323		12,653		9,628		10,027		14,951		22,680	
Split%	35.6		55.8		64.4		44.2					
Day Totals		17,976				19,655				37,631		
Day Splits		47.8				52.2						
Peak Hour	07:30		03:45		07:15		02:15		07:30		02:30	
Volume	1,161		1,552		1,943		1,201		3,054		2,698	
Factor	0.89		0.92		0.93		0.88		0.98		0.90	

* Data File : D1909014

Wednesday, October 02, 2019 Location: Santa Ana PROJECT:

ADT261 WARNER AVE from RAITT ST to BRISTOL ST **Prepared by AimTD tel. 714 253 7888**

AM Period	NB	SB	EB		WB			PM Period	NB	SB	EB		WB		
0:30			32		32			12:00			182		187		
0:15			33		25			12:15			187		186		
0:30			25		29			12:30			177		222		
0:45			31	121	16	102	223	12:45			205	751	204	799	1550
1:00			14		24			13:00			189		232		
1:15			11		19			13:15			223		281		
1:30			13		14			13:30			240		270		
1:45			8	46	12	69	115	13:45			323	975	323	1106	2081
2:00			15		15			14:00			343		311		
2:15			7		14			14:15			386		303		
2:30			15		18			14:30			326		314		
2:45			16	53	13	60	113	14:45			312	1367	341	1269	2636
3:00			22		11			15:00			353		351		
3:15			23		13			15:15			321		376		
3:30			21		15			15:30			329		364		
3:45			34	100	18	57	157	15:45			319	1322	362	1453	2775
4:00			50		21			16:00			277		371		
4:15			55		31			16:15			298		395		
4:30			75		74			16:30			291		378		
4:45			111	291	85	211	502	16:45			321	1187	395	1539	2726
5:00			104		78			17:00			318		420		
5:15			130		96			17:15			348		421		
5:30			185		114			17:30			308		384		
5:45			204	623	127	415	1038	17:45			289	1263	397	1622	2885
6:00			195		121			18:00			296		393		
6:15			184		128			18:15			260		356		
6:30			231		139			18:30			275		324		
6:45			302	912	195	583	1495	18:45			231	1062	285	1358	2420
7:00			313		228			19:00			186		256		
7:15			363		324			19:15			165		241		
7:30			395		423			19:30			143		224		
7:45			408	1479	437	1412	2891	19:45			141	635	179	900	1535
8:00			274		313			20:00			134		165		
8:15			276		245			20:15			125		158		
8:30			241		213			20:30			114		132		
8:45			213	1004	194	965	1969	20:45			124	497	128	583	1080
9:00			202		185			21:00			118		124		
9:15			151		174			21:15			107		110		
9:30			179		170			21:30			114		116		
9:45			152	684	164	693	1377	21:45			96	435	98	448	883
10:00			153		169			22:00			86		94		
10:15			193		187			22:15			66		84		
10:30			175		168			22:30			71		100		
10:45			162	683	167	691	1374	22:45			40	263	78	356	619
11:00			177		189			23:00			45		64		
11:15			157		196			23:15			48		49		
11:30			171		185			23:30			34		51		
11:45			163	668	174	744	1412	23:45			29	156	37	201	357
Total Vol.				6664		6002	**12666**					9913		11634	**21547**

Daily Totals

NB	SB	EB	WB	**Combined**
		16577	17636	**34213**

			AM				**PM**		
Split %			52.6%	47.4%	**37.0%**		46.0%	54.0%	**63.0%**
Peak Hour	0:30	0:30	7:00	7:15	**7:15**		13:45	17:00	**16:45**
Volume			1479	1497	**2937**		1378	1622	**2915**
P.H.F.			0.91	0.86	**0.87**		0.89	0.96	**0.95**

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : BRISTOL STREET — Site: SANTA ANA

Segment : SUNFLOWER AVE TO MACARTHUR BL — Date: 09/17/19

Client : CITY SANTA ANA — Day: Tuesday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	56	191	290	1,165	31	116	300	1,251	87	307	590	2,416
12:15	46		288		27		284		73		572	
12:30	51		296		34		324		85		620	
12:45	38		291		24		343		62		634	
01:00	31	96	290	1,166	24	76	300	1,126	55	172	590	2,292
01:15	26		274		13		260		39		534	
01:30	22		302		22		260		44		562	
01:45	17		300		17		306		34		606	
02:00	31	105	286	1,183	18	67	268	1,003	49	172	554	2,186
02:15	22		293		15		264		37		557	
02:30	20		298		14		232		34		530	
02:45	32		306		20		239		52		545	
03:00	10	43	313	1,196	15	99	268	955	25	142	581	2,151
03:15	14		266		22		230		36		496	
03:30	12		300		27		229		39		529	
03:45	7		317		35		228		42		545	
04:00	18	76	322	1,261	40	333	218	846	58	409	540	2,107
04:15	16		336		75		205		91		541	
04:30	20		303		108		227		128		530	
04:45	22		300		110		196		132		496	
05:00	36	171	338	1,277	96	555	200	872	132	726	538	2,149
05:15	34		323		129		214		163		537	
05:30	43		318		162		220		205		538	
05:45	58		298		168		238		226		536	
06:00	59	313	291	1,193	186	1,017	226	857	245	1,330	517	2,050
06:15	66		322		176		228		242		550	
06:30	84		286		311		183		395		469	
06:45	104		294		344		220		448		514	
07:00	110	549	261	1,019	328	1,535	222	854	438	2,084	483	1,873
07:15	147		234		368		218		515		452	
07:30	124		262		413		197		537		459	
07:45	168		262		426		217		594		479	
08:00	160	629	212	795	424	1,606	204	738	584	2,235	416	1,533
08:15	195		206		371		208		566		414	
08:30	161		208		409		180		570		388	
08:45	113		169		402		146		515		315	
09:00	140	637	180	639	350	1,093	142	500	490	1,730	322	1,139
09:15	155		164		260		126		415		290	
09:30	168		139		233		122		401		261	
09:45	174		156		250		110		424		266	
10:00	179	751	138	491	228	884	112	362	407	1,635	250	853
10:15	169		128		222		82		391		210	
10:30	193		106		252		84		445		190	
10:45	210		119		182		84		392		203	
11:00	240	1,081	93	332	254	1,052	82	231	494	2,133	175	563
11:15	276		104		258		56		534		160	
11:30	279		75		270		45		549		120	
11:45	286		60		270		48		556		108	
Totals	4,642		11,717		8,433		9,595		13,075		21,312	
Split%	35.5		55.0		64.5		45.0					
Day Totals		16,359				18,028				34,387		
Day Splits		47.6				52.4						
Peak Hour	11:00		04:45		07:30		12:00		07:45		12:00	
Volume	1,081		1,279		1,634		1,251		2,314		2,416	
Factor	0.94		0.95		0.96		0.91		0.97		0.95	

Data File : D1909011

2002

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : FLOWER STREET — Site: SANTA ANA

Segment : SEGERSTROM AVE TO WARNER AVE — Date: 09/10/19

Client : CITY SANTA ANA — Day: Tuesday

Interval	SB				NB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	15	46	70	306	14	59	76	282	29	105	146	588
12:15	10		80		13		65		23		145	
12:30	10		78		19		65		29		143	
12:45	11		78		13		76		24		154	
01:00	3	15	80	295	11	29	96	361	14	44	176	656
01:15	2		60		9		78		11		138	
01:30	3		79		6		94		9		173	
01:45	7		76		3		93		10		169	
02:00	6	23	95	524	3	23	124	627	9	46	219	1,151
02:15	2		99		3		116		5		215	
02:30	5		140		8		172		13		312	
02:45	10		190		9		215		19		405	
03:00	3	34	109	425	8	23	275	935	11	57	384	1,360
03:15	7		113		0		217		7		330	
03:30	7		89		6		208		13		297	
03:45	17		114		9		235		26		349	
04:00	9	118	126	448	5	31	224	1,009	14	149	350	1,457
04:15	10		112		3		290		13		402	
04:30	44		105		9		236		53		341	
04:45	55		105		14		259		69		364	
05:00	30	260	120	506	10	90	320	1,217	40	350	440	1,723
05:15	61		124		20		321		81		445	
05:30	89		132		36		288		125		420	
05:45	80		130		24		288		104		418	
06:00	72	424	106	372	38	185	242	718	110	609	348	1,090
06:15	80		92		29		200		109		292	
06:30	122		92		48		156		170		248	
06:45	150		82		70		120		220		202	
07:00	171	727	92	331	84	538	118	418	255	1,265	210	749
07:15	186		79		127		102		313		181	
07:30	194		84		161		106		355		190	
07:45	176		76		166		92		342		168	
08:00	145	638	80	245	118	308	81	316	263	946	161	561
08:15	160		58		76		92		236		150	
08:30	164		61		62		79		226		140	
08:45	169		46		52		64		221		110	
09:00	115	346	41	146	57	186	47	204	172	532	88	350
09:15	78		38		40		53		118		91	
09:30	80		33		48		52		128		85	
09:45	73		34		41		52		114		86	
10:00	70	245	37	134	55	207	52	174	125	452	89	308
10:15	59		39		58		42		117		81	
10:30	56		30		42		42		98		72	
10:45	60		28		52		38		112		66	
11:00	59	262	11	54	48	253	45	109	107	515	56	163
11:15	62		15		78		28		140		43	
11:30	77		15		57		21		134		36	
11:45	64		13		70		15		134		28	
Totals	3,138		3,786		1,932		6,370		5,070		10,156	
Split%	61.9		37.3		38.1		62.7					
Day Totals		6,924				8,302				15,226		
Day Splits		45.5				54.5						
Peak Hour	07:00		02:30		07:15		05:00		07:15		05:00	
Volume	727		552		572		1,217		1,273		1,723	
Factor	0.94		0.73		0.86		0.95		0.90		0.97	

Data File : D1909101

2003

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : EDINGER AVENUE
Segment : FLOWER ST TO BROADWAY
Client : CITY SANTA ANA

Site: SANTA ANA
Date: 09/24/19
Day: Tuesday

Interval	EB				WB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	36	91	176	638	50	123	237	1,094	86	214	413	1,732
12:15	23		136		19		323		42		459	
12:30	17		148		30		262		47		410	
12:45	15		178		24		272		39		450	
01:00	13	60	157	642	30	99	280	1,289	43	159	437	1,931
01:15	18		150		25		304		43		454	
01:30	11		161		23		339		34		500	
01:45	18		174		21		366		39		540	
02:00	20	59	176	892	15	74	403	1,786	35	133	579	2,678
02:15	17		210		12		413		29		623	
02:30	10		232		20		406		30		638	
02:45	12		274		27		564		39		838	
03:00	20	113	250	948	10	56	480	2,087	30	169	730	3,035
03:15	23		206		6		496		29		702	
03:30	32		226		16		564		48		790	
03:45	38		266		24		547		62		813	
04:00	48	381	244	905	15	116	556	2,316	63	497	800	3,221
04:15	72		210		14		568		86		778	
04:30	124		225		38		574		162		799	
04:45	137		226		49		618		186		844	
05:00	93	664	242	1,036	40	254	594	2,327	133	918	836	3,363
05:15	128		246		48		627		176		873	
05:30	190		280		80		598		270		878	
05:45	253		268		86		508		339		776	
06:00	211	1,018	225	839	86	482	566	1,836	297	1,500	791	2,675
06:15	247		232		98		434		345		666	
06:30	254		194		134		438		388		632	
06:45	306		188		164		398		470		586	
07:00	272	1,034	173	671	200	819	394	1,472	472	1,853	567	2,143
07:15	276		174		239		418		515		592	
07:30	234		164		206		341		440		505	
07:45	252		160		174		319		426		479	
08:00	230	997	168	606	180	759	302	1,013	410	1,756	470	1,619
08:15	252		164		226		283		478		447	
08:30	245		146		209		240		454		386	
08:45	270		128		144		188		414		316	
09:00	194	703	110	405	148	708	203	758	342	1,411	313	1,163
09:15	178		104		192		202		370		306	
09:30	155		100		176		215		331		315	
09:45	176		91		192		138		368		229	
10:00	144	576	82	257	184	725	163	558	328	1,301	245	815
10:15	123		65		165		160		288		225	
10:30	168		55		170		125		338		180	
10:45	141		55		206		110		347		165	
11:00	144	598	48	156	216	793	105	339	360	1,391	153	495
11:15	140		44		172		92		312		136	
11:30	164		40		213		74		377		114	
11:45	150		24		192		68		342		92	
Totals	6,294		7,995		5,008		16,875		11,302		24,870	
Split%	55.7		32.1		44.3		67.9					
Day Totals		14,289				21,883				36,172		
Day Splits		39.5				60.5						
Peak Hour	06:30		05:00		07:00		04:45		06:45		04:45	
Volume	1,108		1,036		819		2,437		1,897		3,431	
Factor	0.91		0.93		0.86		0.97		0.92		0.98	

Data File : D1909052

2004

Thursday, October 03, 2019 Location: Santa Ana PROJECT:

ADT160 MAIN ST from EDINGER AVE to McFADDEN AVE. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB		SB		EB	WB		PM Period	NB		SB		EB	WB	
0:00	34		46					12:00	176		167				
0:15	38		36					12:15	204		187				
0:30	30		27					12:30	197		209				
0:45	36	138	40	149			287	12:45	216	793	175	738			1531
1:00	26		26					13:00	197		181				
1:15	25		13					13:15	205		167				
1:30	23		15					13:30	247		159				
1:45	27	101	17	71			172	13:45	206	855	202	709			1564
2:00	21		25					14:00	228		173				
2:15	23		21					14:15	295		153				
2:30	29		17					14:30	278		151				
2:45	22	95	22	85			180	14:45	267	1068	197	674			1742
3:00	15		19					15:00	305		177				
3:15	8		29					15:15	322		205				
3:30	17		23					15:30	290		188				
3:45	25	65	26	97			162	15:45	307	1224	197	767			1991
4:00	29		41					16:00	286		215				
4:15	24		31					16:15	332		181				
4:30	41		42					16:30	341		187				
4:45	40	134	54	168			302	16:45	320	1279	218	801			2080
5:00	48		49					17:00	323		200				
5:15	55		68					17:15	336		217				
5:30	78		78					17:30	307		223				
5:45	89	270	131	326			596	17:45	289	1255	238	878			2133
6:00	75		173					18:00	275		206				
6:15	111		155					18:15	311		190				
6:30	121		173					18:30	271		187				
6:45	137	444	212	713			1157	18:45	229	1086	165	748			1834
7:00	159		317					19:00	184		161				
7:15	194		331					19:15	177		156				
7:30	243		324					19:30	180		152				
7:45	234	830	348	1320			2150	19:45	150	691	162	631			1322
8:00	204		349					20:00	140		127				
8:15	185		326					20:15	169		134				
8:30	172		357					20:30	109		115				
8:45	164	725	325	1357			2082	20:45	131	549	123	499			1048
9:00	131		298					21:00	113		133				
9:15	127		261					21:15	120		105				
9:30	135		208					21:30	104		101				
9:45	152	545	195	962			1507	21:45	110	447	90	429			876
10:00	165		169					22:00	97		86				
10:15	156		164					22:15	76		68				
10:30	146		171					22:30	86		76				
10:45	157	624	178	682			1306	22:45	65	324	58	288			612
11:00	185		180					23:00	73		47				
11:15	169		153					23:15	68		42				
11:30	152		166					23:30	42		41				
11:45	158	664	164	663			1327	23:45	31	214	34	164			378
Total Vol.		4635		6593			**11228**			9785		7326			**17111**

Daily Totals

NB	SB	EB	WB	**Combined**
14420	13919			**28339**

	AM			**PM**		
Split %	41.3%	58.7%	**39.6%**	57.2%	42.8%	**60.4%**
Peak Hour	7:15	7:45	**7:15**	16:30	17:15	**16:45**
Volume	875	1380	**2227**	1320	884	**2144**
P.H.F.	0.90	0.97	**0.96**	0.96	0.93	**0.97**

cs@aimtd.com Tell. 714 253 7888

Thursday, October 10, 2019 CITY: Santa Ana PROJECT: SC

ADT159 MAIN ST from WARNER AVE to EDINGER AVE. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB		SB		EB	WB		PM Period	NB		SB		EB	WB	
0:00	49		24					12:00	160		169				
0:15	45		37					12:15	170		159				
0:30	36		25					12:30	146		139				
0:45	31	161	35	121			282	12:45	184	660	146	613			1273
1:00	26		26					13:00	188		170				
1:15	23		18					13:15	195		157				
1:30	26		19					13:30	235		177				
1:45	21	96	12	75			171	13:45	218	836	139	643			1479
2:00	24		8					14:00	207		167				
2:15	20		16					14:15	271		182				
2:30	37		18					14:30	260		194				
2:45	26	107	19	61			168	14:45	314	1052	204	747			1799
3:00	28		18					15:00	273		182				
3:15	13		15					15:15	274		186				
3:30	21		31					15:30	330		175				
3:45	11	73	41	105			178	15:45	346	1223	187	730			1953
4:00	17		37					16:00	323		171				
4:15	24		65					16:15	353		165				
4:30	27		104					16:30	343		176				
4:45	37	105	116	322			427	16:45	318	1337	173	685			2022
5:00	49		91					17:00	332		187				
5:15	52		119					17:15	342		229				
5:30	63		162					17:30	300		224				
5:45	57	221	197	569			790	17:45	301	1275	211	851			2126
6:00	59		169					18:00	294		170				
6:15	66		195					18:15	337		165				
6:30	90		281					18:30	287		128				
6:45	106	321	317	962			1283	18:45	283	1201	145	608			1809
7:00	106		305					19:00	222		133				
7:15	145		372					19:15	209		144				
7:30	171		292					19:30	170		118				
7:45	193	615	299	1268			1883	19:45	154	755	120	515			1270
8:00	157		327					20:00	141		107				
8:15	137		334					20:15	176		119				
8:30	134		336					20:30	193		105				
8:45	133	561	356	1353			1914	20:45	162	672	102	433			1105
9:00	126		272					21:00	198		90				
9:15	122		204					21:15	174		93				
9:30	139		176					21:30	111		100				
9:45	155	542	165	817			1359	21:45	118	601	80	363			964
10:00	148		162					22:00	105		65				
10:15	151		157					22:15	108		70				
10:30	128		157					22:30	85		68				
10:45	139	566	152	628			1194	22:45	63	361	56	259			620
11:00	143		151					23:00	64		44				
11:15	163		141					23:15	78		47				
11:30	151		146					23:30	48		46				
11:45	164	621	155	593			1214	23:45	50	240	35	172			412
Total Vol.		3989		6874			**10863**			10213		6619			**16832**

Daily Totals

NB	SB	EB	WB	**Combined**
14202	13493			**27695**

	AM			**PM**		
Split %	36.7%	63.3%	**39.2%**	60.7%	39.3%	**60.8%**
Peak Hour	7:15	8:00	**7:15**	15:45	17:00	**17:00**
Volume	666	1353	**1956**	1365	851	**2126**
P.H.F.	0.86	0.95	**0.95**	0.97	0.93	**0.93**

cs@aimtd.com Tell. 714 253 7888

Wednesday, September 11, 2019 Location: Santa Ana PROJECT:

ADT157 MAIN ST from ALTON AVE to DYER RD. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB		SB		EB	WB		PM Period	NB		SB		EB	WB	
0:00	37		30					12:00	230		160				
0:15	25		33					12:15	206		177				
0:30	38		38					12:30	202		191				
0:45	34	134	57	158			292	12:45	226	864	172	700			1564
1:00	30		18					13:00	211		160				
1:15	26		19					13:15	277		171				
1:30	19		27					13:30	316		213				
1:45	16	91	20	84			175	13:45	291	1095	184	728			1823
2:00	33		23					14:00	247		189				
2:15	16		21					14:15	309		191				
2:30	68		21					14:30	364		180				
2:45	34	151	15	80			231	14:45	404	1324	186	746			2070
3:00	15		16					15:00	373		178				
3:15	17		16					15:15	340		174				
3:30	20		32					15:30	435		177				
3:45	20	72	34	98			170	15:45	359	1507	180	709			2216
4:00	16		47					16:00	384		162				
4:15	13		80					16:15	405		191				
4:30	25		123					16:30	379		202				
4:45	49	103	132	382			485	16:45	361	1529	193	748			2277
5:00	39		104					17:00	393		197				
5:15	32		125					17:15	435		191				
5:30	55		173					17:30	410		214				
5:45	79	205	209	611			816	17:45	444	1682	223	825			2507
6:00	75		209					18:00	435		165				
6:15	54		238					18:15	389		132				
6:30	84		319					18:30	400		152				
6:45	119	332	366	1132			1464	18:45	284	1508	118	567			2075
7:00	137		353					19:00	241		108				
7:15	146		348					19:15	210		126				
7:30	190		394					19:30	166		120				
7:45	163	636	395	1490			2126	19:45	158	775	119	473			1248
8:00	167		356					20:00	137		94				
8:15	133		434					20:15	119		101				
8:30	139		402					20:30	120		98				
8:45	132	571	355	1547			2118	20:45	104	480	84	377			857
9:00	140		236					21:00	101		78				
9:15	115		225					21:15	104		78				
9:30	141		208					21:30	111		70				
9:45	133	529	222	891			1420	21:45	74	390	64	290			680
10:00	128		152					22:00	112		75				
10:15	127		137					22:15	78		73				
10:30	144		156					22:30	90		56				
10:45	134	533	143	588			1121	22:45	75	355	62	266			621
11:00	181		165					23:00	83		43				
11:15	170		172					23:15	60		38				
11:30	169		180					23:30	58		35				
11:45	200	720	191	708			1428	23:45	39	240	42	158			398
Total Vol.		4077		7769			**11846**			11749		6587			**18336**

Daily Totals

NB	SB	EB	WB	**Combined**
15826	14356			**30182**

	AM			**PM**		
Split %	34.4%	65.6%	**39.2%**	64.1%	35.9%	**60.8%**
Peak Hour	11:45	7:45	**7:30**	17:15	17:00	**17:15**
Volume	838	1587	**2232**	1724	825	**2517**
P.H.F.	0.91	0.91	**0.96**	0.98	0.92	**0.94**

cs@aimtd.com Tell. 714 253 7888

Wednesday, September 11, 2019 Location: Santa Ana PROJECT:

ADT224 SEGERSTROM AVE from BRISTOL ST to FLOWER ST. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB	SB	EB		WB			PM Period	NB	SB	EB		WB		
0:30			25		25			12:00			127		142		
0:15			20		15			12:15			125		116		
0:30			23		29			12:30			136		117		
0:45			29	97	23	92	189	12:45			144	532	111	486	1018
1:00			17		13			13:00			145		140		
1:15			12		8			13:15			161		166		
1:30			9		16			13:30			196		213		
1:45			6	44	7	44	88	13:45			213	715	206	725	1440
2:00			14		15			14:00			194		192		
2:15			11		6			14:15			165		187		
2:30			9		10			14:30			166		222		
2:45			13	47	12	43	90	14:45			182	707	250	851	1558
3:00			10		12			15:00			219		231		
3:15			11		14			15:15			177		204		
3:30			25		6			15:30			231		268		
3:45			29	75	17	49	124	15:45			194	821	303	1006	1827
4:00			29		17			16:00			209		315		
4:15			45		19			16:15			197		313		
4:30			59		36			16:30			244		332		
4:45			91	224	52	124	348	16:45			218	868	290	1250	2118
5:00			58		26			17:00			229		279		
5:15			97		36			17:15			251		265		
5:30			103		40			17:30			260		280		
5:45			135	393	49	151	544	17:45			225	965	285	1109	2074
6:00			90		57			18:00			212		318		
6:15			130		67			18:15			186		242		
6:30			159		83			18:30			144		214		
6:45			202	581	107	314	895	18:45			153	695	186	960	1655
7:00			261		125			19:00			116		145		
7:15			297		144			19:15			115		148		
7:30			312		191			19:30			124		132		
7:45			296	1166	160	620	1786	19:45			115	470	111	536	1006
8:00			212		123			20:00			111		100		
8:15			203		96			20:15			92		103		
8:30			212		148			20:30			96		90		
8:45			193	820	136	503	1323	20:45			81	380	80	373	753
9:00			153		95			21:00			84		90		
9:15			145		103			21:15			77		81		
9:30			132		100			21:30			60		77		
9:45			104	534	108	406	940	21:45			64	285	77	325	610
10:00			109		101			22:00			64		70		
10:15			97		93			22:15			48		64		
10:30			94		92			22:30			48		47		
10:45			104	404	88	374	778	22:45			38	198	40	221	419
11:00			100		111			23:00			38		43		
11:15			103		108			23:15			35		35		
11:30			103		114			23:30			26		32		
11:45			123	429	128	461	890	23:45			22	121	28	138	259
Total Vol.				4814		3181	**7995**					6757		7980	**14737**

			Daily Totals		
	NB	SB	EB	WB	**Combined**
			11571	11161	**22732**

			AM					**PM**		
Split %			60.2%	39.8%	**35.2%**			45.9%	54.1%	**64.8%**
Peak Hour	0:30	0:30	7:00	7:00	**7:00**			17:00	15:45	**16:00**
Volume			1166	620	**1786**			965	1263	**2118**
P.H.F.			0.93	0.81	**0.89**			0.93	0.95	**0.92**

Thursday, October 10, 2019 | CITY: Santa Ana | PROJECT:

Prepared by: Field Data Services of Arizor

ADT152 MacArthur between Flower and Main

Prepared by AimTD LLC tel. 714 253 7888

AM Period	NB	SB	EB		WB			PM Period	NB	SB	EB		WB		
0:00			24		41			12:00			213		294		
0:15			21		35			12:15			245		288		
0:30			15		37			12:30			257		269		
0:45			14	74	25	138	212	12:45			285	1000	314	1165	2165
1:00			16		23			13:00			308		287		
1:15			11		25			13:15			303		275		
1:30			12		20			13:30			294		303		
1:45			14	53	16	84	137	13:45			312	1217	286	1151	2368
2:00			10		19			14:00			281		321		
2:15			9		15			14:15			313		312		
2:30			13		23			14:30			310		345		
2:45			16	48	25	82	130	14:45			287	1191	331	1309	2500
3:00			21		14			15:00			312		350		
3:15			23		18			15:15			279		398		
3:30			31		22			15:30			253		421		
3:45			36	111	21	75	186	15:45			237	1081	475	1644	2725
4:00			47		17			16:00			231		563		
4:15			62		28			16:15			252		514		
4:30			98		42			16:30			248		557		
4:45			101	308	38	125	433	16:45			240	971	586	2220	3191
5:00			121		52			17:00			227		617		
5:15			129		50			17:15			234		595		
5:30			175		64			17:30			242		582		
5:45			196	621	79	245	866	17:45			231	934	556	2350	3284
6:00			200		76			18:00			214		525		
6:15			234		86			18:15			236		512		
6:30			306		107			18:30			223		456		
6:45			372	1112	96	365	1477	18:45			220	893	423	1916	2809
7:00			421		131			19:00			198		302		
7:15			463		185			19:15			203		253		
7:30			521		197			19:30			187		194		
7:45			535	1940	176	689	2629	19:45			179	767	195	944	1711
8:00			501		175			20:00			176		168		
8:15			461		187			20:15			153		150		
8:30			452		157			20:30			134		178		
8:45			423	1837	162	681	2518	20:45			140	603	145	641	1244
9:00			345		142			21:00			142		133		
9:15			269		147			21:15			144		142		
9:30			204		164			21:30			121		145		
9:45			214	1032	153	606	1638	21:45			93	500	123	543	1043
10:00			196		158			22:00			101		131		
10:15			185		165			22:15			98		120		
10:30			157		172			22:30			76		96		
10:45			175	713	169	664	1377	22:45			53	328	65	412	740
11:00			168		197			23:00			56		79		
11:15			203		235			23:15			43		75		
11:30			184		268			23:30			31		89		
11:45			195	750	286	986	1736	23:45			25	155	53	296	451
Total Vol.			8599		4740		**13339**				9640		14591		**24231**

Daily Totals

NB	SB	EB	WB	Combined
		18239	19331	**37570**

	AM			PM		
Split %	64.5%	35.5%	**35.5%**	39.8%	60.2%	**64.5%**
Peak Hour	7:15	11:45	**7:15**	14:15	16:45	**16:45**
Volume	2020	1137	**2753**	1222	2380	**3323**
P.H.F.	0.94	0.97	**0.96**	0.98	0.96	**0.98**

cs@aimtd.com Tell. 714 253 7888

Wednesday, October 02, 2019 CITY: Santa Ana PROJECT:

Prepared by: Field Data Services of Arizor

ADT155 Main between Sunflower and MacArthur. **Prepared by AimTD LLC tel. 714 253 7888**

AM Period	NB		SB		EB	WB		PM Period	NB		SB		EB	WB	
0:00	23		16					12:00	167		141				
0:15	26		14					12:15	176		151				
0:30	21		12					12:30	206		158				
0:45	18	88	8	50			138	12:45	189	738	142	592			1330
1:00	13		9					13:00	198		152				
1:15	14		7					13:15	223		145				
1:30	15		6					13:30	208		136				
1:45	11	53	4	26			79	13:45	198	827	142	575			1402
2:00	9		6					14:00	206		121				
2:15	13		7					14:15	234		124				
2:30	15		4					14:30	254		117				
2:45	8	45	8	25			70	14:45	259	953	112	474			1427
3:00	11		5					15:00	276		98				
3:15	8		5					15:15	298		110				
3:30	7		11					15:30	342		123				
3:45	11	37	14	35			72	15:45	331	1247	142	473			1720
4:00	5		16					16:00	367		147				
4:15	12		24					16:15	389		136				
4:30	15		47					16:30	434		142				
4:45	16	48	75	162			210	16:45	456	1646	134	559			2205
5:00	21		71					17:00	521		152				
5:15	28		67					17:15	489		157				
5:30	37		86					17:30	456		175				
5:45	35	121	97	321			442	17:45	467	1933	154	638			2571
6:00	48		123					18:00	407		142				
6:15	54		156					18:15	342		113				
6:30	76		178					18:30	314		97				
6:45	70	248	207	664			912	18:45	286	1349	86	438			1787
7:00	87		245					19:00	234		65				
7:15	123		289					19:15	201		76				
7:30	145		305					19:30	142		65				
7:45	179	534	311	1150			1684	19:45	113	690	75	281			971
8:00	168		352					20:00	101		54				
8:15	143		316					20:15	87		46				
8:30	125		308					20:30	97		63				
8:45	112	548	298	1274			1822	20:45	67	352	45	208			560
9:00	104		251					21:00	86		53				
9:15	107		214					21:15	97		51				
9:30	98		156					21:30	76		47				
9:45	97	406	132	753			1159	21:45	86	345	32	183			528
10:00	87		121					22:00	76		25				
10:15	76		123					22:15	65		34				
10:30	73		108					22:30	70		31				
10:45	89	325	127	479			804	22:45	53	264	25	115			379
11:00	111		113					23:00	41		21				
11:15	104		134					23:15	42		16				
11:30	122		123					23:30	28		17				
11:45	136	473	125	495			968	23:45	32	143	20	74			217
Total Vol.	2926		5434				**8360**		10487		4610				**15097**

Daily Totals

NB	SB	EB	WB	Combined
13413	10044			**23457**

	AM			PM		
Split %	35.0%	65.0%	**35.6%**	69.5%	30.5%	**64.4%**
Peak Hour	11:45	7:45	**7:30**	17:00	17:00	**17:00**
Volume	685	1287	**1919**	1933	638	**2571**
P.H.F.	0.83	0.91	**0.92**	0.92	0.91	**0.96**

cs@aimtd.com Tell. 714 253 7888

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : GRAND AVENUE — Site: SANTA ANA

Segment : WARNER AVE TO EDINGER AVE — Date: 09/12/19

Client : CITY SANTA ANA — Day: Thursday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	6	20	120	487	4	22	149	543	10	42	269	1,030
12:15	5		115		6		132		11		247	
12:30	5		130		10		136		15		266	
12:45	4		122		2		126		6		248	
01:00	3	14	144	580	1	8	133	451	4	22	277	1,031
01:15	4		134		2		98		6		232	
01:30	3		169		4		132		7		301	
01:45	4		133		1		88		5		221	
02:00	2	14	164	745	2	11	96	469	4	25	260	1,214
02:15	6		182		2		109		8		291	
02:30	2		213		3		138		5		351	
02:45	4		186		4		126		8		312	
03:00	4	16	224	1,101	4	28	153	582	8	44	377	1,683
03:15	4		256		6		148		10		404	
03:30	6		308		6		137		12		445	
03:45	2		313		12		144		14		457	
04:00	3	44	304	1,263	10	90	160	664	13	134	464	1,927
04:15	3		294		16		170		19		464	
04:30	11		341		16		184		27		525	
04:45	27		324		48		150		75		474	
05:00	18	124	328	1,305	30	177	226	701	48	301	554	2,006
05:15	26		322		34		182		60		504	
05:30	28		346		52		156		80		502	
05:45	52		309		61		137		113		446	
06:00	42	229	258	901	46	320	129	451	88	549	387	1,352
06:15	56		259		70		112		126		371	
06:30	57		220		90		105		147		325	
06:45	74		164		114		105		188		269	
07:00	90	428	131	374	120	554	96	354	210	982	227	728
07:15	96		99		124		80		220		179	
07:30	110		84		158		96		268		180	
07:45	132		60		152		82		284		142	
08:00	114	410	60	175	162	675	52	197	276	1,085	112	372
08:15	114		38		152		48		266		86	
08:30	94		42		175		47		269		89	
08:45	88		35		186		50		274		85	
09:00	101	338	29	124	138	464	46	153	239	802	75	277
09:15	78		36		132		35		210		71	
09:30	78		31		103		44		181		75	
09:45	81		28		91		28		172		56	
10:00	65	304	32	92	92	377	50	136	157	681	82	228
10:15	58		20		100		30		158		50	
10:30	84		22		91		24		175		46	
10:45	97		18		94		32		191		50	
11:00	88	430	18	53	114	482	31	79	202	912	49	132
11:15	104		14		94		18		198		32	
11:30	118		9		126		18		244		27	
11:45	120		12		148		12		268		24	
Totals	2,371		7,200		3,208		4,780		5,579		11,980	
Split%	42.5		60.1		57.5		39.9					
Day Totals		9,571				7,988				17,559		
Day Splits		54.5				45.5						
Peak Hour	07:30		04:45		08:00		04:30		07:45		04:30	
Volume	470		1,320		675		742		1,095		2,057	
Factor	0.89		0.95		0.91		0.82		0.96		0.93	

* Data File : D1909119

2011

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : EDINGER AVENUE — Site: SANTA ANA

Segment : LYON ST TO NEWPORT FREEWAY — Date: 09/24/19

Client : CITY SANTA ANA — Day: Tuesday

Interval	EB				WB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	29	101	286	1,162	47	137	310	1,168	76	238	596	2,330
12:15	30		278		25		284		55		562	
12:30	28		312		38		282		66		594	
12:45	14		286		27		292		41		578	
01:00	18	61	297	1,164	23	70	288	1,159	41	131	585	2,323
01:15	17		293		18		316		35		609	
01:30	16		300		16		286		32		586	
01:45	10		274		13		269		23		543	
02:00	15	67	251	1,217	16	74	315	1,382	31	141	566	2,599
02:15	17		303		27		355		44		658	
02:30	20		338		16		336		36		674	
02:45	15		325		15		376		30		701	
03:00	22	121	324	1,308	11	87	376	1,535	33	208	700	2,843
03:15	26		310		16		375		42		685	
03:30	37		367		26		390		63		757	
03:45	36		307		34		394		70		701	
04:00	47	374	352	1,393	38	329	376	1,582	85	703	728	2,975
04:15	66		324		49		420		115		744	
04:30	113		348		86		402		199		750	
04:45	148		369		156		384		304		753	
05:00	120	744	461	1,655	136	642	368	1,396	256	1,386	829	3,051
05:15	153		435		113		346		266		781	
05:30	235		415		180		320		415		735	
05:45	236		344		213		362		449		706	
06:00	224	1,226	292	1,048	252	1,030	375	1,326	476	2,256	667	2,374
06:15	284		254		220		384		504		638	
06:30	346		266		258		331		604		597	
06:45	372		236		300		236		672		472	
07:00	382	1,650	248	777	354	1,266	208	728	736	2,916	456	1,505
07:15	460		205		291		200		751		405	
07:30	424		162		285		180		709		342	
07:45	384		162		336		140		720		302	
08:00	385	1,439	134	541	338	1,299	174	553	723	2,738	308	1,094
08:15	372		148		378		127		750		275	
08:30	352		135		303		131		655		266	
08:45	330		124		280		121		610		245	
09:00	282	1,022	122	412	277	1,075	114	437	559	2,097	236	849
09:15	254		94		284		128		538		222	
09:30	240		106		264		102		504		208	
09:45	246		90		250		93		496		183	
10:00	260	1,012	108	294	292	1,059	120	342	552	2,071	228	636
10:15	250		68		250		90		500		158	
10:30	256		57		245		62		501		119	
10:45	246		61		272		70		518		131	
11:00	256	1,114	50	169	268	1,083	61	205	524	2,197	111	374
11:15	262		43		264		40		526		83	
11:30	306		42		264		52		570		94	
11:45	290		34		287		52		577		86	
Totals	8,931		11,140		8,151		11,813		17,082		22,953	
Split%	52.3		48.5		47.7		51.5					
Day Totals		20,071				19,964				40,035		
Day Splits		50.1				49.9						
Peak Hour	07:15		04:45		07:45		03:45		07:00		04:30	
Volume	1,653		1,680		1,355		1,592		2,916		3,113	
Factor	0.90		0.91		0.90		0.95		0.97		0.94	

Data File : D1909057

2012

Wednesday, October 02, 2019 Location: Santa Ana PROJECT:

ADT267 WARNER AVE from GR to AVE to PULLMAN ST. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB	SB	EB		WB			PM Period	NB	SB	EB		WB		
0:30			9		12			12:00			177		174		
0:15			8		9			12:15			159		165		
0:30			9		8			12:30			163		152		
0:45			11	37	7	36	73	12:45			150	649	144	635	1284
1:00			7		11			13:00			169		150		
1:15			8		7			13:15			156		167		
1:30			7		9			13:30			154		209		
1:45			6	28	6	33	61	13:45			178	657	187	713	1370
2:00			5		7			14:00			174		180		
2:15			8		9			14:15			170		254		
2:30			9		9			14:30			164		348		
2:45			7	29	5	30	59	14:45			163	671	312	1094	1765
3:00			7		6			15:00			156		301		
3:15			13		7			15:15			153		296		
3:30			27		10			15:30			154		303		
3:45			47	94	4	27	121	15:45			148	611	311	1211	1822
4:00			41		3			16:00			171		274		
4:15			51		5			16:15			155		315		
4:30			95		11			16:30			168		356		
4:45			113	300	14	33	333	16:45			197	691	302	1247	1938
5:00			97		18			17:00			224		324		
5:15			123		21			17:15			229		307		
5:30			175		20			17:30			205		310		
5:45			290	685	43	102	787	17:45			168	826	301	1242	2068
6:00			165		54			18:00			136		294		
6:15			209		59			18:15			146		237		
6:30			223		53			18:30			117		171		
6:45			242	839	79	245	1084	18:45			103	502	103	805	1307
7:00			231		112			19:00			85		78		
7:15			283		153			19:15			78		73		
7:30			272		158			19:30			73		82		
7:45			315	1101	164	587	1688	19:45			64	300	70	303	603
8:00			286		161			20:00			56		64		
8:15			274		158			20:15			46		60		
8:30			255		141			20:30			37		50		
8:45			258	1073	139	599	1672	20:45			55	194	64	238	432
9:00			166		111			21:00			43		44		
9:15			173		123			21:15			37		44		
9:30			142		127			21:30			35		49		
9:45			98	579	110	471	1050	21:45			49	164	46	183	347
10:00			125		116			22:00			32		42		
10:15			100		112			22:15			29		36		
10:30			107		106			22:30			24		37		
10:45			106	438	101	435	873	22:45			24	109	42	157	266
11:00			126		113			23:00			16		35		
11:15			143		92			23:15			14		24		
11:30			176		105			23:30			12		31		
11:45			175	620	125	435	1055	23:45			8	50	15	105	155
Total Vol.				5823		3033	**8856**					5424		7933	**13357**

Daily Totals

NB	SB	EB	WB	Combined
		11247	10966	**22213**

			AM				PM		
Split %			65.8%	34.2%	**39.9%**		40.6%	59.4%	**60.1%**
Peak Hour	0:30	0:30	7:15	7:30	**7:15**		16:45	16:15	**16:30**
Volume			1156	641	**1792**		855	1297	**2107**
P.H.F.			0.92	0.98	**0.94**		0.93	0.91	**0.96**

Thursday, October 03, 2019 Location: Santa Ana PROJECT:

ADT264 WARNER AVE from MAIN ST to HALLADAY ST. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB	SB	EB		WB			PM Period	NB	SB	EB		WB		
0:30			16		27			12:00			167		152		
0:15			20		23			12:15			168		156		
0:30			13		25			12:30			193		164		
0:45			11	60	16	91	151	12:45			204	732	158	630	1362
1:00			5		14			13:00			231		175		
1:15			6		20			13:15			215		198		
1:30			5		18			13:30			244		204		
1:45			11	27	14	66	93	13:45			280	970	197	774	1744
2:00			16		20			14:00			252		188		
2:15			9		17			14:15			234		196		
2:30			18		9			14:30			241		218		
2:45			27	70	13	59	129	14:45			221	948	210	812	1760
3:00			20		19			15:00			236		229		
3:15			18		14			15:15			220		285		
3:30			21		8			15:30			227		270		
3:45			43	102	11	52	154	15:45			260	943	223	1007	1950
4:00			33		22			16:00			245		255		
4:15			54		13			16:15			235		277		
4:30			75		31			16:30			237		312		
4:45			71	233	28	94	327	16:45			213	930	263	1107	2037
5:00			94		34			17:00			241		275		
5:15			101		37			17:15			276		283		
5:30			230		63			17:30			319		267		
5:45			280	705	112	246	951	17:45			311	1147	253	1078	2225
6:00			224		103			18:00			265		248		
6:15			202		113			18:15			231		236		
6:30			209		109			18:30			208		245		
6:45			261	896	123	448	1344	18:45			178	882	213	942	1824
7:00			282		183			19:00			187		189		
7:15			322		231			19:15			153		156		
7:30			284		259			19:30			137		135		
7:45			324	1212	221	894	2106	19:45			123	600	115	595	1195
8:00			328		173			20:00			103		131		
8:15			286		150			20:15			106		96		
8:30			293		155			20:30			100		106		
8:45			267	1174	145	623	1797	20:45			92	401	115	448	849
9:00			196		147			21:00			94		108		
9:15			193		137			21:15			70		102		
9:30			141		153			21:30			87		88		
9:45			167	697	157	594	1291	21:45			52	303	68	366	669
10:00			146		149			22:00			61		92		
10:15			158		127			22:15			54		86		
10:30			151		135			22:30			47		58		
10:45			183	638	119	530	1168	22:45			42	204	47	283	487
11:00			158		158			23:00			26		42		
11:15			169		159			23:15			34		37		
11:30			145		146			23:30			26		36		
11:45			159	631	149	612	1243	23:45			30	116	33	148	264
Total Vol.				6445		4309	**10754**					8176		8190	**16366**

Daily Totals

NB	SB	EB	WB	Combined
		14621	12499	**27120**

			AM				PM		
Split %			59.9%	40.1%	**39.7%**		50.0%	50.0%	**60.3%**
Peak Hour	0:30	0:30	7:15	7:00	**7:15**		17:15	16:30	**17:00**
Volume			1258	894	**2142**		1171	1133	**2225**
P.H.F.			0.96	0.86	**0.97**		0.92	0.91	**0.95**

Wednesday, October 02, 2019 CITY: Santa Ana PROJECT: SC

ADT172 McFADDEN AVE from NEWHOPE ST to HARBOR BL. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB	SB	EB		WB			PM Period	NB	SB	EB		WB		
0:00			18		33			12:00			123		127		
0:15			23		19			12:15			125		119		
0:30			13		12			12:30			96		140		
0:45			10	64	13	77	141	12:45			120	464	106	492	956
1:00			7		16			13:00			115		127		
1:15			5		14			13:15			132		129		
1:30			7		7			13:30			124		106		
1:45			9	28	6	43	71	13:45			148	519	156	518	1037
2:00			11		18			14:00			155		162		
2:15			3		8			14:15			159		159		
2:30			5		9			14:30			145		165		
2:45			6	25	7	42	67	14:45			137	596	155	641	1237
3:00			10		3			15:00			166		180		
3:15			5		8			15:15			158		167		
3:30			11		5			15:30			139		183		
3:45			9	35	12	28	63	15:45			153	616	158	688	1304
4:00			12		12			16:00			151		163		
4:15			7		11			16:15			148		175		
4:30			20		20			16:30			148		194		
4:45			16	55	26	69	124	16:45			142	589	188	720	1309
5:00			25		22			17:00			166		233		
5:15			35		26			17:15			160		200		
5:30			32		40			17:30			171		211		
5:45			41	133	32	120	253	17:45			190	687	210	854	1541
6:00			52		44			18:00			184		178		
6:15			49		45			18:15			150		224		
6:30			69		64			18:30			152		181		
6:45			92	262	77	230	492	18:45			137	623	161	744	1367
7:00			108		95			19:00			136		159		
7:15			147		131			19:15			136		135		
7:30			159		170			19:30			128		129		
7:45			180	594	178	574	1168	19:45			96	496	129	552	1048
8:00			182		176			20:00			93		144		
8:15			207		145			20:15			92		96		
8:30			191		107			20:30			87		97		
8:45			160	740	117	545	1285	20:45			96	368	106	443	811
9:00			174		128			21:00			78		98		
9:15			140		111			21:15			87		108		
9:30			90		105			21:30			68		81		
9:45			106	510	106	450	960	21:45			67	300	70	357	657
10:00			113		115			22:00			60		76		
10:15			107		130			22:15			54		62		
10:30			108		108			22:30			41		75		
10:45			110	438	105	458	896	22:45			40	195	38	251	446
11:00			86		123			23:00			28		37		
11:15			96		107			23:15			34		32		
11:30			106		99			23:30			21		28		
11:45			120	408	114	443	851	23:45			21	104	27	124	228
Total Vol.			3292		3079		**6371**				5557		6384		**11941**

Daily Totals

NB	SB	EB	WB	**Combined**
		8849	9463	**18312**

	AM			**PM**		
Split %	51.7%	48.3%	**34.8%**	46.5%	53.5%	**65.2%**
Peak Hour	7:45	7:30	**7:30**	17:15	17:00	**17:00**
Volume	760	669	**1397**	705	854	**1541**
P.H.F.	0.92	0.94	**0.98**	0.93	0.92	**0.96**

Tuesday, September 24, 2019 CITY: Santa Ana PROJECT: SC

ADT181 McFADDEN AVE from GR to AVE to LYON ST. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB	SB	EB		WB			PM Period	NB	SB	EB		WB		
0:00			12		13			12:00			150		119		
0:15			16		21			12:15			118		118		
0:30			16		12			12:30			132		118		
0:45			10	54	13	59	113	12:45			119	519	130	485	1004
1:00			9		18			13:00			131		121		
1:15			5		13			13:15			126		135		
1:30			10		14			13:30			137		145		
1:45			15	39	13	58	97	13:45			124	518	159	560	1078
2:00			8		19			14:00			120		158		
2:15			9		9			14:15			157		148		
2:30			12		11			14:30			185		189		
2:45			11	40	16	55	95	14:45			186	648	166	661	1309
3:00			6		6			15:00			177		180		
3:15			7		8			15:15			203		196		
3:30			20		10			15:30			204		168		
3:45			33	66	24	48	114	15:45			176	760	221	765	1525
4:00			19		19			16:00			193		211		
4:15			37		20			16:15			172		191		
4:30			54		44			16:30			178		206		
4:45			58	168	51	134	302	16:45			173	716	191	799	1515
5:00			61		43			17:00			216		213		
5:15			88		50			17:15			196		191		
5:30			117		69			17:30			204		187		
5:45			137	403	80	242	645	17:45			182	798	181	772	1570
6:00			136		82			18:00			190		163		
6:15			159		82			18:15			155		193		
6:30			171		96			18:30			165		137		
6:45			195	661	115	375	1036	18:45			160	670	146	639	1309
7:00			180		121			19:00			133		143		
7:15			204		148			19:15			130		122		
7:30			179		150			19:30			117		122		
7:45			180	743	183	602	1345	19:45			127	507	104	491	998
8:00			237		154			20:00			118		98		
8:15			201		127			20:15			99		89		
8:30			145		122			20:30			99		79		
8:45			171	754	141	544	1298	20:45			69	385	93	359	744
9:00			158		97			21:00			79		66		
9:15			116		101			21:15			60		83		
9:30			114		119			21:30			72		57		
9:45			121	509	89	406	915	21:45			52	263	54	260	523
10:00			119		110			22:00			66		66		
10:15			96		95			22:15			45		62		
10:30			118		108			22:30			35		37		
10:45			94	427	96	409	836	22:45			39	185	52	217	402
11:00			125		126			23:00			40		39		
11:15			114		115			23:15			22		41		
11:30			131		126			23:30			19		35		
11:45			125	495	115	482	977	23:45			17	98	25	140	238
Total Vol.			4359		3414		**7773**				6067		6148		**12215**

Daily Totals

NB	SB	EB	WB	Combined
		10426	9562	**19988**

	AM			PM		
Split %	56.1%	43.9%	**38.9%**	49.7%	50.3%	**61.1%**
Peak Hour	7:15	7:15	**7:15**	17:00	15:45	**15:15**
Volume	800	635	**1435**	798	829	**1572**
P.H.F.	0.84	0.87	**0.92**	0.92	0.94	**0.97**

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : DYER ROAD — Site: SANTA ANA

Segment : PULLMAN ST TO RED HILL AVE — Date: 09/24/19

Client : CITY SANTA ANA — Day: Tuesday

Interval	EB				WB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	20	52	214	769	38	198	228	930	58	250	442	1,699
12:15	10		190		38		234		48		424	
12:30	14		183		84		230		98		413	
12:45	8		182		38		238		46		420	
01:00	13	32	204	728	24	73	246	1,032	37	105	450	1,760
01:15	11		162		28		250		39		412	
01:30	2		186		13		285		15		471	
01:45	6		176		8		251		14		427	
02:00	12	66	186	687	16	63	297	1,179	28	129	483	1,866
02:15	7		155		17		278		24		433	
02:30	17		170		18		335		35		505	
02:45	30		176		12		269		42		445	
03:00	18	187	158	741	16	60	286	1,286	34	247	444	2,027
03:15	31		191		20		332		51		523	
03:30	56		192		11		323		67		515	
03:45	82		200		13		345		95		545	
04:00	56	480	183	852	26	115	370	1,350	82	595	553	2,202
04:15	85		206		24		316		109		522	
04:30	137		227		40		342		177		569	
04:45	202		236		25		322		227		558	
05:00	175	904	243	1,101	32	209	389	1,412	207	1,113	632	2,513
05:15	205		276		36		358		241		634	
05:30	228		294		58		308		286		602	
05:45	296		288		83		357		379		645	
06:00	214	900	230	844	88	460	322	1,197	302	1,360	552	2,041
06:15	241		215		134		329		375		544	
06:30	231		234		102		313		333		547	
06:45	214		165		136		233		350		398	
07:00	220	991	146	486	172	777	300	986	392	1,768	446	1,472
07:15	251		106		176		266		427		372	
07:30	264		120		186		232		450		352	
07:45	256		114		243		188		499		302	
08:00	228	950	124	380	245	1,001	214	748	473	1,951	338	1,128
08:15	222		90		253		168		475		258	
08:30	238		74		267		170		505		244	
08:45	262		92		236		196		498		288	
09:00	218	868	82	293	224	785	194	654	442	1,653	276	947
09:15	233		84		187		158		420		242	
09:30	211		60		195		172		406		232	
09:45	206		67		179		130		385		197	
10:00	170	677	42	195	193	739	150	488	363	1,416	192	683
10:15	168		54		191		122		359		176	
10:30	181		57		175		122		356		179	
10:45	158		42		180		94		338		136	
11:00	208	870	22	83	188	776	98	285	396	1,646	120	368
11:15	219		19		201		66		420		85	
11:30	199		25		184		65		383		90	
11:45	244		17		203		56		447		73	
Totals	6,977		7,159		5,256		11,547		12,233		18,706	
Split%	57.0		38.3		43.0		61.7					
Day Totals		14,136				16,803				30,939		
Day Splits		45.7				54.3						
Peak Hour	07:15		05:00		07:45		05:00		07:45		05:00	
Volume	999		1,101		1,008		1,412		1,952		2,513	
Factor	0.95		0.94		0.94		0.91		0.97		0.97	

Data File : D1909047

2017

Tuesday, September 24, 2019 CITY: Santa Ana PROJECT: SC

ADT176 McFADDEN AVE from BRISTOL ST to FLOWER ST. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB	SB	EB		WB			PM Period	NB	SB	EB		WB		
0:00			15		23			12:00			80		79		
0:15			23		15			12:15			103		90		
0:30			16		17			12:30			75		82		
0:45			15	69	14	69	138	12:45			74	332	73	324	656
1:00			12		15			13:00			100		78		
1:15			8		11			13:15			89		63		
1:30			5		11			13:30			79		103		
1:45			11	36	8	45	81	13:45			98	366	106	350	716
2:00			9		12			14:00			126		95		
2:15			7		9			14:15			122		116		
2:30			9		9			14:30			136		129		
2:45			4	29	8	38	67	14:45			113	497	146	486	983
3:00			7		7			15:00			147		160		
3:15			7		10			15:15			120		142		
3:30			17		10			15:30			125		137		
3:45			16	47	10	37	84	15:45			126	518	133	572	1090
4:00			18		10			16:00			115		139		
4:15			35		16			16:15			107		160		
4:30			36		26			16:30			117		154		
4:45			49	138	24	76	214	16:45			122	461	150	603	1064
5:00			50		26			17:00			109		140		
5:15			47		32			17:15			133		147		
5:30			83		45			17:30			143		143		
5:45			105	285	33	136	421	17:45			151	536	143	573	1109
6:00			87		60			18:00			125		131		
6:15			101		61			18:15			129		131		
6:30			103		73			18:30			132		138		
6:45			110	401	75	269	670	18:45			104	490	127	527	1017
7:00			119		96			19:00			116		123		
7:15			175		123			19:15			104		116		
7:30			194		118			19:30			102		100		
7:45			179	667	124	461	1128	19:45			98	420	104	443	863
8:00			161		119			20:00			90		102		
8:15			148		105			20:15			84		94		
8:30			124		66			20:30			85		56		
8:45			94	527	70	360	887	20:45			64	323	73	325	648
9:00			75		58			21:00			80		63		
9:15			85		74			21:15			75		47		
9:30			82		64			21:30			73		48		
9:45			70	312	63	259	571	21:45			63	291	62	220	511
10:00			70		84			22:00			40		61		
10:15			82		74			22:15			57		50		
10:30			87		68			22:30			35		38		
10:45			71	310	85	311	621	22:45			28	160	43	192	352
11:00			89		73			23:00			33		44		
11:15			68		75			23:15			30		33		
11:30			99		86			23:30			32		30		
11:45			84	340	93	327	667	23:45			28	123	15	122	245
Total Vol.			3161		2388		**5549**				4517		4737		**9254**

	Daily Totals			
NB	SB	EB	WB	**Combined**
		7678	7125	**14803**

	AM			**PM**		
Split %	57.0%	43.0%	**37.5%**	48.8%	51.2%	**62.5%**
Peak Hour	7:15	7:15	**7:15**	17:15	16:15	**17:15**
Volume	709	484	**1193**	552	604	**1116**
P.H.F.	0.91	0.98	**0.96**	0.91	0.94	**0.95**

Wednesday, September 25, 2019 CITY: Santa Ana PROJECT: SC

ADT169 MAIN ST from MAINPLACE DR to GARDEN GROVE FWY. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB		SB		EB	WB		PM Period	NB		SB		EB	WB	
0:00	27		46					12:00	233		223				
0:15	20		34					12:15	261		227				
0:30	21		42					12:30	230		211				
0:45	34	102	23	145			247	12:45	231	955	269	930			1885
1:00	31		32					13:00	197		240				
1:15	33		23					13:15	229		266				
1:30	34		19					13:30	199		247				
1:45	26	124	26	100			224	13:45	237	862	236	989			1851
2:00	31		11					14:00	214		230				
2:15	31		28					14:15	270		234				
2:30	34		25					14:30	248		222				
2:45	34	130	10	74			204	14:45	209	941	246	932			1873
3:00	23		30					15:00	174		251				
3:15	32		27					15:15	224		245				
3:30	34		24					15:30	245		254				
3:45	46	135	28	109			244	15:45	233	876	273	1023			1899
4:00	41		25					16:00	217		238				
4:15	67		26					16:15	260		318				
4:30	113		26					16:30	240		295				
4:45	115	336	38	115			451	16:45	309	1026	319	1170			2196
5:00	95		54					17:00	291		327				
5:15	52		58					17:15	341		302				
5:30	89		88					17:30	317		281				
5:45	87	323	88	288			611	17:45	296	1245	256	1166			2411
6:00	101		109					18:00	254		216				
6:15	117		109					18:15	270		211				
6:30	161		196					18:30	225		192				
6:45	211	590	207	621			1211	18:45	230	979	201	820			1799
7:00	144		308					19:00	186		189				
7:15	166		340					19:15	175		210				
7:30	194		373					19:30	160		189				
7:45	233	737	307	1328			2065	19:45	144	665	200	788			1453
8:00	264		353					20:00	129		186				
8:15	206		320					20:15	111		132				
8:30	211		320					20:30	113		137				
8:45	218	899	232	1225			2124	20:45	74	427	130	585			1012
9:00	175		251					21:00	70		143				
9:15	149		206					21:15	88		144				
9:30	163		175					21:30	54		103				
9:45	208	695	186	818			1513	21:45	63	275	87	477			752
10:00	190		169					22:00	155		86				
10:15	186		186					22:15	141		85				
10:30	206		190					22:30	135		76				
10:45	177	759	216	761			1520	22:45	106	537	89	336			873
11:00	194		190					23:00	88		92				
11:15	245		206					23:15	64		65				
11:30	235		203					23:30	69		69				
11:45	209	883	226	825			1708	23:45	56	277	68	294			571
Total Vol.		5713		6409			**12122**			9065		9510			**18575**

Daily Totals

NB	SB	EB	WB	**Combined**
14778	15919			**30697**

	AM			**PM**		
Split %	47.1%	52.9%	**39.5%**	48.8%	51.2%	**60.5%**
Peak Hour	11:30	7:15	**7:30**	16:45	16:15	**16:45**
Volume	938	1373	**2250**	1258	1259	**2487**
P.H.F.	0.90	0.92	**0.91**	0.94	0.96	**0.97**

cs@aimtd.com Tell. 714 253 7888

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : FIRST STREET — Site: SANTA ANA

Segment : BRISTOL ST TO FLOWER ST — Date: 09/11/19

Client : CITY SANTA ANA — Day: Wednesday

Interval	EB				WB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	54	172	264	1,057	56	179	214	903	110	351	478	1,960
12:15	44		275		39		220		83		495	
12:30	40		268		45		221		85		489	
12:45	34		250		39		248		73		498	
01:00	30	108	294	1,201	30	105	260	1,055	60	213	554	2,256
01:15	27		312		30		279		57		591	
01:30	25		321		26		238		51		559	
01:45	26		274		19		278		45		552	
02:00	30	117	308	1,210	32	129	306	1,227	62	246	614	2,437
02:15	24		288		24		298		48		586	
02:30	31		308		36		277		67		585	
02:45	32		306		37		346		69		652	
03:00	20	115	340	1,336	24	104	340	1,445	44	219	680	2,781
03:15	25		318		18		388		43		706	
03:30	30		320		28		338		58		658	
03:45	40		358		34		379		74		737	
04:00	52	286	333	1,393	33	155	374	1,478	85	441	707	2,871
04:15	74		363		24		374		98		737	
04:30	80		342		52		366		132		708	
04:45	80		355		46		364		126		719	
05:00	103	596	350	1,468	51	267	376	1,486	154	863	726	2,954
05:15	127		398		54		339		181		737	
05:30	174		343		76		381		250		724	
05:45	192		377		86		390		278		767	
06:00	213	1,098	326	1,240	92	494	384	1,369	305	1,592	710	2,609
06:15	255		324		130		342		385		666	
06:30	295		270		110		336		405		606	
06:45	335		320		162		307		497		627	
07:00	372	1,684	294	1,009	185	944	258	966	557	2,628	552	1,975
07:15	450		285		212		234		662		519	
07:30	455		218		253		252		708		470	
07:45	407		212		294		222		701		434	
08:00	356	1,452	201	754	253	865	228	719	609	2,317	429	1,473
08:15	362		207		207		184		569		391	
08:30	354		178		208		170		562		348	
08:45	380		168		197		137		577		305	
09:00	349	1,177	181	639	188	832	152	629	537	2,009	333	1,268
09:15	294		154		200		179		494		333	
09:30	269		152		220		142		489		294	
09:45	265		152		224		156		489		308	
10:00	244	992	129	381	224	846	130	464	468	1,838	259	845
10:15	230		98		214		122		444		220	
10:30	280		78		200		106		480		184	
10:45	238		76		208		106		446		182	
11:00	220	1,014	71	241	194	938	104	281	414	1,952	175	522
11:15	263		76		240		74		503		150	
11:30	243		45		236		52		479		97	
11:45	288		49		268		51		556		100	
Totals	8,811		11,929		5,858		12,022		14,669		23,951	
Split%	60.1		49.8		39.9		50.2					
Day Totals		20,740				17,880				38,620		
Day Splits		53.7				46.3						
Peak Hour	07:00		05:00		07:15		05:30		07:15		05:00	
Volume	1,684		1,468		1,012		1,497		2,680		2,954	
Factor	0.93		0.92		0.86		0.96		0.95		0.96	

Data File : D1909091

2020

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : FAIRHAVEN AVENUE
Segment : CAMBRIDGE ST TO TUSTIN AVE
Client : CITY SANTA ANA

Site: SANTA ANA
Date: 10/03/19
Day: Thursday

Interval	WB				EB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	10	25	59	223	6	27	68	280	16	52	127	503
12:15	4		56		7		68		11		124	
12:30	7		46		4		70		11		116	
12:45	4		62		10		74		14		136	
01:00	4	11	66	269	2	13	68	261	6	24	134	530
01:15	4		76		5		54		9		130	
01:30	1		64		4		69		5		133	
01:45	2		63		2		70		4		133	
02:00	1	7	78	376	3	8	73	315	4	15	151	691
02:15	3		88		2		101		5		189	
02:30	0		94		1		78		1		172	
02:45	3		116		2		63		5		179	
03:00	0	6	118	452	3	8	114	360	3	14	232	812
03:15	2		134		4		88		6		222	
03:30	4		104		1		68		5		172	
03:45	0		96		0		90		0		186	
04:00	0	11	96	532	5	17	85	359	5	28	181	891
04:15	2		158		5		92		7		250	
04:30	3		116		5		102		8		218	
04:45	6		162		2		80		8		242	
05:00	5	35	155	657	7	43	108	418	12	78	263	1,075
05:15	9		169		8		106		17		275	
05:30	10		175		11		100		21		275	
05:45	11		158		17		104		28		262	
06:00	12	93	135	457	24	207	70	276	36	300	205	733
06:15	23		124		34		72		57		196	
06:30	26		110		73		74		99		184	
06:45	32		88		76		60		108		148	
07:00	41	354	66	209	118	705	46	187	159	1,059	112	396
07:15	65		58		177		53		242		111	
07:30	100		48		198		46		298		94	
07:45	148		37		212		42		360		79	
08:00	139	362	30	108	172	567	30	117	311	929	60	225
08:15	92		22		164		28		256		50	
08:30	72		34		111		28		183		62	
08:45	59		22		120		31		179		53	
09:00	50	194	18	78	74	304	22	93	124	498	40	171
09:15	44		30		80		28		124		58	
09:30	54		17		70		27		124		44	
09:45	46		13		80		16		126		29	
10:00	66	220	23	57	51	225	14	70	117	445	37	127
10:15	36		14		66		27		102		41	
10:30	56		14		44		20		100		34	
10:45	62		6		64		9		126		15	
11:00	56	221	8	20	68	248	10	32	124	469	18	52
11:15	56		7		54		7		110		14	
11:30	49		3		56		9		105		12	
11:45	60		2		70		6		130		8	
Totals	1,539		3,438		2,372		2,768		3,911		6,206	
Split%	39.4		55.4		60.6		44.6					
Day Totals		4,977				5,140				10,117		
Day Splits		49.2				50.8						
Peak Hour	07:30		04:45		07:15		05:00		07:30		05:00	
Volume	479		661		759		418		1,225		1,075	
Factor	0.81		0.94		0.90		0.97		0.85		0.98	

Data File : D1909063

2021

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : FOURTH STREET — Site: SANTA ANA

Segment : FRENCH ST TO GRAND AVE — Date: 10/09/19

Client : CITY SANTA ANA — Day: Wednesday

Interval	EB				WB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	10	34	78	296	21	71	122	509	31	105	200	805
12:15	9		66		13		125		22		191	
12:30	10		74		21		134		31		208	
12:45	5		78		16		128		21		206	
01:00	6	38	76	396	16	41	137	598	22	79	213	994
01:15	14		111		7		141		21		252	
01:30	12		115		8		180		20		295	
01:45	6		94		10		140		16		234	
02:00	5	24	106	388	10	40	155	575	15	64	261	963
02:15	6		84		12		148		18		232	
02:30	8		103		6		118		14		221	
02:45	5		95		12		154		17		249	
03:00	3	15	102	428	4	22	168	691	7	37	270	1.119
03:15	2		97		8		177		10		274	
03:30	6		120		4		164		10		284	
03:45	4		109		6		182		10		291	
04:00	7	78	116	480	11	47	190	728	18	125	306	1,208
04:15	16		112		12		172		28		284	
04:30	26		116		10		182		36		298	
04:45	29		136		14		184		43		320	
05:00	26	190	152	588	18	98	188	782	44	288	340	1,370
05:15	38		155		24		177		62		332	
05:30	56		135		26		231		82		366	
05:45	70		146		30		186		100		332	
06:00	54	345	116	342	32	162	202	610	86	507	318	952
06:15	67		82		40		135		107		217	
06:30	110		70		32		133		142		203	
06:45	114		74		58		140		172		214	
07:00	142	817	54	222	81	560	121	402	223	1,377	175	624
07:15	185		63		118		97		303		160	
07:30	266		56		167		98		433		154	
07:45	224		49		194		86		418		135	
08:00	202	494	28	156	144	418	68	254	346	912	96	410
08:15	118		32		92		69		210		101	
08:30	98		46		100		60		198		106	
08:45	76		50		82		57		158		107	
09:00	66	284	44	153	106	387	47	203	172	671	91	356
09:15	76		44		92		54		168		98	
09:30	63		33		88		53		151		86	
09:45	79		32		101		49		180		81	
10:00	61	247	50	123	87	396	47	170	148	643	97	293
10:15	61		33		97		36		158		69	
10:30	65		21		104		46		169		67	
10:45	60		19		108		41		168		60	
11:00	46	248	16	57	92	406	20	95	138	654	36	152
11:15	57		15		89		22		146		37	
11:30	76		16		116		30		192		46	
11:45	69		10		109		23		178		33	
Totals	2,814		3,629		2,648		5,617		5,462		9,246	
Split%	51.5		39.2		48.5		60.8					
Day Totals		6,443				8,265				14,708		
Day Splits		43.8				56.2						
Peak Hour	07:15		05:00		07:15		05:15		07:15		05:00	
Volume	877		588		623		796		1,500		1,370	
Factor	0.82		0.95		0.80		0.86		0.87		0.94	

Data File : D1909113

2022

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : BROADWAY
Segment : CIVIC CENTER TO SEVENTEENTH
Client : CITY SANTA ANA
File:

Site: SANTA ANA
Date: 09/19/19
D1909027

Day: Thursday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	3	18	130	481	18	85	142	584	21	103	272	1,065
12:15	2		118		29		136		31		254	
12:30	8		129		18		138		26		267	
12:45	5		104		20		168		25		272	
01:00	8	15	94	440	7	46	162	686	15	61	256	1,126
01:15	5		108		16		164		21		272	
01:30	2		136		11		188		13		324	
01:45	0		102		12		172		12		274	
02:00	1	14	140	615	18	48	161	679	19	62	301	1,294
02:15	4		145		8		169		12		314	
02:30	4		168		9		163		13		331	
02:45	5		162		13		186		18		348	
03:00	0	11	208	908	9	33	186	678	9	44	394	1,586
03:15	1		223		6		170		7		393	
03:30	9		253		11		162		20		415	
03:45	1		224		7		160		8		384	
04:00	1	24	259	1,105	13	98	157	690	14	122	416	1,795
04:15	5		238		15		159		20		397	
04:30	8		316		22		170		30		486	
04:45	10		292		48		204		58		496	
05:00	16	70	312	1,050	48	271	188	723	64	341	500	1,773
05:15	15		228		44		201		59		429	
05:30	23		256		78		171		101		427	
05:45	16		254		101		163		117		417	
06:00	23	144	281	862	120	619	173	583	143	763	454	1,445
06:15	32		226		123		149		155		375	
06:30	54		195		146		139		200		334	
06:45	35		160		230		122		265		282	
07:00	52	368	143	443	276	1,279	114	456	328	1,647	257	899
07:15	58		114		322		126		380		240	
07:30	122		100		367		116		489		216	
07:45	136		86		314		100		450		186	
08:00	193	508	121	402	282	982	90	385	475	1,490	211	787
08:15	136		114		237		126		373		240	
08:30	91		92		213		86		304		178	
08:45	88		75		250		83		338		158	
09:00	67	312	85	300	212	668	100	329	279	980	185	629
09:15	74		82		153		80		227		162	
09:30	95		79		159		82		254		161	
09:45	76		54		144		67		220		121	
10:00	69	300	74	194	138	558	54	239	207	858	128	433
10:15	74		50		146		78		220		128	
10:30	73		44		142		54		215		98	
10:45	84		26		132		53		216		79	
11:00	72	402	15	62	100	493	51	166	172	895	66	228
11:15	102		26		133		47		235		73	
11:30	116		10		130		31		246		41	
11:45	112		11		130		37		242		48	
Totals	2,186		6,862		5,180		6,198		7,366		13,060	
Split%	29.7		52.5		70.3		47.5					
Day Totals		9,048				11,378				20,426		
Day Splits		44.3				55.7						
Peak Hour	07:30		04:15		07:15		04:45		07:15		04:30	
Volume	587		1,158		1,285		764		1,794		1,911	
Factor	0.76		0.92		0.88		0.94		0.92		0.96	

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : BROADWAY

Segment : SANTA ANA BL TO CIVIC CENTER D

Client : CITY SANTA ANA

Site: SANTA ANA

Date: 09/19/19

Day: Thursday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	4	6	78	352	17	62	106	460	21	68	184	812
12:15	0		78		20		124		20		202	
12:30	1		106		13		106		14		212	
12:45	1		90		12		124		13		214	
01:00	3	9	99	389	6	31	126	498	9	40	225	887
01:15	4		86		10		119		14		205	
01:30	1		114		8		135		9		249	
01:45	1		90		7		118		8		208	
02:00	0	5	106	520	12	32	130	531	12	37	236	1,051
02:15	0		110		5		126		5		236	
02:30	1		156		5		136		6		292	
02:45	4		148		10		139		14		287	
03:00	0	8	189	753	4	19	154	566	4	27	343	1,319
03:15	0		196		2		148		2		344	
03:30	4		174		9		126		13		300	
03:45	4		194		4		138		8		332	
04:00	1	18	210	858	10	62	132	595	11	80	342	1,453
04:15	2		198		11		143		13		341	
04:30	5		238		13		148		18		386	
04:45	10		212		28		172		38		384	
05:00	5	33	203	836	24	154	168	641	29	187	371	1,477
05:15	7		209		24		192		31		401	
05:30	13		216		45		137		58		353	
05:45	8		208		61		144		69		352	
06:00	10	56	258	742	74	355	156	523	84	411	414	1,265
06:15	11		188		73		133		84		321	
06:30	17		164		82		124		99		288	
06:45	18		132		126		110		144		242	
07:00	36	200	129	353	154	690	100	405	190	890	229	758
07:15	42		92		186		103		228		195	
07:30	58		84		196		98		254		182	
07:45	64		48		154		104		218		152	
08:00	118	396	96	398	167	675	76	347	285	1,071	172	745
08:15	111		122		186		118		297		240	
08:30	82		100		148		86		230		186	
08:45	85		80		174		67		259		147	
09:00	67	270	82	305	141	482	84	299	208	752	166	604
09:15	70		84		108		86		178		170	
09:30	68		74		113		75		181		149	
09:45	65		65		120		54		185		119	
10:00	66	251	78	184	104	406	58	221	170	657	136	405
10:15	53		59		104		68		157		127	
10:30	80		39		106		50		186		89	
10:45	52		8		92		45		144		53	
11:00	50	284	8	21	88	390	45	156	138	674	53	177
11:15	78		6		113		44		191		50	
11:30	84		6		89		26		173		32	
11:45	72		1		100		41		172		42	
Totals	1,536		5,711		3,358		5,242		4,894		10,953	
Split%	31.4		52.1		68.6		47.9					
Day Totals		7,247				8,600				15,847		
Day Splits		45.7				54.3						
Peak Hour	08:00		05:15		07:15		04:30		08:00		04:30	
Volume	396		891		703		680		1,071		1,542	
Factor	0.84		0.86		0.90		0.89		0.90		0.96	

Data File : D1909026

2024

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : CHESTNUT AVENUE — Site: SANTA ANA

Segment : STANDARD AVE TO GRAND AVE — Date: 09/25/19

Client : CITY SANTA ANA — Day: Wednesday

Interval	EB				WB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	0	1	20	55	0	4	16	70	0	5	36	125
12:15	0		9		1		16		1		25	
12:30	0		14		1		16		1		30	
12:45	1		12		2		22		3		34	
01:00	1	1	22	126	2	4	15	137	3	5	37	263
01:15	0		32		2		14		2		46	
01:30	0		50		0		70		0		120	
01:45	0		22		0		38		0		60	
02:00	0	1	18	69	0	2	30	96	0	3	48	165
02:15	0		16		0		16		0		32	
02:30	0		18		0		34		0		52	
02:45	1		17		2		16		3		33	
03:00	0	2	14	67	0	2	14	78	0	4	28	145
03:15	1		20		1		24		2		44	
03:30	1		13		0		20		1		33	
03:45	0		20		1		20		1		40	
04:00	0	4	20	64	0	9	32	112	0	13	52	176
04:15	2		16		1		24		3		40	
04:30	0		16		5		26		5		42	
04:45	2		12		3		30		5		42	
05:00	10	39	36	105	6	39	42	130	16	78	78	235
05:15	8		20		14		32		22		52	
05:30	11		23		9		26		20		49	
05:45	10		26		10		30		20		56	
06:00	10	75	25	63	15	69	24	78	25	144	49	141
06:15	19		12		14		14		33		26	
06:30	24		16		16		20		40		36	
06:45	22		10		24		20		46		30	
07:00	32	224	6	26	18	192	12	35	50	416	18	61
07:15	54		8		42		2		96		10	
07:30	80		7		78		11		158		18	
07:45	58		5		54		10		112		15	
08:00	13	52	10	23	18	56	13	26	31	108	23	49
08:15	13		3		14		6		27		9	
08:30	14		6		7		4		21		10	
08:45	12		4		17		3		29		7	
09:00	16	54	4	18	10	47	3	22	26	101	7	40
09:15	14		2		19		5		33		7	
09:30	10		6		14		9		24		15	
09:45	14		6		4		5		18		11	
10:00	21	54	2	13	17	63	4	16	38	117	6	29
10:15	18		4		20		3		38		7	
10:30	8		3		19		6		27		9	
10:45	7		4		7		3		14		7	
11:00	11	51	1	8	13	66	3	13	24	117	4	21
11:15	16		3		23		3		39		6	
11:30	14		3		14		5		28		8	
11:45	10		1		16		2		26		3	
Totals	558		637		553		813		1,111		1,450	
Split%	50.2		43.9		49.8		56.1					
Day Totals		1,195				1,366				2,561		
Day Splits		46.7				53.3						
Peak Hour	07:00		01:00		07:00		01:30		07:00		01:15	
Volume	224		126		192		154		416		274	
Factor	0.70		0.63		0.62		0.55		0.66		0.57	

Data File : D1909034

2025

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : CIVIC CENTER DRIVE
Segment : MAIN ST TO SANTIAGO
Client : CITY SANTA ANA

Site: SANTA ANA
Date: 09/18/19
Day: Wednesday

Interval	WB				EB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	4	13	50	217	9	41	84	337	13	54	134	554
12:15	3		53		12		90		15		143	
12:30	2		54		10		94		12		148	
12:45	4		60		10		69		14		129	
01:00	3	9	71	268	4	19	70	357	7	28	141	625
01:15	2		71		6		70		8		141	
01:30	2		66		6		130		8		196	
01:45	2		60		3		87		5		147	
02:00	1	4	53	208	1	14	86	388	2	18	139	596
02:15	1		46		3		100		4		146	
02:30	1		63		8		92		9		155	
02:45	1		46		2		110		3		156	
03:00	2	7	79	315	5	15	116	503	7	22	195	818
03:15	0		98		2		121		2		219	
03:30	4		72		5		148		9		220	
03:45	1		66		3		118		4		184	
04:00	4	18	80	288	7	44	153	744	11	62	233	1,032
04:15	2		80		10		150		12		230	
04:30	4		66		11		191		15		257	
04:45	8		62		16		250		24		312	
05:00	10	63	64	270	18	101	234	654	28	164	298	924
05:15	9		57		17		188		26		245	
05:30	16		75		32		130		48		205	
05:45	28		74		34		102		62		176	
06:00	24	165	62	183	29	207	110	380	53	372	172	563
06:15	29		43		42		105		71		148	
06:30	26		35		66		93		92		128	
06:45	86		43		70		72		156		115	
07:00	76	529	39	144	68	472	74	223	144	1,001	113	367
07:15	116		42		100		56		216		98	
07:30	182		32		144		45		326		77	
07:45	155		31		160		48		315		79	
08:00	140	417	37	99	153	376	56	227	293	793	93	326
08:15	98		28		96		60		194		88	
08:30	109		22		60		57		169		79	
08:45	70		12		67		54		137		66	
09:00	62	226	24	84	54	222	26	124	116	448	50	208
09:15	68		20		60		40		128		60	
09:30	48		26		56		36		104		62	
09:45	48		14		52		22		100		36	
10:00	55	170	10	32	56	228	28	93	111	398	38	125
10:15	32		8		56		32		88		40	
10:30	48		10		56		22		104		32	
10:45	35		4		60		11		95		15	
11:00	50	203	4	15	90	384	11	64	140	587	15	79
11:15	46		4		89		20		135		24	
11:30	55		4		101		18		156		22	
11:45	52		3		104		15		156		18	
Totals	1,824		2,123		2,123		4,094		3,947		6,217	
Split%	46.2		34.1		53.8		65.9					
Day Totals		3,947				6,217				10,164		
Day Splits		38.8				61.2						
Peak Hour	07:15		03:15		07:15		04:30		07:15		04:30	
Volume	593		316		557		863		1,150		1,112	
Factor	0.81		0.81		0.87		0.86		0.88		0.89	

Data File : D1909042

2026

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : CIVIC CENTER DRIVE
Segment : FAIRVIEW ST TO RAITT ST
Client : CITY SANTA ANA

Site: SANTA ANA
Date: 09/18/19
Day: Wednesday

Interval	EB				WB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	9	46	59	291	10	39	90	317	19	85	149	608
12:15	17		64		9		77		26		141	
12:30	14		82		14		76		28		158	
12:45	6		86		6		74		12		160	
01:00	13	30	96	347	13	39	80	403	26	69	176	750
01:15	10		96		14		88		24		184	
01:30	3		82		5		112		8		194	
01:45	4		73		7		123		11		196	
02:00	8	24	96	360	11	25	99	491	19	49	195	851
02:15	7		94		3		134		10		228	
02:30	3		82		6		142		9		224	
02:45	6		88		5		116		11		204	
03:00	2	17	136	482	3	30	142	524	5	47	278	1,006
03:15	3		112		11		154		14		266	
03:30	7		122		8		114		15		236	
03:45	5		112		8		114		13		226	
04:00	6	48	121	508	12	87	120	576	18	135	241	1,084
04:15	10		128		22		144		32		272	
04:30	16		128		24		160		40		288	
04:45	16		131		29		152		45		283	
05:00	32	127	144	661	34	167	162	660	66	294	306	1,321
05:15	26		174		26		196		52		370	
05:30	31		192		59		158		90		350	
05:45	38		151		48		144		86		295	
06:00	44	299	128	477	54	305	137	408	98	604	265	885
06:15	54		128		72		108		126		236	
06:30	82		126		74		91		156		217	
06:45	119		95		105		72		224		167	
07:00	147	831	96	326	107	437	76	310	254	1,268	172	636
07:15	186		70		102		72		288		142	
07:30	262		84		114		82		376		166	
07:45	236		76		114		80		350		156	
08:00	156	560	79	232	132	435	78	271	288	995	157	503
08:15	164		52		100		66		264		118	
08:30	118		53		103		62		221		115	
08:45	122		48		100		65		222		113	
09:00	74	290	54	190	64	272	67	240	138	562	121	430
09:15	61		60		60		68		121		128	
09:30	77		46		82		60		159		106	
09:45	78		30		66		45		144		75	
10:00	78	271	25	124	64	279	41	118	142	550	66	242
10:15	66		40		96		28		162		68	
10:30	70		27		65		32		135		59	
10:45	57		32		54		17		111		49	
11:00	62	252	20	61	64	285	24	64	126	537	44	125
11:15	53		11		65		19		118		30	
11:30	66		15		66		14		132		29	
11:45	71		15		90		7		161		22	
Totals	2,795		4,059		2,400		4,382		5,195		8,441	
Split%	53.8		48.1		46.2		51.9					
Day Totals		6,854				6,782				13,636		
Day Splits		50.3				49.7						
Peak Hour	07:15		05:00		07:15		04:30		07:15		05:00	
Volume	840		661		462		670		1,302		1,321	
Factor	0.80		0.86		0.88		0.85		0.87		0.89	

Data File : D1909036

2027

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : FLOWER STREET — Site: SANTA ANA

Segment : WARNER AVE TO EDINGER AVE — Date: 09/10/19

Client : CITY SANTA ANA — Day: Tuesday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	15	46	52	209	7	36	66	262	22	82	118	471
12:15	12		48		6		78		18		126	
12:30	9		47		13		60		22		107	
12:45	10		62		10		58		20		120	
01:00	11	25	52	248	6	25	60	241	17	50	112	489
01:15	8		46		5		50		13		96	
01:30	3		71		4		62		7		133	
01:45	3		79		10		69		13		148	
02:00	0	11	73	391	8	23	77	381	8	34	150	772
02:15	2		86		2		83		4		169	
02:30	4		92		4		116		8		208	
02:45	5		140		9		105		14		245	
03:00	2	18	135	524	3	34	66	298	5	52	201	822
03:15	2		134		9		66		11		200	
03:30	6		131		10		86		16		217	
03:45	8		124		12		80		20		204	
04:00	2	27	122	481	10	101	74	336	12	128	196	817
04:15	5		121		13		84		18		205	
04:30	7		118		41		84		48		202	
04:45	13		120		37		94		50		214	
05:00	10	91	134	525	33	255	100	408	43	346	234	933
05:15	26		140		46		100		72		240	
05:30	23		127		86		90		109		217	
05:45	32		124		90		118		122		242	
06:00	35	161	130	465	61	342	94	339	96	503	224	804
06:15	29		120		77		96		106		216	
06:30	44		120		106		72		150		192	
06:45	53		95		98		77		151		172	
07:00	43	270	78	308	118	474	70	282	161	744	148	590
07:15	77		73		122		73		199		146	
07:30	79		84		130		72		209		156	
07:45	71		73		104		67		175		140	
08:00	89	302	83	258	132	466	65	207	221	768	148	465
08:15	80		69		104		54		184		123	
08:30	68		60		127		42		195		102	
08:45	65		46		103		46		168		92	
09:00	33	155	45	186	86	263	38	141	119	418	83	327
09:15	36		50		53		29		89		79	
09:30	47		46		62		40		109		86	
09:45	39		45		62		34		101		79	
10:00	48	175	36	138	50	219	30	115	98	394	66	253
10:15	40		31		66		28		106		59	
10:30	42		40		44		36		86		76	
10:45	45		31		59		21		104		52	
11:00	40	214	33	80	68	238	20	61	108	452	53	141
11:15	52		16		56		16		108		32	
11:30	66		18		62		11		128		29	
11:45	56		13		52		14		108		27	
Totals	1,495		3,813		2,476		3,071		3,971		6,884	
Split%	37.6		55.4		62.4		44.6					
Day Totals		5,308				5,547				10,855		
Day Splits		48.9				51.1						
Peak Hour	07:30		02:45		07:15		05:00		07:15		05:00	
Volume	319		540		488		408		804		933	
Factor	0.90		0.96		0.92		0.86		0.91		0.96	

Data File : D1909102

2028

Thursday, September 26, 2019 CITY: Santa Ana PROJECT: SC

ADT138 HAZARD AVE from EUCLID ST to NEWHOPE ST. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB	SB	EB		WB			PM Period	NB	SB	EB		WB		
0:00			10		18			12:00			46		69		
0:15			8		6			12:15			51		47		
0:30			4		5			12:30			55		63		
0:45			4	26	10	39	65	12:45			52	204	45	224	428
1:00			4		5			13:00			59		43		
1:15			6		4			13:15			53		60		
1:30			12		4			13:30			63		55		
1:45			6	28	6	19	47	13:45			77	252	73	231	483
2:00			4		0			14:00			61		74		
2:15			4		3			14:15			85		77		
2:30			0		0			14:30			78		64		
2:45			3	11	3	6	17	14:45			90	314	64	279	593
3:00			4		7			15:00			91		94		
3:15			7		2			15:15			90		81		
3:30			5		6			15:30			90		79		
3:45			0	16	3	18	34	15:45			117	388	76	330	718
4:00			4		4			16:00			86		84		
4:15			10		6			16:15			75		78		
4:30			10		14			16:30			78		96		
4:45			12	36	10	34	70	16:45			97	336	100	358	694
5:00			11		13			17:00			90		96		
5:15			12		12			17:15			89		146		
5:30			19		14			17:30			95		125		
5:45			28	70	15	54	124	17:45			105	379	132	499	878
6:00			27		18			18:00			84		133		
6:15			32		21			18:15			90		106		
6:30			40		31			18:30			88		90		
6:45			54	153	37	107	260	18:45			91	353	83	412	765
7:00			55		40			19:00			65		71		
7:15			94		81			19:15			65		67		
7:30			114		136			19:30			60		49		
7:45			156	419	172	429	848	19:45			46	236	46	233	469
8:00			110		126			20:00			56		33		
8:15			120		81			20:15			54		46		
8:30			121		64			20:30			39		36		
8:45			82	433	60	331	764	20:45			42	191	33	148	339
9:00			68		60			21:00			33		29		
9:15			58		79			21:15			41		27		
9:30			47		67			21:30			28		21		
9:45			50	223	71	277	500	21:45			26	128	40	117	245
10:00			70		42			22:00			23		25		
10:15			47		51			22:15			23		24		
10:30			49		52			22:30			16		17		
10:45			48	214	47	192	406	22:45			12	74	11	77	151
11:00			79		48			23:00			17		14		
11:15			54		66			23:15			8		16		
11:30			51		55			23:30			12		9		
11:45			68	252	34	203	455	23:45			9	46	13	52	98
Total Vol.			1881		1709		**3590**				2901		2960		**5861**

Daily Totals

NB	SB	EB	WB	Combined
		4782	4669	**9451**

	AM			PM		
Split %	52.4%	47.6%	**38.0%**	49.5%	50.5%	**62.0%**
Peak Hour	7:45	7:15	**7:30**	15:00	17:15	**17:15**
Volume	507	515	**1015**	388	536	**909**
P.H.F.	0.81	0.75	**0.77**	0.83	0.92	**0.96**

Tuesday, September 24, 2019 CITY: Santa Ana PROJECT: SC

ADT179 McFADDEN AVE from MAIN ST to STANDARD AVE. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB	SB	EB		WB			PM Period	NB	SB	EB		WB		
0:00			15		24			12:00			76		89		
0:15			12		17			12:15			87		95		
0:30			13		14			12:30			92		79		
0:45			12	52	13	68	120	12:45			92	347	78	341	688
1:00			7		13			13:00			63		81		
1:15			7		14			13:15			87		86		
1:30			6		6			13:30			86		115		
1:45			10	30	11	44	74	13:45			95	331	106	388	719
2:00			15		11			14:00			95		125		
2:15			5		11			14:15			116		152		
2:30			5		6			14:30			130		187		
2:45			7	32	6	34	66	14:45			127	468	167	631	1099
3:00			6		5			15:00			130		181		
3:15			11		7			15:15			151		180		
3:30			23		5			15:30			107		196		
3:45			37	77	14	31	108	15:45			127	515	187	744	1259
4:00			18		7			16:00			108		182		
4:15			44		12			16:15			100		221		
4:30			55		27			16:30			100		196		
4:45			57	174	17	63	237	16:45			113	421	220	819	1240
5:00			60		20			17:00			114		208		
5:15			82		27			17:15			109		211		
5:30			109		41			17:30			120		203		
5:45			168	419	36	124	543	17:45			122	465	154	776	1241
6:00			136		43			18:00			134		175		
6:15			145		51			18:15			116		172		
6:30			157		37			18:30			110		157		
6:45			185	623	49	180	803	18:45			128	488	125	629	1117
7:00			214		67			19:00			114		110		
7:15			187		80			19:15			114		124		
7:30			220		84			19:30			99		103		
7:45			199	820	98	329	1149	19:45			88	415	101	438	853
8:00			164		107			20:00			101		88		
8:15			161		87			20:15			84		84		
8:30			125		77			20:30			71		44		
8:45			149	599	66	337	936	20:45			66	322	67	283	605
9:00			88		72			21:00			66		58		
9:15			97		68			21:15			71		63		
9:30			84		86			21:30			68		53		
9:45			83	352	73	299	651	21:45			55	260	62	236	496
10:00			76		92			22:00			60		69		
10:15			80		72			22:15			36		53		
10:30			87		64			22:30			25		46		
10:45			67	310	80	308	618	22:45			24	145	42	210	355
11:00			95		74			23:00			36		40		
11:15			90		101			23:15			31		28		
11:30			84		98			23:30			12		35		
11:45			85	354	78	351	705	23:45			21	100	20	123	223
Total Vol.			3842		2168		**6010**				4277		5618		**9895**

	Daily Totals				
	NB	SB	EB	WB	**Combined**
			8119	7786	**15905**

	AM			**PM**		
Split %	63.9%	36.1%	**37.8%**	43.2%	56.8%	**62.2%**
Peak Hour	7:00	7:30	**7:00**	14:30	16:15	**16:45**
Volume	820	376	**1149**	538	845	**1298**
P.H.F.	0.93	0.88	**0.94**	0.89	0.96	**0.97**

Thursday, October 03, 2019 Location: Santa Ana PROJECT:

ADT197 RAITT ST from FIRST ST to SANTA ANA BL. **Prepared by AimTD tel. 714 753 7888**

AM Period	NB		SB		EB	WB		PM Period	NB		SB		EB	WB	
0:00	9		5					12:00	56		71				
0:15	6		6					12:15	58		67				
0:30	5		3					12:30	61		56				
0:45	2	22	5	19			41	12:45	65	240	64	258			498
1:00	0		4					13:00	54		70				
1:15	0		0					13:15	53		65				
1:30	1		2					13:30	61		58				
1:45	1	2	4	10			12	13:45	74	242	62	255			497
2:00	0		2					14:00	101		58				
2:15	2		0					14:15	89		67				
2:30	3		1					14:30	86		89				
2:45	1	6	0	3			9	14:45	103	379	79	293			672
3:00	3		1					15:00	109		96				
3:15	1		4					15:15	129		74				
3:30	2		2					15:30	132		67				
3:45	2	8	0	7			15	15:45	152	522	78	315			837
4:00	0		2					16:00	132		91				
4:15	3		4					16:15	164		76				
4:30	1		1					16:30	152		51				
4:45	6	10	5	12			22	16:45	175	623	57	275			898
5:00	9		8					17:00	142		89				
5:15	16		16					17:15	151		93				
5:30	18		13					17:30	158		72				
5:45	23	66	14	51			117	17:45	173	624	59	313			937
6:00	44		24					18:00	168		56				
6:15	36		25					18:15	152		64				
6:30	41		30					18:30	111		76				
6:45	52	173	48	127			300	18:45	85	516	78	274			790
7:00	98		104					19:00	71		51				
7:15	141		132					19:15	58		57				
7:30	195		150					19:30	64		46				
7:45	153	587	114	500			1087	19:45	47	240	56	210			450
8:00	121		65					20:00	53		45				
8:15	98		53					20:15	56		39				
8:30	64		55					20:30	51		37				
8:45	51	334	49	222			556	20:45	42	202	46	167			369
9:00	75		42					21:00	50		35				
9:15	53		45					21:15	34		37				
9:30	48		43					21:30	42		32				
9:45	70	246	35	165			411	21:45	25	151	28	132			283
10:00	57		46					22:00	21		25				
10:15	67		40					22:15	16		21				
10:30	53		36					22:30	14		18				
10:45	69	246	35	157			403	22:45	18	69	16	80			149
11:00	58		43					23:00	9		10				
11:15	59		65					23:15	7		7				
11:30	70		56					23:30	5		5				
11:45	66	253	54	218			471	23:45	7	28	5	27			55
Total Vol.		1953		1491			**3444**			3836		2599			**6435**

Daily Totals

NB	SB	EB	WB	Combined
5789	4090			**9879**

	AM			PM		
Split %	56.7%	43.3%	**34.9%**	59.6%	40.4%	**65.1%**
Peak Hour	7:15	7:00	**7:00**	17:30	14:30	**16:45**
Volume	610	500	**1087**	651	338	**937**
P.H.F.	0.78	0.83	**0.79**	0.96	0.88	**0.96**

pacific@aimtd.com Tell. 714 753 7888

Wednesday, September 11, 2019 Location: Santa Ana PROJECT:

ADT193 RAITT ST from SEGERSTROM AVE to WARNER AVE. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB		SB		EB	WB		PM Period	NB		SB		EB	WB	
0:00	21		10					12:00	77		66				
0:15	12		7					12:15	54		60				
0:30	14		6					12:30	69		59				
0:45	11	58	7	30			88	12:45	71	271	69	254			525
1:00	10		4					13:00	86		98				
1:15	7		5					13:15	100		87				
1:30	6		2					13:30	176		90				
1:45	4	27	5	16			43	13:45	206	568	89	364			932
2:00	4		2					14:00	127		78				
2:15	7		4					14:15	113		57				
2:30	7		7					14:30	139		102				
2:45	5	23	4	17			40	14:45	185	564	112	349			913
3:00	2		9					15:00	250		106				
3:15	5		4					15:15	175		86				
3:30	2		8					15:30	209		69				
3:45	0	9	16	37			46	15:45	239	873	89	350			1223
4:00	5		16					16:00	251		82				
4:15	8		14					16:15	257		70				
4:30	4		34					16:30	299		79				
4:45	4	21	33	97			118	16:45	268	1075	88	319			1394
5:00	6		38					17:00	261		85				
5:15	11		62					17:15	234		90				
5:30	13		82					17:30	301		105				
5:45	12	42	95	277			319	17:45	237	1033	109	389			1422
6:00	20		57					18:00	246		101				
6:15	21		106					18:15	210		89				
6:30	42		95					18:30	151		75				
6:45	103	186	156	414			600	18:45	143	750	89	354			1104
7:00	117		172					19:00	128		91				
7:15	114		197					19:15	117		74				
7:30	176		187					19:30	104		57				
7:45	165	572	184	740			1312	19:45	103	452	61	283			735
8:00	170		163					20:00	90		62				
8:15	74		143					20:15	73		56				
8:30	84		179					20:30	71		59				
8:45	146	474	207	692			1166	20:45	81	315	37	214			529
9:00	104		87					21:00	67		58				
9:15	40		59					21:15	70		38				
9:30	34		65					21:30	57		28				
9:45	48	226	65	276			502	21:45	75	269	24	148			417
10:00	38		51					22:00	51		24				
10:15	31		36					22:15	56		14				
10:30	43		61					22:30	44		19				
10:45	46	158	53	201			359	22:45	36	187	16	73			260
11:00	46		49					23:00	27		9				
11:15	47		55					23:15	31		15				
11:30	52		44					23:30	24		6				
11:45	60	205	57	205			410	23:45	22	104	9	39			143
Total Vol.		2001		3002			**5003**			6461		3136			**9597**

Daily Totals

NB	SB	EB	WB	**Combined**
8462	6138			**14600**

	AM			**PM**		
Split %	40.0%	60.0%	**34.3%**	67.3%	32.7%	**65.7%**
Peak Hour	7:15	7:00	**7:15**	16:15	14:30	**16:45**
Volume	625	740	**1356**	1085	406	**1432**
P.H.F.	0.89	0.94	**0.93**	0.91	0.91	**0.88**

cs@aimtd.com Tell. 714 253 7888

Thursday, October 03, 2019 CITY: Santa Ana PROJECT: SC

ADT211 SANTA ANA BLVD from MAIN ST to FRENCH ST **Prepared by AimTD tel. 714 253 7888**

AM Period	NB	SB	EB		WB			PM Period	NB	SB	EB		WB		
0:00			0		23			12:00			0		44		
0:15			0		14			12:15			0		69		
0:30			0		16			12:30			0		48		
0:45			0	0	13	66	66	12:45			0	0	62	223	223
1:00			0		18			13:00			0		71		
1:15			0		6			13:15			0		60		
1:30			0		13			13:30			0		79		
1:45			0	0	9	46	46	13:45			0	0	75	285	285
2:00			0		6			14:00			0		71		
2:15			0		17			14:15			0		85		
2:30			0		17			14:30			0		86		
2:45			0	0	11	51	51	14:45			0	0	90	332	332
3:00			0		4			15:00			0		88		
3:15			0		7			15:15			0		108		
3:30			0		17			15:30			0		94		
3:45			0	0	19	47	47	15:45			0	0	93	383	383
4:00			0		12			16:00			0		135		
4:15			0		19			16:15			0		151		
4:30			0		18			16:30			0		166		
4:45			0	0	24	73	73	16:45			0	0	171	623	623
5:00			0		27			17:00			0		178		
5:15			0		34			17:15			0		182		
5:30			0		53			17:30			0		178		
5:45			0	0	75	189	189	17:45			0	0	192	730	730
6:00			0		71			18:00			0		146		
6:15			0		55			18:15			0		124		
6:30			0		78			18:30			0		127		
6:45			0	0	117	321	321	18:45			0	0	121	518	518
7:00			0		132			19:00			0		102		
7:15			0		106			19:15			0		94		
7:30			0		118			19:30			0		96		
7:45			0	0	137	493	493	19:45			0	0	83	375	375
8:00			0		107			20:00			0		80		
8:15			0		72			20:15			0		77		
8:30			0		67			20:30			0		80		
8:45			0	0	52	298	298	20:45			0	0	73	310	310
9:00			0		68			21:00			0		38		
9:15			0		56			21:15			0		58		
9:30			0		61			21:30			0		70		
9:45			0	0	56	241	241	21:45			0	0	40	206	206
10:00			0		64			22:00			0		43		
10:15			0		58			22:15			0		46		
10:30			0		48			22:30			0		36		
10:45			0	0	53	223	223	22:45			0	0	42	167	167
11:00			0		68			23:00			0		30		
11:15			0		54			23:15			0		28		
11:30			0		58			23:30			0		26		
11:45			0	0	65	245	245	23:45			0	0	21	105	105
Total Vol.						2293	**2293**							4257	**4257**

Daily Totals				
NB	SB	EB	WB	**Combined**
			6550	**6550**

	AM		PM	
Split %	100.0%	**35.0%**	100.0%	**65.0%**
Peak Hour	7:00	**7:00**	17:00	**17:00**
Volume	493	**493**	730	**730**
P.H.F.	0.90	**0.90**	0.95	**0.95**

cs@aimtd.com Tell. 714 253 7888

Wednesday, October 02, 2019 CITY: Santa Ana PROJECT: SC

ADT206 SANTA ANA BLVD from RAITT ST to BRISTOL ST. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB	SB	EB		WB			PM Period	NB	SB	EB		WB		
0:00			3		4			12:00			20		31		
0:15			0		7			12:15			18		40		
0:30			3		5			12:30			25		37		
0:45			2	8	4	20	28	12:45			23	86	27	135	221
1:00			2		0			13:00			35		26		
1:15			0		3			13:15			23		37		
1:30			0		0			13:30			21		28		
1:45			3	5	0	3	8	13:45			26	105	32	123	228
2:00			0		0			14:00			29		22		
2:15			2		0			14:15			15		28		
2:30			0		0			14:30			26		22		
2:45			0	2	0	0	2	14:45			25	95	40	112	207
3:00			0		0			15:00			28		47		
3:15			2		0			15:15			41		43		
3:30			3		2			15:30			35		45		
3:45			2	7	3	5	12	15:45			30	134	39	174	308
4:00			4		2			16:00			18		45		
4:15			3		2			16:15			27		55		
4:30			8		4			16:30			36		49		
4:45			11	26	9	17	43	16:45			34	115	67	216	331
5:00			8		2			17:00			33		76		
5:15			16		5			17:15			25		82		
5:30			10		19			17:30			33		71		
5:45			14	48	20	46	94	17:45			28	119	64	293	412
6:00			17		22			18:00			27		47		
6:15			21		15			18:15			30		31		
6:30			20		22			18:30			16		37		
6:45			31	89	23	82	171	18:45			27	100	34	149	249
7:00			42		23			19:00			19		30		
7:15			96		29			19:15			17		26		
7:30			108		39			19:30			19		29		
7:45			108	354	51	142	496	19:45			9	64	23	108	172
8:00			66		35			20:00			9		30		
8:15			60		43			20:15			9		16		
8:30			25		20			20:30			10		24		
8:45			37	188	26	124	312	20:45			11	39	14	84	123
9:00			27		18			21:00			12		13		
9:15			26		25			21:15			13		12		
9:30			23		20			21:30			14		21		
9:45			13	89	27	90	179	21:45			7	46	19	65	111
10:00			27		21			22:00			9		16		
10:15			14		28			22:15			6		14		
10:30			18		19			22:30			3		7		
10:45			28	87	21	89	176	22:45			9	27	12	49	76
11:00			18		26			23:00			3		9		
11:15			21		31			23:15			4		8		
11:30			26		33			23:30			2		2		
11:45			15	80	31	121	201	23:45			2	11	4	23	34
Total Vol.			983		739		**1722**				941		1531		**2472**

	NB	SB	EB	WB	**Combined**
Daily Totals			1924	2270	**4194**
			AM	**PM**	
Split %			57.1%	42.9% **41.1%**	38.1% 61.9% **58.9%**
Peak Hour			7:15	7:30 **7:15**	15:00 16:45 **16:45**
Volume			378	168 **532**	134 296 **421**
P.H.F.			0.88	0.82 **0.84**	0.82 0.90 **0.97**

Wednesday, October 02, 2019 Location: Santa Ana PROJECT:

ADT243 STANDARD AVE from McFADDEN AVE to CHESTNUT AVE. **Prepared by AimTD tel. 714 753 7888**

AM Period	NB		SB		EB	WB		PM Period	NB		SB		EB	WB	
0:00	15		11					12:00	77		61				
0:15	11		9	9				12:15	73		56				
0:30	11		6					12:30	82		66				
0:45	12	49	13	39			88	12:45	80	312	85	268			580
1:00	8		5					13:00	75		113				
1:15	9		6					13:15	86		105				
1:30	11		7					13:30	79		109				
1:45	13	41	4	22			63	13:45	91	331	92	419			750
2:00	6		5					14:00	114		83				
2:15	6		4					14:15	115		92				
2:30	2		4					14:30	132		79				
2:45	4	18	8	21			39	14:45	129	490	85	339			829
3:00	3		3					15:00	134		98				
3:15	5		5					15:15	166		96				
3:30	4		6					15:30	172		84				
3:45	7	19	4	18			37	15:45	166	638	92	370			1008
4:00	3		10					16:00	184		106				
4:15	6		5					16:15	188		93				
4:30	10		8					16:30	196		107				
4:45	15	34	14	37			71	16:45	218	786	113	419			1205
5:00	18		18					17:00	209		123				
5:15	15		19					17:15	222		114				
5:30	28		47					17:30	207		100				
5:45	31	92	48	132			224	17:45	197	835	99	436			1271
6:00	24		40					18:00	172		97				
6:15	26		46					18:15	141		122				
6:30	52		60					18:30	153		95				
6:45	85	187	63	209			396	18:45	131	597	89	403			1000
7:00	95		97					19:00	105		72				
7:15	105		112					19:15	96		74				
7:30	113		114					19:30	87		85				
7:45	156	469	146	469			938	19:45	91	379	72	303			682
8:00	134		120					20:00	96		77				
8:15	104		99					20:15	68		64				
8:30	85		84					20:30	57		49				
8:45	103	426	74	377			803	20:45	48	269	51	241			510
9:00	78		55					21:00	55		51				
9:15	70		74					21:15	45		53				
9:30	64		80					21:30	49		40				
9:45	73	285	76	285			570	21:45	32	181	43	187			368
10:00	56		57					22:00	43		34				
10:15	53		46					22:15	35		19				
10:30	49		57					22:30	29		27				
10:45	45	203	63	223			426	22:45	23	130	25	105			235
11:00	50		67					23:00	25		15				
11:15	56		78					23:15	15		23				
11:30	64		64					23:30	17		15				
11:45	74	244	59	268			512	23:45	13	70	16	69			139
Total Vol.		2067		2100			**4167**			5018		3559			**8577**

Daily Totals

NB	SB	EB	WB	**Combined**
7085	5659			**12744**

	AM			**PM**		
Split %	49.6%	50.4%	**32.7%**	58.5%	41.5%	**67.3%**
Peak Hour	7:15	7:15	**7:15**	16:45	16:30	**16:45**
Volume	508	492	**1000**	856	457	**1306**
P.H.F.	0.81	0.84	**0.83**	0.98	0.93	**0.97**

pacific@aimtd.com Tell. 714 753 7888

Wednesday, September 11, 2019 Location: Santa Ana PROJECT:

ADT241 STANDARD AVE from WARNER AVE to EDINGER AVE. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB		SB		EB	WB		PM Period	NB		SB		EB	WB	
0:00	23		12					12:00	92		75				
0:15	12		12					12:15	82		72				
0:30	14		12					12:30	78		65				
0:45	11	60	12	48			108	12:45	70	322	69	281			603
1:00	32		10					13:00	79		66				
1:15	13		9					13:15	91		73				
1:30	15		5					13:30	126		55				
1:45	7	67	10	34			101	13:45	109	405	65	259			664
2:00	8		6					14:00	120		75				
2:15	4		6					14:15	112		76				
2:30	15		5					14:30	147		95				
2:45	6	33	11	28			61	14:45	165	544	96	342			886
3:00	5		3					15:00	198		94				
3:15	4		7					15:15	151		75				
3:30	9		7					15:30	202		77				
3:45	12	30	16	33			63	15:45	188	739	70	316			1055
4:00	10		11					16:00	191		88				
4:15	9		29					16:15	188		88				
4:30	10		50					16:30	208		89				
4:45	23	52	74	164			216	16:45	257	844	86	351			1195
5:00	27		58					17:00	274		79				
5:15	38		67					17:15	241		107				
5:30	39		72					17:30	197		90				
5:45	38	142	123	320			462	17:45	146	858	103	379			1237
6:00	45		83					18:00	136		67				
6:15	51		79					18:15	149		65				
6:30	69		113					18:30	127		67				
6:45	67	232	120	395			627	18:45	102	514	71	270			784
7:00	94		112					19:00	74		60				
7:15	73		149					19:15	84		58				
7:30	89		160					19:30	79		65				
7:45	99	355	152	573			928	19:45	68	305	65	248			553
8:00	79		127					20:00	71		57				
8:15	78		112					20:15	56		47				
8:30	82		65					20:30	64		45				
8:45	72	311	80	384			695	20:45	47	238	44	193			431
9:00	54		68					21:00	42		39				
9:15	68		49					21:15	53		37				
9:30	62		67					21:30	37		35				
9:45	52	236	52	236			472	21:45	36	168	35	146			314
10:00	61		47					22:00	46		43				
10:15	53		50					22:15	35		30				
10:30	55		37					22:30	20		22				
10:45	47	216	48	182			398	22:45	33	134	20	115			249
11:00	67		33					23:00	24		20				
11:15	72		60					23:15	25		9				
11:30	74		59					23:30	18		11				
11:45	87	300	45	197			497	23:45	11	78	8	48			126
Total Vol.		2034		2594			**4628**			5149		2948			**8097**

Daily Totals

NB	SB	EB	WB	**Combined**
7183	5542			**12725**

	AM			**PM**		
Split %	43.9%	56.1%	**36.4%**	63.6%	36.4%	**63.6%**
Peak Hour	7:00	7:15	**7:00**	16:30	17:00	**16:30**
Volume	355	588	**928**	980	379	**1341**
P.H.F.	0.90	0.92	**0.92**	0.92	0.89	**0.95**

cs@aimtd.com Tell. 714 253 7888

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : GREENVILLE STREET — Site: SANTA ANA

Segment : WARNER AVE TO EDINGER AVE — Date: 10/01/19

Client : CITY SANTA ANA — Day: Tuesday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	7	19	21	128	4	9	36	135	11	28	57	263
12:15	8		29		2		31		10		60	
12:30	2		38		2		33		4		71	
12:45	2		40		1		35		3		75	
01:00	7	20	43	172	4	12	23	155	11	32	66	327
01:15	4		43		1		42		5		85	
01:30	4		38		5		46		9		84	
01:45	5		48		2		44		7		92	
02:00	2	10	54	268	0	6	52	238	2	16	106	506
02:15	1		45		2		58		3		103	
02:30	1		77		2		48		3		125	
02:45	6		92		2		80		8		172	
03:00	2	11	110	433	0	14	68	212	2	25	178	645
03:15	2		114		4		46		6		160	
03:30	3		102		7		46		10		148	
03:45	4		107		3		52		7		159	
04:00	4	16	110	466	3	41	48	197	7	57	158	663
04:15	2		121		10		43		12		164	
04:30	6		113		16		56		22		169	
04:45	4		122		12		50		16		172	
05:00	7	45	113	499	15	107	60	226	22	152	173	725
05:15	11		135		16		54		27		189	
05:30	14		128		35		46		49		174	
05:45	13		123		41		66		54		189	
06:00	22	104	99	314	30	173	59	216	52	277	158	530
06:15	18		93		46		50		64		143	
06:30	28		69		43		58		71		127	
06:45	36		53		54		49		90		102	
07:00	50	272	44	149	70	340	50	195	120	612	94	344
07:15	70		35		102		52		172		87	
07:30	84		39		88		50		172		89	
07:45	68		31		80		43		148		74	
08:00	60	163	43	147	69	202	57	156	129	365	100	303
08:15	44		34		52		42		96		76	
08:30	36		35		42		34		78		69	
08:45	23		35		39		23		62		58	
09:00	20	107	34	114	31	127	30	101	51	234	64	215
09:15	32		34		30		28		62		62	
09:30	29		24		38		28		67		52	
09:45	26		22		28		15		54		37	
10:00	34	109	19	58	28	129	18	56	62	238	37	114
10:15	23		20		26		11		49		31	
10:30	20		12		43		17		63		29	
10:45	32		7		32		10		64		17	
11:00	32	138	17	38	24	146	7	27	56	284	24	65
11:15	40		15		33		8		73		23	
11:30	36		2		42		6		78		8	
11:45	30		4		47		6		77		10	
Totals	1,014		2,786		1,306		1,914		2,320		4,700	
Split%	43.7		59.3		56.3		40.7					
Day Totals		3,800				3,220				7,020		
Day Splits		54.1				45.9						
Peak Hour	07:15		05:00		07:00		02:15		07:15		05:00	
Volume	282		499		340		254		621		725	
Factor	0.84		0.92		0.83		0.79		0.90		0.96	

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : GREENVILLE STREET — Site: SANTA ANA

Segment : SEGERSTROM AVE TO WARNER AVE — Date: 10/01/19

Client : CITY SANTA ANA — Day: Tuesday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	6	29	41	158	5	21	32	145	11	50	73	303
12:15	9		39		7		35		16		74	
12:30	4		36		9		36		13		72	
12:45	10		42		0		42		10		84	
01:00	3	15	46	187	3	12	32	193	6	27	78	380
01:15	6		43		2		48		8		91	
01:30	4		43		2		51		6		94	
01:45	2		55		5		62		7		117	
02:00	6	14	67	404	1	7	56	280	7	21	123	684
02:15	1		60		3		76		4		136	
02:30	2		84		1		72		3		156	
02:45	5		193		2		76		7		269	
03:00	1	5	192	642	4	14	66	219	5	19	258	861
03:15	2		138		0		52		2		190	
03:30	2		142		1		43		3		185	
03:45	0		170		9		58		9		228	
04:00	1	8	181	663	6	51	35	199	7	59	216	862
04:15	3		148		6		53		9		201	
04:30	1		178		17		47		18		225	
04:45	3		156		22		64		25		220	
05:00	4	24	174	666	16	122	57	229	20	146	231	895
05:15	8		190		29		54		37		244	
05:30	8		162		27		62		35		224	
05:45	4		140		50		56		54		196	
06:00	14	76	122	408	28	198	70	212	42	274	192	620
06:15	14		114		50		42		64		156	
06:30	21		93		44		50		65		143	
06:45	27		79		76		50		103		129	
07:00	26	231	52	205	94	571	39	184	120	802	91	389
07:15	57		54		147		57		204		111	
07:30	62		43		188		44		250		87	
07:45	86		56		142		44		228		100	
08:00	72	160	52	214	102	310	51	137	174	470	103	351
08:15	40		63		78		32		118		95	
08:30	30		53		54		31		84		84	
08:45	18		46		76		23		94		69	
09:00	31	117	47	136	36	153	22	90	67	270	69	226
09:15	22		41		36		27		58		68	
09:30	30		21		49		22		79		43	
09:45	34		27		32		19		66		46	
10:00	39	107	25	81	41	159	17	54	80	266	42	135
10:15	22		28		38		13		60		41	
10:30	18		10		34		11		52		21	
10:45	28		18		46		13		74		31	
11:00	32	141	15	61	40	153	5	28	72	294	20	89
11:15	30		15		28		4		58		19	
11:30	43		13		40		10		83		23	
11:45	36		18		45		9		81		27	
Totals	927		3,825		1,771		1,970		2,698		5,795	
Split%	34.4		66.0		65.6		34.0					
Day Totals		4,752				3,741				8,493		
Day Splits		56.0				44.0						
Peak Hour	07:15		04:30		07:15		02:15		07:15		04:30	
Volume	277		698		579		290		856		920	
Factor	0.81		0.92		0.77		0.95		0.86		0.94	

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : FLOWER STREET — Site: SANTA ANA

Segment : SANTA CLARA TO MEMORY LN — Date: 09/12/19

Client : CITY SANTA ANA — File: D1909110

Day: Thursday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	3	9	58	187	14	30	82	302	17	39	140	489
12:15	4		50		3		74		7		124	
12:30	2		41		4		76		6		117	
12:45	0		38		9		70		9		108	
01:00	4	7	54	180	0	8	102	373	4	15	156	553
01:15	1		34		2		74		3		108	
01:30	2		49		0		87		2		136	
01:45	0		43		6		110		6		153	
02:00	0	4	68	314	4	16	127	484	4	20	195	798
02:15	2		96		6		148		8		244	
02:30	1		92		4		93		5		185	
02:45	1		58		2		116		3		174	
03:00	0	7	80	332	6	20	118	401	6	27	198	733
03:15	1		78		6		100		7		178	
03:30	1		94		4		73		5		167	
03:45	5		80		4		110		9		190	
04:00	3	9	81	392	2	44	92	466	5	53	173	858
04:15	3		74		10		132		13		206	
04:30	2		112		20		122		22		234	
04:45	1		125		12		120		13		245	
05:00	10	28	132	600	16	121	130	569	26	149	262	1,169
05:15	2		164		23		149		25		313	
05:30	6		162		48		162		54		324	
05:45	10		142		34		128		44		270	
06:00	10	90	128	404	40	299	136	457	50	389	264	861
06:15	18		104		47		113		65		217	
06:30	26		92		80		96		106		188	
06:45	36		80		132		112		168		192	
07:00	33	267	86	232	165	971	108	340	198	1,238	194	572
07:15	68		58		230		94		298		152	
07:30	82		40		284		80		366		120	
07:45	84		48		292		58		376		106	
08:00	132	285	34	129	178	531	81	251	310	816	115	380
08:15	74		32		138		68		212		100	
08:30	46		33		136		54		182		87	
08:45	33		30		79		48		112		78	
09:00	35	123	20	72	87	347	48	213	122	470	68	285
09:15	30		25		92		61		122		86	
09:30	32		13		78		58		110		71	
09:45	26		14		90		46		116		60	
10:00	28	142	8	29	58	262	47	140	86	404	55	169
10:15	52		10		66		50		118		60	
10:30	31		3		68		25		99		28	
10:45	31		8		70		18		101		26	
11:00	38	151	10	33	82	303	19	71	120	454	29	104
11:15	30		10		58		16		88		26	
11:30	46		7		88		30		134		37	
11:45	37		6		75		6		112		12	
Totals	1,122		2,904		2,952		4,067		4,074		6,971	
Split%	27.5		41.7		72.5		58.3					
Day Totals		4,026				7,019				11,045		
Day Splits		36.5				63.5						
Peak Hour	07:30		05:00		07:15		05:15		07:15		05:15	
Volume	372		600		984		575		1,350		1,171	
Factor	0.70		0.91		0.84		0.89		0.90		0.90	

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : DYER ROAD — Site: SANTA ANA

Segment : PULLMAN ST TO RED HILL AVE — Date: 09/24/19

Client : CITY SANTA ANA — Day: Tuesday

Interval	EB				WB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	20	52	214	769	38	198	228	930	58	250	442	1,699
12:15	10		190		38		234		48		424	
12:30	14		183		84		230		98		413	
12:45	8		182		38		238		46		420	
01:00	13	32	204	728	24	73	246	1,032	37	105	450	1,760
01:15	11		162		28		250		39		412	
01:30	2		186		13		285		15		471	
01:45	6		176		8		251		14		427	
02:00	12	66	186	687	16	63	297	1,179	28	129	483	1,866
02:15	7		155		17		278		24		433	
02:30	17		170		18		335		35		505	
02:45	30		176		12		269		42		445	
03:00	18	187	158	741	16	60	286	1,286	34	247	444	2,027
03:15	31		191		20		332		51		523	
03:30	56		192		11		323		67		515	
03:45	82		200		13		345		95		545	
04:00	56	480	183	852	26	115	370	1,350	82	595	553	2,202
04:15	85		206		24		316		109		522	
04:30	137		227		40		342		177		569	
04:45	202		236		25		322		227		558	
05:00	175	904	243	1,101	32	209	389	1,412	207	1,113	632	2,513
05:15	205		276		36		358		241		634	
05:30	228		294		58		308		286		602	
05:45	296		288		83		357		379		645	
06:00	214	900	230	844	88	460	322	1,197	302	1,360	552	2,041
06:15	241		215		134		329		375		544	
06:30	231		234		102		313		333		547	
06:45	214		165		136		233		350		398	
07:00	220	991	146	486	172	777	300	986	392	1,768	446	1,472
07:15	251		106		176		266		427		372	
07:30	264		120		186		232		450		352	
07:45	256		114		243		188		499		302	
08:00	228	950	124	380	245	1,001	214	748	473	1,951	338	1,128
08:15	222		90		253		168		475		258	
08:30	238		74		267		170		505		244	
08:45	262		92		236		196		498		288	
09:00	218	868	82	293	224	785	194	654	442	1,653	276	947
09:15	233		84		187		158		420		242	
09:30	211		60		195		172		406		232	
09:45	206		67		179		130		385		197	
10:00	170	677	42	195	193	739	150	488	363	1,416	192	683
10:15	168		54		191		122		359		176	
10:30	181		57		175		122		356		179	
10:45	158		42		180		94		338		136	
11:00	208	870	22	83	188	776	98	285	396	1,646	120	368
11:15	219		19		201		66		420		85	
11:30	199		25		184		65		383		90	
11:45	244		17		203		56		447		73	
Totals	6,977		7,159		5,256		11,547		12,233		18,706	
Split%	57.0		38.3		43.0		61.7					
Day Totals		14,136				16,803				30,939		
Day Splits		45.7				54.3						
Peak Hour	07:15		05:00		07:45		05:00		07:45		05:00	
Volume	999		1,101		1,008		1,412		1,952		2,513	
Factor	0.95		0.94		0.94		0.91		0.97		0.97	

Data File : D1909047

2040

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : FAIRHAVEN AVENUE — Site: SANTA ANA

Segment : TUSTIN AVE TO NEWPORT FWY — Date: 10/03/19

Client : CITY SANTA ANA — Day: Thursday

Interval	WB				EB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	18	24	89	408	18	50	79	376	36	74	168	784
12:15	2		128		14		94		16		222	
12:30	3		96		7		101		10		197	
12:45	1		95		11		102		12		197	
01:00	8	14	114	452	10	21	109	426	18	35	223	878
01:15	2		114		5		92		7		206	
01:30	0		106		4		101		4		207	
01:45	4		118		2		124		6		242	
02:00	3	9	142	604	6	19	107	516	9	28	249	1,120
02:15	2		134		10		144		12		278	
02:30	0		134		1		144		1		278	
02:45	4		194		2		121		6		315	
03:00	5	22	186	615	10	21	154	583	15	43	340	1,198
03:15	7		146		3		158		10		304	
03:30	3		133		4		147		7		280	
03:45	7		150		4		124		11		274	
04:00	9	62	159	690	5	20	131	607	14	82	290	1,297
04:15	16		203		3		160		19		363	
04:30	20		162		7		166		27		328	
04:45	17		166		5		150		22		316	
05:00	44	178	180	833	11	60	147	664	55	238	327	1,497
05:15	38		238		14		188		52		426	
05:30	48		231		12		139		60		370	
05:45	48		184		23		190		71		374	
06:00	58	437	183	651	24	217	138	463	82	654	321	1,114
06:15	107		170		49		118		156		288	
06:30	126		158		68		90		194		248	
06:45	146		140		76		117		222		257	
07:00	178	994	94	338	98	537	101	400	276	1,531	195	738
07:15	202		78		123		112		325		190	
07:30	284		84		162		89		446		173	
07:45	330		82		154		98		484		180	
08:00	300	900	54	211	112	387	73	272	412	1,287	127	483
08:15	230		51		102		68		332		119	
08:30	194		56		93		66		287		122	
08:45	176		50		80		65		256		115	
09:00	139	506	42	142	76	292	80	250	215	798	122	392
09:15	109		34		64		58		173		92	
09:30	132		35		64		56		196		91	
09:45	126		31		88		56		214		87	
10:00	110	433	42	110	80	298	74	207	190	731	116	317
10:15	104		24		74		47		178		71	
10:30	97		22		70		44		167		66	
10:45	122		22		74		42		196		64	
11:00	114	411	12	49	74	324	35	116	188	735	47	165
11:15	93		16		76		26		169		42	
11:30	102		11		76		32		178		43	
11:45	102		10		98		23		200		33	
Totals	3,990		5,103		2,246		4,880		6,236		9,983	
Split%	64.0		51.1		36.0		48.9					
Day Totals		9,093				7,126				16,219		
Day Splits		56.1				43.9						
Peak Hour	07:30		05:15		07:15		05:00		07:30		05:00	
Volume	1,144		836		551		664		1,674		1,497	
Factor	0.87		0.88		0.85		0.87		0.86		0.88	

Tuesday, September 24, 2019 CITY: Santa Ana PROJECT: SC

ADT219 SANTA CLARA AVE from TUSTIN AVE to NEWPORT FWY. **Prepared by AimTD tel. 714 253 7888**

AM Period	NB	SB	EB		WB			PM Period	NB	SB	EB		WB		
0:00			10		7			12:00			68		47		
0:15			7		6			12:15			50		33		
0:30			3		0			12:30			72		51		
0:45			0	20	0	13	33	12:45			80	270	67	198	468
1:00			2		2			13:00			80		45		
1:15			4		0			13:15			69		66		
1:30			2		2			13:30			59		53		
1:45			2	10	4	8	18	13:45			52	260	87	251	511
2:00			3		3			14:00			58		64		
2:15			3		2			14:15			61		55		
2:30			0		2			14:30			110		72		
2:45			2	8	0	7	15	14:45			85	314	99	290	604
3:00			0		0			15:00			88		94		
3:15			0		2			15:15			85		76		
3:30			0		4			15:30			84		87		
3:45			0	0	4	10	10	15:45			58	315	77	334	649
4:00			0		2			16:00			82		64		
4:15			6		2			16:15			85		79		
4:30			0		5			16:30			88		96		
4:45			0	6	2	11	17	16:45			94	349	85	324	673
5:00			8		7			17:00			90		106		
5:15			9		13			17:15			111		118		
5:30			8		14			17:30			106		106		
5:45			9	34	15	49	83	17:45			87	394	108	438	832
6:00			11		13			18:00			94		81		
6:15			46		30			18:15			86		88		
6:30			55		43			18:30			72		80		
6:45			47	159	55	141	300	18:45			58	310	56	305	615
7:00			63		50			19:00			55		53		
7:15			151		76			19:15			48		46		
7:30			137		106			19:30			53		34		
7:45			130	481	116	348	829	19:45			46	202	41	174	376
8:00			106		122			20:00			51		41		
8:15			57		100			20:15			45		35		
8:30			52		72			20:30			45		31		
8:45			72	287	54	348	635	20:45			25	166	21	128	294
9:00			63		56			21:00			30		20		
9:15			67		53			21:15			27		25		
9:30			45		45			21:30			21		11		
9:45			45	220	47	201	421	21:45			27	105	18	74	179
10:00			35		44			22:00			26		22		
10:15			54		37			22:15			26		12		
10:30			46		35			22:30			18		11		
10:45			50	185	44	160	345	22:45			16	86	9	54	140
11:00			51		39			23:00			14		12		
11:15			49		41			23:15			10		5		
11:30			49		38			23:30			10		7		
11:45			58	207	38	156	363	23:45			10	44	5	29	73
Total Vol.			1617		1452		**3069**				2815		2599		**5414**

Daily Totals

NB	SB	EB	WB	Combined
		4432	4051	**8483**

	AM			PM		
Split %	52.7%	47.3%	**36.2%**	52.0%	48.0%	**63.8%**
Peak Hour	7:15	7:30	**7:15**	16:45	17:00	**17:00**
Volume	524	444	**944**	401	438	**832**
P.H.F.	0.87	0.91	**0.96**	0.90	0.93	**0.91**

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : HARBOR BOULEVARD — Site: SANTA ANA

Segment : EDINGER AVE TO MCFADDEN AVE — Date: 10/03/19

Client : CITY SANTA ANA — Day: Thursday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	78	242	302	1,224	47	169	327	1,319	125	411	629	2,543
12:15	70		304		42		322		112		626	
12:30	46		316		45		312		91		628	
12:45	48		302		35		358		83		660	
01:00	72	160	328	1,382	27	98	286	1,272	99	258	614	2,654
01:15	36		326		25		308		61		634	
01:30	34		344		18		337		52		681	
01:45	18		384		28		341		46		725	
02:00	51	134	384	1,766	19	93	334	1,445	70	227	718	3,211
02:15	26		439		18		363		44		802	
02:30	29		426		34		350		63		776	
02:45	28		517		22		398		50		915	
03:00	35	113	508	2,114	28	234	295	1,197	63	347	803	3,311
03:15	27		548		42		311		69		859	
03:30	26		516		56		298		82		814	
03:45	25		542		108		293		133		835	
04:00	50	162	532	2,086	47	467	282	1,184	97	629	814	3,270
04:15	26		526		86		300		112		826	
04:30	42		520		138		296		180		816	
04:45	44		508		196		306		240		814	
05:00	60	312	541	2,221	168	973	332	1,307	228	1,285	873	3,528
05:15	73		561		198		324		271		885	
05:30	80		555		269		355		349		910	
05:45	99		564		338		296		437		860	
06:00	122	567	518	1,962	273	1,658	298	1,133	395	2,225	816	3,095
06:15	118		556		319		283		437		839	
06:30	154		478		480		268		634		746	
06:45	173		410		586		284		759		694	
07:00	200	1,001	402	1,431	572	2,472	268	1,013	772	3,473	670	2,444
07:15	224		394		612		262		836		656	
07:30	272		330		640		259		912		589	
07:45	305		305		648		224		953		529	
08:00	348	1,146	264	1,026	586	2,128	220	817	934	3,274	484	1,843
08:15	282		268		522		209		804		477	
08:30	248		272		520		222		768		494	
08:45	268		222		500		166		768		388	
09:00	228	945	197	777	444	1,714	184	626	672	2,659	381	1,403
09:15	216		204		427		156		643		360	
09:30	265		200		406		150		671		350	
09:45	236		176		437		136		673		312	
10:00	262	1,075	194	654	332	1,362	153	435	594	2,437	347	1,089
10:15	269		170		320		105		589		275	
10:30	276		166		352		101		628		267	
10:45	268		124		358		76		626		200	
11:00	290	1,193	107	455	306	1,189	98	340	596	2,382	205	795
11:15	303		94		253		80		556		174	
11:30	306		139		340		78		646		217	
11:45	294		115		290		84		584		199	
Totals	7,050		17,098		12,557		12,088		19,607		29,186	
Split%	36.0		58.6		64.0		41.4					
Day Totals		24,148				24,645				48,793		
Day Splits		49.5				50.5						
Peak Hour	07:30		05:00		07:15		02:00		07:15		05:00	
Volume	1,207		2,221		2,486		1,445		3,635		3,528	
Factor	0.87		0.98		0.96		0.91		0.95		0.97	

Transportation Studies, Inc.

2640 Walnut Avenue, Suite L

Tustin, CA. 92780

Location : HARBOR BOULEVARD — Site: SANTA ANA

Segment : WARNER TO SANTA ANA RIVER — Date: 10/09/19

Client : CITY SANTA ANA — Day: Wednesday

Interval	NB				SB				Combined			
Begin	AM		PM		AM		PM		AM		PM	
12:00	74	265	359	1,359	42	128	330	1,343	116	393	689	2,702
12:15	58		330		30		332		88		662	
12:30	63		318		32		357		95		675	
12:45	70		352		24		324		94		676	
01:00	68	204	328	1,473	14	71	333	1,305	82	275	661	2,778
01:15	49		350		12		328		61		678	
01:30	50		390		24		336		74		726	
01:45	37		405		21		308		58		713	
02:00	52	156	494	1,931	30	115	400	1,374	82	271	894	3,305
02:15	34		428		24		324		58		752	
02:30	49		496		30		341		79		837	
02:45	21		513		31		309		52		822	
03:00	37	110	604	2,356	24	229	291	1,175	61	339	895	3,531
03:15	16		533		47		288		63		821	
03:30	29		631		50		300		79		931	
03:45	28		588		108		296		136		884	
04:00	43	149	626	2,489	66	569	237	995	109	718	863	3,484
04:15	24		604		91		237		115		841	
04:30	36		619		164		258		200		877	
04:45	46		640		248		263		294		903	
05:00	48	270	672	2,625	184	1,200	290	1,012	232	1,470	962	3,637
05:15	78		655		241		250		319		905	
05:30	66		624		339		234		405		858	
05:45	78		674		436		238		514		912	
06:00	90	456	584	2,265	338	1,964	236	960	428	2,420	820	3,225
06:15	90		628		420		258		510		886	
06:30	140		549		456		232		596		781	
06:45	136		504		750		234		886		738	
07:00	202	1,014	406	1,437	628	3,573	190	712	830	4,587	596	2,149
07:15	204		384		1,000		191		1,204		575	
07:30	272		340		974		176		1,246		516	
07:45	336		307		971		155		1,307		462	
08:00	274	970	246	925	1,004	3,226	144	598	1,278	4,196	390	1,523
08:15	220		231		703		158		923		389	
08:30	237		218		714		170		951		388	
08:45	239		230		805		126		1,044		356	
09:00	228	928	201	797	759	2,203	112	515	987	3,131	313	1,312
09:15	231		207		562		170		793		377	
09:30	243		187		430		132		673		319	
09:45	226		202		452		101		678		303	
10:00	255	998	238	729	370	1,366	96	300	625	2,364	334	1,029
10:15	228		174		332		66		560		240	
10:30	262		159		314		80		576		239	
10:45	253		158		350		58		603		216	
11:00	261	1,131	148	497	332	1,171	54	207	593	2,302	202	704
11:15	308		142		284		60		592		202	
11:30	260		128		277		46		537		174	
11:45	302		79		278		47		580		126	
Totals	6,651		18,883		15,815		10,496		22,466		29,379	
Split%	29.6		64.3		70.4		35.7					
Day Totals		25,534				26,311				51,845		
Day Splits		49.3				50.7						
Peak Hour	11:00		05:00		07:15		02:00		07:15		04:30	
Volume	1,131		2,625		3,949		1,374		5,035		3,647	
Factor	0.92		0.97		0.98		0.86		0.96		0.95	

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Technical Memorandum

To/Attention	City of Santa Ana	**Date**	October 22, 2020
From	IBI Group	**Project No**	119772
cc			
Subject	Addendum to Santa Ana General Plan Traffic Impact Study		

Overview

This memo serves as an addendum to the Traffic Impact Study (October 2020) prepared in support of the Santa Ana General Plan Update Environmental Impact Report (EIR). Presented in this memo is the analysis for ten (10) additional study intersections identified by the City of Tustin and Orange County Transportation Authority (OCTA) as locations where impacts may occur due to the project. The study intersections are listed below are shown in Exhibit A attached to this memo.

106. Red Hill Avenue and El Camino Real (City of Tustin)
107. Red Hill Avenue and I-5 NB Ramps (City of Tustin)
108. Red Hill Avenue and I-5 SB Ramps (City of Tustin)
109. Red Hill Avenue and Nisson Road (City of Tustin)
110. Red Hill Avenue and Walnut Avenue (City of Tustin)
111. Red Hill Avenue and Valencia Avenue (City of Tustin)
112. Tustin Ranch Road and Warner Avenue North (City of Tustin)
113. Tustin Ranch Road and Walnut Avenue (City of Tustin)
114. SR-55 SB Ramps and Irvine Boulevard (Orange County CMP)
115. SR-55 NB Ramps and Irvine Boulevard (Orange County CMP)

This analysis follows the methodology outlined in the Traffic Impact Study (October 2020). Intersection level of service (LOS) analyses were conducted for the Existing Year (2020) No Project, Future Year (2045) No Project, and Future Year (2045) With Project scenarios. Significant impacts were assessed for the year 2045 scenarios using the impact criteria of the presiding agency.

Existing Year (2020) No Project

A summary of the AM and PM peak hour level of service analysis results for the Existing Year (2020) No Project condition is provided in Table 1. Intersections are considered unacceptable if the level of service (LOS) is “E” or “F”. It should be noted that Caltrans and OC CMP intersections have a minimum LOS of C and E, respectively. The following intersections are forecast to operate at an unacceptable level of service in the Existing Year (2020) No Project condition:

- INT 113 – Tustin Ranch Road and Walnut Avenue (City of Tustin)

Table 1: Existing Year (2020) No Project Intersection LOS Summary

ID	INTERSECTION	AM		PM	
		V/C or Delay	LOS	V/C or Delay	LOS
106	Red Hill Avenue and El Camino Real	0.71	C	0.62	B
107	Red Hill Avenue and I-5 NB Ramps	20.3	C	23.3	C
108	Red Hill Avenue and I-5 SB Ramps	22.0	C	22.8	C
109	Red Hill Avenue and Nisson Road	0.65	B	0.70	B
110	Red Hill Avenue and Walnut Avenue	0.68	B	0.79	C
111	Red Hill Avenue and Valencia Avenue	0.54	A	0.55	A
112	Tustin Ranch Road and Warner Avenue North	0.43	A	0.76	C
113	Tustin Ranch Road and Walnut Avenue	1.19	F	1.63	F
114	SR-55 SB Ramps and Irvine Boulevard	0.97	E	0.79	C
115	SR-55 NB Ramps and Irvine Boulevard	0.80	C	0.81	D

Notes: (1) LOS – Level of Service; (2) V/C – Volume to Capacity

Future Year (2045) No Project

A summary of the AM and PM peak hour level of service analysis results for the Future Year (2045) No Project condition is provided in Table 2. Intersections are considered unacceptable if the level of service (LOS) is “E” or “F”. It should be noted that Caltrans and OC CMP intersections have a minimum LOS of C and E, respectively. The following intersections are forecast to operate at an unacceptable level of service in the Future Year (2045) No Project condition:

- INT 113 – Tustin Ranch Road and Walnut Avenue (City of Tustin)
- INT 114 – SR-55 SB Ramps and Irvine Boulevard (Caltrans - CMP)

Table 2: Future Year (2045) No Project Intersection LOS Summary

ID	INTERSECTION	AM		PM	
		V/C or Delay	LOS	V/C or Delay	LOS
106	Red Hill Avenue and El Camino Real	0.65	B	0.78	C
107	Red Hill Avenue and I-5 NB Ramps	20.8	C	23.7	C
108	Red Hill Avenue and I-5 SB Ramps	23.7	C	22.2	C
109	Red Hill Avenue and Nisson Road	0.65	B	0.69	B
110	Red Hill Avenue and Walnut Avenue	0.64	B	0.74	C
111	Red Hill Avenue and Valencia Avenue	0.65	B	0.74	C
112	Tustin Ranch Road and Warner Avenue North	0.45	A	0.89	D
113	Tustin Ranch Road and Walnut Avenue	1.27	F	1.72	F
114	SR-55 SB Ramps and Irvine Boulevard	1.10	F	0.92	E

ID	INTERSECTION	AM		PM	
		V/C or Delay	LOS	V/C or Delay	LOS
115	SR-55 NB Ramps and Irvine Boulevard	0.91	E	0.93	E

Notes: (1) LOS – Level of Service; (2) V/C – Volume to Capacity

Future Year (2045) With Project

A summary of the AM and PM peak hour level of service analysis results for the Future Year (2045) With Project condition is provided in Table 3. Intersections are considered unacceptable if the level of service (LOS) is "E" or "F". It should be noted that Caltrans and OC CMP intersections have a minimum LOS of C and E, respectively. The following intersections are forecast to operate at an unacceptable level of service in the Future Year (2045) With Project condition:

- INT 112 – Tustin Ranch Road and Warner Avenue North
- INT 113 – Tustin Ranch Road and Walnut Avenue
- INT 114 – SR-55 SB Ramps and Irvine Boulevard (Caltrans - CMP)

Table 3: Future Year (2045) With Project Intersection LOS Summary

ID	INTERSECTION	AM		PM	
		V/C or Delay	LOS	V/C or Delay	LOS
106	Red Hill Avenue and El Camino Real	0.64	B	0.70	B
107	Red Hill Avenue and I-5 NB Ramps	20.1	C	23.6	C
108	Red Hill Avenue and I-5 SB Ramps	24.7	C	21.8	C
109	Red Hill Avenue and Nisson Road	0.66	B	0.72	C
110	Red Hill Avenue and Walnut Avenue	0.64	B	0.75	C
111	Red Hill Avenue and Valencia Avenue	0.68	B	0.75	C
112	Tustin Ranch Road and Warner Avenue North	0.46	A	0.92	E
113	Tustin Ranch Road and Walnut Avenue	1.31	F	1.77	F
114	SR-55 SB Ramps and Irvine Boulevard	1.18	F	0.93	E
115	SR-55 NB Ramps and Irvine Boulevard	0.82	D	0.84	D

Intersection Impact Assessment

Intersection impacts were determined using the criteria established by the presiding agency and further evaluated in order to determine the cause for the impact. Impacts are expected to occur due to the proposed land use changes, reclassifications, or a combination of both. The impacted intersections and the cause of impact is identified below:

- **INT 112 – Tustin Ranch Road and Warner Avenue North (PM peak hour / Tustin)**
    - The impact at this intersection is expected to occur as a result of the proposed land use changes. Specific improvements to this intersection to address this identified impact are presented in Table 5. Additionally, the City of Santa Ana will contribute to improvements based on the calculated fair-share percentage presented in Table 5.
- **113 – Tustin Ranch Road and Walnut Avenue (Tustin)**
    - The worsening of LOS at this intersection is expected to occur as a result of the proposed land use changes. Specific improvements to this intersection to address this identified impact are presented in Table 5. Additionally, the City of Santa Ana will contribute to improvements based on the calculated fair-share percentage presented in Table 5.

Table 6 summarizes intersection LOS for the future year scenario, with and without the project, as well as with the improvement measures implemented. All impacts are expected to be mitigated.

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Table 4: Future Year (2045) LOS Impact Summary

ID	INTERSECTION	PEAK HOUR	2045 No Project		2045 With Project		DELTA	LOS IMPACT?
			V/C or Delay	LOS	V/C or Delay	LOS		
106	Red Hill Avenue and El Camino Real	AM	0.65	B	0.64	B	-0.01	NO
		PM	0.78	C	0.70	B	-0.08	NO
107	Red Hill Avenue and I-5 NB Ramps	AM	20.8	C	20.1	C	-0.7	NO
		PM	23.7	C	23.6	C	-0.1	NO
108	Red Hill Avenue and I-5 SB Ramps	AM	23.7	C	24.7	C	1.0	NO
		PM	22.2	C	21.8	C	-0.4	NO
109	Red Hill Avenue and Nisson Road	AM	0.65	B	0.66	B	0.01	NO
		PM	0.69	B	0.72	C	0.03	NO
110	Red Hill Avenue and Walnut Avenue	AM	0.64	B	0.64	B	0.00	NO
		PM	0.74	C	0.75	C	0.01	NO
111	Red Hill Avenue and Valencia Avenue	AM	0.65	B	0.68	B	0.03	NO
		PM	0.74	C	0.75	C	0.01	NO
112	Tustin Ranch Road and Warner Avenue North	AM	0.45	A	0.46	A	0.01	NO
		PM	0.89	D	0.92	E	0.03	YES
113	Tustin Ranch Road and Walnut Avenue	AM	1.27	F	1.31	F	0.04	NO
		PM	1.72	F	1.77	F	0.05	NO
114	SR-55 SB Ramps and Irvine Boulevard	AM	1.10	F	1.18	F	0.08	NO
		PM	0.92	E	0.93	E	0.01	NO
115	SR-55 NB Ramps and Irvine Boulevard	AM	0.91	E	0.82	D	-0.09	NO
		PM	0.93	E	0.84	D	-0.09	NO

Table 5: Intersection LOS Improvement Measures

ID	INTERSECTION	IMPROVEMENT
112	Tustin Ranch Road and Warner Avenue North	• Convert single NB-RT to dual right-turn with overlap control. • Fair-Share: 29.1% (PM)
113	Tustin Ranch Road and Walnut Avenue	• Modify WB-RT to be a channelized RT. • Modify EB approach to be: 2LT, 2T, and 1RT. • Fair-Share: 84.5% (AM) / 24.0% (PM)

Table 6: Future Year (2045) With Project – Mitigated Impact Summary

ID	INTERSECTION	PEAK HOUR	2045 No Project		2045 With Project		DELTA	IMPACT?	2045 With Project – Mit.		DELTA	IMPACT?
			V/C or Delay	LOS	V/C or Delay	LOS			V/C or Delay	LOS		
112	Tustin Ranch Road and Warner Avenue North	AM	0.45	A	0.46	A	0.01	NO	0.46	A	0.01	NO
		PM	0.89	D	0.92	E	0.03	YES	0.75	C	-0.14	NO
113	Tustin Ranch Road and Walnut Avenue	AM	1.27	F	1.31	F	0.04	YES	1.18	F	-0.09	NO
		PM	1.72	F	1.77	F	0.05	YES	1.59	F	-0.13	NO

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Conclusion

Ten (10) additional intersections were assessed in order to determine project-related impacts. In assessing the intersection LOS it was determined that an impact is expected to occur at one location. The impacted intersection is listed below:

- **INT 112 – Tustin Ranch Road and Warner Avenue North (PM / City of Tustin)**
    - An improvement measure was developed and proposed as part of this study; implementation of the measure is expected to improve intersection LOS and fully mitigate the impact. The fair-share contribution, based on the project's contribution to increased traffic volumes, was calculated to be 29.1% for the PM peak hour.
- **INT 113 – Tustin Ranch Road and Walnut Avenue (AM & PM / City of Tustin)**
    - An improvement measure was developed and proposed as part of this study; implementation of the measure is expected to improve intersection LOS and fully mitigate the impact. The fair-share contribution, based on the project's contribution to increased traffic volumes, was calculated to be 84.5% and 24.0% for the Am and PM peak hours, respectively.

Exhibit A - Study Intersection Locations



City of Santa Ana
20 Civic Center Plaza, Santa Ana, CA 92701
Staff Report
April 19, 2022

TOPIC: EIR No. 2020-03 and GPA No.2020-06 Santa Ana General Plan Update

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AGENDA TITLE

Public Hearing - Final Recirculated Program Environmental Impact Report No. 2020-03 and General Plan Amendment No. 2020-06 for Santa Ana General Plan Update

RECOMMENDED ACTION

1. Adopt a resolution certifying Final Recirculated Program Environmental Impact Report No. 2020-03 (SCH No. 2020029087), including adoption of environmental findings of fact pursuant to the California Environmental Quality Act, adoption of a Statement of Overriding Considerations, and adoption of a Mitigation Monitoring and Reporting Program.

2. Adopt a resolution approving General Plan Amendment No. 2020-06.

3. Adopt a resolution of findings to overrule the John Wayne Airport Orange County Airport Land Use Commission's determination that the Proposed General Plan Update is inconsistent with the Airport Environs Land Use Plan.

BACKGROUND

On February 15, 2022, the City Council considered the adoption of the City of Santa Ana General Plan Update. While the City Council did not reach a final decision during the meeting, the City Council discussed and commented on the draft document as well as the proposed policies and implementation actions. The City Council also encouraged the public to submit written comments to the City. In summary, staff documented over 150 comments received from local environmental justice (EJ) stakeholders and from the various City Councilmembers regarding the draft General Plan Update after the February 15, 2022 City Council meeting. Exhibit 1 is a catalog of the documented comments with a City response to each.

After reviewing and responding to the comments, staff met on several occasions with the EJ stakeholders (MPNA, OCEJ, Rise Up Willowick, THRIVE Santa Ana, UCI) to provide the opportunity for follow-up clarifications and where appropriate, discussed further refinements of the draft policies and implementation actions related to the various environmental justice topics. At the conclusion of the most recent meeting held with the

group on April 4, 2022, the stakeholders expressed their support for the General Plan. Staff also followed up with each Councilmember to obtain final input on the draft Plan.

As a result of the additional input, over 60 modifications and refinements responsive to the comments received from the City Council and from the community are being recommended for incorporation into the draft General Plan Update. The recommended changes are as shown in Exhibit 2. Inclusive in the recommended changes are several clarifications and refinements put forth by staff. If the changes are approved and adopted as recommended, they will be incorporated into the April 2022 draft of the General Plan Update (Exhibit 3) to form the adopted City of Santa General Plan – Golden City and Beyond.

PUBLIC NOTIFICATION AND COMMUNITY OUTREACH

Project notifications were posted, published, and mailed in accordance with City and State regulations. General Plan interested parties were emailed and/ or mailed a notice of this public hearing to consider the General Plan Update and Program Environmental Impact Report (GPEIR). In addition, courtesy notices were mailed to property owners and occupants located within the five Land Use Focus Areas and within 500 feet of their boundaries.

EXHIBIT(S)

1. Catalog of Comments Received Post February 15, 2022
2. Recommended Clarifications to the Draft April 2022 General Plan Update
3. Draft April 2022 General Plan Updated - Web Link: https://www.santa-ana.org/general-plan/draft-documents
4. February 15, 2022 City Council Staff Report - Web Link: Item No. 26 - PrimGov Portal
5. EIR Resolution, including Findings of Fact and Statement of Overriding Considerations and Mitigation Monitoring and Reporting Program (MMRP)
6. General Plan Amendment Resolution
7. Orange County Airport Land Use Commission Override Resolution
8. Final Recirculated Environmental Impact Report - Web Link: https://www.santa-ana.org/general-plan/general-plan-environmental-documents
9. November 8, 2021 Planning Commission Report - Web Link: Item No. 3 – PrimeGov Portal
10. List of Parcel with General Plan Land Use Designation Changes

Submitted By: Minh Thai, Executive Director, Planning and Building Agency

Approved By: Kristine Ridge, City Manager



Mobility Element

Santa Ana General Plan

FINAL, APRIL 2022

2055

A SHARED VISION

Santa Ana is a city that promotes the health and wellness of all residents, with a civic culture that actively embraces the power of diversity. Our city invests in resources that create economic opportunities for the next generation, and it is a community that celebrates our past while working together to create a sustainable future.

OUR CORE VALUES

H HEALTH

The people of Santa Ana value a physical environment that encourages healthy lifestyles, a planning process that ensures that health impacts are considered, and a community that actively pursues policies and practices that improve the health of our residents.

Eq Eq EQUITY

Our residents value taking all necessary steps to ensure equitable outcomes, expanding access to the tools and resources that residents need, and to balance competing interests in an open and democratic manner.

The value of equity that includes "EJ" in the upper corner indicates a policy related to environmental justice.

S SUSTAINABILITY

Santa Ana values land use decisions that benefit future generations, plans for the impacts of climate change, and incorporates sustainable design practices at all levels of the planning process.

C CULTURE

Our community values efforts that celebrate our differences as a source of strength, preserve and build upon existing cultural resources, and nurture a citywide culture of empowered residents.

Ed EDUCATION

We are a city that values the creation of lifelong learners, the importance of opening up educational opportunities to all residents and investing in educational programs that advance our residents' economic wellbeing.

MOBILITY VISION

Santa Ana envisions a balanced multimodal transportation system that supports community values. These include a vibrant local economy, healthy neighborhoods, health and wellness, and an attractive environment. Our transportation system connects major activity centers with regional labor, education, and goods markets. Context-sensitive design solutions strengthen the livability, vitality, and safety of our neighborhoods, districts, and corridors. Safe and pleasant bicycling and walking options encourage active lifestyles. While building a 21st century transportation system, the City employs best practices to protect our environment, natural resources, health, and safety.



▲ **OC Streetcar Visual Simulation [cover photo]**

OC Streetcar is the first modern streetcar project to be built in Orange County and will provide improved transit access to important employment, restaurant, and retail centers in Santa Ana, Garden Grove, and surrounding areas. The Streetcar will also provide a convenient "last mile" connection between Metrolink trains and other transportation modes at the Santa Ana Regional Transit Center.

Visual simulation by OCTA

MOBILITY
ELEMENT

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RELATED ITEMS UNDER SEPARATE COVER

TRAFFIC IMPACT STUDY, OCTOBER 2020 (APPENDIX K, R-DEIR)

EIR CHAPTER 5.16, TRANSPORTATION



SOMOS - Sunday on Main Open Streets. In 2014, the City of Santa Ana hosted Orange County's first ever ciclovia, an open streets event where the streets were closed to motorized vehicles and open for people to walk and bicycle. The route connected Santa Ana's vibrant downtown to its historic South Main Corridor, with plenty of family-friendly activities, games, and information booths along the three mile route.



MOBILITY

This Mobility Element is the City's blueprint for moving people, goods, and resources throughout the community. Moving beyond mere functionality, the City seeks to improve the quality of life in Santa Ana by providing more complete streets, offering ways to be more active, and conserving our natural resources. In planning the city's transportation system for the 21st century, the City is also making our community safer, more affordable, and more livable.

INTRODUCTION

Santa Ana is a city in motion. Santa Ana's circulation system of over 500 miles of roads keeps this thriving city of over 335,000 people on the move. Yet each of us thinks about our transportation choices in very different ways. For youth, it is about walking or biking to a school, a park, or a friend's home. For adults, it is about getting safely and conveniently around Santa Ana or making sure children are safe in their travels. For commerce, it is about ensuring access to goods, services, and customers to improve business opportunities.

Santa Ana's circulation system has been a vital part of its history, dating back to 1869 when the city was originally founded. At that time, the circulation system consisted primarily of dirt roads that connected agricultural fields and farms to the shops and post office. In the 1880s, the Southern Pacific Railroad and Santa Fe Railroad brought rail transportation to the community, opening it to commerce and housing development. Seventy-five years later, freeways were constructed and later expanded, which provided convenient access for Santa Ana residents and businesses to the greater metropolitan area.

Santa Ana's circulation system continued to evolve, with added commuter rail and transit routes not only offering options for getting around without a car, but also making Santa Ana the transit and rail hub of Orange County. Looking forward, the City will continue to advance mobility through the OC Streetcar and significant investments in bikeways and pedestrian infrastructure.

Other City planning documents related to circulation are the Alternative Transportation, Safe Mobility, Safe Routes to School, and Street Standard plans.



▲ **Crossing the Street in the Lacy Neighborhood**

A crossing guard helps a family safely cross Brown Street from Garfield Elementary School and its new Wellness Center (opened in 2020), which is housed in a historic Pacific Electric substation that once served the Red Car railways system.

COMMUNITY CONSIDERATIONS

Through the community engagement process for this General Plan, participants identified numerous circulation and mobility considerations and values that they believe should form the basis of and be addressed by this element. Specifically, participants highlighted the following topics, areas of concern, and community strengths:

- **Increase the amount of active transportation opportunities**
- **Alleviate parking congestion in commercial areas and neighborhoods**
- **Improve safety for bicyclists and pedestrians**
- **Expand the availability and frequency of transit**
- **Address high traffic speeds and congestion**





▲ **Programs and Activities to Promote Walking and Biking**

To make bicycling and walking safer, more fun, and easy to do, the City teams with transit, health, and other local organizations to organize and promote activities such as open streets events, family-friendly bike rides, walk-to-school days, winter evening walk and ride events (pictured at top), food-focused bike rides, female-focused bike rides (pictured above), and walking tours.



▲ **Community Input**

Santa Ana's engagement for transportation planning has been a progressive effort, starting in 2012 and extending through 2019. Multiple forums were held throughout the city on a variety of topics that began to set the stage for a transformation in how the City, its residents, and its business community think and plan for transportation. Efforts included a wide range of community open houses, workshops (pictured above), youth bike forum, health fairs, General Plan Advisory Group meetings, and many other venues to hear first-hand the concerns regarding the usability, safety, and desirability of the city's transportation system.

POLICY FRAMEWORK

GOAL M-1: Comprehensive Circulation

A comprehensive and multimodal circulation system that facilitates the safe and efficient movement of people, enhances commerce, and promotes a sustainable community.

POLICY M-1.1
SAFETY

Achieve zero fatalities from traffic collisions through education, enforcement, and infrastructure design.

H Eq Ed

POLICY M-1.2
BALANCED MULTIMODAL NETWORK

Provide a balanced and equitable multimodal circulation network that reflects current and changing needs.

H Eq(EJ) S

POLICY M-1.3
TRAFFIC MANAGEMENT SYSTEMS

Utilize technology to efficiently move people and vehicles and manage motor vehicle speeds.

S

POLICY M-1.4
MOTOR VEHICLE LEVEL OF SERVICE

Maintain at least a vehicle level of service "D" for intersections of arterial streets, except in areas planned for high intensity development or traffic safety projects.

H S

POLICY M-1.5
MULTIMODAL LEVEL OF SERVICE

Ensure that new development and City projects maintain or improve the current level of service for all modes of transportation.

H Eq S

POLICY M-1.6
COMPLETE STREETS

Transform travelways to accommodate all users through street design and amenities, such as sidewalks, trees, landscaping, street furniture, and bus shelters.

H Eq S

POLICY M-1.7
PROACTIVE MITIGATION

Proactively mitigate existing and new potential air quality, noise, congestion, safety, and other impacts from the transportation network on residents and business, especially in environmental justice communities.

H Eq(EJ) S

POLICY M-1.8
ENVIRONMENTAL SUSTAINABILITY

Consider air and water quality, noise reduction, neighborhood character, and street-level aesthetics when making improvements to travelways.

H Eq S

POLICY M-1.9
REGIONAL CONSISTENCY

Ensure the street network is consistent with standards set in the OCTA Master Plan of Arterial Highways and the Congestion Management Program.

S

POLICY M-1.10
INTERGOVERNMENTAL COORDINATION

Collaborate with federal, state, SCAG, OCTA, rail authorities, and other agencies to fund and improve the regional transportation system.

S

CONTINUED ON NEXT PAGE >>

POLICY M-1.11
EMERGING SERVICES

Promote the development of innovative and safe travel and delivery services through partnerships with business and industry leaders.

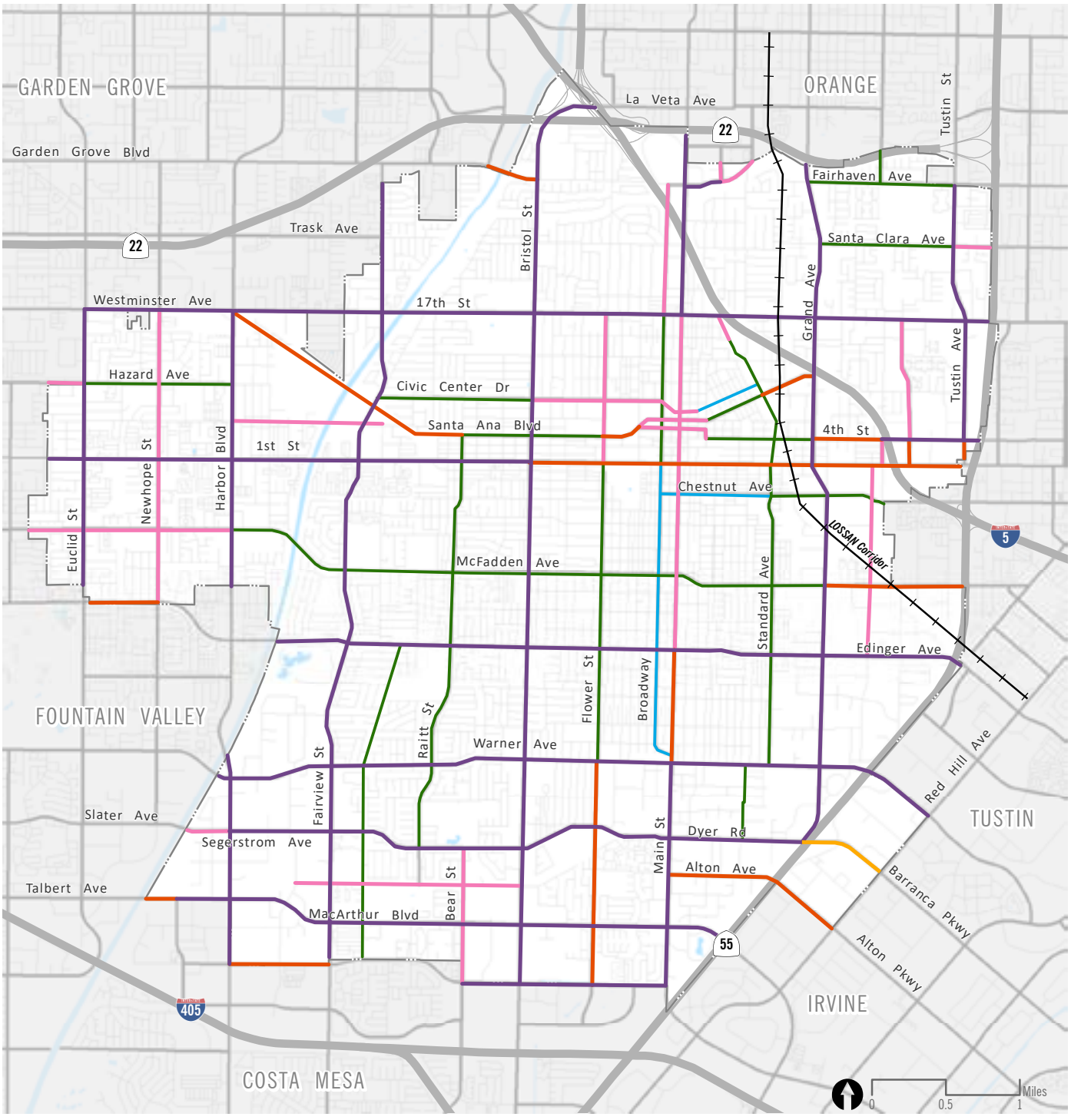
H Eq S



▲ **Complete Streets Improvements along Harbor Boulevard**

After the Harbor Corridor Specific Plan was adopted in 2014, the City began implementing improvements (painting, striping, and spacing for a Class 4 Cycle Track) and applying streetscape requirements to new development (wide sidewalks with street trees and lighting) to make Harbor Boulevard a more complete street and accommodate vehicles, transit users, bicyclists, and pedestrians in a safe, efficient, and attractive manner.

ABOUT THE MAP. This map displays the City's long-term plan for an integrated system of streets and highways, with classifications that correspond to the nature and purpose of each roadway. This map reflects the City's efforts to create and maintain a roadway system that fosters the safe and efficient movement of people and vehicles throughout Santa Ana. See Table M-1 for a description of each street classification. Within city limits, the thin grey lines represent local streets.



Source: OCTA Master Plan of Arterial Highways (06/20/2020) and the City of Santa Ana Date Published: 7/12/2021

- Principal Arterial
- Major Arterial
- Primary Arterial
- Secondary Arterial
- Divided Collector
- Collector

FIGURE M-1
MASTER PLAN OF STREETS AND HIGHWAYS

TABLE M-1. STREET CLASSIFICATIONS

Classification	Description	Examples
Principal Arterial	A street with eight travel lanes and a center median. Typically includes bus transit, pedestrian sidewalks, and bicycle lanes. *Typical ROW: 144' / 126' curb-to-curb / 14' median / 10' sidewalk*	Dyer Road
Major Arterial	A street with six travel lanes and a center median. Typically includes bus transit, pedestrian sidewalks, and bicycle lanes. *Typical ROW: 120' / 100' curb-to-curb / 14' median / 10' sidewalk*	Bristol Street, Harbor Boulevard, and Edinger Avenue
Primary Arterial	A street with four travel lanes and a center median. Typically includes pedestrian sidewalks and may include bus transit services and bicycle lanes. *Typical ROW: 100' / 84' curb-to-curb / 14' median / 10' sidewalk*	4th Street (between Grand Avenue and Interstate 5), and Sunflower Avenue (west of Raitt Street)
Secondary Arterial	A street with four travel lanes and no center median. Typically includes pedestrian sidewalks and may include bus transit and bicycle lanes. Serves more local traffic than a Primary Arterial. *Typical ROW: 80' / 64' curb-to-curb / 8' sidewalk*	Main Street (through Downtown), and Newhope Street
Divided Collector Arterial	A street with two travel lanes and a continuous center two-way left-turn lane, but may be divided by raised median, with an expanded right-of-way to accommodate bike lanes. *Typical ROW: 80' / 64' curb-to-curb / 8' sidewalk*	Raitt Street and Standard Avenue
Collector Street	A street with two travel lanes and no center median, typically includes pedestrian sidewalks, may include shared bicycle routes. *Typical ROW: 56' / 40' curb-to-curb / 10' sidewalk*	Broadway south of 1st Street (north of Civic Center)
Local Street	A street with two travel lanes serving residences and businesses. Typically includes pedestrian sidewalks and on-street parking. May include shared bicycle routes. Local streets are the most common street type. ** Not part of the City's MPSH.*	Lacy Street, Orange Avenue, Raitt Street, Santa Clara Avenue, Wilshire Avenue

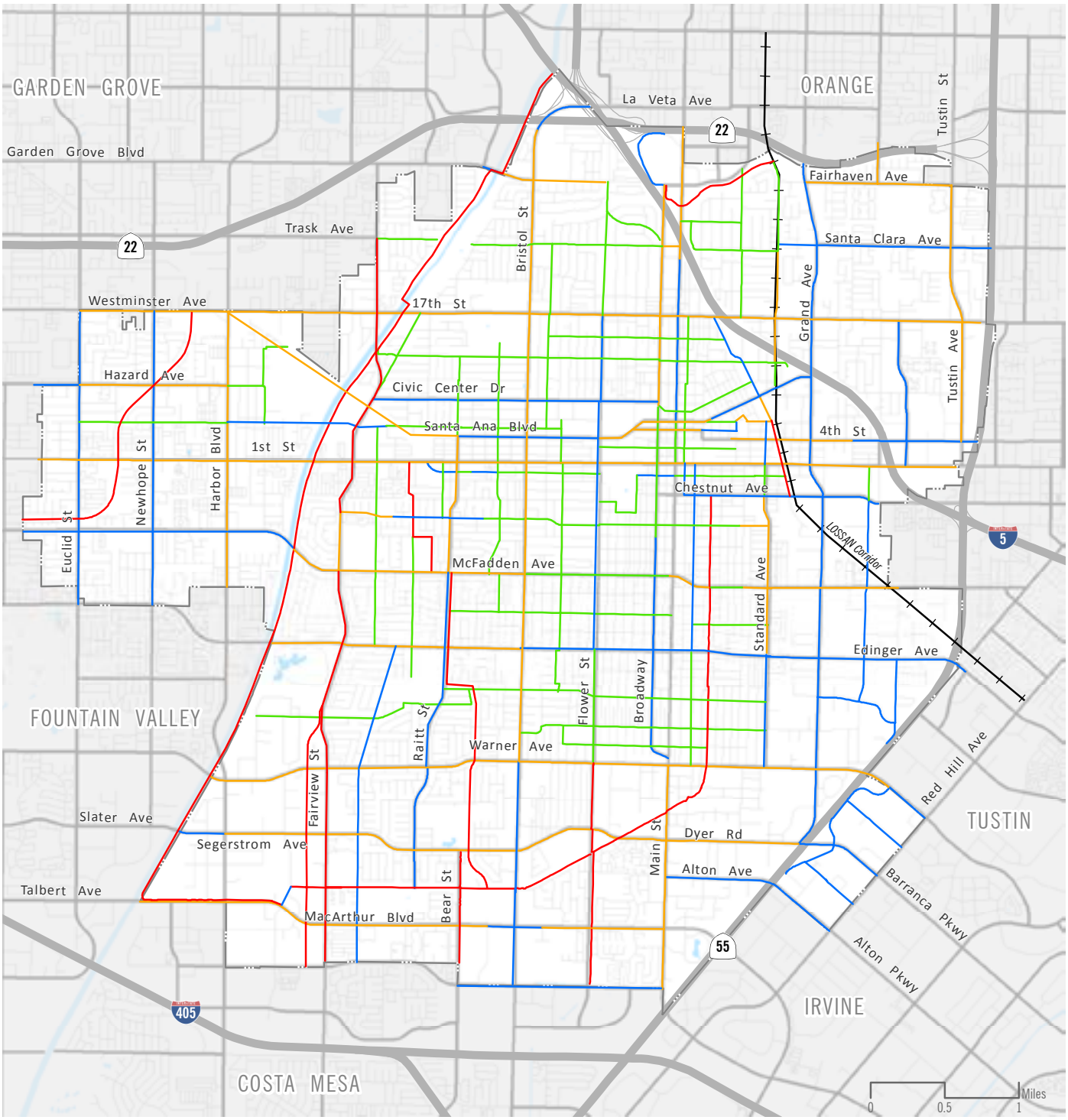


▲ **Local Streets in Wilshire Square**

Shown here is Russell Avenue at the intersection of Van Ness Avenue in the Wilshire Square neighborhood. Both streets offer two travel lanes, on-street parking, sidewalks, and pedestrian-scaled lighting. Russell Avenue offers wide-canopied street trees while Van Ness Avenue is lined with tall palm trees.

Photo courtesy of Lisandro Orozco

ABOUT THE MAP. This map displays the City's long-term plan for a comprehensive bicycle network, with a hierarchy of facilities that provide varying levels of design, signage, and separation from vehicles. The City intends to provide a safe and efficient system that reduces the reliance on the automobiles and facilitates an increase in bicycling by choice. Table M-2 describes each bicycle classification.



Source: City of Santa Ana (2020) Date Published: 11/8/2021

Class I Path

Class II Bike Lane

Class III Bike Route / Boulevard

Class IV Cycle Track

TABLE M-2. BIKEWAY CLASSIFICATIONS

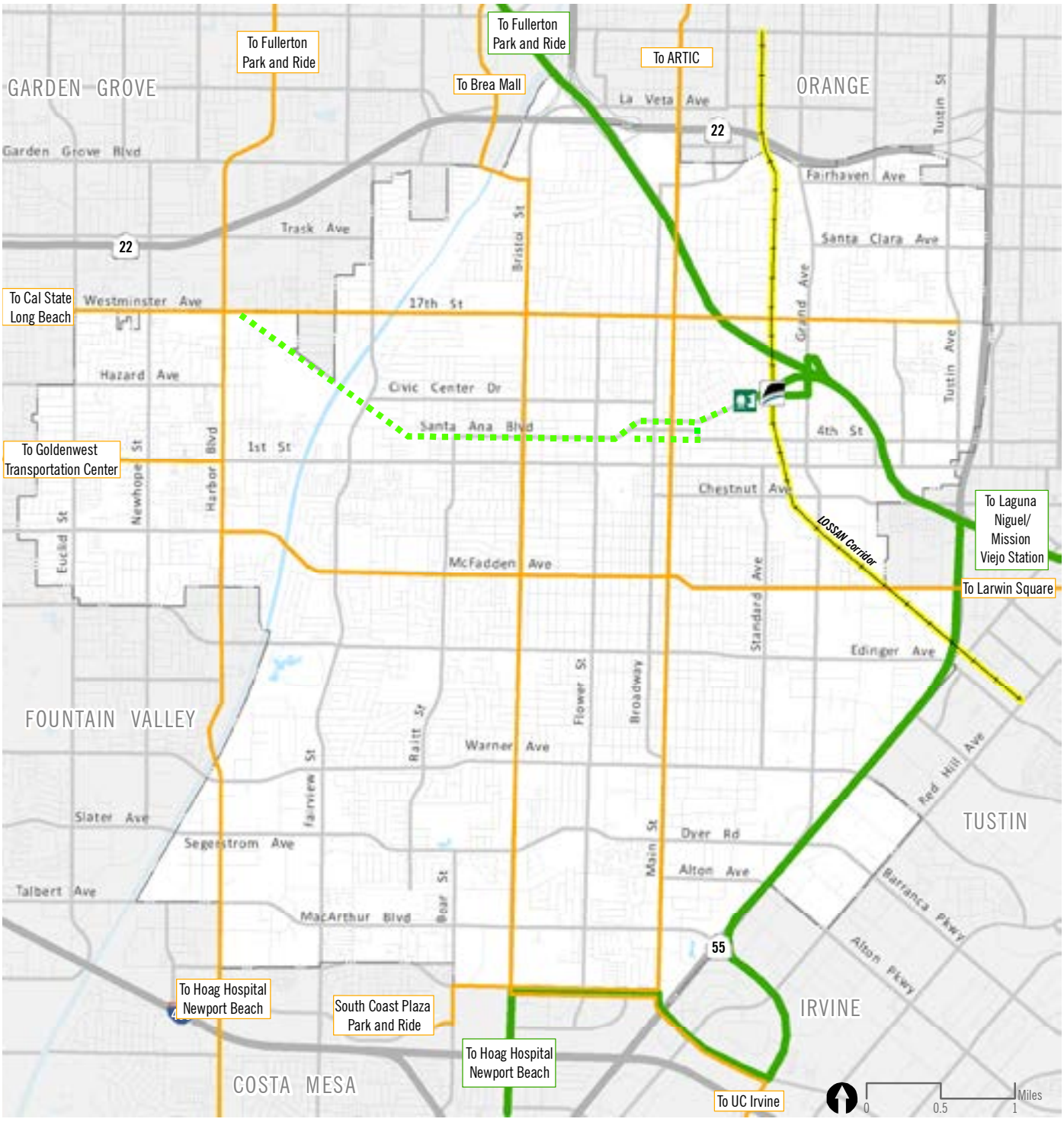
Classification	Description	Examples
Class I Bicycle Path	Paved rights-of-way for the exclusive use of bicyclists and pedestrians. Physically separated from vehicle traffic and generally built in locations not served by streets or where vehicular cross-flows are minimized.	Santa Ana River Trail and several segments of Alton Avenue/Maple Street, Santiago Creek Trail, Flower Street, Santa Ana Gardens Channel/Bear Street, and MacArthur Boulevard
Class II Bicycle Lane	One-way routes denoted by a striped lane on a roadway to delineate the rights-of-way assigned to vehicles and bikes. Can be striped adjacent to the curb where no parking exists or striped to the left side of on-street parking lanes.	Bristol Street, Greenville Street, Memory Lane, and Ross Street
Class III Bicycle Route	Bikeways where cyclists share the travel lane with motor vehicles. Typically on low-volume roadways, such as local streets in residential neighborhoods, and may be designated by signage or roadway markings (called sharrows).	Although not always designated by signage, most streets in low-traffic-volume residential neighborhoods are classified as Class 3 routes
Class IV Cycle Track	Local roads that have been enhanced with treatments that prioritize bicycle travel. These treatments might include wayfinding signage, bollards, and traffic-calming features that facilitate safe and convenient bicycle travel, slow vehicle speeds, and minimize vehicular traffic volumes.	Bristol Street and Edinger Avenue



▲ **Cycle Track along Bristol Street**

As part of the Bristol Street Specific Plan, the City constructed (in 2018) a six-foot-wide bike lane, a concrete curb with plantings separating the bike lane from general traffic lanes, and a greenway separating the bike lane from the sidewalk.

ABOUT THE MAP. This map shows the long-term plans of the City and regional transit operators to expand and enhance ways for people to travel within, to, and from Santa Ana by rail, streetcar, and bus. OCTA Transit and Freeway Transit Opportunity Corridors represent bus rapid transit lines; other OCTA bus lines (not shown) run along most major streets in the city.



Source: OCTA OCstreetcar Overview, OCTA Transit Vision Plan (June 2020) Date Published: 9/20/2020

OCTA Transit Opportunity Corridor

OCTA Freeway Transit Opportunity Corridor

Future Alignment of OC Street Car

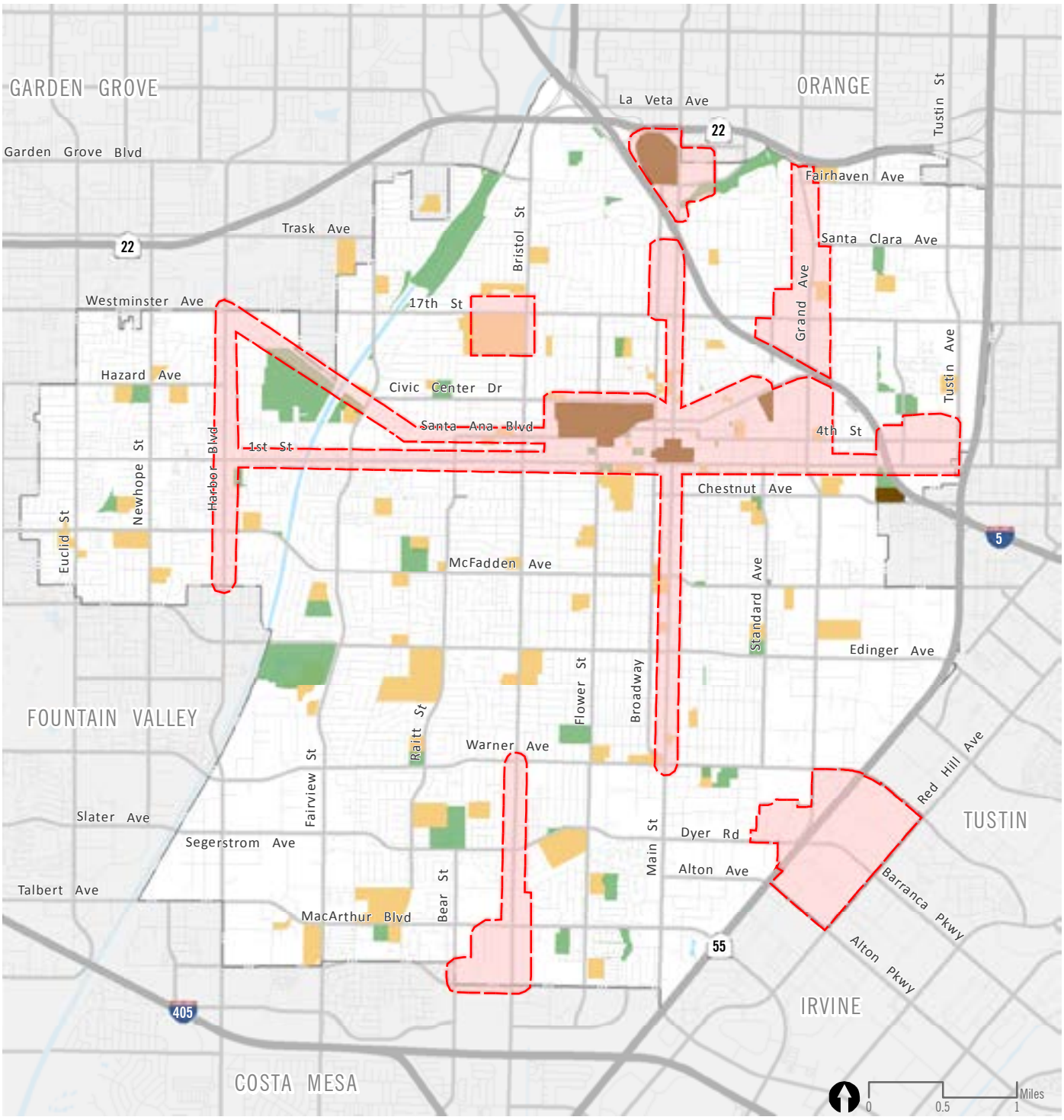
Santa Ana Regional Transportation Center

Metrolink and Amtrack Commuter Rail

Metrolink Station

FIGURE M-3
MASTER PLAN OF TRANSIT

ABOUT THE MAP. This map shows areas that currently or should support high levels of pedestrian activity based on proximity to activity centers, mixed-use development, and/or transit stops. The intent is to foster a more walkable and active public realm through improvements such as wider sidewalks, safer street crossings, and enhanced streetscapes.

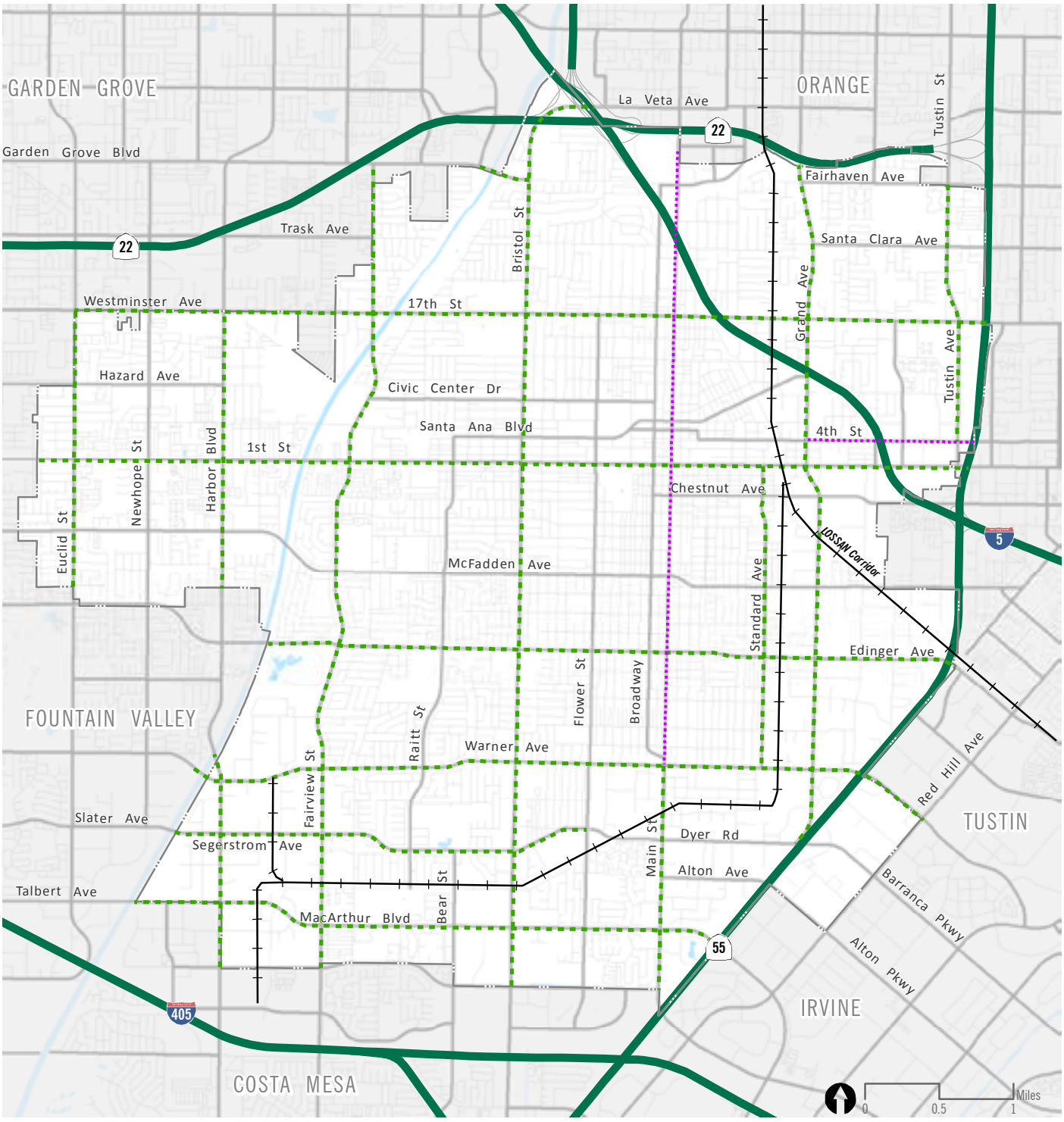


Source: City of Santa Ana 2020 Date Published: 9/20/2020

Pedestrian Opportunity Zone
Parks & Recreation
Landmark
School

FIGURE M-4
PEDESTRIAN OPPORTUNITY ZONES

ABOUT THE MAP. This map shows the network for transporting materials and goods to and from businesses in Santa Ana, and throughout southern California. The City's street system supports the national network and local businesses via designated truck routes, generally along major or primary arterials that connect to freeways. Except for local deliveries, trucks are prohibited from driving on residential streets or low-volume roadways.



Source: Truck Networks on State Highways District 12 (CalTrans 6/7/2018), California Rail Network (CalTrans 10/31/2018), U.S. Department of Transportation - Federal Highway Administration, Cityof Santa Ana.

Date Published: 9/21/2020

Truck/Freight Rail Routes

- National Network Route
- Local Truck Route
- Local Truck Route / Terminal Access Truck Route
- Freight Rail Line

Notes: National Network Routes are designated truck routes regulated by federal regulations (Code of Federal Regulations, Title 23, Section 658), and allow trucks that may exceed the limits established under state regulations. Local truck routes designated as a Terminal Access route allow vehicles that exceed the limits established under state regulations (California Vehicle Code Sections 35400/35401). Local truck routes allow any vehicle that exceeds 3 tons, but vehicles must meet other state regulations regarding length and width.

FIGURE M-5
GOODS MOVEMENT NETWORI 2070



Santa Ana around 1920. Although much has changed in the world, the need for a multimodal transportation system remains constant. Shown here are people walking, driving, biking, and riding the Red Car along 4th Street in Santa Ana. Within the next couple of years, those living and working in Santa Ana will once again enjoy these options.

Photo courtesy of Orange County Archives

GOAL M-2: Regional Mobility

An integrated system of travelways that connects the city to the region, employment centers, and key destinations, making Santa Ana the leader in regional transportation.

POLICY M-2.1
INTERSTATE FREEWAYS

Support Caltrans and OCTA efforts to improve freeway safety, while addressing impacts to neighborhoods.

H Eq S

POLICY M-2.2
TRANSIT SERVICES

Support OCTA, Caltrans, and other regional and local transportation providers to enhance existing transit services to provide residents, workers and visitors with safe, affordable, accessible, convenient, reliable, and attractive transit services.

H Eq S

POLICY M-2.3
REGIONAL TRANSPORTATION CENTER

Continue to promote and develop the Santa Ana Regional Transportation Center (SARTC) as a major transportation hub linking Amtrak, Metrolink, the OC Streetcar, other regional systems, and first and last mile connections.

H S

POLICY M-2.4
COMMUTER RAIL

Support the expansion of commuter rail services and Santa Ana's role as a destination along the Los Angeles–San Diego–San Luis Obispo (LOSSAN) rail corridor.

H S

POLICY M-2.5
OC STREETCAR

Support development and expansion of the OC Streetcar project, connecting neighborhoods, employment centers, and Downtown Santa Ana to activity centers in Orange County.

H S

POLICY M-2.6
HIGH FREQUENCY TRANSIT CORRIDORS

Work with OCTA to support the improvement of transit opportunity corridors to facilitate high frequency transit (e.g., bus rapid transit and other modes) along designated corridors in Santa Ana.

H S

POLICY M-2.7
REGIONAL MOBILITY ACCESS

Enhance access to regional transit, including first and last mile connections, to encourage the use of public transit.

H Eq S

POLICY M-2.8
GRADE SEPARATIONS

Encourage the installation and improvement of grade separations at rail crossings that minimize impacts to adjacent properties and nonmotorized users.

H Eq S

POLICY M-2.9
GOODS MOVEMENT

Maintain a network of truck routes limited to arterial streets to allow for goods movement and protect residential neighborhoods from adverse impacts.

H Eq S

GOAL M-3: Active Transportation

A safe, balanced, and integrated network of travelways for nonmotorized modes of transportation that connects people to activity centers, inspiring healthy and active lifestyles.

POLICY M-3.1
NONMOTORIZED TRAVELWAY NETWORK

Expand and maintain a citywide network of nonmotorized travelways within both the public and private realms that create linkages between neighborhoods, recreational amenities, schools, employment centers, neighborhood serving commercial, and activity centers.

H Eq S

POLICY M-3.2
NONMOTORIZED TRAVELWAY AMENITIES

Enhance nonmotorized travelways with amenities such as landscaping, shade trees, lighting, benches, crosswalks, rest stops, bicycle parking, and support facilities that promote a pleasant and safe experience.

H Eq S

POLICY M-3.3
SAFE ROUTES TO SCHOOLS AND PARKS

Lead the development and implementation of safe routes to schools and parks by partnering with the school districts, residents, property owners, and community stakeholders.

H Eq (EJ) S

POLICY M-3.4
REGIONAL COORDINATION

Coordinate development of the City's active transportation and transit network with adjacent jurisdictions, OCTA, and other appropriate agencies.

H Eq S

POLICY M-3.5
EDUCATION AND ENCOURAGEMENT

Encourage active transportation choices through education, special events, and programs.

H Eq S Ed

POLICY M-3.6
TRANSIT CONNECTIVITY

Enhance first and last mile connectivity to transit facilities through safe, accessible, and convenient linkages.

H Eq S

POLICY M-3.7
COMPLETE STREETS DESIGN

Enhance streets to facilitate safe walking, bicycling, and other nonmotorized forms of transportation through community participatory design.

H Eq S

POLICY M-3.8
SANTA ANA RIVER AND GOLDEN LOOP

Proactively pursue the improvement and restoration of the Santa Ana River natural habitat and the completion of the Golden Loop to serve as a multiuse recreational amenity.

H S

POLICY M-3.9
NEIGHBORHOOD TRAFFIC

Develop innovative strategies to calm neighborhood traffic, increase safety, and eliminate collisions, while also maintaining access for emergency response.

H

GOAL M-4: Transportation, Land Use, and Design

Coordinated transportation planning efforts with land use and design strategies that encourage sustainable development and achieve broader community goals.

POLICY M-4.1
INTENSE DEVELOPMENT AREAS

Program multimodal transportation and public realm improvements that support new development in areas along transit corridors and areas planned for high intensity development.

H S

POLICY M-4.2
PROJECT REVIEW

Encourage active transportation, transit use, and connectivity through physical improvements and public realm amenities identified during the City's Development Review process.

H Eq S

POLICY M-4.3
TRANSPORTATION MANAGEMENT

Coordinate with OCTA, employers, and developers to utilize TDM (transportation demand management) strategies and education to reduce vehicle trips and parking demands.

H S

POLICY M-4.4
FAIR SHARE IMPACTS

Ensure that all development projects pay their fair share of the system improvements necessary to accommodate the transportation needs of their projects.

Eq S

POLICY M-4.5
LAND USE DEVELOPMENT DESIGN

Ensure that building placement and design features create a desirable and active streetscape, by prioritizing pedestrian access directly from the street and placing parking lots to the rear of a development site.

H S

POLICY M-4.6
ROADWAY CAPACITY ALTERNATIVES

Promote reductions in automobile trips and vehicle miles traveled by encouraging transit use and nonmotorized transportation as alternatives to augmenting roadway capacity.

H S

POLICY M-4.7
PARKING

Explore and implement a flexible menu of parking options and other strategies to efficiently coordinate the response to parking demands.

S

POLICY M-4.8
NOISE MITIGATION

Encourage physical and operational improvements to reduce noise levels around major roads, freeways, and rail corridors, in particular around sensitive land uses.

H Eq

POLICY M-4.9
AIR POLLUTION MITIGATION

Utilize land use, building, site planning, and technology solutions to mitigate exposure to transportation-related air pollution, especially in environmental justice focus areas.

H Eq EJ S

GOAL M-5: Sustainable Transportation Design

A transportation system that is attractive, safe, state-of-the-art, and supports community, environmental, and conservation goals.

POLICY M-5.1
ENHANCED STREET DESIGN

Improve the beauty, character, and function of travelways with amenities such as landscaped parkways and medians, bike lanes, public art, and other amenities.

H S

POLICY M-5.2
RAIL CORRIDORS

Coordinate with rail service providers to improve and maintain the aesthetics of rail corridors, reduce noise levels, and mitigate traffic conflicts and other environmental hazards.

H Eq S

POLICY M-5.3
TRAVEL VIEWS

Promote the undergrounding of utilities and the reduction of visual clutter along travelways.

S

POLICY M-5.4
GREEN STREETS

Leverage opportunities along streets and public rights-of-way to improve water quality through use of landscaping, permeable pavement, and other best management practices.

S

POLICY M-5.5
STREET DESIGN

Design and retrofit streets based on their combined land use context and road function to achieve safety objectives.

H Eq S

POLICY M-5.6
CLEAN FUELS AND VEHICLES

Encourage the use of alternative fuel vehicles and mobility technologies through the installation of supporting infrastructure.

S

POLICY M-5.7
INFRASTRUCTURE CONDITION

Enhance travelway safety by maintaining streets, alleys, bridges, sidewalks, lighting, and other transportation infrastructure in excellent condition.

S Eq(EJ)

POLICY M-5.8
TRAFFIC SAFETY

Prioritize the safety of all travelway users when designing transportation improvement and rehabilitation projects.

H S

Transit Hub. In 1985, the Santa Ana Regional Transportation Center (SARTC) was erected on the site of the former Atchison, Topeka and Santa Fe Railway depot that was built in 1933 and closed in 1982. The SARTC now serves as the hub of Amtrak's Pacific Surfliner and Metrolink's Orange County lines, and shares use of the railroad with the Burlington-Northern Santa Fe and Union Pacific, which serve the city's industrial sector. Santa Ana continues to lead the rail revolution—most importantly, the planned OC Streetcar, set to begin operations in 2022/23.



RELATED GENERAL PLAN POLICIES

Each policy in the City of Santa Ana General Plan is listed under the most related goal and element. Some policies, however, also reinforce other goals housed in different elements.

To provide a more complete picture of all of the policies that support the goals of this element, Table M-1 lists each Mobility goal in the far left column and, in columns to the right, identifies related policy numbers from other elements.

Goal M-2, for example, is supported not only by the policies in this element (M-1.1 through M-5.8), but also by Policy CN-3.11 in the Conservation Element, Policy N-1.3 in the Noise Element, Policy S-2.3 in the Safety Element, and Policy UD-7.5 in the Urban Design Element.

TABLE M-3. RELATED GENERAL PLAN POLICIES

	Volume 1 Services & Infrastructure				Volume 2 Natural Environment				Volume 3 Built Environment			
Mobility Goals	CM	M	EP	PS	CN	OS	N	S	LU	HE	HP	UD
M-1: Comprehensive Circulation A comprehensive and multimodal circulation system that facilitates the safe and efficient movement of people, enhances commerce, and promotes a sustainable community.	CM-1.1	Current Element	EP-3.4	PS-1.8	CN-1.9 CN-1.10 CN-1.14	OS-2.6 OS-2.14	N-2.1	-	LU-4.5	HE-1.13 HE-1.14 HE-1.8	-	-
M-2: Regional Mobility An integrated system of travelways that connects the city to the region, employment centers, and key destinations, making Santa Ana the leader in regional transportation.	-	Current Element	-	-	CN-3.11	-	N-1.3	S-2.3	-	-	-	UD-7.5
M-3: Active Transportation A safe, balanced, and integrated network of travelways for nonmotorized modes of transportation that connects people to activity centers, inspiring healthy and active lifestyles.	CM-3.4 CM-3.7	Current Element	-	-	-	OS-1.2 OS-1.6 OS-1.7 OS-2.3	-	-	LU-1.7	-	-	UD-3.6 UD-5.4

Notes:

CM: Community Element
M: Mobility Element
EP: Economic Prosperity Element
PS: Public Services Element
CN: Conservation Element
OS: Open Space Element
N: Noise Element
S: Safety Element
LU: Land Use Element
HE: Housing Element
HP: Historic Preservation Element
UD: Urban Design Element

TABLE M-3. RELATED GENERAL PLAN POLICIES

	Volume 1 Services & Infrastructure				Volume 2 Natural Environment				Volume 3 Built Environment			
Mobility Goals	**CM**	**M**	**EP**	**PS**	**CN**	**OS**	**N**	**S**	**LU**	**HE**	**HP**	**UD**
M-4: Transportation, Land Use, and Design Coordinated transportation planning efforts with land use and design strategies that encourage sustainable development and achieve broader community goals.	-	Current Element	-	-	CN-1.10	-	-	-	LU-1.6 LU-2.10 LU-3.6	HE-1.5	-	UD-3.4 UD-3.5
M-5: Sustainable Transportation A transportation system that is attractive, safe, state-of-the-art, and supports community, environmental, and conservation goals.	-	Current Element	-	-	CN-1.11 CN-1.12	-	-	-	-	-	-	UD-3.1 UD-3.2 UD-7.2

Notes:

CM: Community Element
M: Mobility Element
EP: Economic Prosperity Element
PS: Public Services Element
CN: Conservation Element
OS: Open Space Element
N: Noise Element
S: Safety Element
LU: Land Use Element
HE: Housing Element
HP: Historic Preservation Element
UD: Urban Design Element



▲ Santa Ana River Trail

Nearly four linear miles of trails run along the Santa Ana River through the city's western area. The City's vision for the river includes preserving and restoring natural habitat, acquiring land for improvements, protecting water quality, and promoting the Santa Ana River Trail as a nonmotorized transportation route to serve the region. This photo shows a bicyclist along the trail as it passes under the 1st Street bridge.

IMPLEMENTATION

An implementation plan is a coordinated series of actions the City desires to take in the future that are intended to advance, over the long term, the City's Shared Vision, Core Values, and the General Plan goals and policies. An implementation plan is thus a follow-up measure for this element. Taken as a whole, these programs represent the City's best thinking today on what actions should be taken to address the considerations and concerns of the community and make sure that the plan's aspirations are achieved.

Implementation is in large part contingent upon adequate funding. Many of these actions can be pursued through initiatives already underway. Other programs will require additional resources. Therefore, the exact mix and timing of programs the City may pursue will in part be opportunity driven, dependent on the availability of funding, staffing, and other necessary resources. The Time Frame in the Implementation Table below is the target for completion of the Action.

This element may be implemented by amendments to existing plans, ordinances, development standards and design guidelines; capital investments/projects; and interagency/interjurisdictional coordination. The following table identifies the implementation action, the responsible City agency, and targeted timeline for accomplishment.

TABLE M-4. MOBILITY ELEMENT IMPLEMENTATION

Ref #	Implementation Action	Agency / Time Frame
GOAL M-1: Comprehensive Circulation A comprehensive circulation system that facilitates the safe and efficient movement of people, enhances commerce, and promotes a sustainable community.		
1.1	**Cross-agency collaboration.** Coordinate with external agencies to address the impacts of new regional transportation projects on the local network and accommodate complete street practices.	PWA Ongoing
1.2	**MPAH.** Coordinate with external agencies to ensure the OCTA Master Plan of Arterial Highways accommodates current and future demand for all users while prioritizing the safety of people utilizing non-automobile modes of transportation.	PWA 2022-2035
1.3	**Complete streets design.** Update the City's standard plans to include complete streets design strategies.	PWA 2025
1.4	**Reduce collisions.** Minimize parking from arterial streets to reduce vehicular, bicycle, and pedestrian conflicts.	PWA Ongoing
1.5	**Capital Improvement Program.** Prepare the annual Capital Improvement Program that corresponds with the City's general plan goals and implementation actions.	PWA Annually
1.6	**Asset Management database.** Explore the benefits of an asset management database to coordinate ongoing maintenance of streets, parkway and the public realm.	PWA Ongoing
1.7	**Transportation network safety.** Continue ITS to provide enhanced safety and efficiency features on the transportation network, including traffic signal synchronization.	PWA Ongoing
1.8	**Impact fees.** Secure development impact fees and dedications for project-related transportation improvements during City review and approval processes.	PWA 2021-2035
GOAL M-2: Regional Mobility Integrated system of travelways that connects the City to the region, employment centers, and key destinations, making Santa Ana the leader in regional transportation.		
2.1	**Cross-agency planning.** Work with OCTA and regional transportation agencies on the Long-Range Transportation Plan and to expand bus and rail transit services, particularly transit priority corridors.	PWA/PBA- Ongoing

TABLE M-4. MOBILITY ELEMENT IMPLEMENTATION

Ref #	Implementation Action	Agency / Time Frame
2.2	**Cross-agency coordination.** Coordinate with OCTA and transportation agencies to identify a funding, operation, and maintenance plan for the OC Streetcar.	PWA Ongoing
2.3	**Bus turnouts.** Construct bus turnouts on arterial streets according to OCTA design standards.	PWA Ongoing
2.4	**Freeway improvement projects.** Participate in Caltrans Advisory Working Groups freeway improvement projects to evaluate benefits and impacts to the Santa Ana local streets.	PWA Ongoing
2.5	**Grade separation.** Pursue implementation of grade separation between rail crossings and roadways at Santa Ana Boulevard, Seventeenth Street, and Grand Avenue.	PWA 2022-2035
2.6	**Regional transit planning.** Develop the Santa Ana Regional Transportation Center (SARTC) Master Plan.	PWA/PBA/ CDA 2022-2035
2.7	**Wayfinding program.** Refine the City's wayfinding signage program to direct travelers to activity centers, Downtown, and Civic Center.	PWA 2022-2035
GOAL M-3: Active Transportation A safe, balanced, and integrated network of travelways for nonmotorized modes of transportation that connects people to activity centers, inspiring healthy and active lifestyles.		
3.1	**Active transportation planning.** Develop and pursue implementation of a comprehensive active transportation plan.	PWA 2022-2035
3.2	**Circulation connectivity.** Close the Golden Loop network between Memory Lane and Santiago Creek and between Santiago Park and Lincoln Avenue/Santiago Street.	PWA/PRCSA Ongoing
3.3	**Pedestrian accessibility.** Implement the City ADA Transition Plan to cost-effectively enhance pedestrian accessibility, with guidance from the Sidewalk Connectivity Plan.	PWA Ongoing
3.4 EJ	**Pedestrian opportunity zones.** Prepare public realm plans in pedestrian opportunity zones.	PWA/PBA 2022-2035
3.5	**Safe routes to schools and parks.** Develop and pursue implementation of a Safe Routes to School Plan and a Safe Routes to Parks Plan.	PWA/PRCSA Ongoing
3.6	**Drinking water access.** Create a network of public water fountains / bottle fill stations that promote the City's award-winning water, conservation, and substitution of bottled water for affordable public drinking water.	PWA 2022-2035
GOAL M-4: Transportation, Land Use, and Design Coordinated transportation planning efforts with land use and design strategies that encourage sustainable development and achieve broader community goals.		
4.1	**Municipal Code update.** Amend the code to require parking studies to evaluate requests for parking reductions in all development projects.	PBA 2024
4.2	**Downtown transportation planning.** Prepare comprehensive Downtown transportation and parking management plan that involves Downtown stakeholders and addresses downtown activity, economic growth, and operational improvements.	PWA/PBA/ CDA 2025
4.3	**Downtown transportation improvements.** Study the Downtown parking structure facilities, involving Downtown stakeholders, to identify potential operational improvements and monitor the program.	CDA/PWA 2025
4.4 EJ	**Residential parking.** Reevaluate the Residential Permit Parking Program to ensure it complies with state law and best practices.	PWA Ongoing
4.5	**Citywide Design Guidelines update.** Update the Citywide Design Guidelines to strengthen pedestrian and cyclist linkages to development centers and residential neighborhoods and coordinate on-site landscape with public realm landscaping.	PBA/PWA 2022-2027
4.6	**Safe travelways.** Provide City interagency review of physical improvements and related design standards within travelways and the public realm.	PWA Ongoing

TABLE M-4. MOBILITY ELEMENT IMPLEMENTATION

Ref #	Implementation Action	Agency / Time Frame
4.7	**Bike parking and amenities.** Update the City bike parking ordinance to require new major development to provide long-term bike locker parking and amenities, and incentives for inclusion in existing development.	PBA 2022-2027
4.8	**Ensure safety.** Utilize the City's project review processes to identify and maintain landscaped setbacks along public rights-of-way.	PWA Ongoing
4.9	**Trip reduction.** Continue to evaluate alternative trip reduction program best practices for City employees, and implement a program that is cost-effective.	HR Ongoing
4.10	**Vehicle miles traveled.** Maintain vehicle miles traveled (VMT) guidelines in compliance with SB 743.	PWA/PBA Ongoing
4.11	**Cross-agency coordination.** Maintain ongoing coordination of land use and transportation impacts through joint powers authority agreements with adjacent jurisdictions.	PWA Ongoing
4.12	**School partnerships.** Pursue partnerships with local school districts to improve coordination of review process for new and rehabilitated school facility improvements, to promote safe and well-designed student pick-up/drop-off.	PWA/PD Ongoing
GOAL M-5: Sustainable Transportation A transportation system that is attractive, safe, state-of-the-art, and supports community, environmental, and conservation goals.		
5.1	**Safe travelways.** Continue design practices that facilitate the safe use of the travelways.	PWA Ongoing
5.2	**Street designs.** Participate in state and regional transportation agency forums to affect policies for universal street design standards to ensure standards are user friendly, cost-effective, and sustainable.	PWA Ongoing
5.3	**Street improvements.** Incorporate low impact development (LID) strategies to infiltrate, treat, or harvest urban stormwater runoff in street improvement projects.	PWA Ongoing
5.4	**Municipal/development code update.** Update the City's design standards and municipal codes regarding landscaping and amenities in the public realm and street parkways, to promote aesthetically pleasing and sustainable corridors.	PWA 2025
5.5	**Tree preservation.** Re-evaluate the City's street tree maintenance and preservation programs to ensure fiscal sustainability and aesthetically pleasing trees over the long term.	PWA 2022-2035
5.6	**Traffic calming.** Develop or update neighborhood traffic management plans to facilitate traffic calming measures appropriate and reasonable for the area.	PWA Ongoing
5.7 EJ	**Parking management strategies.** Evaluate parking management strategies, such as parking assessment districts, to facilitate parking in areas programmed for future development.	CDA 2022
5.8	**Air quality improvements.** Participate in inter-jurisdictional efforts to promote improvements in air quality and to meet state and federal mandates through advanced technology and TDM programs.	PBA/PWA 2021-2035
5.9	**Rail service.** Coordinate with rail and transit service providers to address aesthetics, ongoing maintenance, safety, and noise concerns along rail corridors.	PWA Ongoing
5.10	**Street medians.** Complete construction of medians on major arterial streets.	PWA 2022-2035
5.11	**Underground utilities program.** Continue to implement the program to underground utilities on arterial streets.	PWA 2022-2035
5.12	**Transportation network.** Provide upkeep of the City's transportation network based upon an established maintenance schedule.	PWA Ongoing

Notes:
CDA - Community Development Agency
CMO - City Manager's Office
HR- Human Resources Department
PBA - Planning and Building Agency
PWA - Public Works Agency
PRCSA - Parks, Recreation and Community Services Agency
PD - Police Department
EJ associated with environmental justice policies


golden city beyond
A SHARED VISION FOR SANTA ANA

OCTA

November 2, 2020

To: Regional Planning and Highways Committee

From: Darrell E. Johnson, Chief Executive Officer

Subject: Amendments to the Master Plan of Arterial Highways

Overview

The Orange County Transportation Authority administers the Master Plan of Arterial Highways, including the review and approval of amendments requested by local agencies. The City of Santa Ana has requested multiple amendments to the Master Plan of Arterial Highways. These amendments are subject to approval by the Board of Directors and are recommended to be contingent on an executed memorandum of understanding to address potential impacts. A status update on the active Master Plan of Arterial Highways amendments is also provided.

Recommendations

A. Authorize the Chief Executive Officer, or his designee, to negotiate and execute a final memorandum of understanding specifying roles and responsibilities for implementation of proposed actions related to the Master Plan of Arterial Highways amendment request. Participating agencies include the cities of Fountain Valley, Santa Ana, Tustin, and the Orange County Transportation Authority.

B. Conditionally approve the following amendment to the Master Plan of Arterial Highways to:

   1. Reclassify the following streets from a secondary (four-lane, undivided) arterial to a divided collector (two-lane divided):
      i. Cambridge Street between Fairhaven Avenue and State Route 22.
      ii. Santa Clara Avenue between Grand Avenue and Tustin Avenue.
      iii. Broadway Street between 1st Street and 17th Street.
      iv. Penn Way between Interstate 5 southbound on/off-ramps and Washington Avenue.

v. Santiago Avenue between Washington Avenue and 6th Street.
vi. Standard Avenue between 6th Street and Warner Avenue.
vii. Civic Center Drive between Fairview Street and Bristol Street.
viii. Hazard Avenue between Euclid Street and Harbor Boulevard.
ix. Raitt Street between Segerstrom Avenue and Santa Ana Boulevard.
x. McFadden Avenue between Harbor Boulevard and Grand Avenue.
xi. Flower Street between Warner Avenue and First Street.
xii. Halladay Avenue between Warner Avenue and Dyer Road.
xiii. Chestnut Avenue between Standard Avenue and Grand Avenue.

2. Reclassify Civic Center Drive between French Street and Santiago Street, from a secondary (four-lane, undivided) arterial to a collector (two-lane, undivided) arterial.
3. Reclassify 1st Street between Bristol Street and Tustin Avenue, from a major (six-lane, divided) to a primary (four-lane, divided) arterial.
4. Reclassify Chestnut Avenue between Grand Avenue the eastern city limit, from a primary (four-lane, divided) to a divided collector (two-lane, divided) arterial.
5. Remove the following facilities from the Master Plan of Arterial Highways:
   i. Flower Street between 17th Street and its northern terminus.
   ii. Logan Street between Stafford Street and Santa Ana Boulevard.
   iii. Stafford Street between proposed Logan Street and Santiago Street.

The proposed amendment will become final contingent upon the Orange County Transportation Authority (1) fully executing a final memorandum of understanding with the cities of Fountain Valley, Santa Ana, and Tustin, and (2) receiving documentation that the City of Santa Ana has complied with the requirements of the California Environmental Quality Act and have amended their general plan.

If the original proposed Master Plan of Arterial Highways amendment is modified as a result of the California Environmental Quality Act and/or general plan amendment process, the modified Master Plan of Arterial Highways amendment shall be returned to the Orange County Transportation Authority Board of Directors for consideration and action.

C. Conditionally approve the amendment to the Master Plan for Arterial Highways for Fairhaven Avenue between Grand Avenue to Tustin Avenue from a secondary (four-lane, undivided) arterial to a divided collector (two-lane, divided) arterial. The proposed amendment will become final contingent upon the Orange County Transportation Authority receiving documentation that the cities of Santa Ana and Orange have complied with the requirements of the California Environmental Quality Act and have amended their respective general plans.

   If the cities of Santa Ana and Orange do not update their respective general plans within three years to reflect the Master Plan of Arterial Highway amendment, the contingent amendment will expire, but can be returned to the Orange County Transportation Authority Board of Directors for reconsideration and action.

   If the original proposed Master Plan of Arterial Highways amendment is modified as a result of the California Environmental Quality Act and/or general plan amendment process, the modified Master Plan of Arterial Highways amendment shall be returned to the Orange County Transportation Authority Board of Directors for consideration and action.

D. Direct the Executive Director of Planning, or his designee, to file a Notice of Exemption from the California Environmental Quality Act in support of the Master Plan of Arterial Highways amendment.

E. Receive and file a status report on the active Master Plan of Arterial Highways amendments.

Background

In January 2020, the City of Santa Ana (City) initiated the Circulation Element Update in combination with the overall General Plan Update. The proposed Circulation Element Update is the culmination of community outreach efforts undertaken by the City since 2011, input from neighboring local agencies, and various planning documents. The goal of these efforts is to provide active transportation options for residents through complete street improvements and vision zero policies. The City is requesting amendments to the Master Plan of Arterial Highways (MPAH) to reclassify 17 segments in order to accommodate complete street projects and remove three segments from the MPAH (Attachment A). Complete streets refer to street features that accommodate all users, including pedestrians, bicyclists, public transit users, and drivers. The 17 segments represent those arterials which require MPAH reclassifications in order to accommodate complete street improvements and are depicted in Attachment B.

Amendment to the MPAH require a review of the potential circulation implications of the change. The City utilized the General Plan Update Program Environmental Impact Report (PEIR)-Traffic Impact Study (TIS) for the MPAH amendment process. The TIS reviews existing and future traffic conditions of the current general plan and of the proposed land use and circulation element updates. Additional analysis was completed that focused on the identification of the impacts and mitigations associated with the changes to the MPAH, as opposed to the overall General Plan Update evaluated by the PEIR. The City plans to adopt the General Plan Update and PEIR in late fall 2020. The proposed MPAH amendments are recommended for conditional approval by the Orange County Transportation Authority (OCTA) Board of Directors (Board) contingent on compliance with the MPAH Guidance, and additional terms identified in the Recommendations.

Discussion

The 17 segments proposed for reclassification would result in a reduction of approximately 30 existing lane miles. Deletions and downgrades on the MPAH may be allowed if the increased traffic volumes in the affected agencies do not result in the unmitigated peak-hour intersection impacts (Guidance for Administration of the MPAH, Section 4.0 MPAH Amendment Policies). If impacts to the MPAH system are identified as a result of the proposed amendment, approval of an amendment may be subject to the execution of a memorandum of understanding (MOU) between OCTA and affected agencies, which specify roles and responsibilities for implementation of any identified mitigations. The TIS analyzed 105 intersections with 18 identified that could be impacted due to the MPAH amendment request (Attachment C). Of the 18 impacted intersections, four are located outside the City in the following jurisdictions: City of Fountain Valley (1), City of Orange (1), and the City of Tustin (2).

The City proposed a draft mitigation program to ensure that mitigations for impacted intersections, if needed in the future, would be implemented. The program would fully mitigate impacts at eight intersections (including the four intersections outside of the City) and partially mitigate impacts at two intersections. Feasible mitigations could not be developed at the remaining eight impacted intersections, all located within the City, due to right-of-way needs for proposed complete streets projects. The TIS also identified potential significant impacts to an additional five intersections due to the City’s proposed Land Use Element Update. These impacts are not associated with the change to the MPAH; therefore, they are excluded from the MPAH amendment process.

Furthermore, potential impacts to high-quality transit corridors were identified. OCTA provides high-frequency (15-minute service intervals) fixed-route bus service during peak commute hours on two segments that are part of the MPAH amendment - McFadden Avenue (Route 66) and 1st Street (Route 64).

The City's proposal would remove existing and planned travel lanes to incorporate complete streets infrastructure. OCTA and City staff agreed that additional mitigation measures on these corridors are necessary to maintain or improve transit service performance.

An MOU was prepared to define the roles and responsibilities of the parties the mitigations for the proposed MPAH amendment (Attachment D). Currently, the City and the affected agencies are in agreement on a draft mitigation program. For the impacts located at the cities of Fountain Valley and Tustin, the City has agreed to pay, at minimum, their fair share cost of the mitigation measures. The proposed reclassification on Fairhaven Avenue, between Grand Avenue and Tustin Avenue, is shared with the City of Orange. For this reclassification on Fairhaven Avenue to become final, both the cities of Orange and Santa Ana will need to update their circulation elements. The reclassification results in an impact at the Tustin Avenue and Fairhaven Avenue intersection (City of Orange). The existing intersection can accommodate the proposed reclassification to a divided collector (two-lane, divided) arterial, and therefore, the City of Orange has requested not be a party to the MOU. Lastly, the City has also agreed to fund a cooperative study, led by OCTA. The study will identify feasible transit improvements for impacts on high-quality transit corridors. Roles and responsibilities for funding and implementation would be identified and coordinated with OCTA as well. The affected agencies have reviewed the TIS, addressed remaining questions with the City, and all have provided letters of support for the City's amendment request (attachments E, F, and G).

The City also shared the TIS with immediately adjacent agencies - the cities of Costa Mesa, Garden Grove, Irvine, Westminster, the County of Orange, and the California Department of Transportation. Except for the City of Garden Grove (Garden Grove), all immediately adjacent agencies have indicated their support for the City's amendment request (attachments H, I, J, K, and L). Garden Grove is the owner of the Willowick Golf Course property, which is near the proposed Hazard Avenue reclassification. Garden Grove would like to preserve existing vehicular access in the vicinity of the Willowick property while the City and Garden Grove solidify a vision for the property. Currently, the property remains classified as open space in the City's proposed Land Use Element Update. Moreover, the TIS did not identify any impacts in the vicinity of Hazard Avenue, and it is projected to operate at a segment link level of service "A" in the future year with reclassification. Therefore, there are no technical MPAH-related issues with the proposed reclassification to Hazard Avenue.

Further, the City has discussed the matter with City of Garden Grove, and there is an understanding that the parties will work to address this issue as the long-term vision for the property develops.

Based on the support from the affected agencies and technical analysis of the proposed reclassifications, staff recommends conditional approval of the MPAH amendments, authorization to negotiate and execute the final MOU, and a cooperative agreement for a transit cooperative study. If approved by the Board, the proposed amendments will not become final until OCTA receives: (1) a fully signed and executed MOU among OCTA and the cities of Fountain Valley, Santa Ana, and Tustin; and (2) documentation that the City has amended their general plan and have complied with the requirements of the California Environmental Quality Act (CEQA). Additionally, the City of Orange must also provide documentation that their general plan has been amended in compliance with CEQA before finalizing the reclassification of Fairhaven Avenue on the MPAH.

CEQA

Amendments to the MPAH are exempt from the CEQA review. As such, if the Board approves the recommendations, OCTA will file a Notice of Exemption from CEQA in support of the proposed amendment to the MPAH.

MPAH Status Update

As indicated in the status report in Attachment M, there are currently 22 active amendments proposed for the MPAH.

The City's General Plan Update also includes the conditionally-approved MPAH amendment in 2016 to support planned bikeways and accommodate the development of the OC Streetcar on Fourth Street from French Street to Grand Avenue, and on three segments of Santa Ana Boulevard:(1) Flower Street to Ross Street, (2) French Street to Santiago Street, and (3) Raitt Street to Flower Street. As indicated in Attachment A, the adoption of the General Plan Update will finalize the amendment on these four facilities.

In June 2020, OCTA received appropriate documentation to finalize and update the MPAH map to reflect conditionally-approved amendments within several other jurisdictions:

- Huntington Beach – Graham Street and Talbert Street (shared with the County of Orange), Delaware Street, 6th Street;
- Yorba Linda – Esperanza Road (shared with the County of Orange), Fairmont Boulevard Connector; and
- Westminster – Garden Grove Boulevard, Edwards Street, Trask Avenue, and Hoover Street.

Many of the other remaining amendments are awaiting local action to amend their respective general plans. Others are either under review, pending resolution of issues with other agencies, or awaiting refinement of development plans.

Summary

The City of Santa Ana has requested amendments to the Master Plan of Arterial Highways to reflect the City of Santa Ana's General Plan Update. The technical analysis indicates that -- with the recommended mitigations -- implementation of the amendments would not adversely impact the integrity of the Master Plan of Arterial Highways. Therefore, staff recommends Board of Directors' conditional approval of the requested amendments.

Attachments

A. Letter from Mr. Nabil Saba, P.E., Acting Executive Director, Public Works Agency, City of Santa Ana, to Mr. Charles Larwood, Manager of Transportation Planning, Orange County Transportation Authority, Dated January 21, 2020, re: Request for Amendment to the Master Plan of Arterial Highways in Santa Ana
B. City of Santa Ana Proposed MPAH Amendments Maps
C. City of Santa Ana General Plan Update, Amendment to the Master Plan of Arterial Highways, Intersection Impacts
D. Draft Memorandum of Understanding No. C-X-XXXX Between Orange County Transportation Authority and Cities of Santa Ana, Fountain Valley, Tustin for Amendment of the Master Plan of Arterial Highways
E. Letter from Ms. Hye Jin Lee, MS, PE, Director of Public Works, City of Fountain Valley, to Mr. Nabil Saba, P.E., Executive Director, Public Works Agency, City of Santa Ana, Dated September 8, 2020, re: City of Santa Ana Proposed Master Plan of Arterial Highways (MPAH) Reclassifications
F. Letter from Mr. Christopher S. Cash, Public Works Director, City of Orange, to Mr. Nabil Saba, P.E., Executive Director, Public Works Agency, City of Santa Ana, Dated September 17, 2020, re: City of Santa Ana Proposed Master Plan of Arterial Highways (MPAH) Reclassifications
G. Letter from Ms. Krys Saldivar, Public Works Manager - Traffic/ Transportation, City of Tustin, to Mr. Nabil Saba, P.E., Executive Director, Public Works Agency, City of Santa Ana, Dated October 6, 2020, re: City of Santa Ana Master Plan of Arterial Highways (MPAH) Reclassifications Letter of Support

H. Letter from Mr. Raja Sethuraman, Public Services Director, City of Costa Mesa, to Mr. Nabil Saba, P.E., Executive Director, Public Works Agency, City of Santa Ana, Dated September 3, 2020, re: City of Santa Ana Proposed Master Plan of Arterial Highways (MPAH) Reclassifications

I. Letter from Mr. Mark A. Steuer, Director of Public Works and Transportation, City of Irvine, to Mr. Nabil Saba, P.E., Executive Director, Public Works Agency, City of Santa Ana, Dated September 15, 2020, re: City of Santa Ana Proposed Master Plan of Arterial Highways (MPAH) Reclassifications

J. Letter from Mr. Marwan Youssef, Ph.D., P.E., Public Works Director/ City Engineer, City of Westminster, to Mr. Nabil Saba, P.E., Executive Director, Public Works Agency, City of Santa Ana, September 9, 2020, re: City of Santa Ana Proposed Master Plan of Arterial Highways (MPAH) Reclassifications

K. Letter from Ms. Nardy Khan, PE/PMP, Deputy Director, Infrastructure Programs, Orange County Public Works, to Mr. Nabil Saba, P.E., Executive Director, Public Works Agency, City of Santa Ana, Dated September 3, 2020, re: City of Santa Ana Proposed Master Plan of Arterial Highways (MPAH) Reclassifications

L. Letter from Mr. Scott Shelley, Branch Chief, Regional-IGR-Transit Planning, California Department of Transportation District 12, to Mr. Verny Carvajal, Principal Planner, City of Santa Ana, Dated August 17, 2020

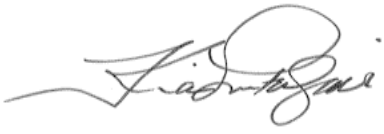
M. Status Report on Active Master Plan of Arterial Highways Amendments

Prepared by:



Stephanie Chhan
Transportation Analyst
(714) 560-5572

Approved by:



Kia Mortazavi
Executive Director, Planning
(714) 560-5741

Agenda Item

Planning Commission

Item #: 4.2. 2/2/2026 **File #:** 26-0007

TO: **Chair and Members of the Planning Commission**

THRU: **Hayden Beckman, Planning Manager**

FROM: **Ryan Agbayani, Senior Planner**

1. SUBJECT

Public Hearing to consider a request to operate a bed and breakfast inn at 616 W. Chapman Avenue and finding of CEQA Exemption (CUP-3208 and SP-1159).

2. SUMMARY

The applicant proposes to operate a bed and breakfast inn ("Bonjour Bed & Breakfast") within an existing one-story single-family residential dwelling located in the Old Towne Historic District.

3. RECOMMENDED ACTION

1. Conduct and close the Public Hearing.

2. Adopt Planning Commission Resolution No. PC 04-26, approving Conditional Use Permit No. 3208 and Minor Site Plan Review No. 1159 for a request to operate a bed and breakfast inn at 616 W. Chapman Avenue.

3. Find the request categorically exempt from the California Environmental Quality Act (CEQA) in accordance with Section 15301 (Class 1 - Existing Facilities) of the State CEQA Guidelines.

4. PROJECT BACKGROUND

Property Information	
Applicant	Bonjour Bed & Breakfast
Property Owner	Ahmed Joseph Mahmoud
Property Location	616 W. Chapman Avenue
Existing General Plan Land Use Element Designation	Old Towne Mixed Use Spoke Street 15S (
Existing Zoning Classification	Old Towne Mixed Use-15S (Specific Plan) (SP)
Old Towne	Yes
Specific Plan/PC	Santa Fe Depot Specific Plan
Site Size	6,751 Square Feet

Circulation	Access to the project site is from an existin driveway on W. Chapman Avenue to the n from an existing alley along the southern p
Existing Conditions	The site is currently developed with an exis one-story single-family dwelling.
Surrounding Land Uses and Zoning	**North:** OTMU-15S (SP) Existing commerc religious institution and coffee house withir Victorian structure (CUP-3210)
	South: Single-Family Residential-6 (R1-6) unit residential dwellings.
	East: OTMU-15S (SP) Existing commercia establishment with surface parking.
	West: OTMU-15S (SP) Existing commerci development (psychology offices) with surf the rear, adjacent to the alley.
Previous Applications/Entitlements	None.

5. PROJECT DESCRIPTION

The applicant proposes to operate a bed and breakfast inn that would host one to two adults, traveling together, within an existing one-story single-family dwelling in the Old Towne Historic District. Per the applicant's proposal, only one room will be dedicated to guests. No existing residential structures will be removed to operate the proposed bed and breakfast use or to provide the required parking, and no alterations to the existing dwelling are proposed with this request. The applicant's business narrative letter is included as Attachment 4.

6. PROJECT ANALYSIS

The proposed use is appropriate at this location and will provide a desired accommodation amenity for both local residents and visitors to the City. A bed and breakfast inn is a conditionally permitted use for properties located within the Old Towne Mixed Use-15S zoning district and the Santa Fe Depot Specific Plan.

Owner Occupancy Requirement

Per OMC Chapter 17.04 Definitions, a "bed and breakfast inn means a building of residential design determined to be architecturally or historically significant to the City of Orange wherein lodging is provided to tourists. The operator of the inn shall live on the same property."

Additionally, per OMC Section 17.13.050 Conditional Use Regulations, "an owner, manager, proprietor, or caretaker of the property must reside on the subject premises at all times."

The subject property is designated as a "contributor" in the Historic District. Furthermore, the property

owner has stated their intent to live there. The applicant is required to meet these criteria as part of their business operation. To ensure long-term compliance with the occupancy requirement, a condition has been included in the attached Planning Commission Resolution (Attachment 1).

Off-Street Parking Requirements

Per OMC Section 17.34.060 Required Number of Parking Spaces, the minimum off-street parking requirement is “one space per rentable room, plus any other additional spaces required by the underlying zone. The existing historic property is legal non-conforming, because there is no enclosed garage. As such, two parking spaces are required for the dwelling, and one designated parking space is required for the single rentable room. Per the project plans (Attachment 3), a total of three striped parking spaces that meet minimum dimension and access requirements are proposed along the southern property line, adjacent to the alley.

Kitchen and Meals

Per OMC Section 17.13.050 Conditional Use Regulations, “no cooking facilities shall be permitted in any guest room,” and “guest meals shall be served only to persons registered as overnight guests.”

Per the applicant’s business narrative letter, meals will be prepared through meal vouchers or off-site cloud kitchens. Through implementation of recommended conditions of approval, the existing kitchen within the residential dwelling shall not be converted into a commercial kitchen, and no commercial kitchen operations for the public will be allowed. Meals will only be served to the residents and overnight guests.

7. AUTHORIZING GUIDELINES

Orange Municipal Code Section 17.08.020 B.2.a. authorizes the Planning Commission to review and take final action on applications for a Conditional Use Permit and Minor Site Plan Review. Footnote (b) of Table 17.08.020 of the OMC - Reviewing Bodies states that, “When more than one type of application is filed for a single project, the application requiring the highest level of approval shall dictate the review process for the entire group of applications.” The Planning Commission is the highest-level approving body for this application.

8. PUBLIC NOTICE

Notice was provided to property owners and tenants within 300 feet of the project and posted at the site on or before January 22, 2026.

9. ENVIRONMENTAL REVIEW

The proposed project is categorically exempt from the provisions of the California Environmental Quality Act (CEQA) per State CEQA Guidelines Section 15301 (Class 1 - Existing Facilities) because the project consists of operation of a bed and breakfast inn at an existing one-story single-family residential dwelling. There is no environmental public review required for a CEQA categorically exempt project.

10. ADVISORY BOARD ACTION

Staff Review Committee:
The Staff Review Committee reviewed the project on Wednesday, December 10, 2025 and recommended approval, subject to conditions of approval.

11. ATTACHMENTS

- Attachment 1 Planning Commission Resolution No. PC 04-26
- Attachment 2 Vicinity Map
- Attachment 3 Project Plans
- Attachment 4 Applicant Business Narrative Letter
- Attachment 5 Site Photos

Agenda Item

Planning Commission

Item #: 4.2. 2/2/2026 **File #:** 26-0007

TO: **Chair and Members of the Planning Commission**

THRU: **Hayden Beckman, Planning Manager**

FROM: **Ryan Agbayani, Senior Planner**

1. SUBJECT

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2. SUMMARY

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3. RECOMMENDED ACTION

1. Conduct and close the Public Hearing.
2. Adopt Planning Commission Resolution No. PC 04-26, approving Conditional Use Permit No. 3208 and Minor Site Plan Review No. 1159 for a request to operate a bed and breakfast inn at 616 W. Chapman Avenue.
3. Find the request categorically exempt from the California Environmental Quality Act (CEQA) in accordance with Section 15301 (Class 1 - Existing Facilities) of the State CEQA Guidelines.

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Property Information	
Applicant	Bonjour Bed & Breakfast
Property Owner	Ahmed Joseph Mahmoud
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6. PROJECT ANALYSIS

The proposed use is appropriate at this location and will provide a desired accommodation amenity for both local residents and visitors to the City. A bed and breakfast inn is a conditionally permitted use for properties located within the Old Towne Mixed Use-15S zoning district and the Santa Fe Depot Specific Plan.

Owner Occupancy Requirement

Per OMC Chapter 17.04 Definitions, a "bed and breakfast inn means a building of residential design determined to be architecturally or historically significant to the City of Orange wherein lodging is provided to tourists. The operator of the inn shall live on the same property."

Additionally, per OMC Section 17.13.050 Conditional Use Regulations, "an owner, manager, proprietor, or caretaker of the property must reside on the subject premises at all times."

The subject property is designated as a "contributor" in the Historic District. Furthermore, the property

owner has stated their intent to live there. The applicant is required to meet these criteria as part of their business operation. To ensure long-term compliance with the occupancy requirement, a condition has been included in the attached Planning Commission Resolution (Attachment 1).

Off-Street Parking Requirements

Per OMC Section 17.34.060 Required Number of Parking Spaces, the minimum off-street parking requirement is “one space per rentable room, plus any other additional spaces required by the underlying zone. The existing historic property is legal non-conforming, because there is no enclosed garage. As such, two parking spaces are required for the dwelling, and one designated parking space is required for the single rentable room. Per the project plans (Attachment 3), a total of three striped parking spaces that meet minimum dimension and access requirements are proposed along the southern property line, adjacent to the alley.

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Per the applicant’s business narrative letter, meals will be prepared through meal vouchers or off-site cloud kitchens. Through implementation of recommended conditions of approval, the existing kitchen within the residential dwelling shall not be converted into a commercial kitchen, and no commercial kitchen operations for the public will be allowed. Meals will only be served to the residents and overnight guests.

7. AUTHORIZING GUIDELINES

Orange Municipal Code Section 17.08.020 B.2.a. authorizes the Planning Commission to review and take final action on applications for a Conditional Use Permit and Minor Site Plan Review. Footnote (b) of Table 17.08.020 of the OMC - Reviewing Bodies states that, “When more than one type of application is filed for a single project, the application requiring the highest level of approval shall dictate the review process for the entire group of applications.” The Planning Commission is the highest-level approving body for this application.

8. PUBLIC NOTICE

Notice was provided to property owners and tenants within 300 feet of the project and posted at the site on or before January 22, 2026.

9. ENVIRONMENTAL REVIEW

The proposed project is categorically exempt from the provisions of the California Environmental Quality Act (CEQA) per State CEQA Guidelines Section 15301 (Class 1 - Existing Facilities) because the project consists of operation of a bed and breakfast inn at an existing one-story single-family residential dwelling. There is no environmental public review required for a CEQA categorically exempt project.

10. ADVISORY BOARD ACTION

Staff Review Committee:
The Staff Review Committee reviewed the project on Wednesday, December 10, 2025 and recommended approval, subject to conditions of approval.

11. ATTACHMENTS

- Attachment 1 Planning Commission Resolution No. PC 04-26
- Attachment 2 Vicinity Map
- Attachment 3 Project Plans
- Attachment 4 Applicant Business Narrative Letter
- Attachment 5 Site Photos

RESOLUTION NO. PC 04-26

A RESOLUTION OF THE PLANNING COMMISSION OF THE CITY OF ORANGE APPROVING CONDITIONAL USE PERMIT NO. 3208 AND MINOR SITE PLAN REVIEW NO. 1159 TO ALLOW THE OPERATION OF A BED AND BREAKFAST INN AT 616 W. CHAPMAN AVENUE

APPLICANT: AHMED JOSEPH MAHMOUD

WHEREAS, the Planning Commission has authority per Orange Municipal Code (OMC) Table 17.08.020 and Section 17.10.030.C to take action on Conditional Use Permit No. 3208 and Minor Site Plan Review No. 1159, for a proposal to operate a bed and breakfast inn within an existing historic one-story single-family residential dwelling at 616 W. Chapman Avenue; and

WHEREAS, the application for Conditional Use Permit No. 3208 and Minor Site Plan Review No. 1159 was filed by the applicant in accordance with the provisions of the OMC; and

WHEREAS, the application for Conditional Use Permit No. 3208 and Minor Site Plan Review No. 1159 was processed in the time and manner prescribed by state and local law; and

WHEREAS, the application for Conditional Use Permit No. 3208 and Minor Site Plan Review No. 1159 is Categorically Exempt from the provisions of the California Environmental Quality Act (CEQA) per State CEQA Guidelines Section 15301 (Class 1 – Existing Facilities), as the project consists of operation of a bed and breakfast inn within an existing one-story single-family residential dwelling; and

WHEREAS, the Planning Commission conducted one duly advertised public hearing on February 2, 2026, at which time interested persons had an opportunity to testify either in support of or opposition to Conditional Use Permit No. 3208 and Minor Site Plan Review No. 1159 upon property described in Exhibit A, attached hereto and incorporated herein.

NOW, THEREFORE, BE IT RESOLVED that the Planning Commission approves Conditional Use Permit No. 3208 and Minor Site Plan Review No. 1159, for a request to operate a bed and breakfast inn within an existing historic residential dwelling based on the following findings:

SECTION 1 – FINDINGS

General Plan Finding:

1. *The project must be consistent with the goals and policies stated within the City's General Plan.*

   The project is consistent with the Land Use and Economic Development Elements of the City's General Plan in that the establishment and operation of a bed and breakfast inn promotes compatible commercial activity, supports local economic vitality, and enhances visitor-serving uses within appropriate mixed-use and historic areas. Bed and breakfast inns are specifically identified as conditionally permitted uses within the Old Towne Mixed Use-15S (Specific Plan) zoning district and the Santa Fe Depot Specific Plan area, subject to review to ensure compatibility with surrounding land uses.

   The proposed bed and breakfast inn is designed to operate at a scale and intensity that is compatible with the surrounding neighborhood and consistent with the City's vision for mixed-use development that balances economic activity with neighborhood character. The use will provide short-term lodging opportunities that support tourism, local businesses, and pedestrian activity, while maintaining the architectural character and historic context of the area.

   Additionally, the project supports General Plan objectives to strengthen the local economy by encouraging small-scale, locally operated businesses and diversifying lodging options within the City. By attracting visitors to the area, the project contributes to increased patronage of nearby restaurants, shops, and services, which is consistent with the City's economic development goals.

Conditional Use Permit Findings:

1. *A Conditional Use Permit shall be granted upon sound principles of land use and in response to services required by the community.*

   Consideration has been given as to whether the proposed use will detrimentally affect adjacent land uses. The proposed use provides an opportunity for patrons seeking specialized lodging and accommodations in close proximity to the Orange Transit Center and Old Towne Orange. It will occur on a site that has been zoned for a mix of restaurant, retail, service, and residential uses around a commercial destination that serves City residents as well as visitors. Per the OMC Chapter 17.04 Definitions, owner occupancy is required for a bed and breakfast inn. Additionally, per OMC Section 17.13.050 Conditional Use Regulations, no cooking facilities shall be permitted in any guest room, and guest meals shall only be served to persons registered as overnight guests. As conditioned, its operation is not anticipated to create a nuisance to the community or a burden on police services.

2. *A Conditional Use Permit shall not be granted if it will cause deterioration of bordering land uses or create special problems for the area in which it is located.*

   The request to operate a bed and breakfast inn is not anticipated to have adverse impacts on surrounding land uses or create a special problem, because the site is located in the Old Towne Mixed Use-15S zoning district, which is intended to function as a mixed-use restaurant, retail, service, and residential district.

3. *A Conditional Use Permit must be considered in relationship to its effect on the community or neighborhood plan for the area in which it is located.*

   The project location is in close proximity to the Orange Transit Center and the Old Towne Orange Historic District, a popular commercial destination where shopping, dining, and lodging establishments are expected to be concentrated. The proposed bed and breakfast use is somewhat accessory to the existing historic residential dwelling, where the owner, manager, proprietor, or caretaker is required to reside on the subject premises at all times. The proposed small-scale operation is not anticipated to have an adverse impact on neighboring land uses and the community, because conditions of approval have been designed to minimize the potential for adverse effects on sensitive land uses in the immediate area.

4. *A Conditional Use Permit, if granted, shall be made subject to those conditions necessary to preserve the general welfare, not the individual welfare of any particular applicant.*

   The conditions of approval for the Conditional Use Permit are put forth with the purpose of preserving the general welfare of the community. If the City of Orange Community Development Department finds that the business is not operating as required, or if there is a history of criminal or nuisance behavior, the Department will recommend that the Planning Commission reconsider this application.

SECTION 2 – ENVIRONMENTAL REVIEW

The proposed project is categorically exempt from the provisions of CEQA per State CEQA Guidelines Section 15301 (Class 1 – Existing Facilities) because the project consists of operation of a bed and breakfast inn within an existing one-story single-family residential dwelling. No environmental public review is required for a CEQA categorically exempt project.

SECTION 3 – CONDITIONS OF APPROVAL

BE IT FURTHER RESOLVED that the following conditions are imposed with approval:

General:

1. The final conditions of approval shall be reprinted on a dedicated sheet of the construction documents when submitted to the Building Division for the plan check process.

2. The term “applicant” shall refer to the entity that requests approval of this action or any successor in interest to this approval.

3. Any future change in the nature and operation of the use approved by Conditional Use Permit (CUP) No. 3208 and Minor Site Plan Review No. 1159 shall require an application for a new or amended CUP.

4. The applicant agrees, as a condition of City's approval of , to indemnify, defend, and hold harmless, at applicant's expense, the City, its officers, agents, and employees (City) from and against any claim, action, or proceeding brought against the City, including, but not limited to, any claim, action or proceeding commenced within the time period provided in Government Code Section 66499.37 to attack, review, set aside, void or annul the City's approval, to challenge the determination made by the City under the California Environmental Quality Act (CEQA) or to challenge the reasonableness, legality or validity of any condition attached hereto. City shall promptly notify applicant of any such claim, action or proceeding to which the City receives notice and to cooperate fully with the applicant in the defense thereof. Applicant shall reimburse the City for any and all costs and expenses, including, but not limited to, court costs and attorney's fees that the City may be required to pay, including any expenses ordered by a court or expenses incurred through the Office of the City Attorney in connection with said claim, action or proceeding. City may, in its sole discretion, participate in the defense of any claim, action or proceeding but such participation shall not relieve applicant of the obligations of this condition. In the event the applicant is required to defend City in connection with such claim, action or proceeding, City shall have the right to approve counsel to so defend the City, approve all significant decisions concerning the manner in which the defense is conducted and approve any all settlements, which approval(s) shall not be unreasonably withheld. The obligations set forth herein remain in full force and effect throughout all stages of litigation including any and all appeals of any lower court judgment rendered in the proceeding. Further, applicant agrees to indemnify, defend and hold harmless the City for all costs and expenses incurred in enforcing this provision.

5. The applicant shall comply with all federal, state, and local municipal laws, including local City ordinances and regulations. Any violations of these laws in conjunction with this use may be cause for revocation of this permit.

6. Prior to the operation of the business, the applicant shall file for, or if applicable, amend a business license with the Business License Division. Failure to obtain the required business license may be cause for revocation of this approval.

Planning Division:

7. This CUP shall apply only to the project parcel (APN: 390-673-27), with the address of 616 W. Chapman Avenue, regardless of any change in ownership.

8. Per OMC Section 17.13.050 Conditional Use Regulations, the property owner, manager, proprietor, or caretaker is required to reside the subject premises at all times.

9. Per OMC Section 17.13.050 Conditional Use Regulations, no cooking facilities shall be permitted in any guest room.

10. Per OMC Section 17.13.050 Conditional Use Regulations, guest meals shall be served only to persons registered as overnight guests.

11. The project site shall be developed and maintained in accordance with the approved plans and conditions of approval.

12. The activity level of the business shall be monitored by the Police Department to establish the level of police services used for the business. Should the of police services demonstrate that the applicant has not controlled excessive, or unnecessary activity resulting in high use of police services then the Conditional Use Permit shall be reviewed for consideration of further conditions, modifications or revocation.

13. All required parking spaces shall be clearly identified and maintained at all times. A minimum of three designated parking spaces is required. Parking shall meet the minimum required dimensions as outlined in OMC Section 17.34.110 Parking Area Dimensions, and shall be in double-stripe formation as outlined in OMC Section 17.34.130 Maintenance and Operation of Permanent Parking Areas.

14. In conjunction with the operation of the business, the property owner shall be responsible to maintain the property to a level deemed adequate by the Community Development Director, or designee. This includes, but is not limited to, the building(s), on-site landscaping, trash areas, signed, utilities, property fences and gates, etc.

Building Division

15. The final approved plans shall include the "General Notes" as shown below:

    a) On-premises food service shall be for residents and guests only. No public food service is permitted.
    b) NFPA 13D automatic fire sprinkler system is required to be installed.
    c) Per CBC Chapter 2, 202 Definitions: "A place of public accommodation includes a facility operated by a private entity whose operations affect commerce and fall within at least the category of a place of lodging, except for an establishment located within a facility that contains no more than five rooms for rent or hire and that actually is occupied by the proprietor of the establishment as the residence of the proprietor.
    d) The operator shall be limited to a single room for lodging within the owner-occupied residential unit. As such, the operation is not considered a "Place of Public Accommodation" pursuant to the California Building Code (CBC) Chapter 2, 202 Definitions.
    e) CBC 11B202.3. Where existing elements or spaces are altered, each altered element or space shall comply with the applicable requirements of Division 2, including 11B-202.4.
    f) Exception #3. Residential dwelling units not required to be accessible in compliance with this code shall not be required to comply with Section 11B-202.3.

16. Should the business operation exceed the one room for lodging, or if the owner occupancy requirement is not met, the project shall be considered a "place of public accommodation" and will be subject to any and all California Building Code requirements related to Accessibility and the Americans with Disabilities Act (ADA).

Fire Department

17. Automatic Fire Sprinklers are required per NFPA13D. Deferred submittal for plan and permit by licensed C-16 contractor. Permit is required prior to water service approval.

Traffic Division

18. Prior to the issuance of building permits, the applicant will be responsible for paying Transportation Systems Improvement Program (TSIP) fees based on the proposed net change in land use (proposed project minus existing trips). Final TSIP fees will be calculated using the final approved building plans and current rates at the time of building permit issuance.

Water Division

19. Prior to issuance of any grading permit or building permit, the applicant must submit improvement plans to the Water Division for review and approval for any new fire hydrants, domestic water services, fire services, landscape services, backflow prevention devices, and any other proposed improvements or relocations affecting the public water system facilities.

20. Prior to issuance of any grading permit, the applicant must construct all public and/or private improvements to the satisfaction of the Water Division. The applicant may be required to enter into an agreement with the City of Orange, and post security in a form and amount acceptable to the City Engineer and/or Water Division to ensure construction of said improvements.

21. Prior to issuance of any grading permit or building permit, the applicant will be responsible for the installation and/or relocation of the proposed or existing public water system facilities to a location and of a design per the improvement plans approved by the Water Division.

22. Prior to issuance of building permit, the Water Division will approve the type and location of landscaping services, fire services, and backflow prevention devices for proposed City services.

23. Prior to issuance of building permit, for the first phase of work, the applicant will be responsible for obtaining approval of all necessary encroachment permits from affected agencies for all public water construction work.

24. Prior to approval of a water improvement plan, the applicant must satisfy all water main connection, plan check, and inspections charges as determined by the Water Division.

25. Plans submitted during plan check must show that the water improvement plans are consistent with the fire suppression plans and or fire master plan. The applicant's consultant preparing the water improvement plans must coordinate their plans with the consultant preparing the fire suppression plans and/or fire master plan so that their designs concur.

Surface Water Quality

26. Prior to approval of grading or building permits, the applicant shall submit a Non-Priority Project Water Quality Management Plan (WQMP) for review and approval to the Public Works Department that:

    a) Describes the project site.
    b) Describes the potential pollutants.
    c) Incorporates the applicable Site Design, Routine Source and Structural Control BMPs as defined in the Model Water Quality Management Plan (MWQMP) and Technical Guidance Document (TGD).
    d) Generally describes the long-term operation and maintenance requirements for structural Control BMPs
    e) Identifies the entity that will be responsible for long-term operation, maintenance, repair, and/or replacement of the BMPs,
    f) A copy of the forms to be used in conducting maintenance and inspection activities
    g) Includes record keeping requirements (forms to be kept for 5 years).

Miscellaneous:

27. Graffiti shall be removed from the exterior walls, doors, windows, and any additional part of the property (including signage, etc.) within 72 hours of the time the City of Orange Notice of Violation is received by the business operator.

ADOPTED this 2nd day of February 2026

_____
Alison Vejar, Planning Commission Chair

I hereby certify that the foregoing Resolution was adopted by the Planning Commission of the City of Orange at a regular meeting thereof held on the 2nd day of February 2026, by the following vote:

AYES:
NOES:

RECUSED:
ABSENT:

_____

Hayden Beckman
Planning Manager

Exhibit A

LEGAL DESCRIPTION

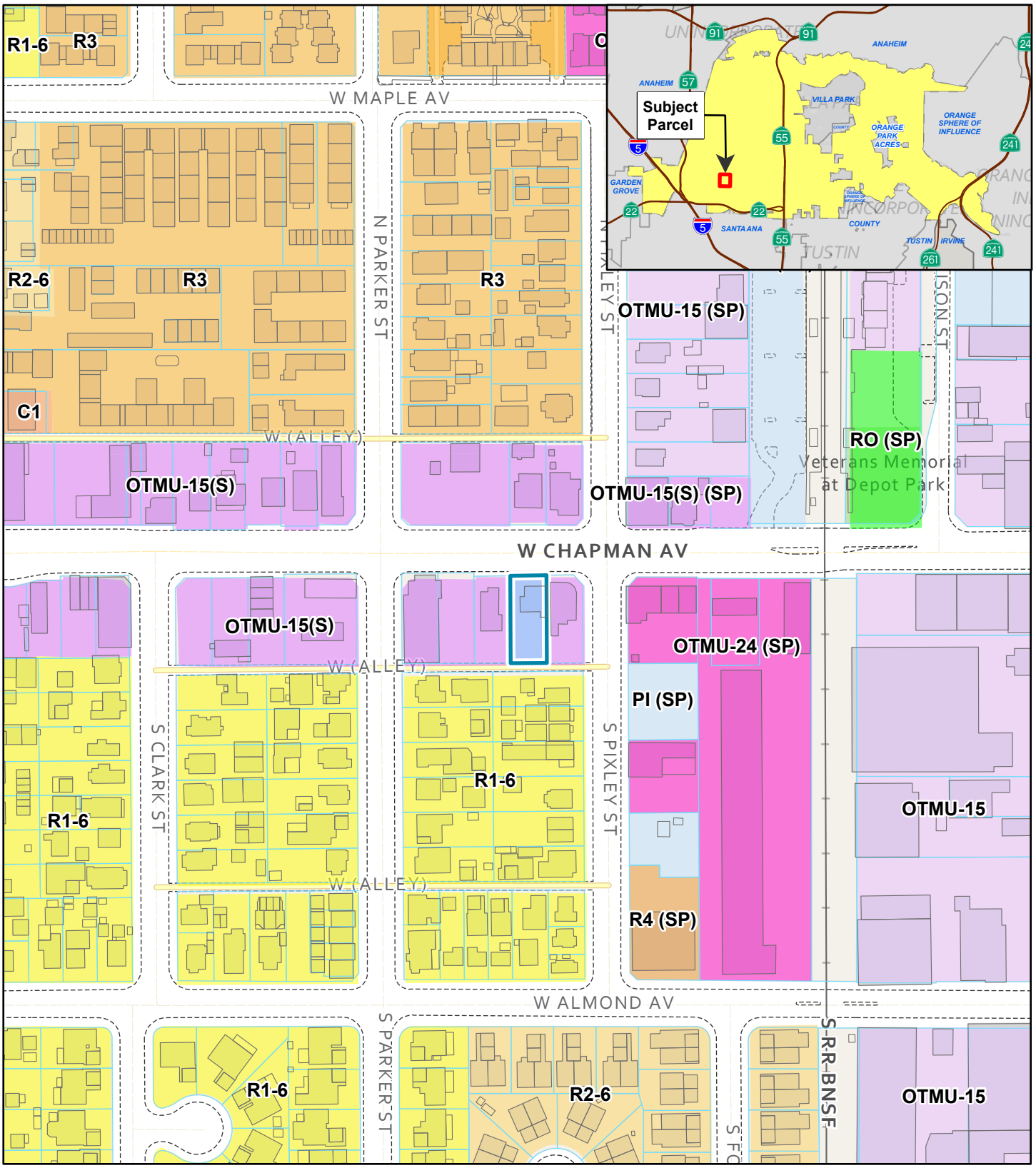
The land referred to herein is situated in the State of California, County of Orange, City of Orange and described as follows:

Lot 2 in Block B of Kordes Tract, in the City of Orange, County of Orange, State of California, as per Map recorded in Book 18 Page 2 of Miscellaneous Records in the Office of the County Recorder of said County.

Except therefrom the North 5 feet as granted to the City of Orange in Deed recorded July 5, 1988 as Instrument No. 88-321764, Official Records.

FOR INFORMATIONAL PURPOSES ONLY: APN: 390-673-27

(End of Legal Description)



City of Orange
Geographic Information System

Legend

Parcels

Subject Parcel

Zoning

- C1
- PI
- PI (SP)
- R1-6
- R2-6
- R3
- R4 (SP)
- RO (SP)
- OTMU-15(S)
- OTMU-15(S) (SP)
- OTMU-15
- OTMU-15 (SP)
- OTMU-24 (SP)
- UNZONED

N W E S

0 100 200 Feet

1/12/2026

Source: City of Orange (2026)

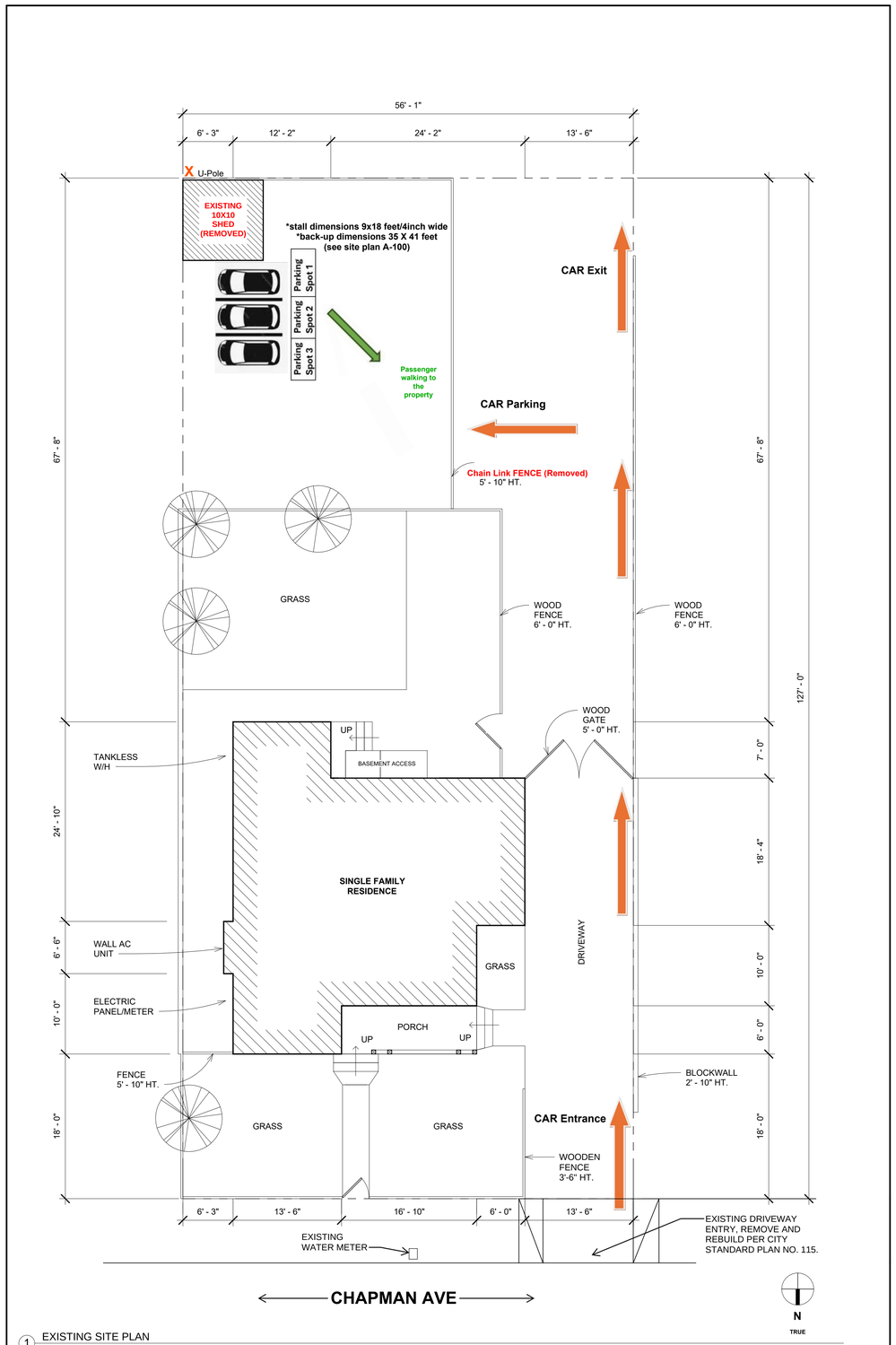
CUP-3208 and SP-1159
616 W. Chapman Avenue
Zoning: OTMU-15(S)

CITY OF ORANGE

BONJOUR BED & BREAKFAST

VICINITY MAP

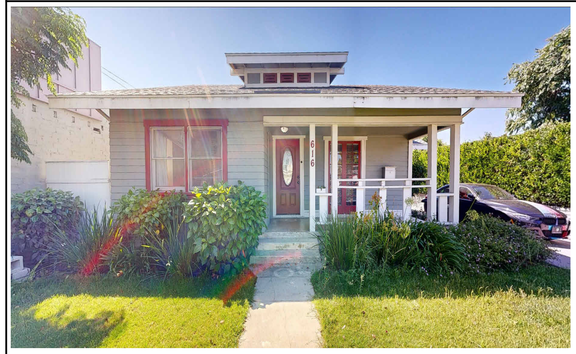
616 W Chapman Ave, Orange, CA 92868



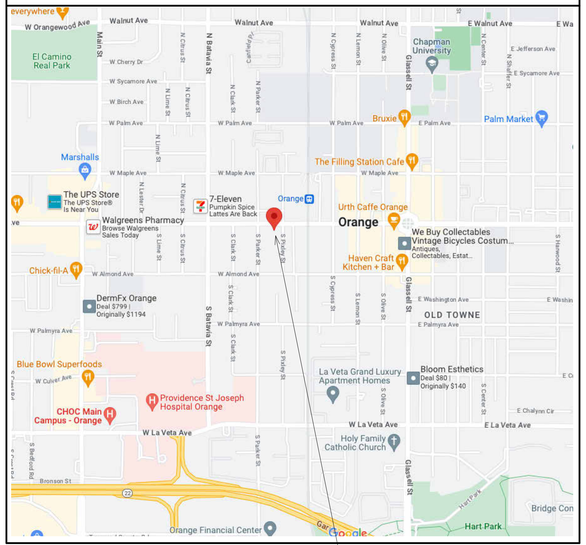
1 EXISTING SITE PLAN
1/8" = 1'-0"

*** SITE PLAN SCHEMATIC ONLY - NOT A LEGAL SURVEY**

STREET VIEW



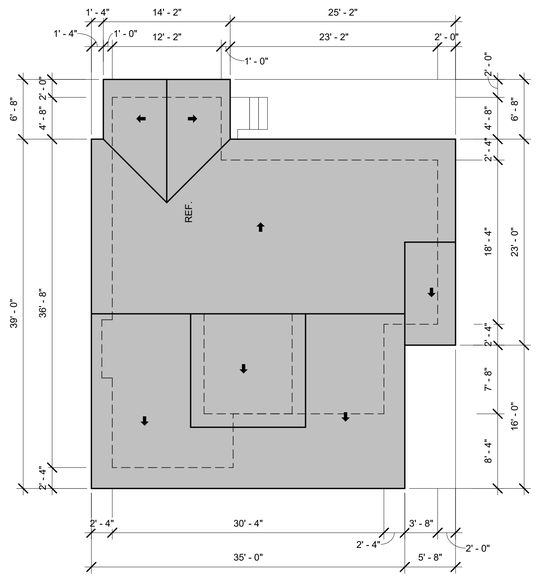
VICINITY MAP



SITE

AREA CALCULATION

EXISTING RESIDENCE - INTERIOR	1065 SF
EXISTING RESIDENCE - EXTERIOR	1143 SF
Note: All design and plans based on the 2022 CBC, CMC, CEC, CPC, CGBSC and Building Energy Code."	-
-	-
-	-



2 EXISTING ROOF PLAN
1/8" = 1'-0"

LEGAL DESCRIPTION & BUILDING RECORDS

PROPERTY ADDRESS	616 W Chapman Ave, Orange, CA 92868
AIN #	39067327
LAND USE	SFD
YEAR BUILT	1920
LOT AREA	6,861 Sq. Ft.
GROSS FLOOR AREA	1,111
ZONING	OTMU-15S
NUMBER OF LEVELS	1
FIRE SPRINKLED	NFPA AUTOMATIC FIRE SPRINKLER SYSTEM WILL BE INSTALLED
PROPOSED OCCUPANCY	BED AND BREAKFAST (1 ROOM OUT OF 3 WILL BE B & B) MAX 2 GUESTS, OWNER WILL OCCUPY MASTER BEDROOM (129 SQF),GUEST WILL OCCUPY THE MIDDLE ROOM 127 SQF.
FLOOR PLAN INTERIOR	NO INTERIOR FLOOR PLAN WILL BE PROPOSED
PROPSED SITE PLAN IMPROVEMENT	REMOVING CHAINLINK FENCE FROM REAR
SINAGE & PARKING	NO SINAGE REQUIRED
FAMILY OCCUPANTS	JOSEPH M. (HUSBAND) MOUNIA M. (WIFE) ZACK M. (SON)

DEXIGN XYNTHESIS, INC.
3744 E. CHAPMAN AVE. SUITE F2022
ORANGE, CA 92859
714.712.0069

PROJECT CONTACT: RAFAEL INIGUEZ
EMAIL: riniguez@dexignxynthesis.com

2109

PROJECT ADDRESS:

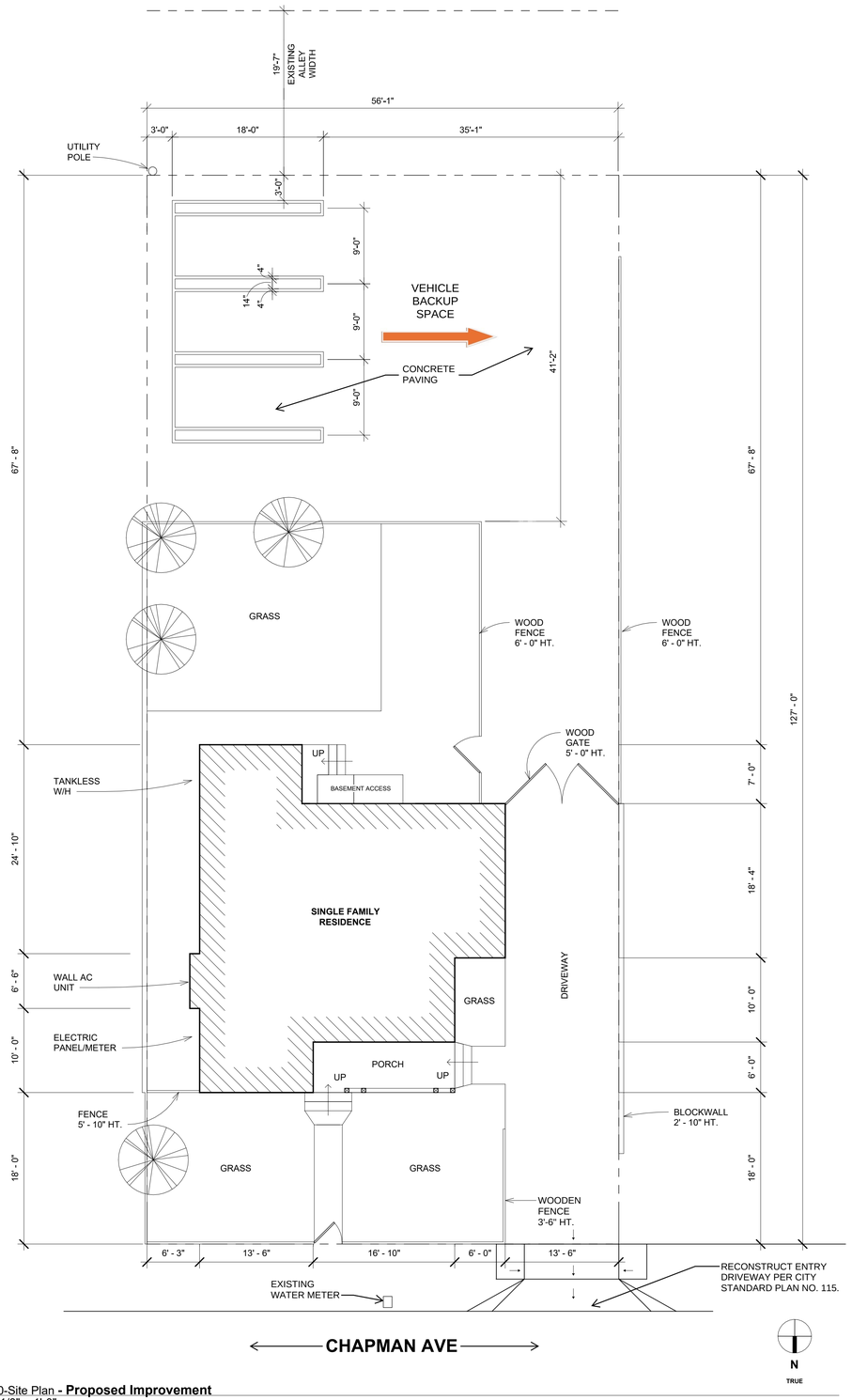
616 W Chapman AvE
Orange, CA 92868

SHEET TITLE:

COVER SHEET

SCALE: AS NOTED

SHEET NUMBER: A-000



2 0-Site Plan - **Proposed Improvement**
1/8" = 1'-0"

GENERAL NOTES

1. NO SIGNAGE IS BEING PROPOSED FOR THIS PROJECT.

2. THERE WILL BE NO FOOD SERVED ON THE PREMISES FOR THIS PROJECT.

3. NFPA 13D AUTOMATIC FIRE SPRINKLER SYSTEM REQUIRED TO BE INSTALLED.

CALIFORNIA BUILDING CODE NOTES

PER CBC CHAPTER 2, 202 DEFINITIONS:
A **PLACE OF PUBLIC ACCOMODATION** INCLUDES A FACILITY OPERATED BY A PRIVATE ENTITY WHOSE OPERATIONS AFFECT COMMERCE AND FALL WITHIN AT LEAST THE CATEGORY OF A PLACE OF LODGING, ***EXCEPT FOR AN ESTABLISHMENT LOCATED WITHIN A FACILITY THAT CONTAINS NOT MORE THAN FIVE ROOMS FOR RENT OR HIRE AND THAT ACTUALLY IS OCCUPIED BY THE PROPRIETOR OF THE ESTABLISHMENT AS THE RESIDENCE OF THE PROPRIETOR.***

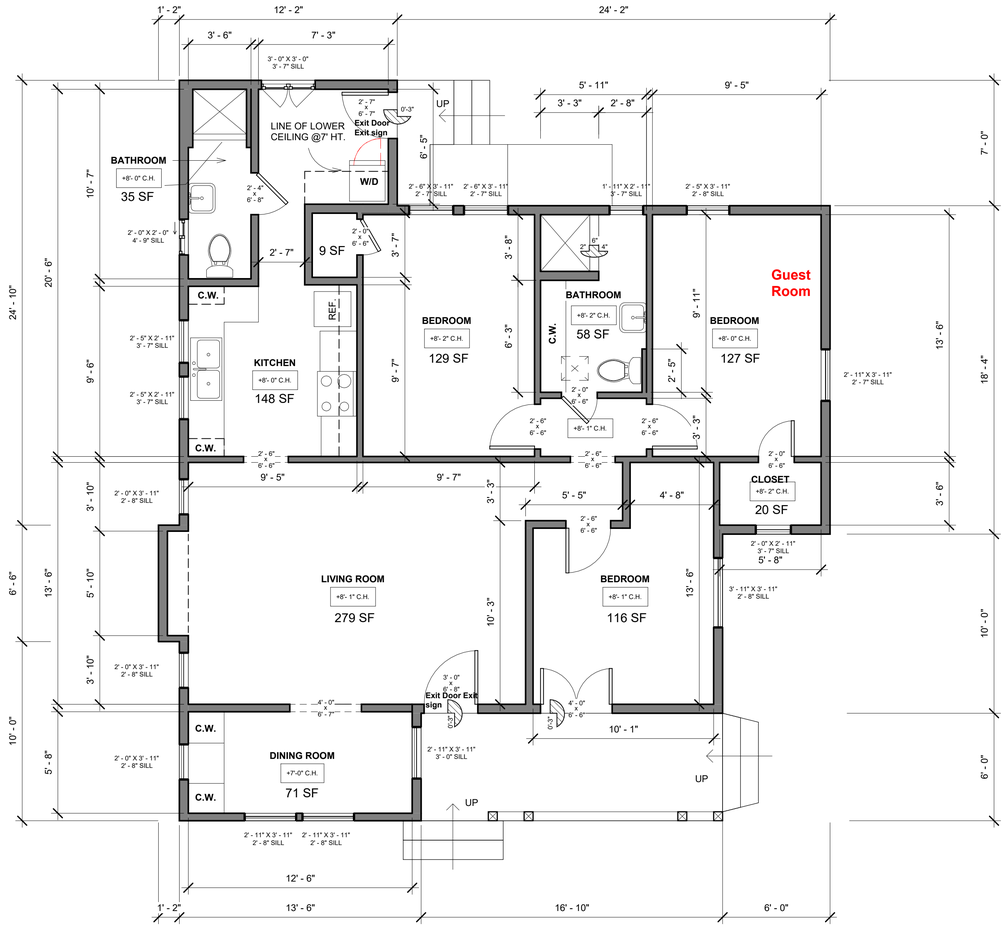
THIS PROJECT WILL ONLY HAVE 1 ROOM FOR LODGING AND THE ESTABLISHMENT WILL BE OCCUPIED BY THE OWNER, THEREFORE THIS PROJECT IS NOT CONSIDERED A PLACE OF PUBLIC ACCOMODATION.

CBC 11B202.3
WHERE EXISTING ELEMENTS OR SPACES ARE ALTERED, EACH ALTERED ELEMENT OR SPACE SHALL COMPLY WITH THE APPLICABLE REQUIREMETNS OF DIVISION 2, INCLUDING 11B-202.4

EXCEPTION #3.
RESIDENTIAL DWELLING UNITS NOT REQUIRED TO BE ACCESSIBLE IN COMPLIANCE WITH THIS CODE SHALL NOT BE REQUIRED TO COMPLY WITH SECTION 11B-202.3.

TERMS, SYMBOLS & ABBREVATIONS GLOSSARY

1' - 0" X 1' - 0" Sill Height SILL	WINDOW WIDTH X HEIGHT - SILL HEIGHT
C.W.	CASEWORK / CABINETRY / BUILT-INS
F.P.	FIREPLACE
T.O.	TOP OF
B.O.	BOTTOM OF
+X'-X" C.H.	CEILING HEIGHT WHERE MARKED
+X'-X"	LEVEL CHANGE
+X'-X" C.H. / +X'-X" C.H.	ILLUSTRATION OF ROOM OUTLINE WHERE MARKED
	ATTIC ACCESS/CEILING OPENING
X"	THRESHOLD DIRECTION & HEIGHT



1 1-1st Floor
1/4" = 1'-0"

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2110

PROJECT ADDRESS:

616 W Chapman AvE
Orange, CA 92868

SHEET TITLE:

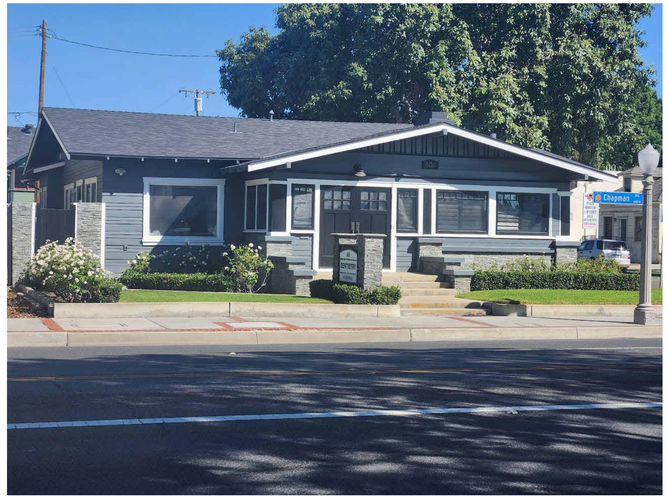
FLOOR PLANS

SCALE: AS NOTED

SHEET NUMBER: A-100



North (Rear) elevation, view looking south



South (Front) elevation, view looking North East



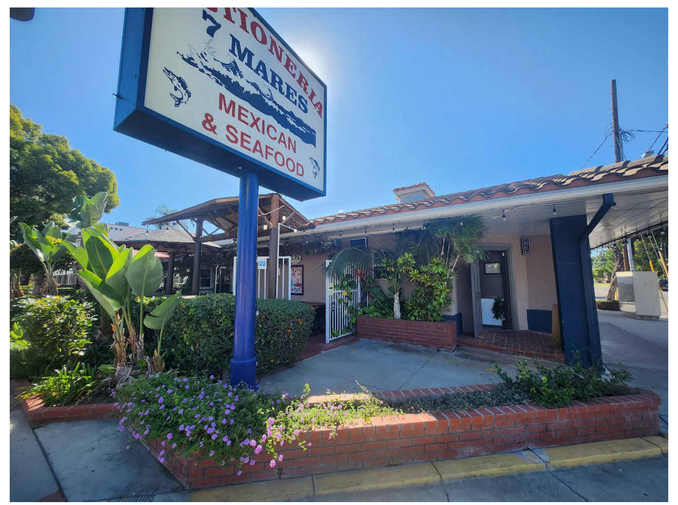
South (Front) elevation, view looking North



West (Right) elevation, view looking East



South (Front) elevation, view looking North



East (Right) elevation, view looking East

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PROJECT CONTACT: RAFAEL INIGUEZ
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PROJECT ADDRESS:

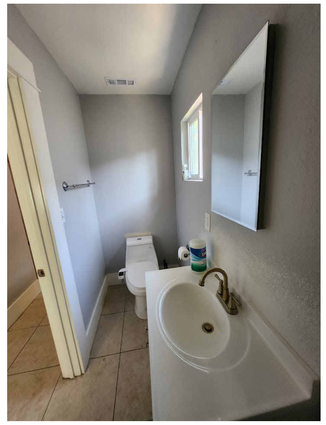
616 W Chapman AvE
Orange, CA 92868

SHEET TITLE:

EXTERIOR PHOTOS

SCALE: AS NOTED

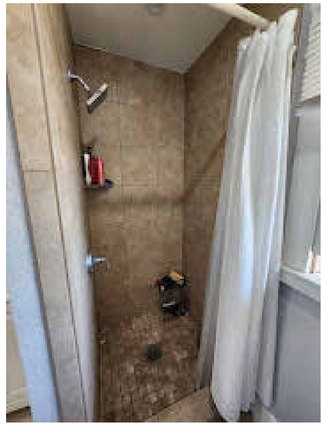
SHEET NUMBER: A-200



BATHROOM "1"



KITCHEN



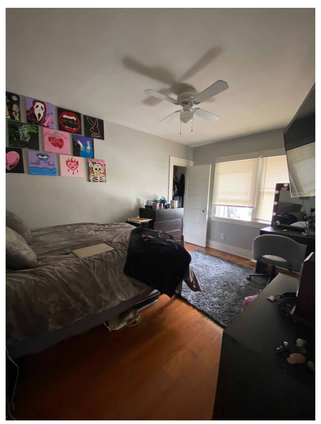
BATHROOM "2"



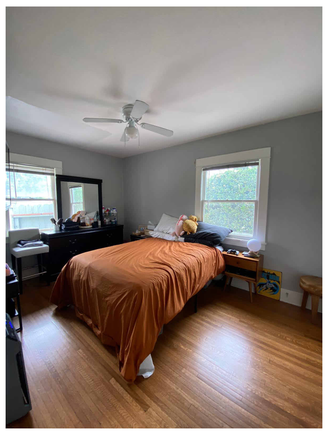
LIVING ROOM - PHOTO "A"



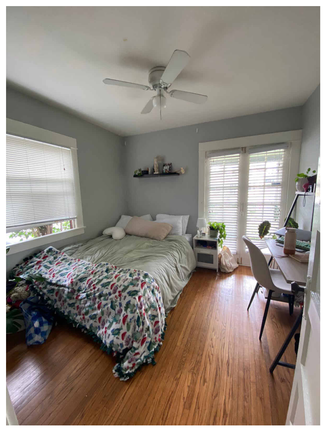
LIVING ROOM - PHOTO "B"



ROOM "1"



ROOM "2"



ROOM "3"

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PROJECT CONTACT: RAFAEL INIGUEZ
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PROJECT ADDRESS:

616 W Chapman AvE
Orange, CA 92868

SHEET TITLE:

INTERIOR PHOTOS

SCALE: AS NOTED

SHEET NUMBER: A-201

LETTER OF EXPLAINATION

1. **For non-residential projects**

- Company name : BB&B stands for Bonjour Bed & Breakfast
- Type of business : Bed & Breakfast
- Use : Bed & Breakfast
- Hours of operation : 24/7
- Check-in time is 3 PM, Check-out is 11 AM . No minimum night stay or overnight requirements.
- Company owned vehicles : None
- Number of employees : 2 (Wife & Husband)
- Rooms capacity: only one room will be dedicated to guests.
- Type of materials handled : None. Meals provided through meal vouchers or offsite cooking (cloud kitchen)
- History of the business : Family-owned creperie in Cavaillon, France, since 1965 then catering services for family and friends in OC since 2020.
- No Live music.
- Owner, wife & 2 children will reside full time at the property.

2. **Our property characteristics**

Craftsman single family home that was built in 1915 (historical/Mills Act), one level, backyard, front yard, porch, 1 tree in the front yard and couple of trees in the backyard. (No proposed site plans for interior or exterior changes will be submitted to the city, only minimal improvements like removing the chain fence in the rear)

3. **Surrounding properties are all commercial uses.**

Our house is located on Chapman Avenue, on the right side there is a Mexican coffee shop, on the left side there is an administrative office and a Mexican restaurant. In the back there is an empty alley for all the businesses. Most of the surrounding properties are approximately 9 to 12 feet in height commercial joints.

4. **Unique conditions**

Location: Close to the train station and bus station (ideal for local and tourists), close to the Circle, within the Historic downtown. Big open-air backyard for guests to enjoy their breakfast.

5. **Phasing**

No future expansion planned, or any site plans altercations will be requested.

6. **For Alcohol beverages**

We are not serving alcohol.

Executive summary

Bonjour Bed & Breakfast will be serving breakfasts through cloud kitchen or locally sourced meals (vouchers) for some authentic French breakfast pastries like Croissants, Tarte aux Pommes, petits fours and bread, tartines a la confiture, crepes sucrees et salees and baguettes for the stay-in guests (No cooking at the house). introducing guests to the history of Orange through city tours, books and documentaries. We wish to create a unique experience that offers delicious and memorable stays. My wife grew up in the South of France (La Provence) where authentic French cuisine has shined; she has been in the hosting and pastries business for over 15 years. Our B&B will be targeting those in the age bracket of 21 to 80. Previously, we have been doing home deliveries and catering for family and friends through cloud kitchens; we have got positive reviews from them due to the quality of our food and service.

Company description

Bonjour Bed & Breakfast's mission is to provide our guests with a comfortable, quality, and satisfying stay-in experience. The pastries will sourced locally or cooked in a cloud kitchen using locally made and seasonal ingredients. My wife is a previous chef and currently a history teacher and finds a pleasure in being a food tourism advocate and food guide enthusiast. Our primary goal is to provide a unique experience for people visiting the Old Downtown of Orange and fully introduce them to the History of the city which will be beneficial to the community, having a quality B&B with a theme that does not exist currently in the city of Orange.

We are dedicated to serving the City of Orange’s community, locals and visitors who will be coming from other regions. The business structure consists of the husband and the wife, the husband will be in charge of all the administrative tasks (finances, calendar management, website

management, marketing…) while the wife will be the French local food expert leading the city tours and home stay comfort arrangements for guests. Our objectives for the next two years are:

I. Honor our guests and consistently exceed their expectations to solidify a glowing reputation for our business.
II. Be a unique B&B with authentic pastries that will leave our customers with great memories.
III. Reach a 70% occupancy rate within the first year and 80% within the second year.

Marketing and sales research

BB&B's prospective customers will be customers between 21 and 80 of age, either they are tourists, students and their families or even locals, we have something for everyone. One of the reasons for choosing a Historical Craftsman house and spending so much money to restore it is to introduce and cherish the deep History of the Old Town of Orange. The host (husband) grew up in Orange and is fully aware of the rich History of the town, hence he will easily spread the word and guide visitors to the best spots and joints around to enjoy that great city.

We intend to advertise our business through different channels. We plan on having our own website, besides utilizing third parties' platforms, social media and word-of-mouth.

Competitive analysis

In terms of competitive landscape, There's no B&B that has city tours led by a local food expert and a history advocate which will introduce the city to more unique type of business in Old Town Orange.

Furthermore, according to Orange Coast magazine, tourists visiting Orange County in 2023 preferred meals presented through la ocal food tourism guide which gives us an edge.

SWOT Analysis

The following table shows the SWOT analysis of the business.

Strengths	**Weaknesses**
● A strong network of customers from our previous catering business. ● The wife has explicit know-how of the baking industry. ● We have initiated SMART business objectives and goals. ● Close to all vivid spots including the Train station, bus station and downtown (Location +++). ● Multiple parking spaces.	● Economy of scale
Opportunities	**Threats**
● A growing number of customers who desire a charming place to discover	● Seasonality ● Competing with corporate businesses.

the old town, while having a unique experience offered by their host. ● Most of the B&Bs, cafes and bakeries around the California region are cuisine-based. They do not offer city tour and a culinary guide.	● Financing. Unforeseen expenditures.

Service and Product description

At BB&B, the guests will enjoy a stay in a Historical house (The Heins' home has been always a trademark in the Confectionary and Pastries business), in the heart of the old town, at a walking distance to the Train station and to all the many cafes, bars, restaurants and boutiques that the city has to offer. Their stay will include recommendations of sites to visit, and locally sourced French pastries for every morning spent at the house. Guests will enjoy a variety of city tours with a local food advisor to boost the local businesses. The food vouchers will consist of hot beverages such as coffee, tea and hot chocolate, besides juices and pastries or brunches. In the future,we will acclimate according to the demand & VOC (Voice Of Customer), we will have different local sourced options for viennoiseries such as croissants, pains au chocolat, chaussons aux pommes, and also baguettes to make tartines aux beurre et a la confiture.

BB&B will have a competitive advantage due to product authenticity,giving our brand unique access to the surrounding residents who are most likely eager to enjoy that new delicious stay. Our primary marketing channel will be social media

platforms, our website, third-parties' platforms, word of mouth and referrals. All of our ingredients are organic and fresh (sourced from local markets and some are imported from France like flour).

Financial plan

Our 1 year financial plan below sums up our regular future expenses that are utilized in our operations. Our initial ops budget is $50,000, which will cater to our regular expenses. Our fiscal year for 2026 is below.

	Jan-2026	Feb-2026	March-2026	Apr-2026	May-2026	June-2026	Juy-2026	Aug-2026	Sep-2026	Oct-2026	Nov-2026	Dec-2026
Kitchen supplies	4000	4000	4000	4000	4000	4000	4000	4000	4500	4500	4500	4500
Utilities	3100	3100	3100	3100	3100	3100	3100	3100	3200	3300	3300	3300
Cleaning	1500	1500	1500	1500	1500	1500	1500	1500	1600	1600	1600	1600
Deliveries	1500	1500	1500	1500	1500	1500	1500	1500	1600	1600	1600	1600
Waste/Over Head	1700	1700	1700	1700	1700	1700	1700	1800	1800	1800	1800	1800

Morgage	3000	3000	3000	3000	3000	3000	3000	3200	3200	3200	3200	3200
City fees	500	500	500	500	500	500	500	600	600	600	600	600
Other expenses	2500	3000	3000	3500	3600	3600	3600	4000	4200	4500	4500	4500

Based on the estimates above, the business will break even for the first 12 months due to the old associated start-up costs like furniture, equipment, fees, materials and renovations. Cash flow will be around $1000 a month in the second year after all expenses and associated costs. (Salaries aren't included)

Organization and management plan

BB&B is a family sole proprietor business (wife, husband and son) who will reside on the premises at all times. The business structure consists of the husband and the wife, the husband will be in charge of all the administrative tasks (finances, calendar management, website management, marketing…) while the wife will be the food conceirge and the history guide for city tours. We will ensure that the management set up meets the expectation of our customers and achieve the highest service level possible.



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